GENERAL NOTES:

1) CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE PROJECT SITE PRIOR TO STARTING WORK AND SHALL NOTIFY THE ARCHITECT AND/OR STRUCTURAL ENGINEER IMMEDIATELY OF ANY DISCREPANCIES. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY EXISTING SITE CONDITIONS THAT ARE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS.

2) ALL REFERENCED STANDARDS REFER TO THE LATEST EDITION. 3) DESIGN CRITERIA: A) BUILDING CODE: IBC 2012 WITH GEORGIA AMENDMENTS B) FLOOR LIVE LOAD: 20 PSF C) ROOF LIVE LOAD:

D) GROUND SNOW LOAD: 0 PSF E) RISK CATEGORY: F) WIND LOAD DATA: 2) WIND EXPOSURE FRONT: 3) WIND EXPOSURE BACK: 4) INTERNAL PRESSURE COEFF

5) EDGE ZONE DISTANCE: COMPONENTS AND CLADDING LOADS (PSF) a) ROOF INTERIOR ZONE: ROOF EDGE ZONE: 9.60 -36.52 c) ROOF CORNER EDGE ZONE: 9.60 -54.96

d) WALL INTERIOR ZONE: 19.92 -21.58 19.92 -26.56 e) WALL EDGE ZONE: H) EARTHQUAKE DESIGN DATA: SEISMIC IMPORTANCE FACTOR: 0.324 SDS: 0.333 6) SITE CLASS:

7) SEISMIC DESIGN CATEGORY:

8) RESPONSE MODIFICATION FACTOR R:

J) DESIGN SOIL BEARING PRESSURE: 1500 PSF

9) SEISMIC RESPONSE COEFFICIENT CS: 10) DESIGN BASE SHEAR 11) BASIC SEISMIC FORCE RESISTING SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR WALLS 12) ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE FLOOD LOADS:

4) THE DESIGN LOADING INFORMATION PROVIDED IS FOR INFORMATIONAL PURPOSES ONLY. ALL COMPONENTS, CLADDING, FINISHES, VENEERS, MECHANICAL UNITS, ARCHITECTURAL FEATURES, ETC. SHALL BE DESIGNED FOR ADEQUATE CONNECTIVE CAPABILITY UNDER CODE SPECIFIED LOADING. THESE MANUFACTURER'S SHALL BE RESPONSIBLE TO DETERMINE THE REQUIRED LOADING CONDITIONS FOR THEIR PRODUCT, INCLUDING ANY SUPERIMPOSED LOADS, DEAD LOADS, CONCENTRATED LOADS AND ANY LOADS SHOWN ON THE PLANS.

5) ALL STRUCTURAL COMPONENTS AND SYSTEMS SHALL BE DESIGNED FOR THE FOLLOWING DEFLECTION LIMITS, UNLESS NOTED OTHERWISE ON THE PLANS AND DETAILS.

WALLS W/ FLEXIBLE FINISH WALLS W/ BRITTLE FINISH WALLS W/ BRICK VENEER L/600 ROOF BEAMS OR JOISTS ROOF TRUSSES OR GIRDERS WINDOW HEADERS MATCH WALL FLOOR JOIST OR BEAMS L/360 L/360 FLOOR GIRDERS *D = DEAD LOAD; L = LIVE LOAD; W = WIND LOAD; S = SEISMIC LOAD

6) CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL DISCIPLINES, INCLUDING BUT NOT LIMITED TO GROUNDING WIRES, CONDUITS, PIPE AND DUCT PENETRATIONS, ELECTRICAL, MECHANICAL AND PLUMBING OPENINGS, EQUIPMENT LOADS, ETC. SEE ARCHITECTURAL, CIVIL, ELECTRICAL AND MECHANICAL DRAWINGS FOR ITEMS NOW SHOWN ON THE STRUCTURAL DRAWINGS.

7) THE CONTRACTOR SHALL PROTECT ANY EXISTING FACILITIES, UTILITIES OR STRUCTURES FROM

8) THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND ERECTION OF ALL TEMPORARY BRACING AND SHORING AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION.

9) CONTRACT DRAWINGS, DOCUMENTS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF CONSTRUCTION INCLUDING, BUT NOT LIMITED TO, SHORING AND TEMPORARY BRACING.

10) DO NOT SCALE THE DRAWINGS.

11) THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO ENSURE THE SAFETY OF ALL PERSONS AND STRUCTURES AT THE SITE AND ADJACENT TO THE SITE. VISITS TO THE SITE BY THE STRUCTURAL ENGINEER OR THE STRUCTURAL ENGINEER'S REPRESENTATIVE SHALL NOT RELIEVE THE CONTRACTOR OF THIS RESPONSIBILITY.

12) CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY UNUSUAL AND OR EXCESSIVE LOADS DUE TO EQUIPMENT OR CONSTRUCTION REQUIREMENTS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OR ARCHITECT OF ANY LOADS FROM EQUIPMENT HAT ARE DIFFERENT FROM THE DESIGN LOADS SHOWN ON THESE PLANS.

13) THE CONTRACTOR AND ALL SUBCONTRACTORS SHALL CONSTRUCT THIS PROJECT IN ACCORDANCE WITH ALL APPLICABLE BUILDING CODES AND ALL APPLICABLE FEDERAL, STATE AND LOCAL CODES,

14) WORK NOT INDICATED AS PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT AT CORRESPONDING PLACES SHALL BE REPEATED.

15) IN CASE OF CONFLICT BETWEEN THE DRAWINGS AND/OR SPECIFICATIONS, THE MORE RIGID REQUIREMENT SHALL BE ASSUMED TO GOVERN UNTIL A RULING IS MADE BY THE ARCHITECT/

16) THE CONTRACTOR SHALL REFER TO ELECTRICAL, MECHANICAL, ARCHITECTURAL AND OTHER DISCIPLINES DRAWINGS FOR LOCATIONS OF ALL OPENINGS. CONTRACTOR IS RESPONSIBLE TO COORDINATE THESE DRAWINGS WITH THE DRAWINGS OF OTHER DISCIPLINES. THE STRUCTURAL ENGINEER AND ARCHITECT SHALL BE IMMEDIATELY NOTIFIED OF ANY OPENINGS FOUND BY THIS COORDINATION THAT ARE REQUIRED IN THE STRUCTURAL MEMBERS. NO CUTS OR MODIFICATIONS OF ANY MEMBERS SHALL BE MADE THAT ARE NOT APPROVED BY THE STRUCTURAL ENGINEER.

17) THE ENGINEER'S APPROVAL OF SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR DEVIATIONS FROM REQUIREMENTS IN THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS. DEVIATION FROM THE CONTRACT DOCUMENTS SHALL BE SUBMITTED SEPARATELY

18) THE CONTRACTOR SHALL SUBMIT ALL SHOP DRAWINGS IN A TIMELY MANNER, ALLOWING THE ENGINEER AT LEAST TWO WEEKS TO REVIEW THE SHOP DRAWINGS. THE CONTRACTOR SHALL SUBMIT ALL SHOP DRAWINGS IN A DIGITAL PDF FORMAT. ANY ITEMS NOT SPECIFICALLY DESIGNED OR DETAILED ON THESE DESIGN DRAWINGS SHALL BE SUBMITTED WITH CALCULATIONS TO THE STRUCTURAL ENGINEER FOR REVIEW.

19) PRIOR TO SUBMITTING ANY ITEMS FOR APPROVAL, INCLUDING SHOP DRAWINGS, THE CONTRACTOR SHALL REVIEW THE MATERIALS AND COORDINATE ALL TRADES. ALL COORDINATION REQUIREMENTS SHALL BE NOTED ON THE SUBMITTALS. NO SUBMITTALS OR OTHER ITEMS FOR REVIEW SHALL BE FORWARDED TO THE STRUCTURAL ENGINEER OR ARCHITECT UNTIL THEY HAVE BEEN REVIEWED AND APPROVED BY THE CONTRACTOR.

20) SUBMITTALS SHALL NOT BE THE REPRODUCTION OF THE CONTRACT DOCUMENTS.

21) THE CONTRACTOR SHALL NOT PROCEED WITH FABRICATION WITHOUT APPROVED SHOP DRAWINGS.

22) THE CONTRACTOR SHALL VERIFY SIZES AND LOCATIONS OF ALL SLOTS, PIPE SLEEVES, ANCHOR BOLTS, ETC. AS REQUIRED FOR ALL TRADES PRIOR TO CONSTRUCTION.

23) SEE MECHANICAL, PLUMBING, FIRE PROTECTION, HVAC, ELECTRICAL AND OTHER TRADES DRAWINGS FOR ADDITIONAL INFORMATION AFFECTING THE STRUCTURAL WORK, INCLUDING:

A) HANGERS, SUSPENDED PIPING, SUSPENDED EQUIPMENT, SUSPENDED DUCT WORK. B) ELECTRICAL CONDUIT, ELECTRICAL BOXES

C) INSERTS, EMBEDMENTS AND OTHER SUPPORTED EQUIPMENT D) SLAB ON GRADE OR FLOOR EQUIPMENT AND ANCHORS E) UNDERGROUND DUCT, ELECTRICAL TRENCHES, PITS, MANHOLES, PIPING

F) SEISMIC TIES FOR EQUIPMENT REQUIRING ADDITIONAL SEISMIC STABILITY

24) ONCE THE PROJECT IS COMPLETED. IT IS THE OWNER'S RESPONSIBILITY TO PROVIDE THE APPROPRIATE MAINTENANCE TO PROTECT THE STRUCTURAL INTEGRITY OF THE STRUCTURE. AS PART OF THE CONTRACT, THE CONTRACTOR IS REQUIRED TO INFORM THE OWNER(S) OF THIS IN WRITING.

25) THE STRUCTURAL DESIGN OF THIS BUILDING TAKES INTO CONSIDERATION THE ANTICIPATED GRAVITY, LATERAL AND UPLIFT LOADS BASED ON SOUND ENGINEERING JUDGEMENT. THE ENGINEER OF RECORD RESERVES THE RIGHT TO VERIFY AND MODIFY THE STRUCTURE AS NEEDED AS A RESULT OF THESE LOADS IN THE SHOP DRAWING PROCESS.

26) THESE STRUCTURAL PLANS ARE BASED ON THE LATEST INFORMATION PROVIDED TO THE STRUCTURAL ENGINEER PRIOR TO THE DATE ON THESE DRAWINGS. IF THERE IS A CONFLICT BETWEEN THESE DRAWINGS AND ANY OTHER DISCIPLINE'S DRAWINGS OR A CHANGE HAS BEEN MADE TO THIS JOB AFTER THE DATE OF THESE DRAWINGS. THE CONTRACTOR SHALL CONTACT THE ARCHITECT AND THE STRUCTURAL ENGINEER TO INSURE THESE CHANGES ARE INCORPORATED INTO THE STRUCTURAL PLANS.

27) THE ARCHITECT OF RECORD SHALL BE COPIED ON ALL EMAILS.

CONCRETE NOTES:

B) SIDEWALKS: 2500 PSI

STRUCTURAL PLANS.

1) ALL CONCRETE SHALL CONFORM TO ACI 301.

2) ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICALE

3) ALL CONCRETE SHALL HAVE THE FOLLOWING STRENGTHS: A) SLABS AND FOOTINGS: 3000 PSI

4) SUBMIT MIX DESIGN TO ENGINEER FOR APPROVAL.

5) THE CONCRETE SLUMP SHALL FALL WITHIN THE FOLLOWING RANGES: A) FOOTINGS AND SLABS: 4 TO 6 INCHES

B) WALLS: 5 TO 7 INCHES 6) THE CONCRETE AIR ENTRAINMENT SHALL FALL WITHIN THE FOLLOWING RANGES:

A) FOOTINGS AND SLABS: 1 TO 4% B) SIDEWALKS: 5 TO 7%

C) STRUCTURAL CONCRETE SUBJECTED TO FREEZE AND THAW ACTION: 5 TO 7% 7) FLYASH MAY BE USED IN THE CONCRETE MIX. FLYASH SHALL ONLY BE USED AS A 2 TO 1 REPLACEMENT OF CEMENT (2 FLYASH PER 1 POUND CEMENT) UP TO 120 POUNDS OF

8) ALL CONCRETE REINFORCING STEEL TO BE ASTM A615 GRADE 60.

9) ALL WELDED WIRE MESH TO BE ASTM A185 65 KSI.

OTHERWISE ON THE STRUCTURAL PLANS.

10) REBAR SHOP DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH ACI DETAILING MANUAL LATEST EDITION.

 MINIMUM LAP ON ALL REBAR SHALL BE 50 BAR DIAMETERS, UNLESS NOTED OTHERWISE. 12) ALL REINFORCING BARS SHOWN TO BE HOOKED SHALL HAVE A STANDARD HOOK PER CRSI AND ACI STANDARDS, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE

13) CORNER BARS ARE REQUIRED AT ALL CORNERS UNLESS SPECIFICALLY NOTED

14) REINFORCEMENT SHALL BE HELD IN PLACE DURING CONCRETE PLACEMENT. IF REQUIRED, ADDITIONAL BARS SHALL BE PROVIDED BY THE CONTRACTOR TO FURNISH SUPPORT FOR ALL BARS.

15) SUBMIT REBAR SHOP DRAWINGS TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION.

17) THE CONTRACTOR SHALL TAKE THE PRECAUTIONS SPECIFIED BY ACI WHEN PLACING CONCRETE IN HOT OR COLD WEATHER CONDITIONS.

18) NO WATER SHALL BE ADDED TO THE CONCRETE AT THE SITE OR IN ROUTE TO THE SITE. 19) LAP ALL WELDED WIRE FABRIC 12" MINIMUM.

20) PROVIDE REBAR SUPPORTS AND TIES IN THE CONCRETE PER ACI AND CRSI SPECIFICATIONS.

21) REINFORCING SHALL BE INSTALLED IN THE CONCRETE IN ACCORDANCE WITH THE FOLLOWING COVER REQUIREMENTS

A) CONCRETE POURED AGAINST THE GROUND: 3" B) CONCRETE EXPOSED TO THE GROUND OR WEATHER: 2"

16) NO CALCIUM CHLORIDE SHALL BE USED IN THE CONCRETE MIX.

C) CONCRETE NOT EXPOSED TO THE WEATHER OR IN CONTACT WITH THE GROUND: 1) #3 - #11 BARS: 3/4" 2) LARGER THAN #11 BARS: 1 1/2"

D) CONCRETE IN BEAMS OR COLUMNS: 1 1/2"

22) A QUALIFIED TESTING LABORATORY SHALL BE RETAINED BY THE GENERAL CONTRACTOR TO COLLECT CYLINDER AND PERFORM THE NECESSARY CONCRETE TESTS. A MINIMUM OF FOUR CYLINDERS SHALL BE TAKEN FOR EVERY 50 CUBIC YARDS OR PORTION THEREOF OF EACH DAY'S POUR. ONE CYLINDER SHALL BE TESTED AT 7 DAYS. 2 CYLINDERS SHALL BE TESTED AT 28 DAYS AND THE REMAINING CYLINDER SHALL BE HELD IN RESERVE IF NEEDED. ONE COPY OF THE TEST REPORTS SHALL BE SENT TO THE ARCHITECT AND STRUCTURAL ENGINEER. NO ADDITIONAL ELEMENTS SHALL BE ADDED TO THE CONCRETE AFTER THE CONCRETE FOR THE CYLINDERS IS TAKEN.

23) IN ADDITION TO THE CONCRETE CYLINDERS THE TESTING LABORATORY SHALL PERFORM THE FOLLOWING TEST EACH TIME CONCRETE CYLINDERS ARE TAKEN: A) STANDARD SLUMP TEST B) AIR ENTRAINMENT TEST

24) THE CONTRACTOR SHALL REPAIR AND PATCH DEFECTIVE AREAS IMMEDIATELY AFTER REMOVAL OF FORMS.

25) ALL PLUMBING SLOTS SHALL BE FILLED WITH CONCRETE TO THE SAME DEPTH AS THE FLOOR SLAB AFTER PIPING IS INSTALLED.

26) REBAR DOWELS SHALL MATCH VERTICAL REINFORCING, ALL SLAB DOWELS SHALL BE STRAIGHT, SMOOTH AND FREE OF BURRS AT THE ENDS. DOWELS SHALL BE PROPERLY SUPPORTED DURING CONSTRUCTION AND PROPERLY ALIGNED TO KEEP DOWELS PARALLEL TO THE DIRECTION OF EXPECTED MOTION.

27) THE GENERAL CONTRACTOR IS RESPONSIBLE FOR THE PROPER DESIGN OF ALL TEMPORARY FRAMEWORK, FORMWORK AND SHORING.

28) ALL NON-SHRINK GROUT SHALL BE PLACED UNDER COLUMN BASE PLATES ONCE THE STEEL COLUMN IS IN PLACE AND PLUMB.

SHALLOW FOUNDATION NOTES:

C) TEMPERATURE

1) FOUNDATION DESIGN IS BASED ON A SOIL BEARING CAPACITY OF 1500 PSF. CONTRACTOR IS RESPONSIBLE TO INSURE THIS CONDITION EXISTS.

2) PLACE CONCRETE IN FOOTINGS ON SAME DAYS AS FOOTINGS ARE EXCAVATED. IF THIS IS NOT POSSIBLE, CONTRACTOR SHALL PROTECT THE EXCAVATION FROM ANY DISTURBANCE UNTIL THE CONCRETE IS PLACED IN THE FOOTINGS. ANY TESTING OF THE SUBGRADE UNDER THE FOOTINGS SHALL BE MADE ON THE SAME DAY AS THE FOOTINGS ARE POURED.

3) THE CONTRACTOR SHALL OBTAIN A COPY OF THE SOILS REPORT BY WHITAKER LABORATORY, INC., DATED NOVEMBER 19, 2015. THE CONTRACTOR SHALL BE RESPONSIBLE TO FOLLOW ALL RECOMMENDATIONS IN THIS REPORT.

4) REMOVE A MINIMUM OF 24 INCHES OF SOIL BELOW THE GRANULAR AGGREGATE BASE SUBGRADE SPECIFIED BELOW THE SLAB FOR THE ENTIRE BUILDING SITE, INCLUDING FIVE FEET BEYOND THE BUILDING PERIMETER ON ALL SIDES. PROOF-ROLL THIS AREA UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER AND REMOVE AND REPLACE, AS DIRECTED, ANY UNSUITABLE

MATERIAL WITH STRUCTURAL FILL. 5) UNLESS SPECIFIED OTHERWISE IN THE SOILS REPORT, ALL SUBSOILS UNDER FOOTINGS SHALL BE COMPACTED TO A MINIMUM OF 98% OF THE MAXIMUM DENSITY AS MEASURED BY THE STANDARD PROCTOR METHOD (ASTM D-698) AT OPTIMUM MOISTURE CONTENT.

6) ALL COLUMNS AND WALLS SHALL BE CENTERED ON THEIR SUPPORTING FOOTING, UNLESS SPECIFICALLY SHOWN OTHERWISE ON THE STRUCTURAL PLANS.

7) WHERE WALL FOOTINGS ALIGN WITH COLUMN FOOTINGS, THE REINFORCING FOR THE WALL FOOTING SHALL RUN CONTINUOUS THROUGH THE COLUMN FOOTING.

8) ALL REINFORCING IN FOOTINGS SHALL BE EQUALLY SPACED UNLESS SPECIFICALLY SHOWN OTHERWISE ON THE STRUCTURAL PLANS.

WITH SIMPSON SPECIFICATIONS FOR THE SPECIFIC EPOXY USED.

9) ALL REINFORCING STEEL SHALL BE SUPPORTED BY WIRE CHAIRS AND ADDITIONAL STEEL RODS AS NEEDED. DO NOT SUPPORT REINFORCING STEEL ON BRICKS, BLOCKS OR OTHER SOLID ITEMS.

11) HOLE DIAMETER, DEPTH, CLEANING AND INSTALLATION OF EPOXY SHALL BE IN ACCORDANCE

10) SIMPSON SET-XP EPOXY SHALL BE USED TO INSTALL ALL POST-INSTALLED THREADED RODS IN

12) THE ANCHOR ROD MAY BE ADJUSTED DURING THE SPECIFIED GEL TIME, ACCORDING TO SIMPSON. DO NOT ADJUST OR DISRUPT THE THREADED ROD AFTER THIS GEL TIME HAS PASSED.

13) DO NOT INSTALL THE EPOXY IN THE CONCRETE WHEN ENVIRONMENTAL CONDITIONS SPECIFIED BY SIMPSON FOR THE EPOXY CANNOT BE MET.

14) PROVIDE ADEQUATE EXPOSED THREADING OF THE ANCHOR ROD TO PROVIDE FULL NUT ENGAGEMENT, FLUSH WITH THE OUTSIDE FACE. CARE SHOULD BE TAKEN TO INSURE THAT LENGTHS FOR ADDITIONAL PLATE WASHERS FOR OVERSIZED HOLES, SLOTTED HOLES FOR LATERAL LOADING PLATE WASHERS ARE TAKEN INTO ACCOUNT.

MASONRY NOTES:

1) ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH ACI 530.

5) GROUT LIFTS IN VERTICAL CELLS SHALL NOT EXCEED 4'-0".

TIES SHALL BE RJ 7-11 OR EQUIVALENT SEISMIC BRICK TIES.

2) ALL CMU SHALL BE F'M = 1500 LIGHTWEIGHT ASTM C90.

3) MASONRY SHALL BE INSTALLED IN A RUNNING BOND PATTERN, UNLESS SPECIFICALLY

NOTED OTHERWISE ON THE STRUCTURAL OR ARCHITECTURAL PLANS. 4) FILL ALL REINFORCED MASONRY UNITS, ALL UNITS BELOW GRADE, AND ALL UNITS

SUPPORTING SOIL WITH 3000 PSI GROUT PER ASTM C476.

6) MORTAR FOR MASONRY SHALL BE ASTM C270 TYPE "S". MASONRY JOINTS SHALL BE 3/8" THICK FULL BED ON ALL HORIZONTAL AND VERTICAL JOINTS.

7) HORIZONTAL JOINT REINFORCEMENT SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION PER ASTM A153 CLASS B2.

8) HORIZONTAL JOINT REINFORCEMENT SHALL BE HEAVY DUTY LADDER TYPE CERTIFIED FOR USE IN SEISMIC DESIGN CATEGORIES C OR D. HORIZONTAL JOINT REINFORCING SHALL BE INSTALLED AT A MAXIMUM VERTICAL SPACING OF 16 INCHES. PREFABRICATED CORNER AND "T" SECTIONS SHALL BE USED AT WALL CORNERS AND INTERSECTIONS. JOINT REINFORCING SHALL HAVE A MINIMUM LAP OF 12".

9) ALL BRICK TIES SHALL BE SPACED AT 16" O.C. VERTICALLY AND HORIZONTALLY. ALL

10) PROVIDE FACTORY MADE SECTIONS AT CORNERS AND WALL INTERSECTIONS.

11) SEE OTHER DISCIPLINES' DRAWINGS FOR SIZE AND LOCATION OF OPENINGS.

12) SPACE CONTROL JOINTS IN WALL AT 30'-0" ON CENTER. ALIGN WITH CONTROL JOINTS

IN SLAB WHERE POSSIBLE. 13) THE CONTRACTOR SHALL TAKE ADDITIONAL PRECAUTIONS WHEN MASONRY IS TO BE CONSTRUCTED DURING COLD WEATHER (AMBIENT TEMPERATURE BELOW 40 DEGREES FAHRENHEIT). DURING HOT CONDITIONS (ABOVE 90 DEGREES FAHRENHEIT) PRECAUTIONS SHALL BE TAKEN TO MINIMIZE EXCESS HEAT IN THE MASONRY UNITS, WATER AND MORTAR. THE CONTRACTOR SHALL FOLLOW THE RECOMMENDATION

14) PREFORMED BED JOINT REINFORCEMENT SHALL BE USED AT ALL WALL CORNERS AND

PRESCRIBED BY THE PORTLAND CEMENT ASSOCIATION FOR COLD OR HOT WEATHER

15) ALL VERTICAL AND HORIZONTAL REBAR SHALL BE THE SAME SIZE AS SHOWN ON THE

STRUCTURAL PLANS AS THE MAIN WALL REINFORCING. 16) DO NOT LOCATE MORE THAN 1 VERTICAL BAR IN EACH CELL UNLESS SPECIFICALLY

17) PROVIDE REBAR SPACERS IN THE WALL TO SUPPORT AND HOLD THE VERTICAL REBAR

IN THE CENTER OF THE CELL SPACED NOT MORE THAN 48" ON CENTER. 18) PROVIDE (2) CELLS WITH VERTICAL REINFORCING GOR 8" WALLS AND (3) CELLS WITH VERTICAL REINFORCING FOR 12" WALLS AT END OF ALL WALLS. THIS REINFORCING SHALL BE LOCATED

19) PROVIDE REINFORCING IN 2 CELLS EACH SIDE OF ALL OPENING IN THE MASONRY WALL.

WITHIN 16" ON THE END OF THE WALL FOR 8" WALLS AND 24" FOR 12" WALLS.

20) PROVIDE 1 BOND BEAM OVER AND UNDER ALL MASONRY OPENINGS. THESE BOND BEAMS SHALL HAVE A MINIMUM OF 2 HORIZONTAL REBARS. EXTEND THE BOND BEAM 16 INCHES BEYOND THE EDGES OF THE OPENING. HOOK THE HORIZONTAL REINFORCING AROUND THE VERTICAL SIDE BARS OF THE OPENING WITH A STANDARD 180 DEGREE

21) PROVIDE BOND BEAMS IN THE WALL WITH 2 HORIZONTAL BARS SPACED NOT MORE THAN 4'-0" O.C. VERTICALLY.

22) HOOK THE ENDS OF ALL HORIZONTAL REBAR AROUND THE VERTICAL BARS WITH A

DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.

23) DO NOT RUN ANY ELECTRICAL CONDUIT, PLUMBING PIPES OR OTHER TRADE'S MATERIALS IN THE SAME CELLS OR BOND BEAMS AS THE REINFORCING STEEL.

SLAB ON GRADE NOTES:

1) ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 302.1R AND OTHER APPLICABLE ACI CODES.

2) THE SLAB ON GRADE SHALL BE PLACED ON A COMPACTED GRANULAR AGGREGATE BASE MATERIAL FILL HAVING LESS THAN 10% FINES. THIS G.A.B. MATERIAL SHALL BE COMPACTED TO 98% OF THE MAXIMUM DENSITY AS MEASURE BY THE STANDARD PROCTOR METHOD (ASTM D-698) AT OPTIMUM MOISTURE CONTENT. THE 4" AND 6" SLABS SHALL BE PLACED ON 6" OF G.A.B. THE 8" SLAB SHALL BE PLACED ON 8" G.A.B. COMPACTION SHALL BE VERIFY BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO POURING CONCRETE. THE COMPACTION TEST RESULTS, WITH THE SEAL AND SIGNATURE OF THE GEOTECHNICAL ENGINEER, SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.

3) SAW CUTTING OF THE CONTROL JOISTS SHALL BE PERFORMED: 1) BEFORE THE CONCRETE BEGINS TO COOL; 2) AS SOON POSSIBLE TO SAWCUT THE CONCRETE WITHOUT DAMAGING THE FINISH OR DAMAGE OR TEAR THE CONCRETE WITH THE SAW BLADE; 3) BEFORE SHRINKAGE CRACKS BEGIN

4) MAXIMUM SPACING OF SAW-CUT CONTROL JOINTS SHALL BE 15'-0" FOR A 4" SLAB ON GRADE AND 20'-0" FOR A 6" SLAB ON GRADE. DO NOT ALIGN SAW-CUT CONTROL JOINTS WITH THICKENED SLABS THAT RUN PARALLEL TO THE JOINT.

5) ALL SAW-CUT CONTROL JOINTS SHALL SECTION THE SLAB ON GRADE INTO RECTANGULAR SECTIONS. THE WIDTH TO LENGTH RATIO OF THESE SEGMENTS SHALL NOT EXCEED 1.25.

6) ISOLATION JOINTS SHALL BE PLACED AT ALL COLUMNS THAT PENETRATE THE SLAB. THE CORNERS OF THE ISOLATION JOINTS SHALL ALIGN WITH THE SAW-CUT CONTROL JOINTS IN THE SLAB, UNLESS SHOWN OTHERWISE ON THE STRUCTURAL PLANS.

7) INSTALL EXPANSION JOINTS WHERE SHOWN ON THE PLANS.

8) LOCATE CONSTRUCTION JOINTS UNDER WALLS.

9) INSTALL CONSTRUCTION JOINTS TO LIMIT EACH POUR TO NO MORE THAN 7,500 SQUARE FEET. THE LENGTH OF ANY SINGLE POUR SHALL NOT EXCEED 100 FEET. ALLOW A MINIMUM OF 24 HOURS BETWEEN ADJACENT POURS.

10) THE VAPOR BARRIER UNDER THE SLAB ON GRADE SHALL BE A MINIMUM OF 15 MIL. LAP EDGES OF SHEETS A MINIMUM OF 12" AND CLOSE JOINTS WITH TAPE.

11) ALL SOIL DISTURBED BY PLUMBING. ELECTRICAL OR OTHER TRADE INSTALLATION SHALL BE PROPERLY REFILLED IN 6" LIFTS, COMPACTED AND TESTED IN ACCORDANCE WITH SPECIFICATIONS

STEEL JOISTS AND GIRDERS NOTES:

1) ALL STEEL JOIST SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH SJI SPECIFICATIONS.

2) SUBMIT STEEL JOIST SHOP DRAWINGS TO ENGINEER FOR APPROVAL PRIOR TO

3) JOIST AND GIRDER DESIGNERS SHALL VERIFY THE EXACT LOCATION AND WEIGHT OF ALL MECHANICAL EQUIPMENT PRIOR TO SHOP DRAWING SUBMITTAL AND JOIST

4) ALL BOTTOM CHORDS OF JOIST AND JOIST GIRDERS SHALL HAVE BOTTOM CHORD EXTENSIONS FASTENED AFTER APPLICATION OF ALL DEAD LOADS ON THE JOISTS

5) PROVIDE L4X4X3/8 ANGLES SURROUNDING ALL ROOF PENETRATIONS IN THE METAL

6) ALL STEEL JOISTS SHALL HAVE A MINIMUM OF 4" BEARING ON STEEL BEAMS, STAGGER SPACING AS REQUIRED TO PROVIDE BEARING ON BOTH SIDES OF THE BEAM. ALL STEEL JOISTS BEARING ON MASONRY WALL SHALL HAVE A MINIMUM OF 4" BEARING. STEEL JOISTS BEARING ON MASONRY SHALL BEAR ON BEARING PLATES (3/8"X6"X8" WITH 2 3/4" DIAMETER

BY 8" LONG STUDS) UNLESS OTHERWISE NOTED IN PLANS. 7) JOIST DESIGNER/SUPPLIER SHALL VERIFY THE EXACT LOCATION AND WEIGHT OF ALL MECHANICAL EQUIPMENT PRIOR TO SHOP DRAWING SUBMITTAL AND JOIST FABRICATION ALL JOISTS SHALL BE DESIGNED PER THE LOADS SHOWN ON THE PLANS. WITH ANY ADJUSTMENTS NEEDED AFTER MECHANICAM COORDINATION. THE CONTRACTOR SHALL OBTAIN AND SUBMIT CALCULATIONS RELATING TO THE DESIGN OF THE BAR JOISTS AND GIRDERS INCLUDING "SPECIAL JOISTS."

8) CONTRACTOR IS RESPONSIBLE TO COORDINATE THE TRADES (E.G. MECHANICAL, FIRE PROTECTIONS, KITCHEN EQUIPMENT, ETC) WITH THE JOIST MANUFACTURER TO INSURE THE JOIST MANUFACTURER HAS THE PROPER DIMENSIONS, LOCATIONS AND WEIGHTS. BASED ON SUPPLIERS SUPPLIED DATA, PRIOR TO THE DESIGN OF THE JOIST.

GAGE METAL NOTES:

1) METAL DECK FOR USE ON THE ROOF SHALL BE 20 GAGE TYPE "B" VENTED ROOF DECK IN ACCORDANCE WITH THE STEEL DECK INSTITUTE STANDARDS: IP = .205; SP = .227; IN = .213; SN = .238; FY = 33 KSI.

2) THE METAL ROOF DECK SHALL BE GALVANIZED WITH A G90 GALVANIZED FINISH.

3) THE METAL ROOF DECK SHALL BE PAINTED WITH A STANDARD GRAY SHOP PRIMER PAINT.

INSTALL IN A 36/7 PATTERN. 5) ATTACH THE ROOF DECK TO THE LIGHT GAUGE ROOF TRUSSES WITH #12 TEK SCREWS.

4) ATTACH THE ROOF DECK TO THE STEEL JOISTS WITH 5/8" DIAMETER PUDDLE WELD.

INSTALL IN A 36/7 PATTERN.

6) END LAPS SHALL BE A MINIMUM OF 3" AND SHALL OCCUR CENTERED OVER STRUCTURAL

7) SIDE LAPS SHALL BE A MINIMUM OF 1 RIB.

8) PROVIDE (8)#10 SIDELAP SCREWS EQUALLY SPACED BETWEEN STEEL BAR JOIST.

9) PROVIDE (9)#10 SIDELAP SCREWS EQUALLY SPACED BETWEEN LIGHT GAUGE ROOF TRUSSES. 10) THE CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO PREVENT DEFORMATION OF THE METAL DECK DURING CONSTRUCTION.

11) SUBMIT METAL DECK SHOP DRAWINGS TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION. 12) ALL LIGHT GAGE METAL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISI SPECIFICATIONS, LATEST EDITION.

13) ALL METAL 18 GAGE AND LIGHTER SHALL BE FY = 33 KSI MINIMUM.

14) ALL METAL 16 GAGE AND HEAVIER SHALL BE FY = 50 KSI MINIMUM.

15) ALL SCREWS FOR CONNECTIONS SHALL BE #10 TEK.

HEIGHTS, UNLESS NOTED OTHERWISE:

A) 0' TO 8' 1 ROW AT MID HEIGHT

16) ALL P.A.F. INTO CONCRETE SHALL BE RAMSET TE 1" PINS, OR EQUIVALENT.

17) ALL P.A.F. INTO STEEL SHALL BE RAMSET TE (KNURLED) PINS, OR EQUIVALENT. 18) SUBMIT LIGHT GAGE METAL SHOP DRAWINGS TO ENGINEER FOR APPROVAL PRIOR TO

19) ALL LIGHT GAGE FABRICATION AND/OR ERECTION SHALL BE MADE BY PERSONNEL EXPERIENCED IN LIGHT GAGE METAL FABRICATION AND/OR ERECTION.

20) ALL ATTACHMENT SHALL BE BY WELDING OR SCREWS. NO WIRE TYING OF LIGHT GAGE METAL SHALL BE USED. 21) WALL BRIDGING SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING WALL

B) OVER 8' SPACED AT 4'-0" O.C. 22) ALL BRIDGING SHALL BE INSTALLED PRIOR TO ADDING ANY LOADS TO THE LIGHT GAGE

23) ALL STUDS AND JOISTS SHALL BE OF THE SIZE, TYPE, GAGE AND SPACING AS INDICATED IN THE DRAWINGS AND TABLES.

24) THE CONTRACTOR SHALL PROVIDE THE MANUFACTURER'S STANDARD STEEL RUNNERS, TRACKS, BLOCKING, LINTELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS AND ACCESSORIES AS RECOMMENDED BY THE MANUFACTURER FOR THE PARTICULAR APPLICATION TO PROVIDE A COMPLETE STRUCTURAL SYSTEM.

25) THE EXTERIOR WALL SYSTEMS SHALL BE DESIGNED TO WITHSTAND BOTH POSITIVE AND NEGATIVE WIND PRESSURES AS INDICATED IN THE LATEST EDITION OF THE APPLICABLE BUILDING CODE. CARE SHALL BE TAKEN IN THE DESIGN TO CONSIDER DEFLECTIONS OF THE WALL SYSTEMS UNDER LOADING AS IT RELATES TO THE PRESCRIBED DEFLECTION LIMITS AS INDICATED IN THE APPLICABLE BUILDING CODE. LIGHT GAGE SUPPLIER SHALL PROVIDE LIGHT GAGE SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE JURISDICTION OF THE PROJECT.

27) ALL INTERIOR STUD PARTITION WALLS SHALL BE FRAMED IN ACCORDANCE WITH THE LATEST EDITION OF ASTM C754.

ERECTION OF THE STEEL FRAMING SYSTEMS ARE COMPLETE.

26) THE CONTRACTOR SHALL INSTALL SUFFICIENT TEMPORARY BRACING, AS NEEDED, UNTIL

PROFESSIONAL

03/07/19

DESIGNED DRAWN

DEM DATE: 08/02/2019 JOB NO. 119273573 03/07/19

SIUM YMNA TION CO

DRAWING NUMBER

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