

## **TECHNICAL MEMORANDUM**

# Atlanta Beltline Inc. (ABI) Northeast Trail (Task C)

TO: Nathan Currier, P.E., S.E., Senior Structural Engineer Kimley-Horn & Associates (KHA)

**FROM:** Akli Hibouche, PhD and Prashanth Vaddu, P.E.

MC Squared, Inc.

**DATE:** October 22, 2021

## SUBJECT: Addendum to Task C WFI Report for Walls 101A, 101B and 101C

Global and External Stability Analyses

This technical memorandum is in reference to your e-mail dated October 20, 2021, requesting additional evaluation and analyses for Walls 101A thru 101C (Task C) based on updated wall plans and change in wall design (strap length) to overcome ROW issues. This may serve as an addendum to our original Task C WFI report submittal (dated March 30, 2018).

Per your request we completed global stability and external stability analysis based on the following assumptions:

- Maximum wall height for Wall 101C is approximately 15 feet.
- Strap length at Wall 101C is equal to 70% height of the wall (**10 feet strap length** was assumed).
- Used updated wall plans and cross-sections, dated October 2021.
- Shoring will be used to construct the walls. It is assumed that wall excavations are stable and shoring system will be designed by others.
- Wall 101A has been removed and the design has been changed to a 2(H):1(V) slope as shown in the provided cross-sections. It is assumed that the 2(H):1(V) slope is stable.

Based on these assumptions, three (3) critical Wall Sections were selected for geotechnical engineering evaluations; Section 1 (Station 101+04 to 101+30), Section 2 (Station 101+30 to 101+80), and Section 3 (Station 101+80 to 102+15). Evaluations include global stability, sliding, overturning and bearing capacity.

Based on our analysis and the updated assumptions stated for this new model, the results satisfy the minimum required 1.5 factor of safety for global stability, in accordance with AASHTO Standard Specifications for Highway Bridges, 17th edition, 2002.

External stability analyses including sliding, overturning and bearing capacity were assessed for the new strap length for Wall 101C. These analyses were performed in accordance with AASHTO Standard Specifications for Highway Bridges, 17th edition, 2002. The results satisfy the minimum required factors of safety of 1.5, 2 and 3 for sliding, overturning and bearing capacity, respectively.



Internal stability calculations for MSE walls must be performed by the specialty wall designer in accordance with applicable standards. Internal stability analyses are not a part of our scope of services due to the proprietary nature of the wall system.

#### Attachments:

Global Stability Analyses Results Client Provided Documents

Copies to: KHA: Sean Johnston, P.E., Vice President. sean.johnston@kimley-horn.com

Project Folder: A051707.058









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- 1. STATIONS ARE MEASURED ALONG € CONSTRUCTION. OFFSETS ARE MEASURED TO FRONT FACE OF WALL.
- 2. LENGTHS ARE MEASURED ALONG FRONT FACE OF WALL AND INCLUDE GRANITE VENEER.
- 3. PROVIDE TEMPORARY SHORING AS NECESSARY FOR WALL
- 4. FOR WALL NOTES, DESIGN DATA, AND SUMMARY OF QUANTITIES, SEE "GENERAL NOTES".
- 5. FOR TYPICAL SECTIONS AND DETAILS, SEE "WALL DETAILS".
- 6. ELEVATIONS SHOWN ARE AT THE TOP OF CAPSTONE AT THE FRONT FACE OF WALL.
- 7. GRANITE VENEER NOT SHOWN FOR CLARITY, SEE 38
- 8. EJ INDICATES EXPANSION JOINT IN WALL AND VENEER.
- 9. JOINT SPACINGS SHOWN ARE APPROXIMATE. EXACT LOCATIONS TO BE COORDINATED WITH STONE VENEER JOINT SPACING BY THE CONTRACTOR.
- 10. ™ INDICATES 42" METAL SAFETY RAIL.

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