

Retaining Wall Foundation Investigation Report

**Atlanta Beltline Inc. (ABI) Northeast Trail (Task C)
MC² Project No. A051707.058
Atlanta, Fulton County, Georgia**

Prepared For: Mr. Sean Johnston, P.E.
Vice President
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Prepared By: MC Squared, Inc.
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Project No.: A051707.058
Prepared: March 2018





March 30, 2018

Mr. Sean Johnston, P.E.
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Atlanta, GA 30308

**Subject: Retaining Wall Foundation Investigation Report
Atlanta Beltline Inc. (ABI) Northeast Trail (Task C)
MC² Project No. A051707.058
Atlanta, Fulton County, Georgia**

Dear Mr. Johnston:

MC Squared, Inc. (MC²) is pleased to present this Wall Foundation Investigation (WFI) Report for the proposed ABI Northeast Trail (Task C) in Atlanta, Fulton County, Georgia. This WFI was performed in general accordance with the latest GDOT guidance documents for WFI. The report summarizes our findings, the subsurface conditions we encountered and our conclusions and recommendations as they relate to the project design and construction.

Thank you for giving us the opportunity to work with **Kimley-Horn & Associates** for ABI. Please let us know if you have any comments or require additional information.

Respectfully submitted,
MC²

Amir Moussly
Staff Engineer

Akli Hibouche, Ph.D.
Staff Geotechnical Engineer

Prashanth Vaddu, P.E.
Project Manager
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Sam Moussly
CEO

Attachments:

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RETAINING WALL FOUNDATION REPORT
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1. Location

This report is for the design of six Soldier Pile walls (Wall No. 100B, 101D, 113A, 113B, 114A and 114B) and five Mechanically Stabilized Earth (MSE) walls (Wall No. 98, 99, 101A, 101B and 101D) for the construction of ABI Northeast Trail (Task C) from Westminster Drive Northeast to approximately 250 feet north of the Clear Creek pedestrian bridge. The trail improvement project begins at Station 93+19 and continues north to approximately Station 115+00 in Atlanta, Fulton County, Georgia. Information about the proposed walls is tabulated below:

Wall ID	Station to Station	Approx. Length (ft.)	Assumed Max Height (ft.)	Wall Type
Wall 98	98+16, 9' LT to 100+22, 9' LT	205	12	MSE
Wall 99	99+68, 13' RT to 100+41, 129' RT	168	16	MSE
Wall 100A	100+18, 28' RT to 100+38, 50' RT	-	<2.4	-
Wall 100B	100+40, 44' RT to 100+33, 18' RT	37	13	SOLDIER PILE
Wall 100C	100+03, 33' RT to 100+36, 62' RT	-	<2.4	-
Wall 100D	100+18, 28' RT to 101+14, 9' RT	-	<2.4	CIP CANTILEVER
Wall 101A	100+85, 9' LT to 103+75, 9' LT	289	12	MSE
Wall 101B	101+30, 9' RT to 102+51, 9' RT	122	10	MSE
Wall 101C	101+04, 23' RT to 102+16, 17' RT	113	16	MSE
Wall 101D	101+14, 9' RT to 101+20, 19' RT	16	8	SOLDIER PILE
Wall 113A	112+77, 9' LT to 113+00, 9' LT	23	10	SOLDIER PILE
Wall 113B	112+77, 9' RT to 113+00, 9' RT	23	10	SOLDIER PILE
Wall 114A	114+29, 9' LT to 115+01, 9' LT	73	11	SOLDIER PILE
Wall 114B	114+29, 9' RT to 115+01, 9' RT	71	11	SOLDIER PILE

Note: Wall details obtained from plans provided by KHA dated December 20, 2017.

Based on information provided by **KHA**, walls 100A, 100C and 100D are relatively short (29 inches or shorter) stairway walls. Accordingly, these walls have not been evaluated in this report.

2. Geology

This project will be geologically sited in a Biotitic Gneiss Undifferentiated rock type in the Georgia Piedmont Region.

3. Subsurface and Groundwater Information

Subsurface information for all walls were obtained from thirteen (13) Standard Penetration Test (SPT) Borings, as generally summarized in the table below:

Wall ID	Boring ID	Station / Offset	Existing Ground Elevation (ft.)	Boring Termination Elevation (ft.)	GWT Depth (ft.)	Soils Description
98	TC-WB-01	98+79, 7' LT	840.3	810.3	20	Track ballast to 2'. Alternating layers of firm to stiff CL and very loose to medium dense SM to 30'.
98	TC-WB-02	99+54, 10' LT	839.4	824.4	GNE	Track ballast with pieces of brick to 2'. Loose SC with gravel to 6'. Medium dense to very dense SM to 15'. Auger refusal at 15'.
99	TC-WB-03	99+60, 34' RT	840.1	811.6	19	Loose to medium dense SM with gravel to 8'. Stiff CL to 13.5'. Medium dense to very dense SM to 28.5'.
99	TC-WB-04	99+89, 111' RT	840.1	810.1	19	Track ballast to 2'. Loose to medium dense SM to 13.5'. Firm CL to 23.5'. PWR and very dense SM to 30'.
100B and 100D	TC-WB-05	100+04, 59' RT	840.2	815.2	13.5	Loose to very dense SM with gravel to 25'. Auger refusal at 25'.
100B and 100D	TC-WB-06	99+71, 64' RT	840.2	811.7	19	Loose to medium dense SM/SC to 13.5'. Stiff to very hard CL to 23.5'. Dense to very dense SM to 28.5'.

Wall ID	Boring ID	Station / Offset	Existing Ground Elevation (ft.)	Boring Termination Elevation (ft.)	GWT Depth (ft.)	Soils Description
100B and 100D	TC-WB-07	101+23, 14' RT	859.8	851.8	GNE	(Hand auger) SC to 1.5'. SM to 8'.
101A, 101B, 101C and 101D	TC-WB-08	101+99, 9' LT	838.7	826.2	9.5	Track ballast to 2'. Stiff CL to 4'. Firm ML to 8'. Very dense SM to 12.5'. Auger refusal at 12.5'.
101A, 101B, 101C and 101D	TC-WB-09	102+40, 13' RT	840.0	834.0	GNE	(Hand auger) SM to 1'. SC to 5'. SP-SM to 6'.
101A, 101B, 101C and 101D	TC-WB-10	102+99, 13' LT	837.4	824.4	GNE	Track ballast to 2'. Medium dense to very dense SM with gravel to 13'. Auger refusal at 13'.
101A, 101B, 101C and 101D	TC-WB-11	103+70, 10' LT	836.3	811.3	22	Track ballast to 2'. Firm CL to 4'. Firm to stiff ML to 8'. Loose SC to 10'. Hard CL to 18.5'. Medium dense to very dense SM to 25'. Auger refusal at 25'.
Wall 113A and 113B	TC-WB-12	112+66, 11' RT	836.3	821.3	GNE	Track ballast to 2'. Firm to stiff ML to 8'. Medium dense to very dense SM to 15'. Auger refusal at 15'.
Wall 114A and 114B	TC-WB-13	114+32, 11' RT	837.5	817.5	GNE	Track ballast to 4'. Medium dense SM to 6'. Stiff ML to 13.5'. Medium dense SM to 20'.

Refer to individual soil profiles (**Appendix I**) for details.

All surface waters should be properly drained away from walls during and post construction.

4. Pile Elevations Pile elevations for Soldier Pile walls have been summarized in the table below.

Wall ID	Assumed Max Height (ft.)	Pile Type	Estimated Embedment Depth (ft.)	Lateral Deflection at Top of Pile (in.)	Estimated Minimum Tip Elevation (ft.)
Wall 100B	12	HP 12x53	15	2.65	833.0
Wall 101D	8	HP 12x53	11	1.05	833.0
Wall 113A	10	HP 12x53	13	1.32	814.0
Wall 113B	10				
Wall 114A	11	HP 12x53	19	2.30	807.0
Wall 114B	11				

5. Soil Parameters

The following soil design parameters are recommended for use for the proposed MSE walls:

Cohesion	C= 0 psf
Soil Unit Weight,	γ = 120 pcf
Angle of Internal Friction	ϕ = 30 °
Coefficient of Sliding Friction	μ = 0.4

6. Slopes

Maximum 2:1 slopes will be safe for this project. Shoring may be required to construct the leveling pads at Walls 101B and 101C if the excavations cannot be performed safely.

7. Evaluations

MSE Walls

The maximum allowable soil bearing pressure is 2000 psf. Should the design pressure exceed the maximum allowable soil bearing pressure, a waiting period may be required to accommodate for consolidation of soils. Waiting periods will depend on structural settlement tolerances including any planned wall facing. Settlement monitoring points should be established for every 100 linear feet of wall length. Frequent settlement measurements should be taken until most of the undesired long-term settlement has occurred. Once the estimated future settlement is within the design tolerance limits, wall construction may be completed. Refer to the table summarizing our estimates in the Analyses and Assumptions section of this report.

All surface waters should be properly drained away from walls during and post construction.

Soldier Pile Walls

Very dense (auger refusal) material was encountered above the estimated minimum tip elevations at Walls 113A and 113B. Pre-drilling and/or pilot holes may be required to achieve the estimated minimum tip elevations to help advance piles through these dense layers. This elevation represents an embedment of 12 feet into very dense material and may be adjusted by the Engineer during construction. Temporary casing may also be required to prevent the collapse of the pilot holes prior to driving the piles. If pilot holes are used, a maximum pilot hole diameter of 24 inches is recommended.

8. Analyses and Assumptions

Analyses – MSE Walls

Stability against possible failure by bearing, sliding and overturning, as well as overall stability analyses were assessed for the proposed MSE Walls 98, 99, 101A, 101B and 101C using the “Preliminary Layout” dated January 2018 provided by **KHA**. Global stability was completed using GeoStudio Slope/W software, 2012. Potential failure surfaces were computed using limiting equilibrium method of analysis developed by Spencer. Stability analyses against bearing, sliding and overturning were completed in accordance with AASHTO Standard Specifications for Highway Bridges, 17th edition, 2002. Walls were analyzed using ASD methodology.

For MSE walls, strap lengths were assumed to be equal to 100% of the wall height, where failure slip surface initiation points are assumed to start past the strap length.

For Walls 101A, 101B and 101 C, four (4) different sections were analyzed for the worst case. See **Appendix III** for more details. Additionally, Walls 101B and 101C are assumed to be built on 2.5:1 slopes. Fill soil design parameters used in analysis can be found in Section 5 of this report. Where applicable, a live load of 250 psf was used to account for maintenance vehicles accessing the future trail. Based on our analysis, the minimum required (per AASHTO 2002) factors of safety, 1.5 (sliding and global stability), 2.0 (overturning in soil) and 3.0 (bearing capacity in soil) are achieved, as summarized in the tables below.

Wall ID	Assumed Max Height (ft.)	Wall Type	Global Stability (FS>1.5)	Estimated Settlement (in.)	Sliding (FS>=1.5)	Overturning (FS>=2)	Bearing Capacity (FS>=3)
Wall 98	12	MSE	2.1	1.5	2.0	9.0	5.0

Wall ID	Assumed Max Height (ft.)	Wall Type	Global Stability (FS>1.5)	Estimated Settlement (in.)	Sliding (FS>=1.5)	Overturning (FS>=2)	Bearing Capacity (FS>=3)
Wall 99	16	MSE	1.9	3.4 (2-in. immediate; 1.4-in. long term)	2.0	9.0	7.4
Wall 101A	12	MSE	SEE TABLE BELOW	SEE TABLE BELOW	SEE TABLE BELOW	SEE TABLE BELOW	SEE TABLE BELOW
Wall 101B	10	MSE					
Wall 101C	15	MSE					

Wall ID	Section	Station to Station	Global Stability (FS>1.5)	Settlement (in.)	Sliding (FS>=1.5)	Overturning (FS>=2)	Bearing Capacity (FS>=3)
Walls 101A + 101B + 101C	1	101+04 to 101+30	1.6	1.6	2.2	9	3.6
	2	101+30 to 101+64	1.9	1.6			
	3	101+64 to 102+15	1.6	1.6			
	4	102+15 to 102+51	2.6	0.7			

Analyses – Soldier Pile Walls

Lateral stability and global stability analyses were assessed for the proposed soldier pile retaining wall using the Preliminary Wall Layout dated May 2017. Global stability was completed using GeoStudio Slope/W software, 2012. Potential failure surfaces are computed using limiting equilibrium method of analysis developed by Spencer. Lateral stability was completed according to section 5.6 of AASHTO 17th edition, 2002. Earth pressures supported by each pile for lateral stability analysis were calculated based on spacing between piles of 5 feet. Soldier Pile wall results are tabulated below.

Wall ID	Assumed Max Height (ft.)	Pile Type	Estimated Embedment Depth (ft.)	Lateral Deflection at Top of Pile (in.)	Estimated Minimum Tip Elevation (ft.)	Global Stability (FS>1.5)
Wall 100B	12	HP 12x53	15	2.65	833.0	2.86
Wall 101D	8	HP 12x53	11	1.05	833.0	3.81
Wall 113A	10	HP 12x53	13	1.32	814.0	2.49

Wall ID	Assumed Max Height (ft.)	Pile Type	Estimated Embedment Depth (ft.)	Lateral Deflection at Top of Pile (in.)	Estimated Minimum Tip Elevation (ft.)	Global Stability (FS>1.5)
Wall 113B	10					
Wall 114A	11	HP 12x53	19	2.30	807.0	2.17
Wall 114B	11					

Pile type HP 12x53, with a spacing of 5 feet, was assumed based on information provided by KHA during a phone call on February 21, 2018. Where applicable, a live load of 250 psf was used to account for maintenance vehicles accessing the future trail. Walls 100B and 101D are assumed to be built on a 2.5:1 slope. Based on our analysis, the minimum required (per AASHTO 2002) factor of safety of 1.5 for global stability is achieved. Walls were analyzed using ASD methodology. Refer to Appendix IV for lateral stability analyses results.

Assumptions

MSE Wall 98

Wall Height: 12 ft.

Strap Length: 12 ft.

Section Analyzed: Station 100+21

MSE Wall 99

Wall Height: 16 ft.

Strap Length: 16 ft.

Section Analyzed: Station 100+14

Soldier Pile Wall 100B

Wall Height: 12 ft.

Spacing: 5 ft.

Section Analyzed: Station 100+33

MSE Wall 101A

Wall Height: 12 ft.

Strap Length: 12 ft.

Sections Analyzed: Various (see table under Analyses – MSE Walls in this section of the report)

MSE Wall 101B

Wall Height: 10 ft.

Strap Length: 10 ft.

Sections Analyzed: Various (see table under Analyses – MSE Walls in this section of the report)

MSE Wall 101C

Wall Height: 15 ft.

Strap Length: 15 ft.

Sections Analyzed: Various (see table under Analyses – MSE Walls in this section of the report)

Soldier Pile Wall 101D

Wall Height: 8 ft.

Spacing: 5 ft.

Section Analyzed: Station 101+19

Soldier Pile Walls 113A and 113B

Wall Height: 10 ft.

Spacing: 5 ft.

Section Analyzed: Station 112+92

Soldier Pile Walls 114A and 114B

Wall Height: 11 ft.

Spacing: 5 ft.

Section Analyzed: Station 114+83

Applicable Design Standard/Guidance: AASHTO Standard Specifications for Highway Bridges, 17th edition, 2002.

9. Special Conditions

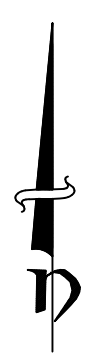
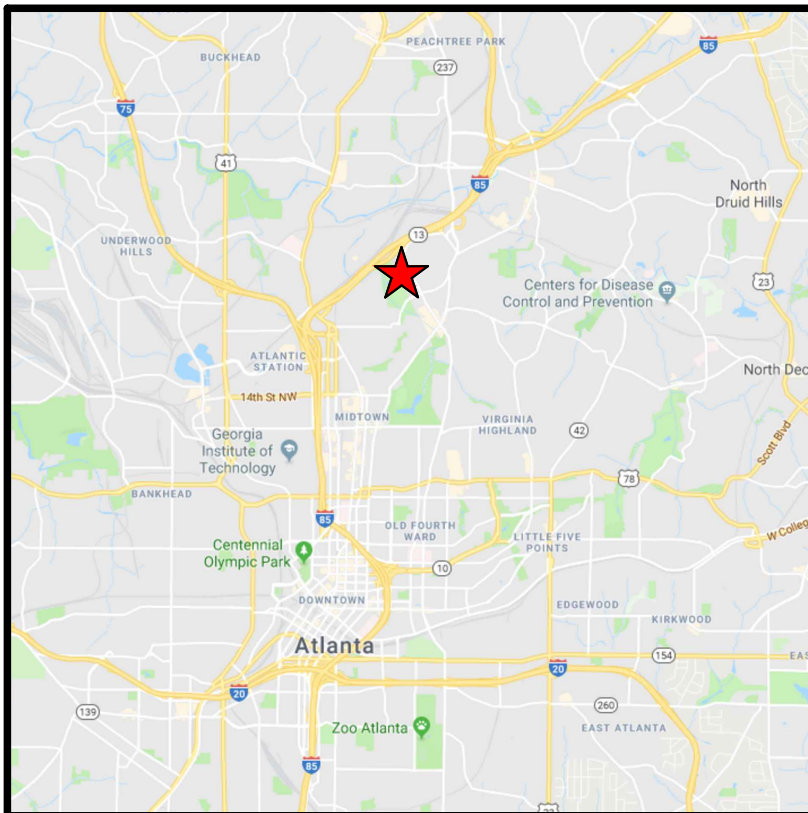
- A. Numerous structures are located very close to the construction limits of this project. Vibration monitoring will be required due to vibrations from the construction activities which may cause some concern with property owners. GDOT Special Provision (SP) 154 may be used as a guideline to determine location of seismographs, crack gauges, etc. Pre- and post-construction crack survey reports recording observations of structural distresses shall be completed.

Prepared By: Amir Moussly
Akli Hibouche, Ph.D.

Reviewed By: Prashanth Vaddu, P.E.
Nicholas Diorio, P.E.

FIGURES

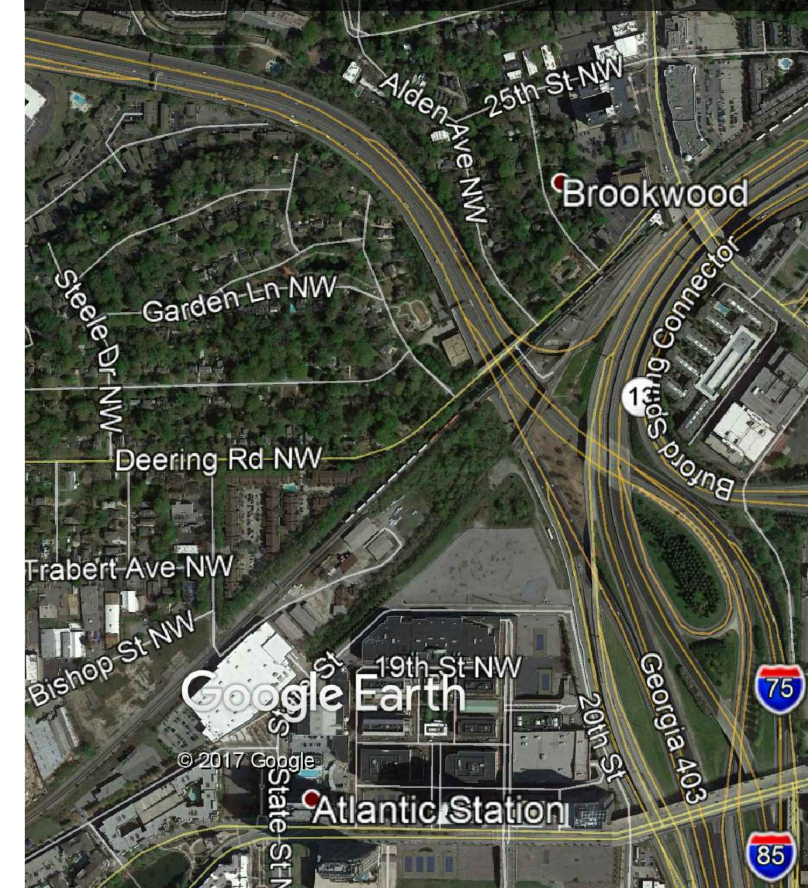
- Project Location Map
- Boring Location Plan



Approximate
Project Alignment

Task A

Task C



LEGEND:
★ Project Location



DATE	NAME	REVISION	APPROVED BY:	NAME	DATE	Project Location Map ABI NE Trail Atlanta, Fulton County, Georgia	MC ² PROJ. NO. A051707.058	SHEET NO. 1
				DESIGNED BY:	TC 11/16/2017			
				DRAWN BY:	TC 12/12/2017			
				CHECKED BY:	JJ 12/15/2017			
				SUPERVISED BY:	PV			

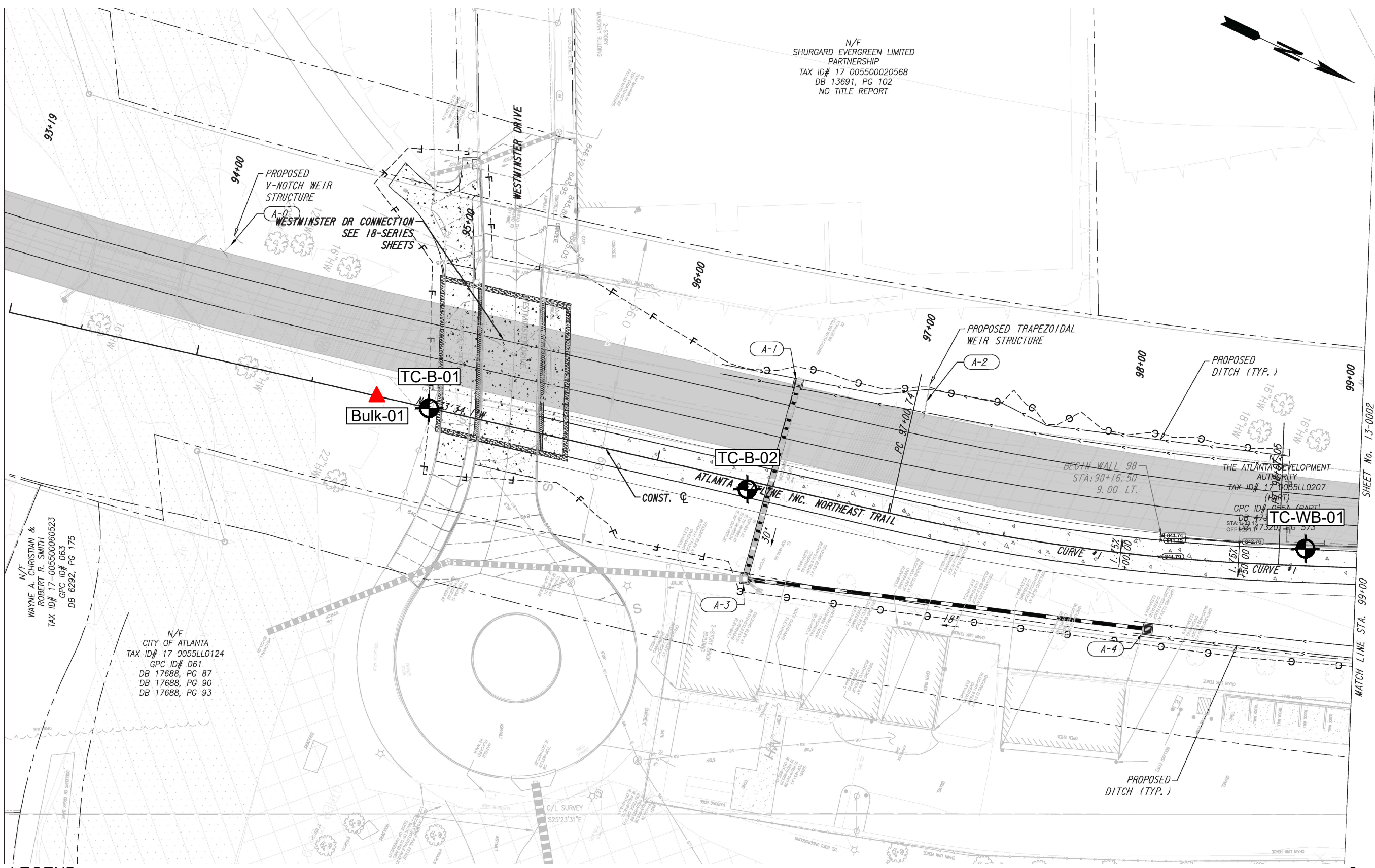


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GEORGIA ENGINEERING CERTIFICATE OF AUTHORIZATION No. PEF00482
Prashanth Vaddu, P.E.
GEORGIA LICENSE No. PE039820

NAME DATE
DESIGNED BY: TC 11/16/2017
DRAWN BY: TC 12/12/2017
CHECKED BY: JJ 12/15/2017
SUPERVISED BY: PV

N/F
SHURGARD EVERGREEN LIMITED
PARTNERSHIP
TAX ID# 17 005500020568
DB 13691, PG 102
NO TITLE REPORT



N/F
WAYNE A. CHRISTIAN &
ROBERT R. SMITH
TAX ID# 17-005500060523
GPC ID# 063
DB 6292, PG 175

N/F
CITY OF ATLANTA
TAX ID# 17 0055LL0124
GPC ID# 061
DB 17688, PG 87
DB 17688, PG 90
DB 17688, PG 93

THE ATLANTA DEVELOPMENT
AUTHORITY
TAX ID# 17 0035LL0207
(PART)
GPC ID# 065A (PART)
DB 473
STA: 150+17
OFF: 177320, PG 573

SHEET No. 13-0002
MATCH LINE STA. 99+00

LEGEND:

- Approximate Boring Location
- Approximate Bulk Sample Retrieval Location
- Approximate Infiltration Test Location

Source: Kimley-Horn
& Associates
Date: 11/1/2017

DATE	NAME	REVISION	APPROVED BY:



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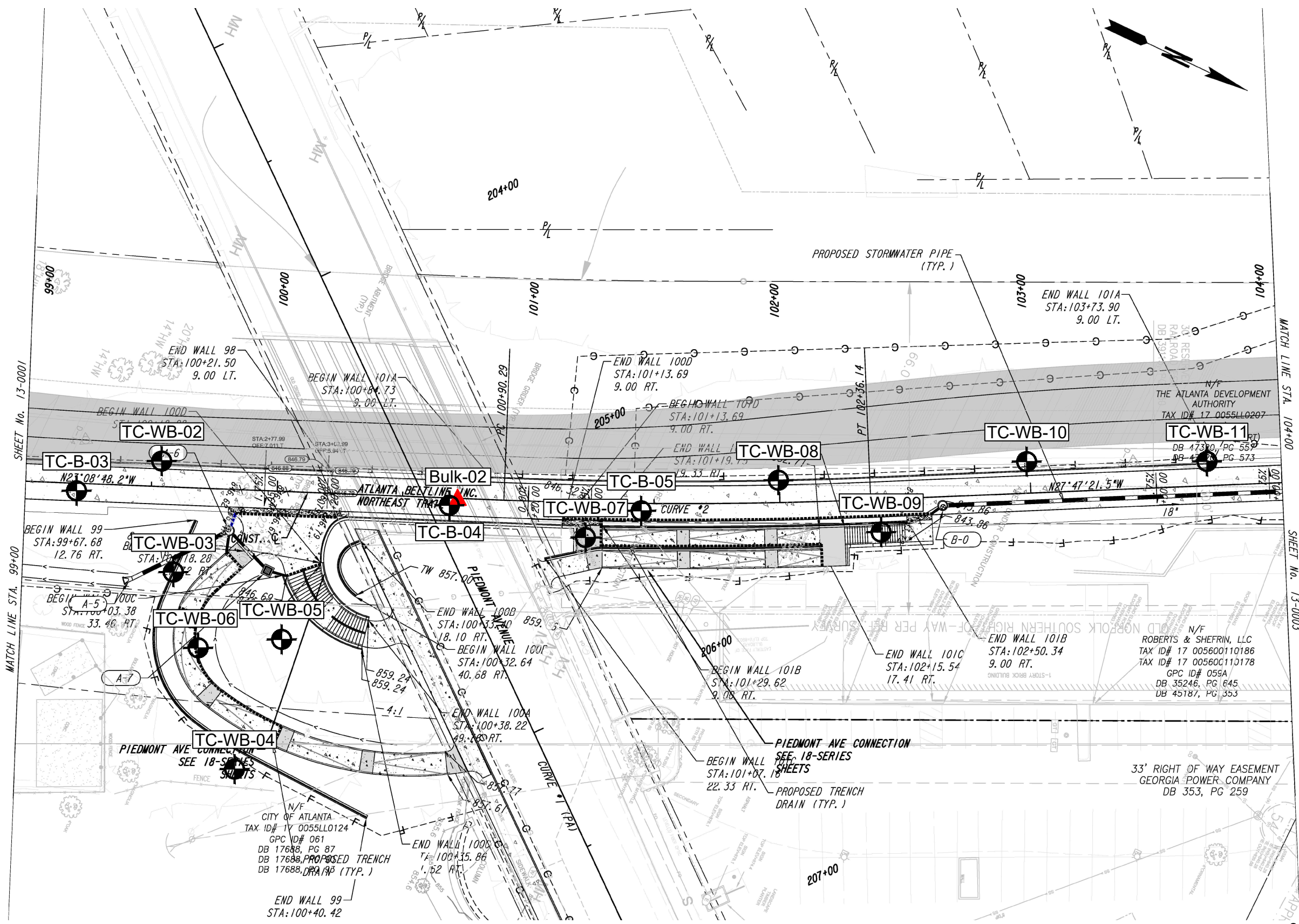
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NAME	DATE
DESIGNED BY: TC	11/16/2017
DRAWN BY: TC	12/12/2017
CHECKED BY: JJ	12/15/2017
SUPERVISED BY: PV	

Boring Location Plan

ABI NE Trail (Task C)
Atlanta, Fulton County, Georgia

MC² PROJ. NO.	SHEET NO.
A051707.058	2



LEGEND:

-  Approximate Boring Location
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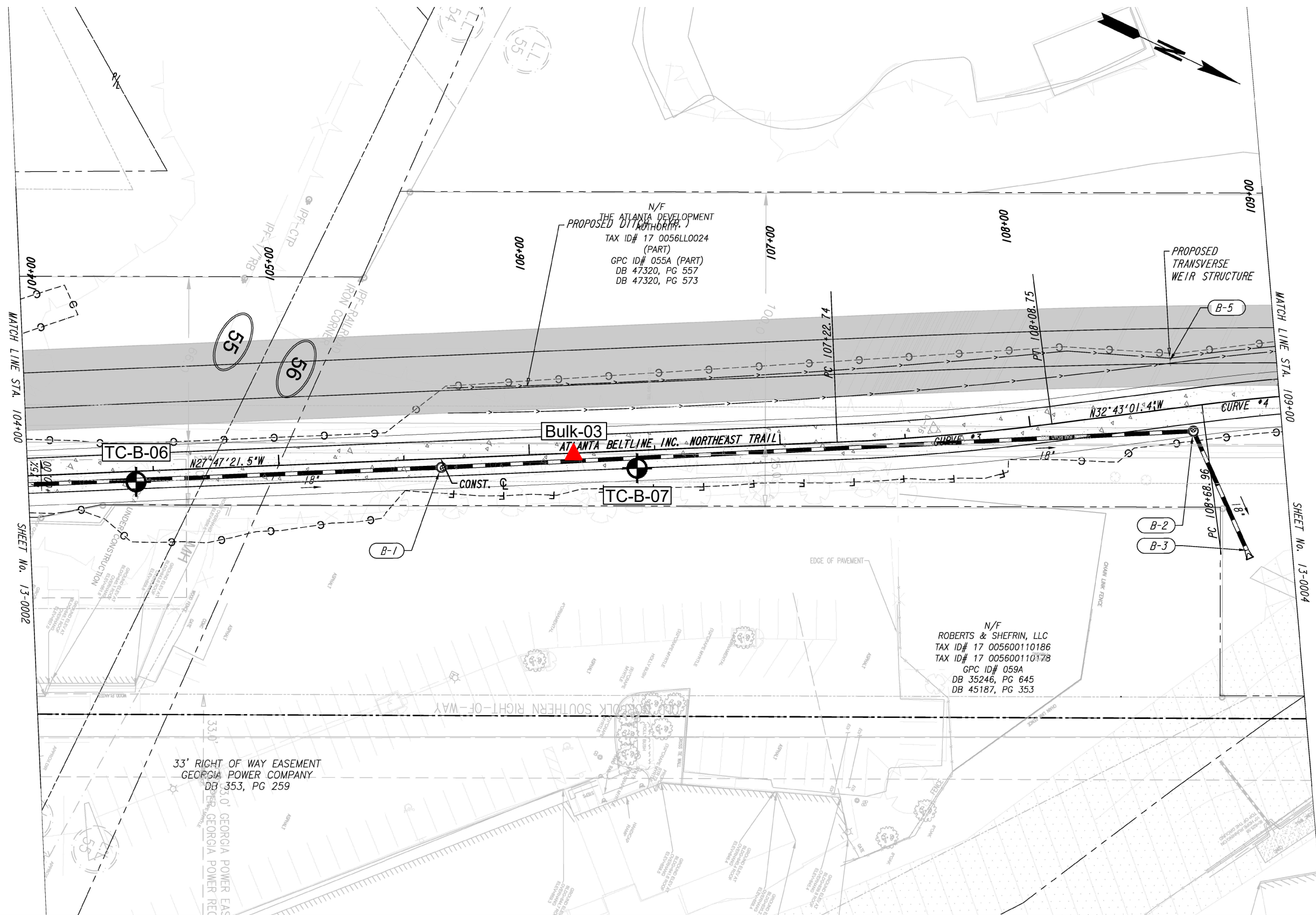
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Boring Location Plan

ABI NE Trail (Task C)
Atlanta, Fulton County, Georgia

MC² PROJ. NO.	SHEET NO.
A051707.058	3



LEGEND:

- Approximate Boring Location
- Approximate Bulk Sample Retrieval Location
- Approximate Infiltration Test Location

Source: Kimley-Horn & Associates
Date: 11/1/2017

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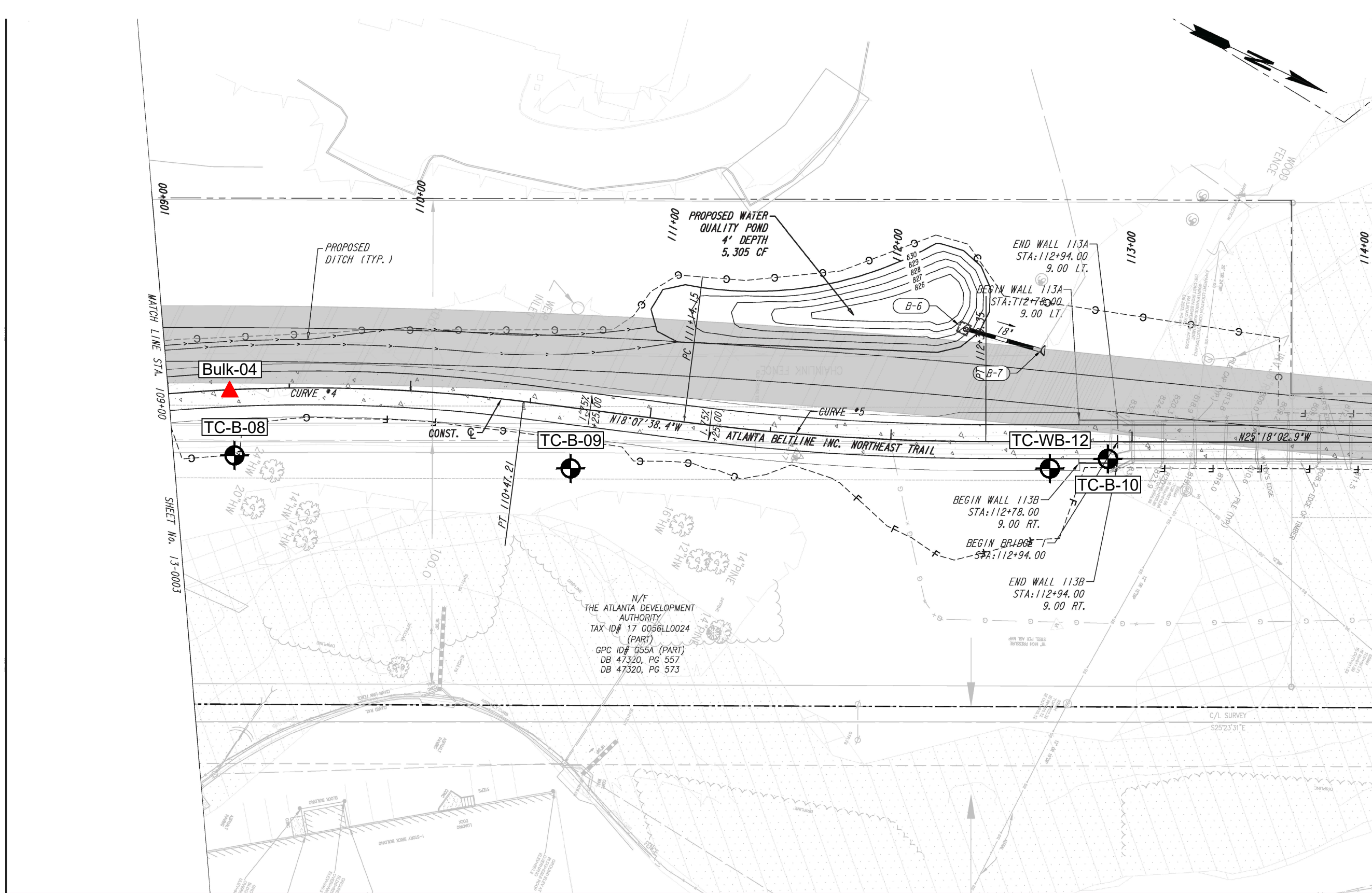
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NAME	DATE
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DRAWN BY: TC	12/12/2017
CHECKED BY: JJ	12/15/2017
SUPERVISED BY: PV	

Boring Location Plan

ABI NE Trail (Task C)
Atlanta, Fulton County, Georgia


MC² PROJ. NO.	SHEET NO.
A051707.058	4



SHEET No. 13-0005

MATCH LINE STA. 114+00

LEGEND:

-  Approximate Boring Location
-  Approximate Bulk Sample Retrieval Location
-  Approximate Infiltration Test Location

Source: Kimley-Horn & Associates
Date: 11/1/2017

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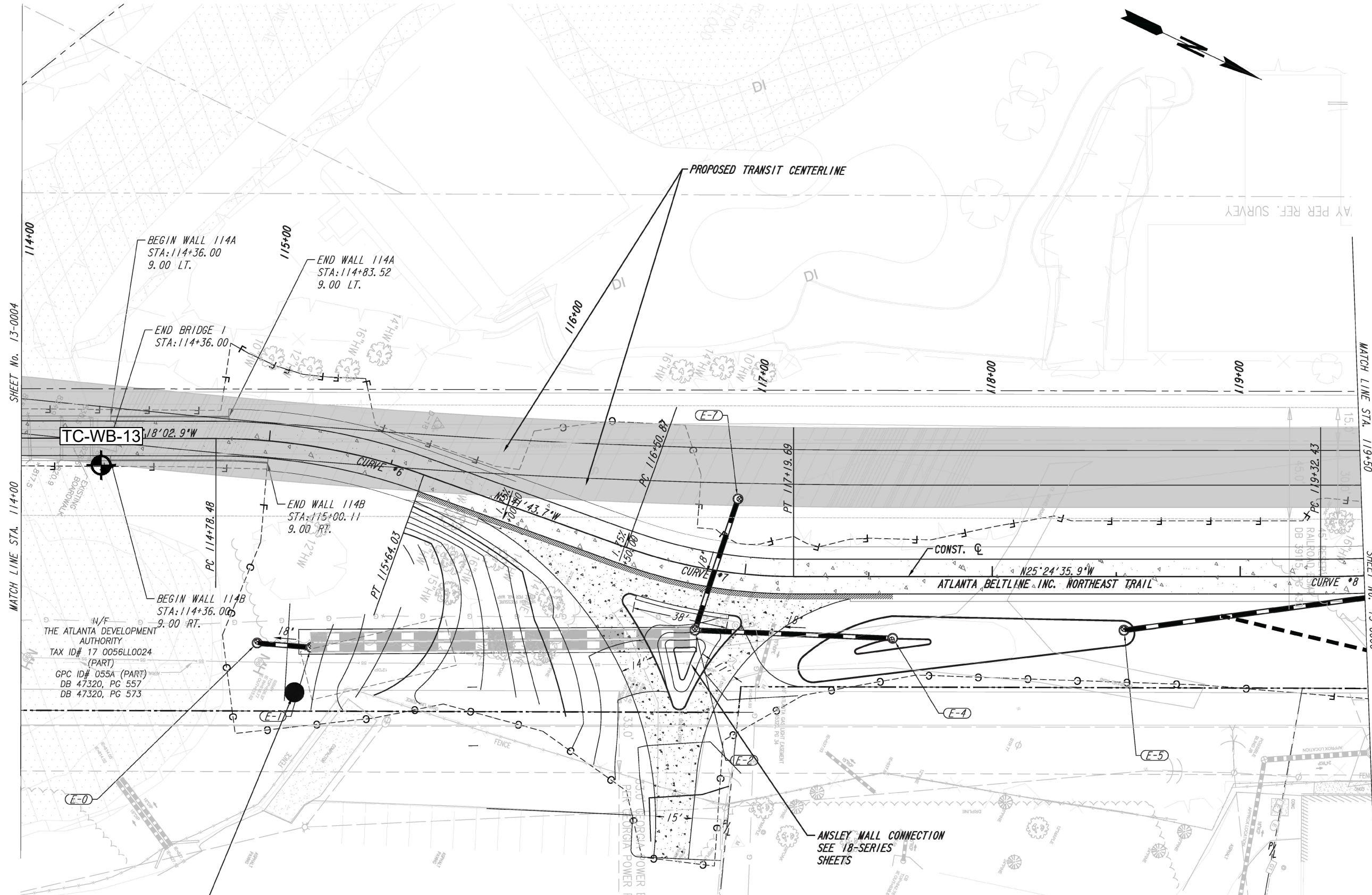
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SUPERVISED BY: PV	

Boring Location Plan

ABI NE Trail (Task C)
Atlanta, Fulton County, Georgia

MC ² PROJ. NO.	SHEET NO.
A051707.058	5



SHEET No. 13-0004

MATCH LINE STA. 114+00

MATCH LINE STA. 119+50

SHEET No. 13-0006

TC-WB-13 18'02.9"W

N/F
THE ATLANTA DEVELOPMENT
AUTHORITY
TAX ID# 17 0056LLO024
(PART)
GPC ID# 055A (PART)
DB 47320, PG 557
DB 47320, PG 573

N25°24'35.9"W
ATLANTA BELTLINE INC. NORTHEAST TRAIL

ANSLEY MALL CONNECTION
SEE 18-SERIES
SHEETS

LEGEND:

- Approximate Boring Location
- Approximate Bulk Sample Retrieval Location
- Approximate Infiltration Test Location

Source: Kimley-Horn
& Associates
Date: 11/1/2017

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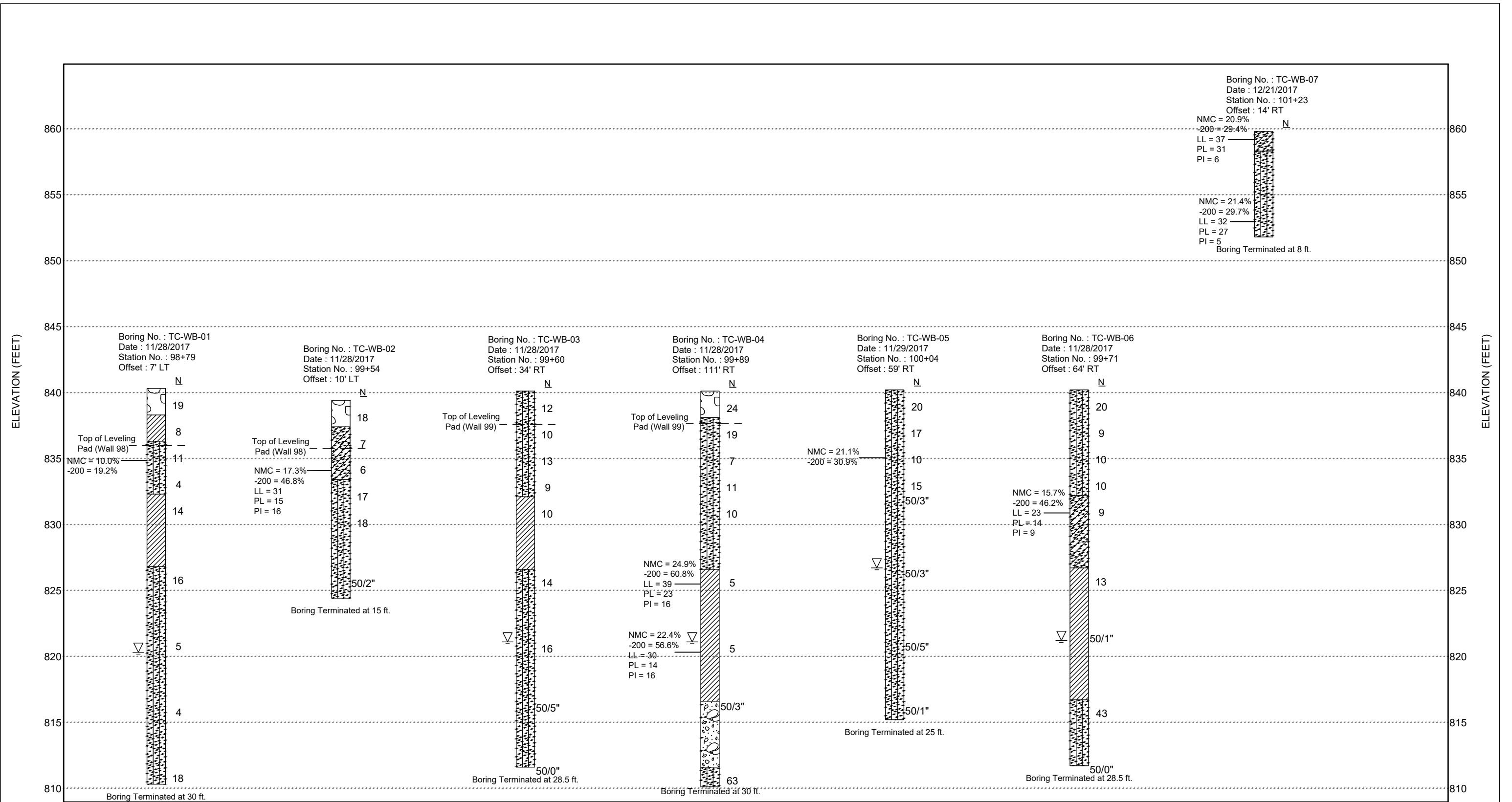
Boring Location Plan

ABI NE Trail (Task C)
Atlanta, Fulton County, Georgia


MC ² PROJ. NO. A051707.058	SHEET NO. 6
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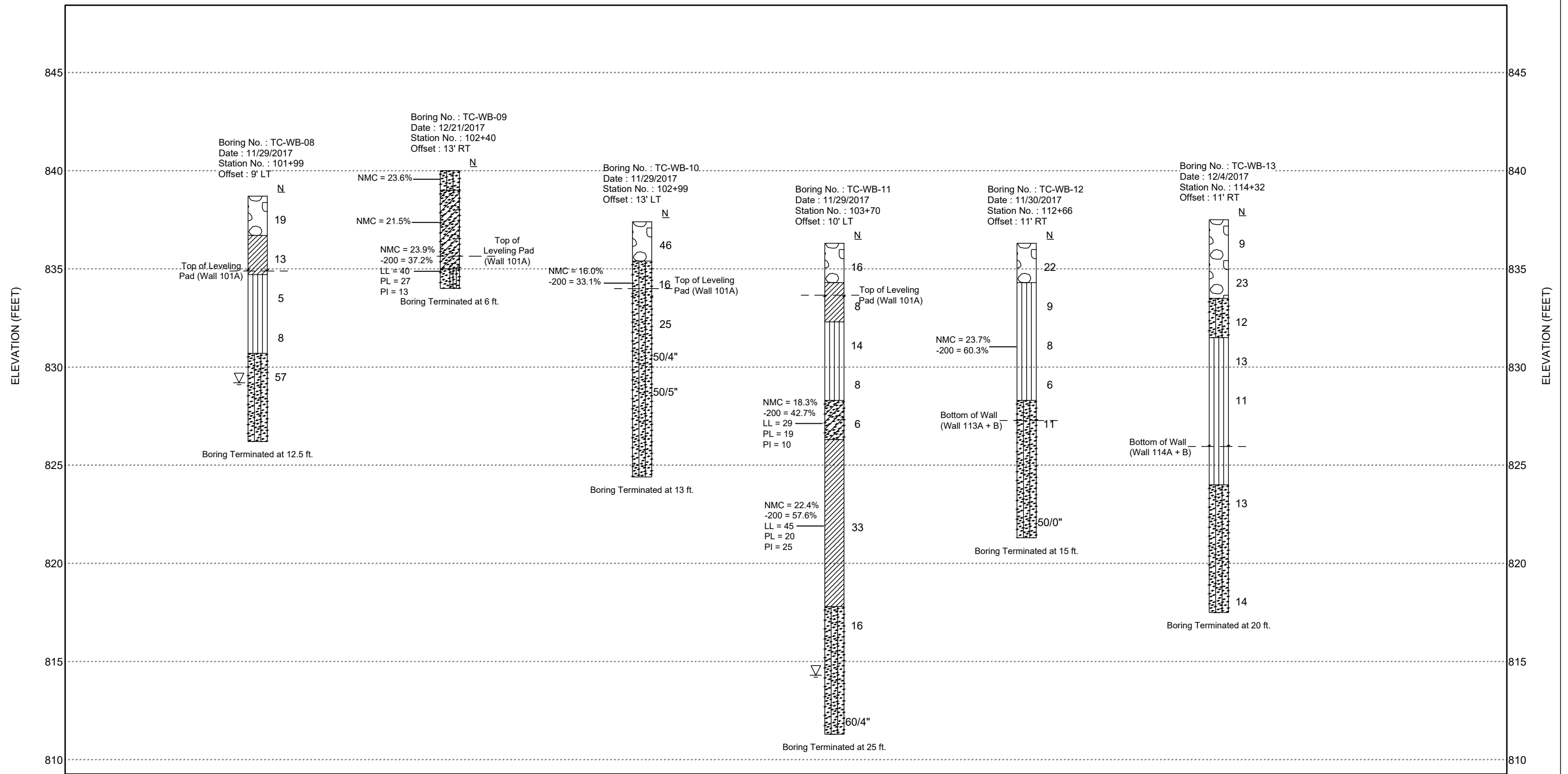
APPENDIX I

- Subsurface Boring Profiles
 - Legend
 - Soil Profiles (gINT)
- Summary of Laboratory Test Results
 - Atterberg Limits' Results
 - Grain Size Distribution




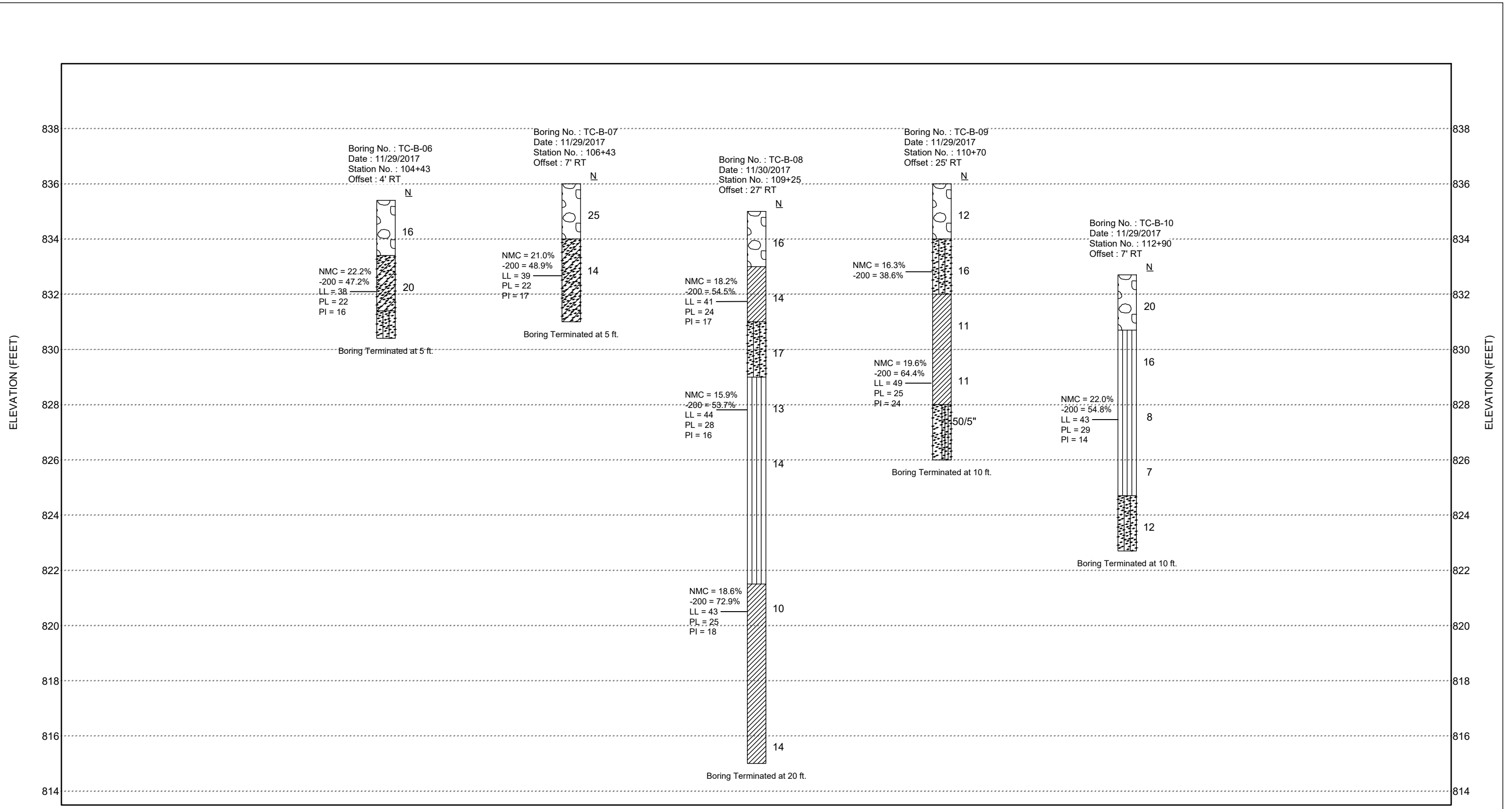
*N Values Drawn At Top Of Interval

DATE	NAME	REVISION	APPROVED BY	 MC SQUARED INC. Geotechnical Consultants 1275 Shiloh Road, Suite 2620 Kennesaw, Georgia 30144 P - (770) 650 0873 F - (770) 650 7825	GEORGIA ENGINEERING CERTIFICATE OF AUTHORIZATION No. PEF004822 Prashanth Vaddu PE GEORGIA LICENSE No. PE039820	NAME	DATE	SUBSURFACE BORING PROFILES	MC ² PROJ. NO.	SHEET NO.	
						DESIGNED BY:	TC				11/7/17
						DRAWN BY:	KH				12/13/17
						CHECKED BY:	AM				12/15/17
				SUPERVISED BY:	PV						
								ABI NE Trail (Task C) Atlanta, Fulton County, Georgia	A051707.058	7	




*N Values Drawn At Top Of Interval

DATE	NAME	REVISION	APPROVED BY	 MC SQUARED INC. Geotechnical Consultants 1275 Shiloh Road, Suite 2620 Kennesaw, Georgia 30144 P - (770) 650 0873 F - (770) 650 7825	DESIGNED BY:		GEORGIA ENGINEERING CERTIFICATE OF AUTHORIZATION No. PEF004822 Prashanth Vaddu PE GEORGIA LICENSE No. PE039820	SUBSURFACE BORING PROFILES		MC ² PROJ. NO. A051707.058	SHEET NO. 8	
						TC		11/7/17	ABI NE Trail (Task C)			
						KH		12/13/17	Atlanta, Fulton County, Georgia			
						AM		12/15/17				
						PV						



*N Values Drawn At Top Of Interval

DATE	NAME	REVISION	APPROVED BY	 MC SQUARED INC. Geotechnical Consultants 1275 Shiloh Road, Suite 2620 Kennesaw, Georgia 30144 P - (770) 650 0873 F - (770) 650 7825	GEORGIA ENGINEERING CERTIFICATE OF AUTHORIZATION No. PEF004822 Prashanth Vaddu PE GEORGIA LICENSE No. PE039820	DESIGNED BY:	NAME	DATE	SUBSURFACE BORING PROFILES ABI NE Trail (Task C) Atlanta, Fulton County, Georgia	MC² PROJ. NO. A051707.058	SHEET NO. 10
						DRAWN BY:	KH	12/13/17			
						CHECKED BY:	JJ	12/15/17			
						SUPERVISED BY:	PV				



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Kennesaw, GA 30144

SUMMARY OF LABORATORY RESULTS

CLIENT Kimley-Horn & Associates

PROJECT NAME ABI NE Trail (Task C)

PROJECT NUMBER A051707.058

PROJECT LOCATION Atlanta, Fulton County, Georgia

Sample No.	Station/ Offset (C/L)	Soil Description	GDOT Class	% < Finer Sieve					D75 (mm)	Clay Count %	N M C (%)	Total Vol- ume Change %	Swell %	Shrink age %	M D D pcf	O M C %	LL %	PL %	PI %	Eros- ion In- dex	Soil sup- port Value
				1.5"	3/4"	#10	#40	#60													
Bulk-1 (0-1 ft)	96+15, CL	Brown silty C to F SAND (micaceous)	SM	100.0	84.5	65.3	54.1	32.7	0.797	10.5		18.3	16.4	1.9	113.1	13.6				6.11	
Bulk-2 (0-1 ft)	100+72, CL	Light brown M to F sandy CLAY (micaceous)	CL	100.0	99.6	93.8	86.8	66.3	0.122	39.0		26.1	22.0	4.1	101.2	18.9				1.97	
Bulk-3 (0-1 ft)	106+18, CL	Brown M to F sandy SILT (micaceous)	ML	100.0	97.5	85.4	74.7	50.7	0.254	19.4		32.0	26.9	5.1	88.6	27.4				3.90	
Bulk-4 (0-1 ft)	109+25, CL	Reddish brown silty C to F SAND (micaceous)	SM	100.0	87.8	68.8	58.6	34.8	0.625	13.8		12.2	10.1	2.0	98.9	20.3				5.85	
TC-B-02 (2-4 ft)	96+40, 3' RT	Brown M to F sandy SILT (micaceous)	ML	100.0	99.4	91.0	83.5	54.3	0.171		30.2									3.45	
TC-B-02 (8-10 ft)	96+40, 3' RT	Brown M to F sandy fat CLAY (micaceous)	CH	100.0	95.2	88.9	82.1	59.5	0.163		23.1						51	28	23	2.81	
TC-B-03 (2-4 ft)	99+20, 3' RT	Dark brown silty M to F SAND (micaceous)	SM	100.0	91.0	78.6	67.4	41.2	0.358		18.4									5.06	
TC-B-03 (6-8 ft)	99+20, 3' RT	Reddish brown fine sandy elastic SILT (micaceous)	MH	100.0	98.9	94.4	87.9	59.7	0.132		31.4						56	33	23	2.79	
TC-B-05 (2-4 ft)	101+45, 3' RT	Light gray silty M to F SAND	SM	100.0	96.1	67.8	52.0	18.9	0.572		6.5									7.80	
TC-B-06 (2-4 ft)	104+43, 4' RT	Reddish brown clayey M to F SAND (micaceous)	SC	100.0	95.3	80.4	69.2	47.2	0.329		22.2						38	22	16	4.33	
TC-B-07 (2-4 ft)	106+43, 7' RT	Brown clayey M to F SAND (micaceous)	SC	100.0	96.3	79.2	69.3	48.9	0.34		21.0						39	22	17	4.11	
TC-B-08 (2-4 ft)	109+25, 27' RT	Brown M to F sandy lean CLAY (micaceous)	CL	100.0	98.4	85.4	76.9	54.5	0.227		18.2						41	24	17	3.42	
TC-B-08 (6-8 ft)	109+25, 27' RT	Brown M to F sandy SILT (micaceous)	ML	100.0	98.9	91.8	83.3	53.7	0.174		15.9						44	28	16	3.53	
TC-B-08 (13.5-15 ft)	109+25, 27' RT	Brown lean CLAY with fine sand (micaceous)	CL	100.0	99.8	98.4	96.4	72.9	0.082		18.6						43	25	18	1.16	
TC-B-09 (2-4 ft)	110+70, 25' RT	Dark gray silty M to F SAND (micaceous)	SM	100.0	90.4	68.7	58.0	38.6	0.607		16.3									5.38	
TC-B-09 (6-8 ft)	110+70, 25' RT	Brown M to F sandy lean CLAY (micaceous)	CL	100.0	98.3	92.3	84.7	64.4	0.139		19.6						49	25	24	2.21	
TC-B-10 (4-6 ft)	112+90, 7' RT	Reddish brown fine sandy SILT (micaceous)	ML	100.0	93.5	88.8	81.8	54.8	0.178		22.0						43	29	14	3.39	
TC-WB-01 (4-6 ft)	98+79, 7' LT	Brown silty C to F SAND	SM	100.0	78.1	54.5	43.8	19.2	1.593		10.0									7.77	
TC-WB-02 (4-6 ft)	99+54, 10' LT	Light brown clayey M to F SAND	SC	100.0	99.2	86.6	73.8	46.8	0.263		17.3						31	15	16	4.37	
TC-WB-04 (13.5-15 ft)	99+60, 34' RT	Brown M to F sandy lean CLAY (micaceous)	CL	100.0	99.3	92.3	84.4	60.8	0.151		24.9						39	23	16	2.65	
TC-WB-04 (19-21 ft)	99+89, 111' RT	Brown, gray M to F sandy lean CLAY (slightly micaceous)	CL	100.0	99.7	91.4	82.2	56.6	0.18	31.0	22.4						30	14	16	3.17	
TC-WB-05 (4-6 ft)	100+04, 59' RT	Tan silty M to F SAND (micaceous)	SM	100.0	96.2	81.5	68.1	30.9	0.329		21.1									6.33	
TC-WB-06 (8-10 ft)	99+71, 64' RT	Brown clayey M to F SAND (micaceous)	SC	100.0	96.3	86.8	76.2	46.2	0.238		15.7						23	14	9	4.45	
TC-WB-07 (0-1.5 ft)	101+23, 14' RT	Reddish brown silty SAND (micaceous)	SM					29.4			20.9						37	31	6	6.52	
TC-WB-07 (7.5-8 ft)	101+23, 14' RT	Reddish brown silty SAND (micaceous)	SM					29.7			21.4						32	27	5	6.48	
TC-WB-09 (0-1 ft)	102+40, 13' RT	Gray silty SAND (micaceous)	SM								23.6										
TC-WB-09 (1-3.5 ft)	102+40, 13' RT	Gray silty SAND (micaceous)	SM								21.5										
TC-WB-09 (3.5-5 ft)	102+40, 13' RT	Reddish brown silty SAND (micaceous)	SM					37.2			23.9						40	27	13	5.56	
TC-WB-10 (2-4 ft)	102+99, 13' LT	Dark gray silty M to F SAND (micaceous)	SM	100.0	90.9	75.3	64.1	33.1	0.419		16.0									6.06	
TC-WB-11 (8-10 ft)	103+70, 10' LT	Dark gray clayey M to F SAND (micaceous)	SC	100.0	97.5	84.4	72.7	42.7	0.278		18.3						29	19	10	4.88	



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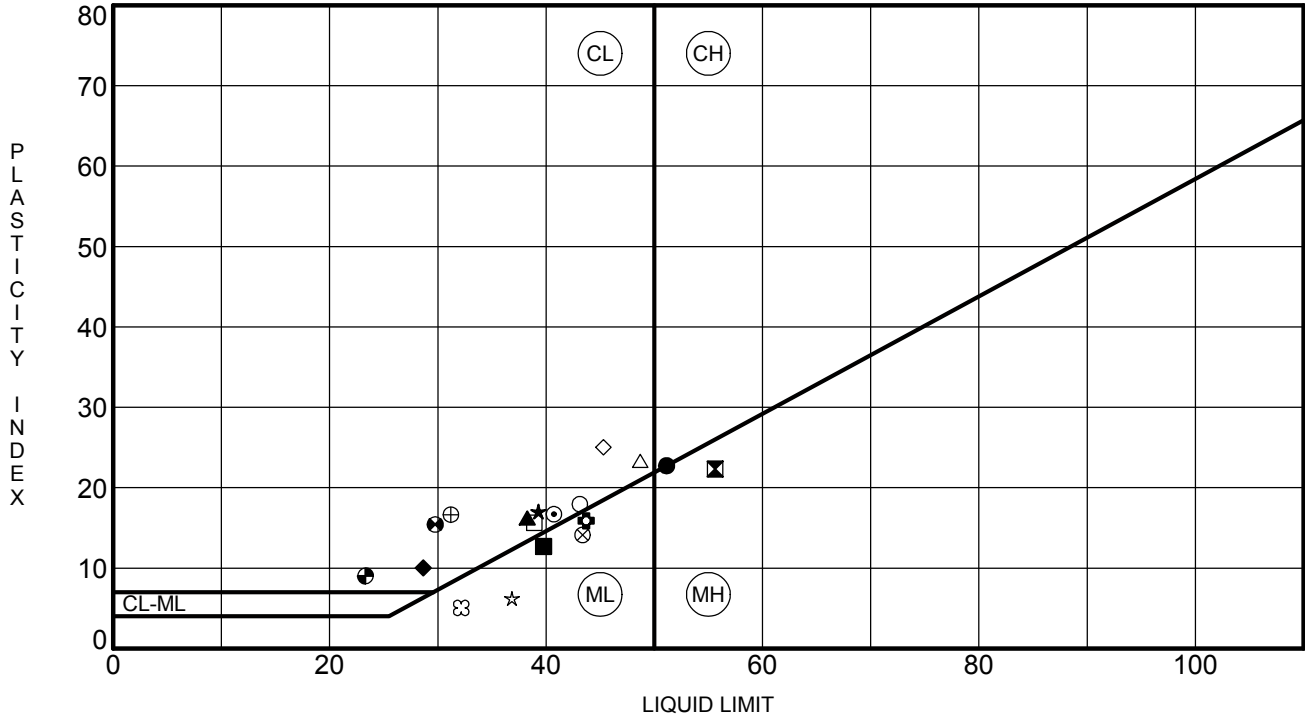
ATTERBERG LIMITS' RESULTS

CLIENT Kimley- Horn & Associates

PROJECT NAME ABI NE Trail (Task C)

PROJECT NUMBER A051707.058

PROJECT LOCATION Atlanta, Fulton County, Georgia



ATTERBERG LIMIT FOR GDOT - GINT STD US LAB.GDT - 2/23/18 16:52 - C:\USERS\GINT.MC2\DESKTOP\A17.058 LAB\LABORATORY TESTING A17.058 SEGMENT-C.GPJ

Specimen Identification	LL	PL	PI	%Fines	Classification
● TC-B-02 (8-10 ft)	51	28	23	60	Brown M to F sandy fat CLAY (micaceous)
⊠ TC-B-03 (6-8 ft)	56	33	23	60	Reddish brown fine sandy elastic SILT (micaceous)
▲ TC-B-06 (2-4 ft)	38	22	16	47	Reddish brown clayey M to F SAND (micaceous)
★ TC-B-07 (2-4 ft)	39	22	17	49	Brown clayey M to F SAND (micaceous)
⊙ TC-B-08 (2-4 ft)	41	24	17	54	Brown M to F sandy lean CLAY (micaceous)
⊕ TC-B-08 (6-8 ft)	44	28	16	54	Brown M to F sandy SILT (micaceous)
○ TC-B-08 (13.5-15 ft)	43	25	18	73	Brown lean CLAY with fine sand (micaceous)
△ TC-B-09 (6-8 ft)	49	25	24	64	Brown M to F sandy lean CLAY (micaceous)
⊗ TC-B-10 (4-6 ft)	43	29	14	55	Reddish brown fine sandy SILT (micaceous)
⊕ TC-WB-02 (4-6 ft)	31	15	16	47	Light brown clayey M to F SAND
□ TC-WB-04 (13.5-15 ft)	39	23	16	61	Brown M to F sandy lean CLAY (micaceous)
⊕ TC-WB-04 (19-21 ft)	30	14	16	57	Brown, gray M to F sandy lean CLAY (slightly micaceous)
⊕ TC-WB-06 (8-10 ft)	23	14	9	46	Brown clayey M to F SAND (micaceous)
★ TC-WB-07 (0-1.5 ft)	37	31	6	29	Reddish brown silty SAND (micaceous)
⊗ TC-WB-07 (7.5-8 ft)	32	27	5	30	Reddish brown silty SAND (micaceous)
■ TC-WB-09 (3.5-5 ft)	40	27	13	37	Reddish brown silty SAND (micaceous)
◆ TC-WB-11 (8-10 ft)	29	19	10	43	Dark gray clayey M to F SAND (micaceous)
◇ TC-WB-11 (13.5-15 ft)	45	20	25	58	Brown M to F sandy lean CLAY (micaceous)

%Fines- % of total soil (by weight) passing U.S. No. 200 sieve



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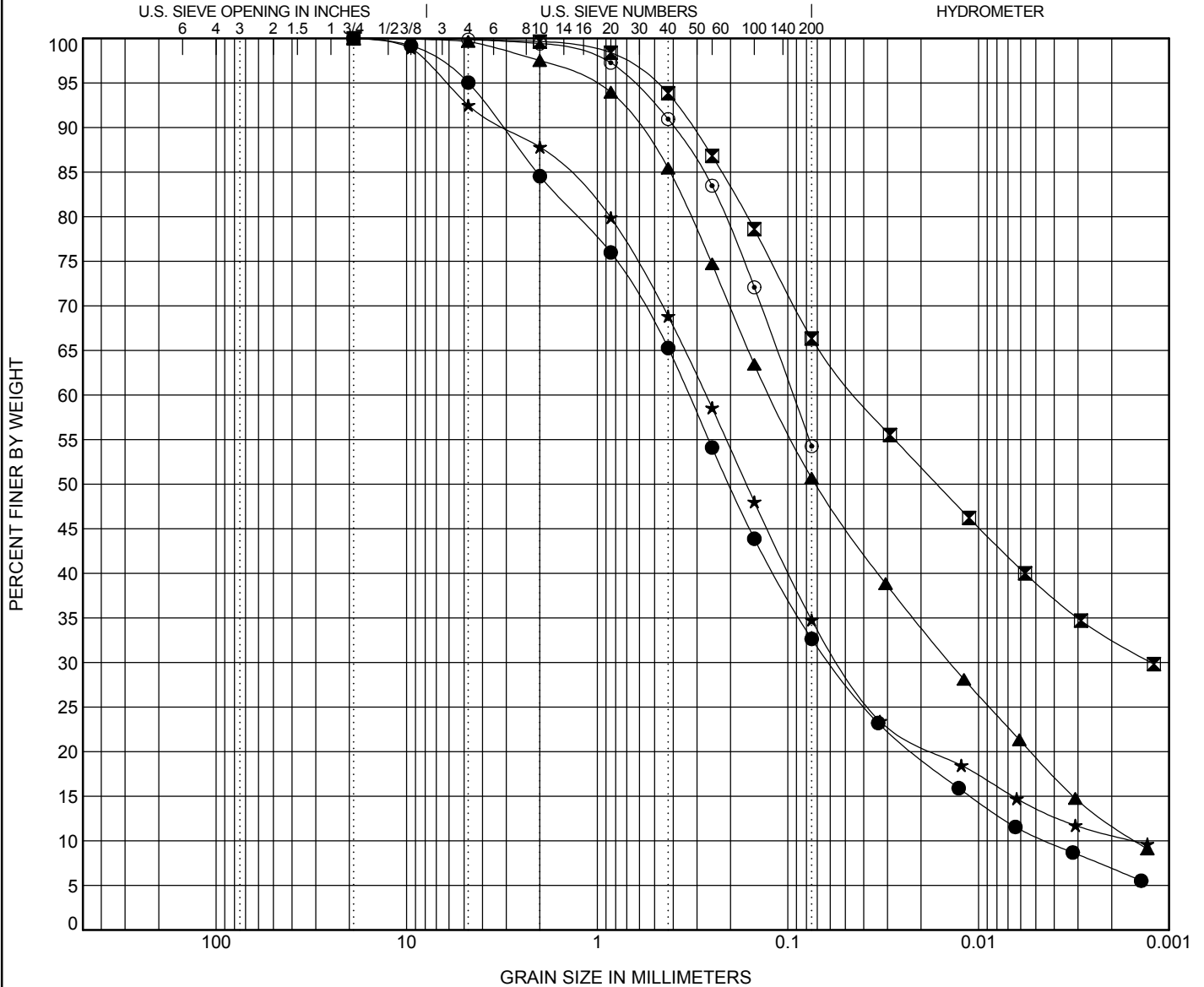
GRAIN SIZE DISTRIBUTION

CLIENT **Kimley-Horn & Associates**

PROJECT NAME **ABI NE Trail (Task C)**

PROJECT NUMBER **A051707.058**

PROJECT LOCATION **Atlanta, Fulton County, Georgia**



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● Bulk-1 (0-1 ft)	Brown silty C to F SAND (micaceous)								2.46	75.26
☒ Bulk-2 (0-1 ft)	Light brown M to F sandy CLAY (micaceous)									
▲ Bulk-3 (0-1 ft)	Brown M to F sandy SILT (micaceous)								1.06	83.24
★ Bulk-4 (0-1 ft)	Reddish brown silty C to F SAND (micaceous)								6.92	177.94
⊙ TC-B-02 (2-4 ft)	Brown M to F sandy SILT (micaceous)									
Specimen Identification	D100	D60	D30	D10	NMC	%Gravel	%Sand	%Silt	%Clay	
● Bulk-1 (0-1 ft)	19	0.331	0.06	0.004		5.0	62.4	22.1	10.5	
☒ Bulk-2 (0-1 ft)	19	0.043	0.001			0.2	33.5	27.3	39.0	
▲ Bulk-3 (0-1 ft)	19	0.124	0.014	0.001		0.3	49.0	31.3	19.4	
★ Bulk-4 (0-1 ft)	19	0.269	0.053	0.002		7.5	57.7	21.0	13.8	
⊙ TC-B-02 (2-4 ft)	19	0.094			30.2	0.1	45.6		54.3	

Note-Sample soaked for 16 hrs (+/- 10 min)



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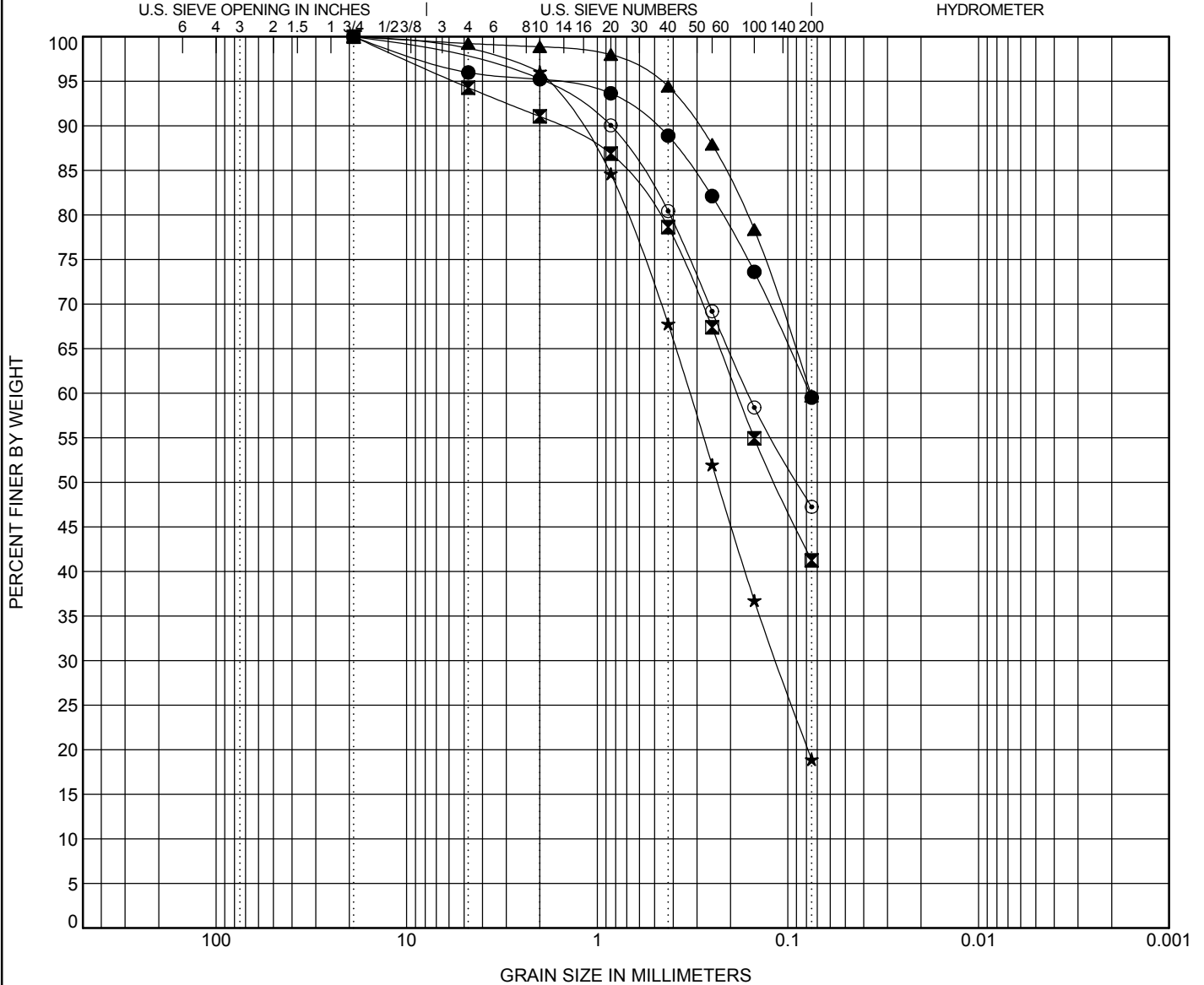
GRAIN SIZE DISTRIBUTION

CLIENT **Kimley-Horn & Associates**

PROJECT NAME **ABI NE Trail (Task C)**

PROJECT NUMBER **A051707.058**

PROJECT LOCATION **Atlanta, Fulton County, Georgia**



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● TC-B-02 (8-10 ft)	Brown M to F sandy fat CLAY (micaceous)					51	28	23		
☒ TC-B-03 (2-4 ft)	Dark brown silty M to F SAND (micaceous)									
▲ TC-B-03 (6-8 ft)	Reddish brown fine sandy elastic SILT (micaceous)					56	33	23		
★ TC-B-05 (2-4 ft)	Light gray silty M to F SAND									
◎ TC-B-06 (2-4 ft)	Reddish brown clayey M to F SAND (micaceous)					38	22	16		
Specimen Identification	D100	D60	D30	D10	NMC	%Gravel	%Sand	%Silt	%Clay	
● TC-B-02 (8-10 ft)	19	0.077			23.1	4.0	36.5	59.5		
☒ TC-B-03 (2-4 ft)	19	0.185			18.4	5.7	53.0	41.2		
▲ TC-B-03 (6-8 ft)	19	0.076			31.4	0.8	39.6	59.7		
★ TC-B-05 (2-4 ft)	19	0.327	0.115		6.5	2.4	78.7	18.9		
◎ TC-B-06 (2-4 ft)	19	0.162			22.2	2.9	49.8	47.2		

Note-Sample soaked for 16 hrs (+/- 10 min)



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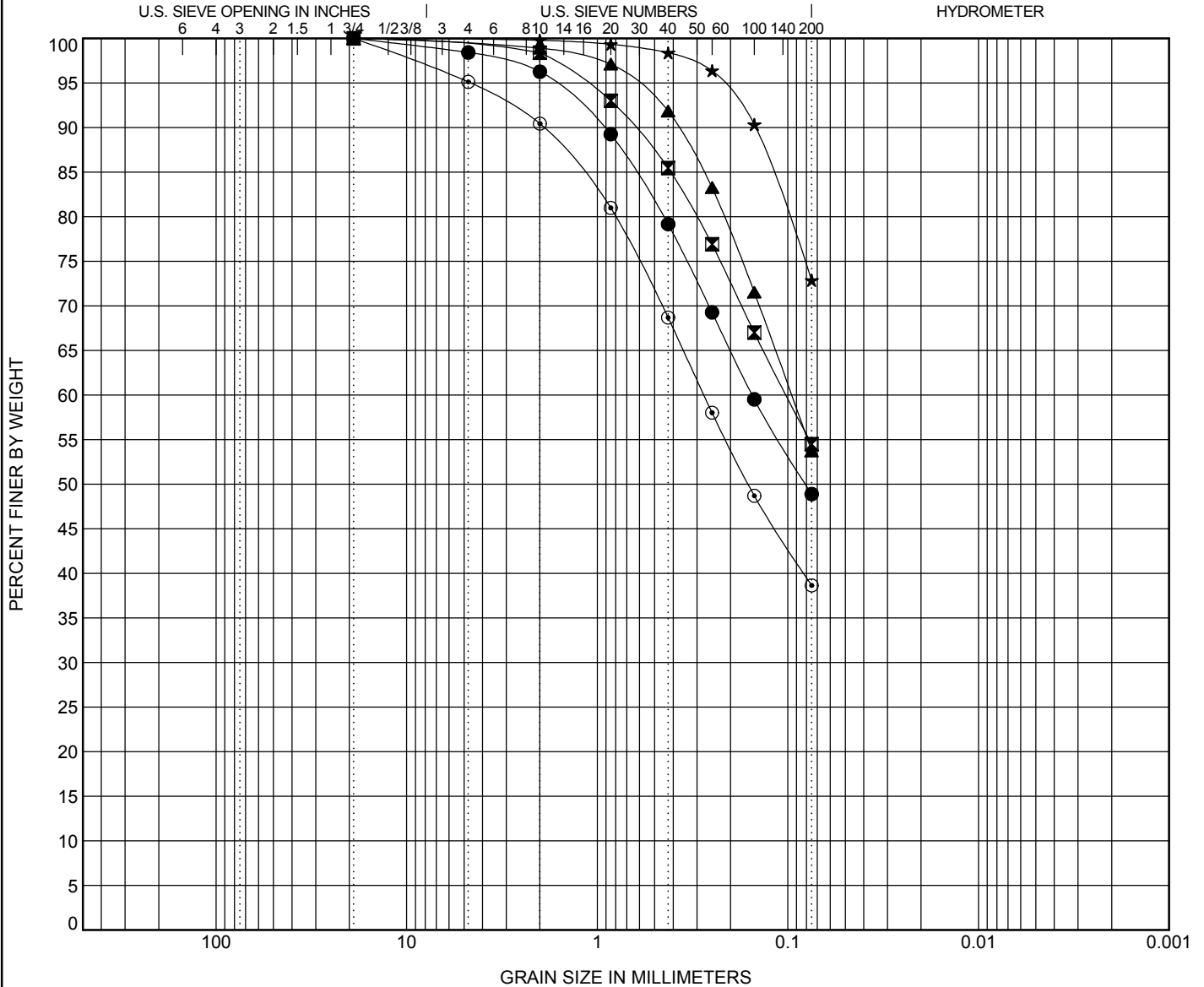
GRAIN SIZE DISTRIBUTION

CLIENT **Kimley-Horn & Associates**

PROJECT NAME **ABI NE Trail (Task C)**

PROJECT NUMBER **A051707.058**

PROJECT LOCATION **Atlanta, Fulton County, Georgia**



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● TC-B-07 (2-4 ft)	Brown clayey M to F SAND (micaceous)	39	22	17		
⊠ TC-B-08 (2-4 ft)	Brown M to F sandy lean CLAY (micaceous)	41	24	17		
▲ TC-B-08 (6-8 ft)	Brown M to F sandy SILT (micaceous)	44	28	16		
★ TC-B-08 (13.5-15 ft)	Brown lean CLAY with fine sand (micaceous)	43	25	18		
⊙ TC-B-09 (2-4 ft)	Dark gray silty M to F SAND (micaceous)					

Specimen Identification	D100	D60	D30	D10	NMC	%Gravel	%Sand	%Silt	%Clay
● TC-B-07 (2-4 ft)	19	0.154			21.0	1.6	49.5		48.9
⊠ TC-B-08 (2-4 ft)	19	0.102			18.2	1.0	44.5		54.5
▲ TC-B-08 (6-8 ft)	19	0.096			15.9	0.7	45.6		53.7
★ TC-B-08 (13.5-15 ft)	19				18.6	0.1	27.0		72.9
⊙ TC-B-09 (2-4 ft)	19	0.276			16.3	4.9	56.5		38.6

Note-Sample soaked for 16 hrs (+/- 10 min)



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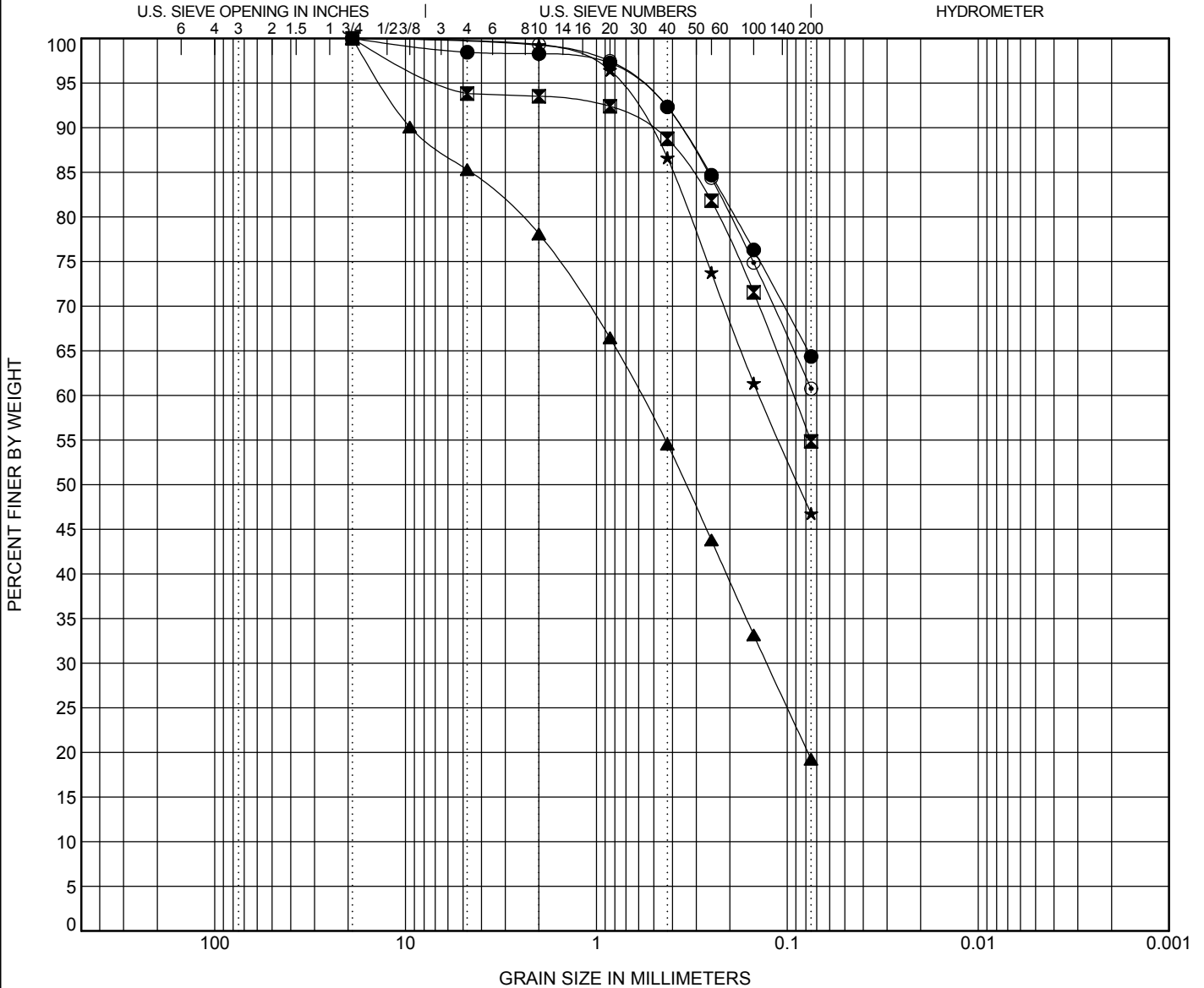
GRAIN SIZE DISTRIBUTION

CLIENT Kimley-Horn & Associates

PROJECT NAME ABI NE Trail (Task C)

PROJECT NUMBER A051707.058

PROJECT LOCATION Atlanta, Fulton County, Georgia



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● TC-B-09 (6-8 ft)	Brown M to F sandy lean CLAY (micaceous)	49	25	24		
☒ TC-B-10 (4-6 ft)	Reddish brown fine sandy SILT (micaceous)	43	29	14		
▲ TC-WB-01 (4-6 ft)	Brown silty C to F SAND					
★ TC-WB-02 (4-6 ft)	Light brown clayey M to F SAND	31	15	16		
◎ TC-WB-04 (13.5-15 ft)	Brown M to F sandy lean CLAY (micaceous)	39	23	16		

Specimen Identification	D100	D60	D30	D10	NMC	%Gravel	%Sand	%Silt	%Clay
● TC-B-09 (6-8 ft)	19				19.6	1.5	34.1	64.4	
☒ TC-B-10 (4-6 ft)	19	0.093			22.0	6.2	39.0	54.8	
▲ TC-WB-01 (4-6 ft)	19	0.584	0.128		10.0	14.7	66.1	19.2	
★ TC-WB-02 (4-6 ft)	19	0.141			17.3	0.5	52.8	46.8	
◎ TC-WB-04 (13.5-15 ft)	19				24.9	0.4	38.8	60.8	

Note-Sample soaked for 16 hrs (+/- 10 min)



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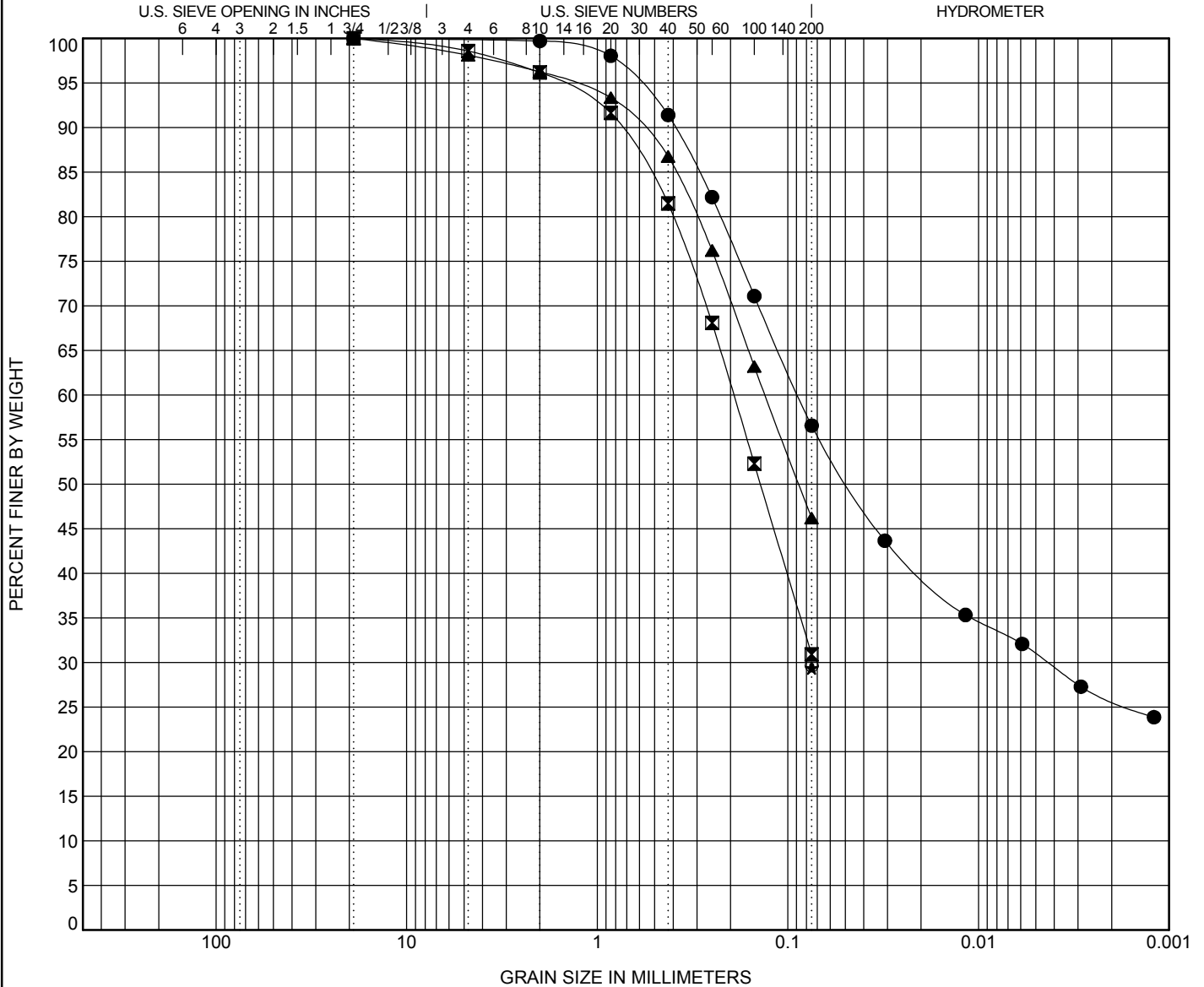
GRAIN SIZE DISTRIBUTION

CLIENT Kimley-Horn & Associates

PROJECT NAME ABI NE Trail (Task C)

PROJECT NUMBER A051707.058

PROJECT LOCATION Atlanta, Fulton County, Georgia



Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● TC-WB-04 (19-21 ft)	Brown, gray M to F sandy lean CLAY (slightly micaceous)					30	14	16		
☒ TC-WB-05 (4-6 ft)	Tan silty M to F SAND (micaceous)									
▲ TC-WB-06 (8-10 ft)	Brown clayey M to F SAND (micaceous)					23	14	9		
★ TC-WB-07 (0-1.5 ft)	Reddish brown silty SAND (micaceous)					37	31	6		
⊙ TC-WB-07 (7.5-8 ft)	Reddish brown silty SAND (micaceous)					32	27	5		

Specimen Identification	D100	D60	D30	D10	NMC	%Gravel	%Sand	%Silt	%Clay
● TC-WB-04 (19-21 ft)	19	0.088	0.004		22.4	0.2	43.3	25.6	31.0
☒ TC-WB-05 (4-6 ft)	19	0.193			21.1	1.4	67.7		30.9
▲ TC-WB-06 (8-10 ft)	19	0.132			15.7	1.9	51.9		46.2
★ TC-WB-07 (0-1.5 ft)	0.075				20.9				29.4
⊙ TC-WB-07 (7.5-8 ft)	0.075				21.4				29.7

Note-Sample soaked for 16 hrs (+/- 10 min)

APPENDIX II

- Special Provision 520 Piling – Pilot Holes

**DEPARTMENT OF TRANSPORTATION
STATE OF GEORGIA**

SPECIAL PROVISION

**ABI Northeast Trail (Task C)
MC Squared Project No. A051707.058
Atlanta, Fulton County, Georgia**

SECTION 520—PILING

Delete Sub-Section 520.3.05.B and substitute the following:

520.3.05.B. Drill Pilot Holes

When pilot holes are required, drill them to the diameter and approximate depth specified on the Plans.

Backfill voids and holes with Class A or better concrete. Furnishing and placing backfill concrete is an incidental part of the work.

The following are not considered pilot holes:

- Holes created by spudding (punching)
- Holes dug to drive piling that is too long to fit leads
- Holes dug to replace a template (if permitted)

Where pilot holes are required in granular material and the material cannot be sealed off using “mudding” drilling methods, drill the pilot hole as follows:

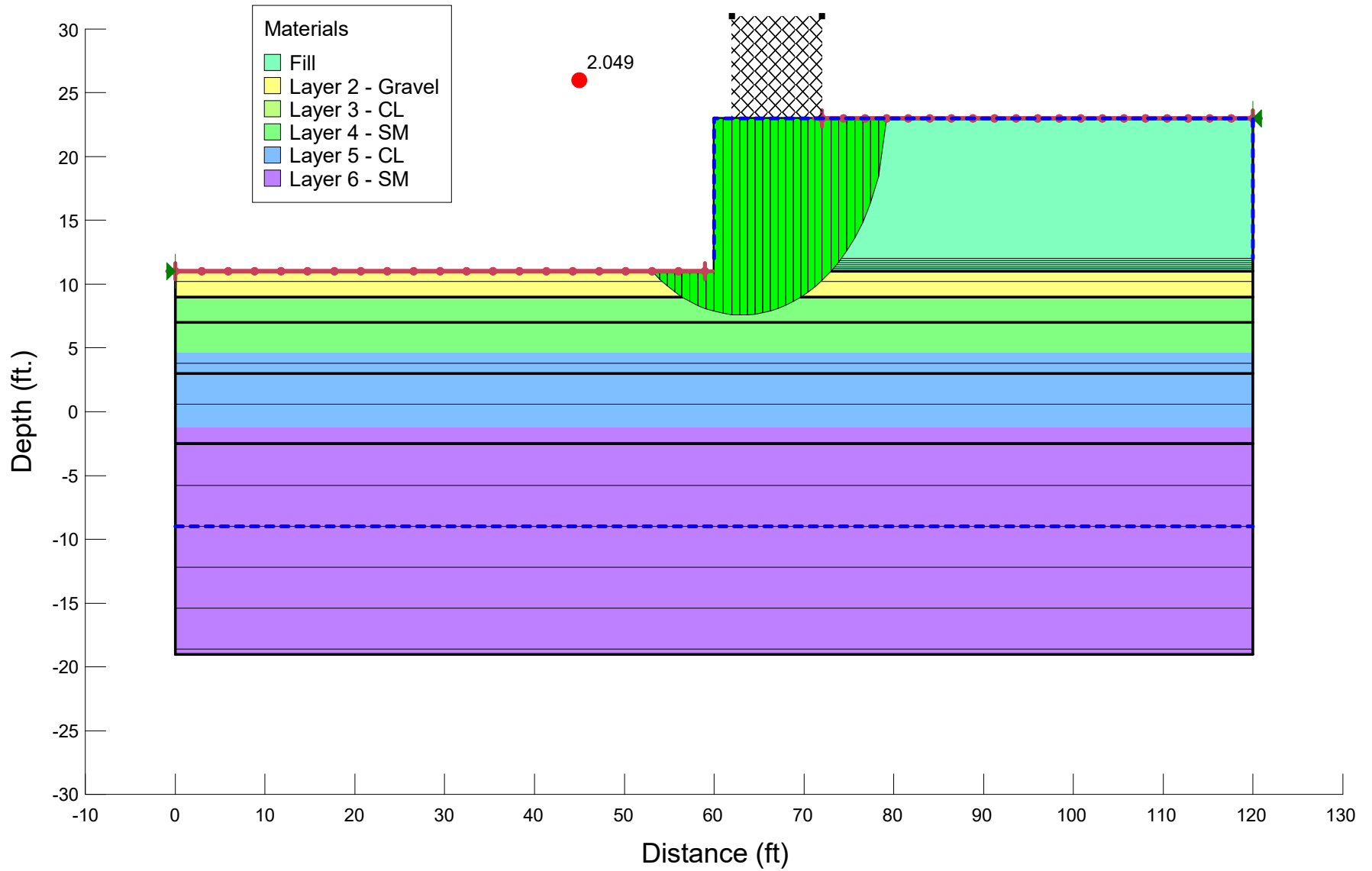
1. Place a casing pipe with a large enough diameter around the boring device.
2. Hold the casing in position until the pilot hole is completed and the pile driving progresses deep enough into the hard material to keep loose material out of the pilot hole.

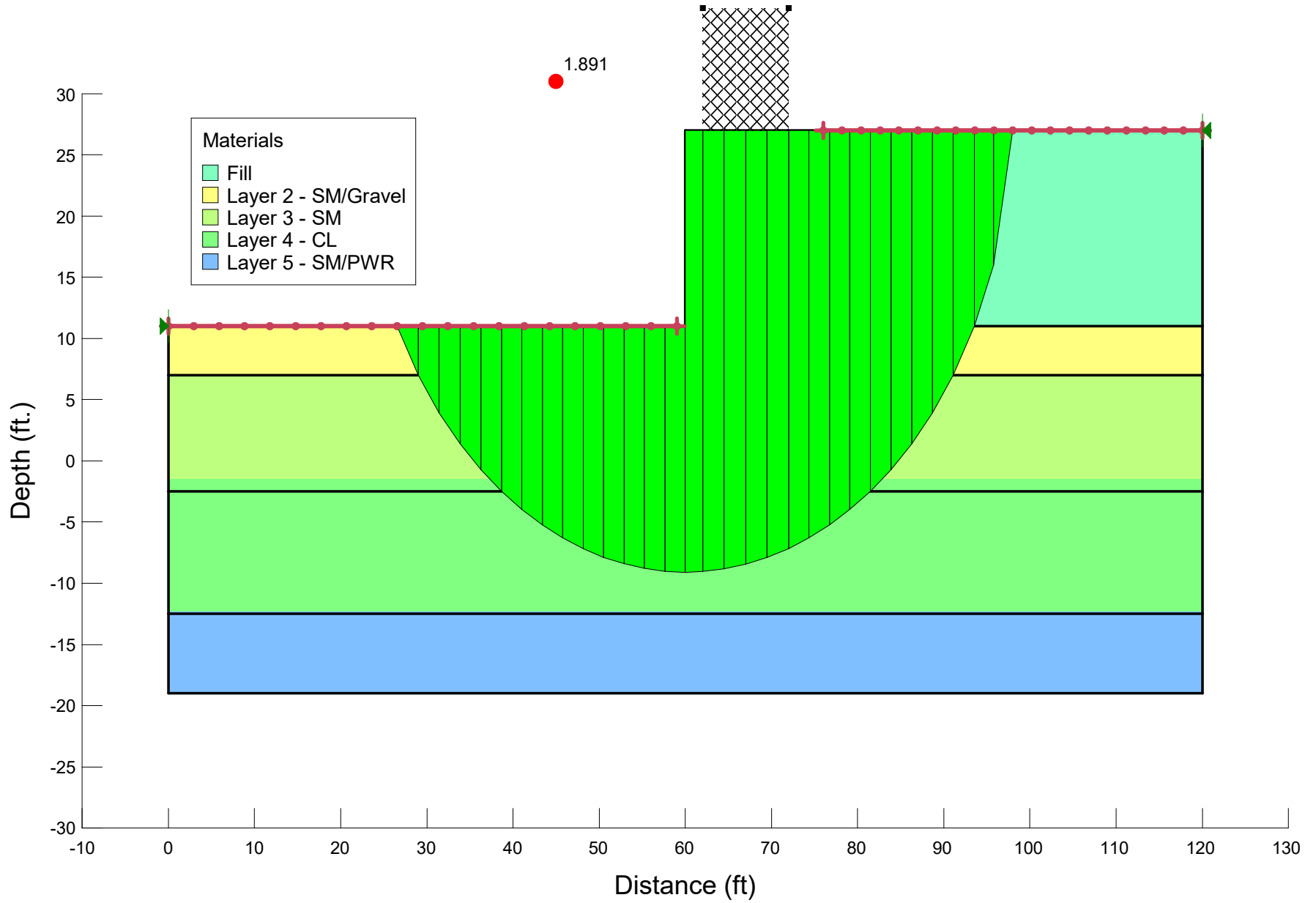
The use of casing is incidental to the work.

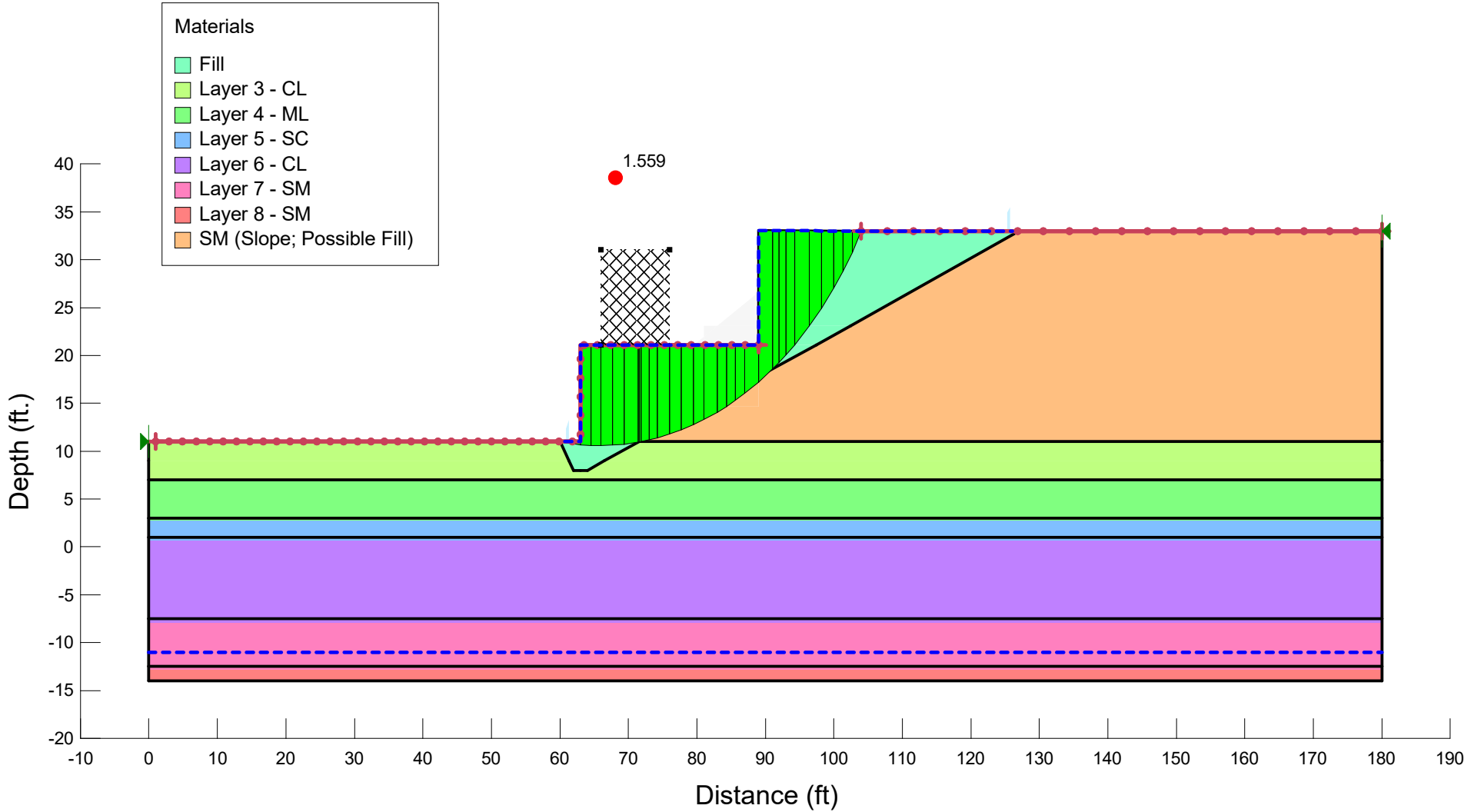
Office of Materials and Testing

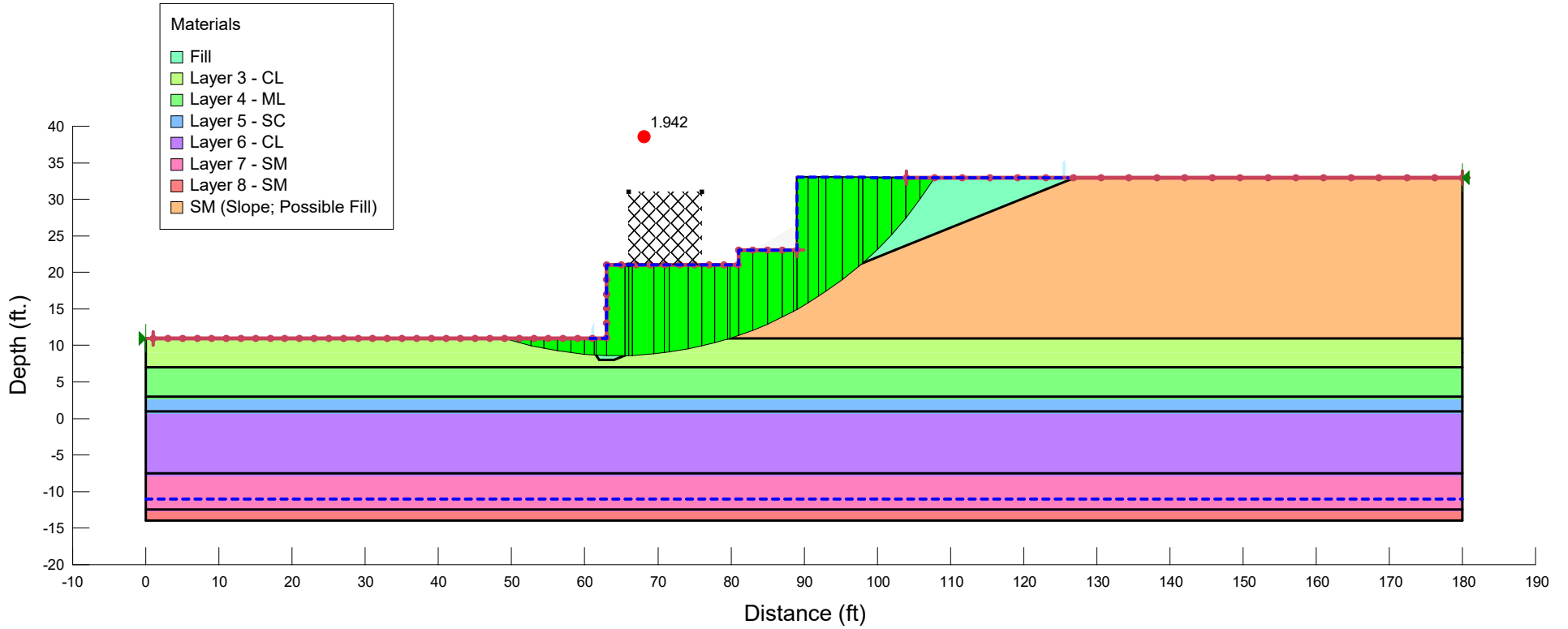
APPENDIX III (MSE WALLS)

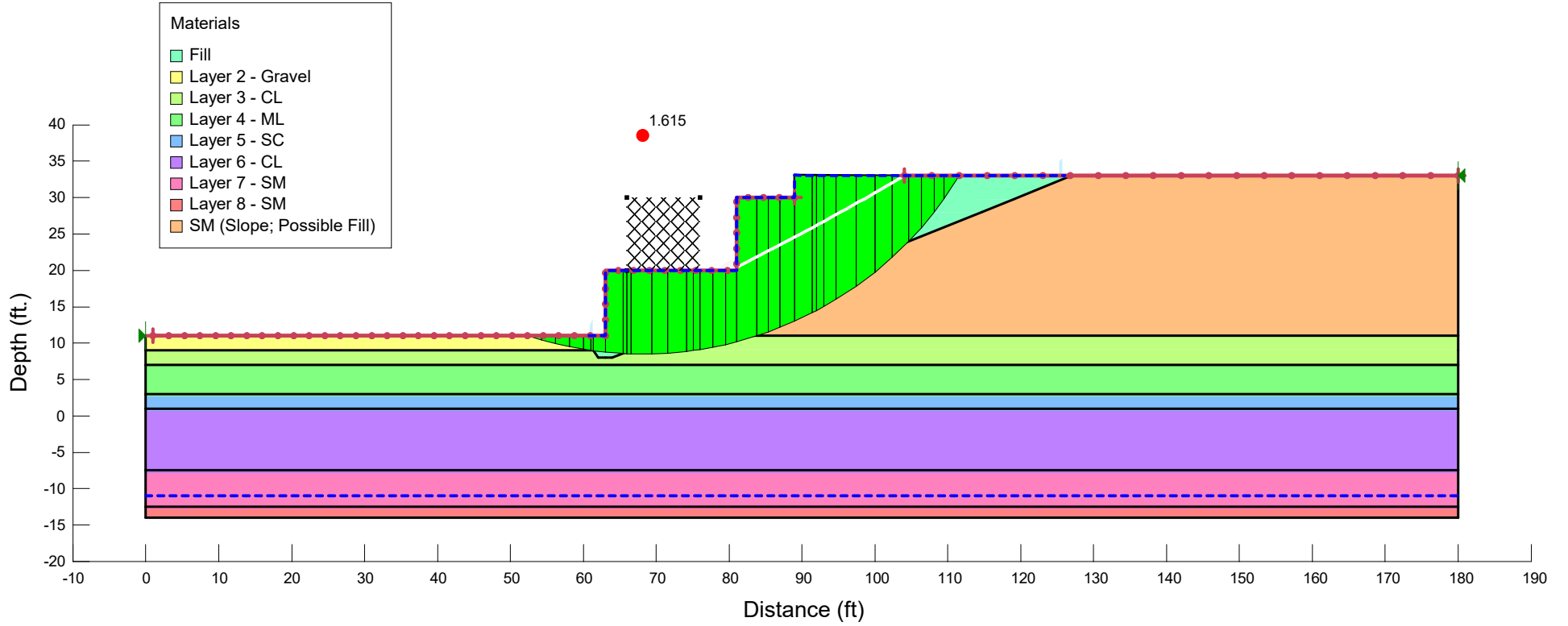
- Global Stability Analysis – Wall 98
 - Global Stability Analysis – Wall 99
- Global Stability Analysis – Walls 101A thru C (Section 1)
- Global Stability Analysis – Walls 101A thru C (Section 2)
- Global Stability Analysis – Walls 101A thru C (Section 3)
- Global Stability Analysis – Walls 101A thru C (Section 4)

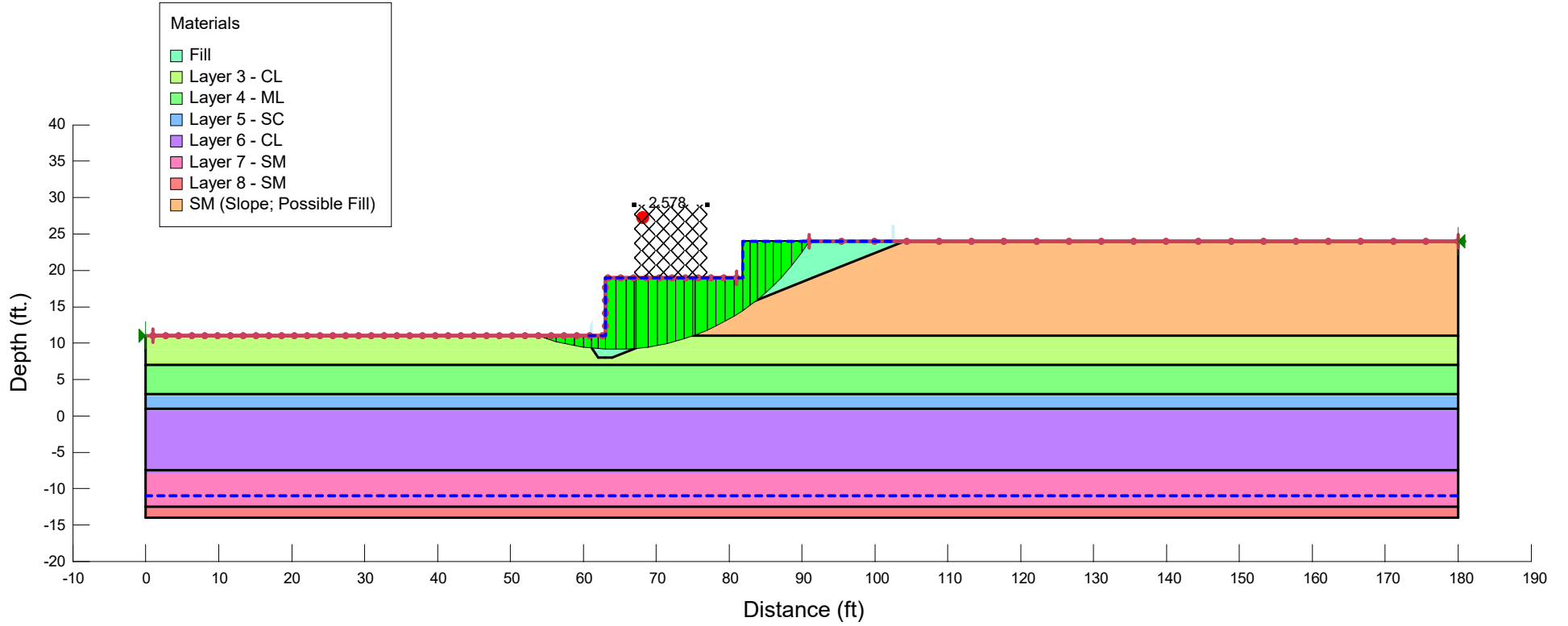












APPENDIX IV (SOLDIER PILE WALLS)

- Lateral Stability Analysis Summary
- Global Stability Analysis – Wall 100B
- Global Stability Analysis – Wall 101D
- Global Stability Analysis – Wall 113A+B
 - Global Stability Analysis – 114A+B
- AASHTO Lateral Stability References

Lateral Stability Analysis Summary

1. Analysis assumptions:

Lateral stability of the soldier pile wall was completed according to section 5.6 of AASHTO 17th edition, 2002. The simplified earth pressure distribution for permanent flexible cantilevered walls with discrete vertical wall elements in granular soils, known as **Case a. Embedment in Soil** in Figure 5.6.2A (attached, see **Appendix IV (Soldier Pile Walls)**), was used for the analysis. The pile type used for the analysis is the 14-inch steel H-pile. The spacing center to center between piles was assumed to be 5 feet. The resultant net passive resistance of the pile was assumed to be mobilized across a maximum of three times the pile flange width. A portion of 2 feet of the embedment depth below finished grade was assumed ineffective in providing passive lateral support.

2. Analysis results

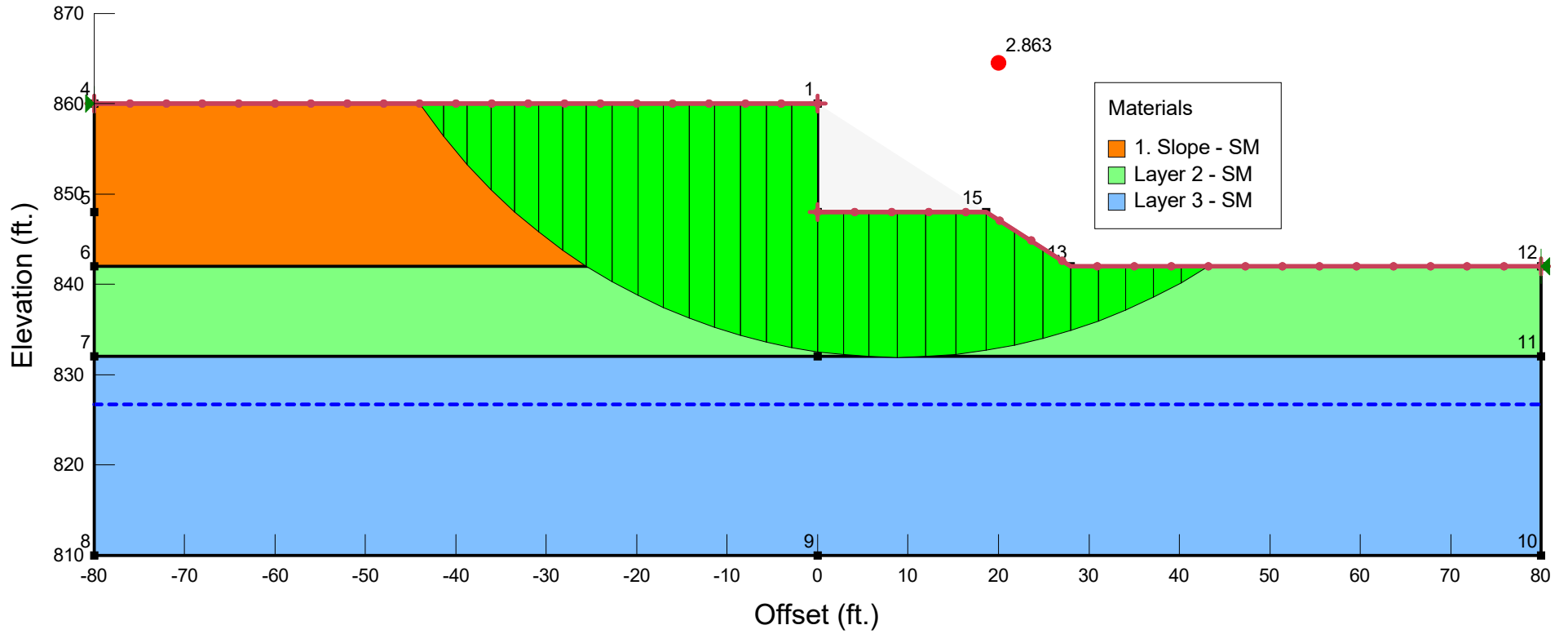
A summary of the analysis parameters and results for all Soldier Pile walls is presented in **Table 1**.

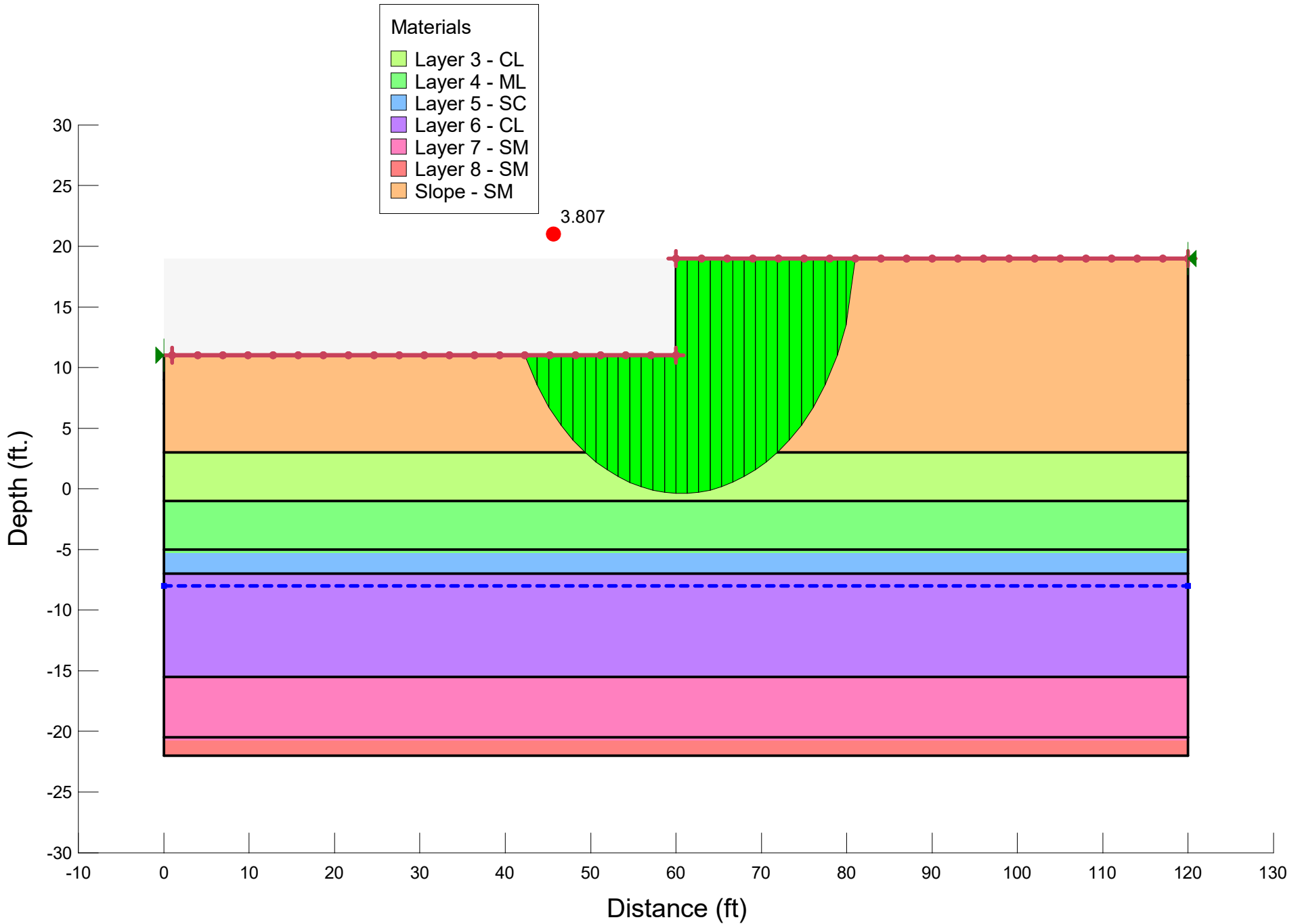
The analysis parameters are presented in Figures 5.6.2A and Table 5.6.2A in AASHTO 17th edition, 2002. The figure and table are shown in **Appendix IV (Soldier Pile Walls)** under the "AASHTO Lateral Stability References" section.

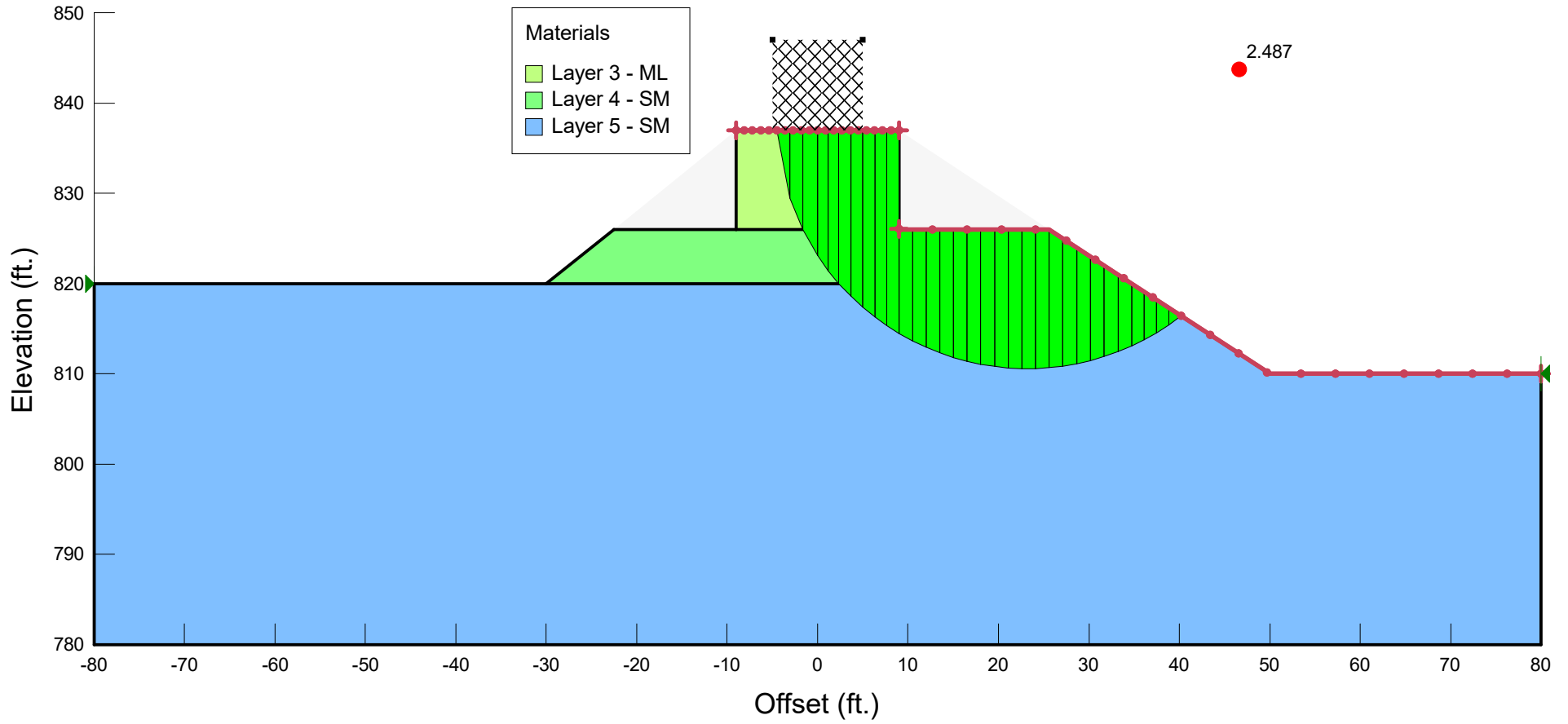
Table 1. Summary of Lateral Stability Analysis

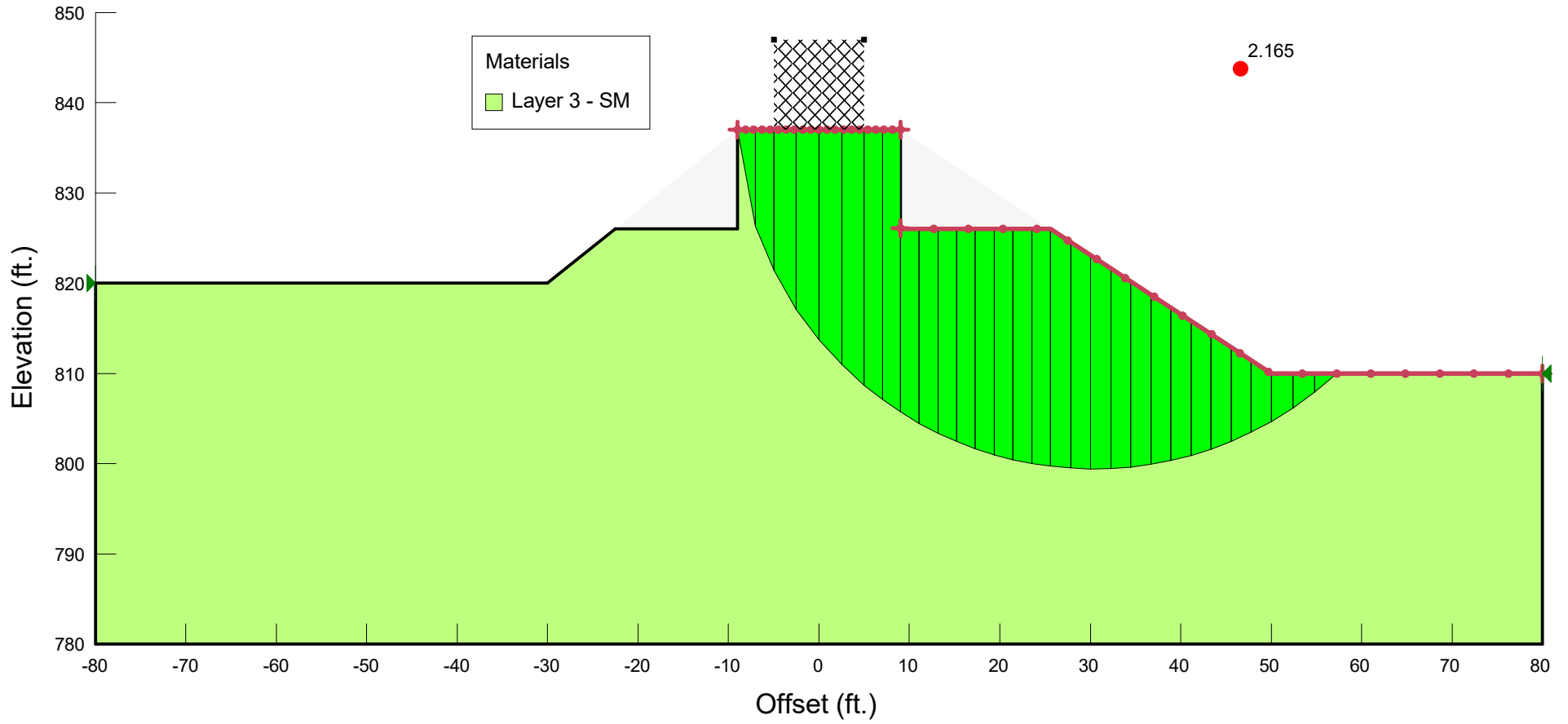
Wall ID	$\gamma'1$ (pcf)	$\gamma'2$ (pcf)	$\phi'1$ (°)	$\phi'2$ (°)	H (ft.)	ℓ (ft.)	b (ft.)	β (°)	β' (°)	K_{a1}	K_{a2}	K_{p2}	P_{a1} (psf)	P_{a2} (psf)	P_p (psf)	D (ft.)	Minimum Tip Elevation (ft)
100B	110	120	30	35	12	5	3	0	0	0.333	0.271	3.000	13200	81216	334125	15	833.30
101D	110	110	30	30	8	5	3	0	0	0.333	0.333	3.000	5867	49005	179685	11	833.10
113A + B	115	115	28	28	10	5	3	0	0	0.361	0.361	2.770	10380	80152	242242	13	814.00
114A + B	115	130	28	40	11	5	3	0	0	0.361	0.217	2.770	10380	88681	517452	19	807.00

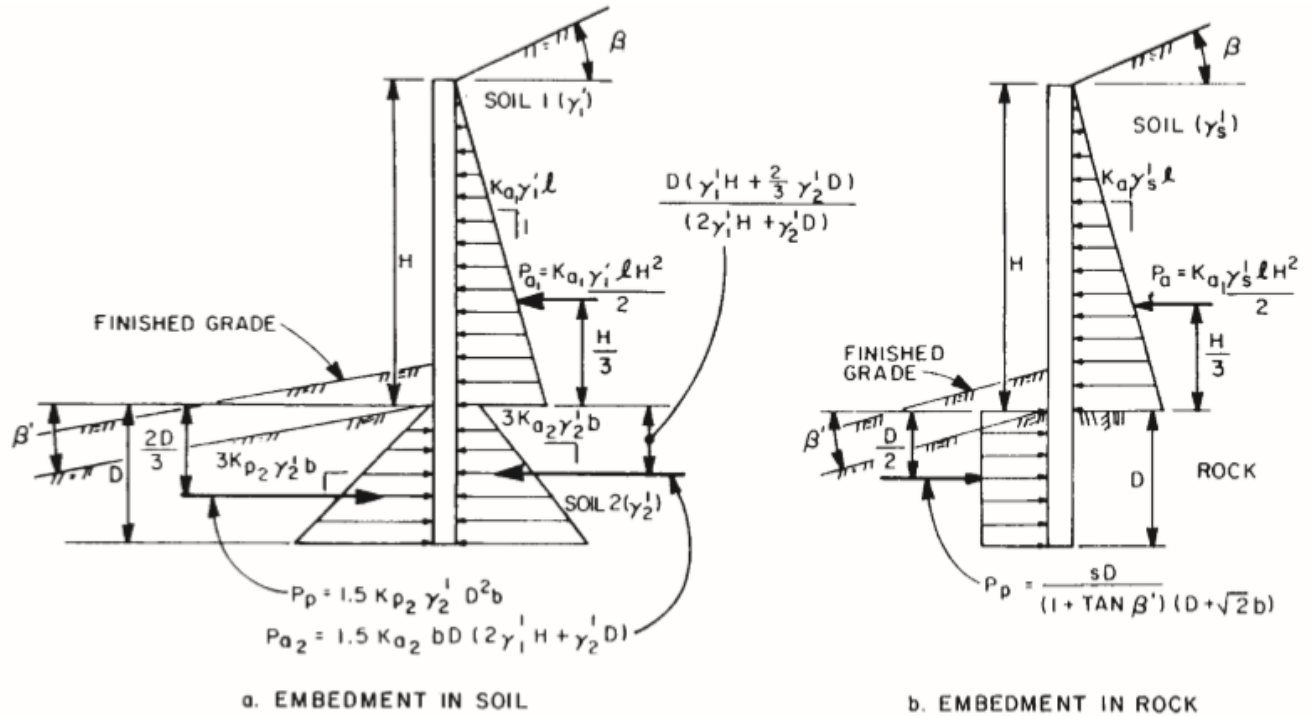
Refer to the Appendix for parameters descriptions.











NOTE: REFER TO TABLE 5.6.2A FOR
 GENERAL NOTES AND LEGEND

FIGURE 5.6.2A Simplified Earth Pressure Distributions for Permanent Flexible Cantilevered Walls With Discrete Vertical Wall Elements

Reference AASHTO 17th edition, 2002.

TABLE 5.6.2A General Notes and Legend Simplified Earth Pressure Distributions for Permanent and Temporary Flexible Cantilevered Walls with Discrete Vertical Wall Elements

LEGEND:

γ'	=	Effective unit weight of soil
b	=	Vertical element width
ℓ	=	Spacing between vertical wall elements (c/c)
S_u	=	Undrained shear strength of cohesive soil
s	=	Shear strength of rock mass
P_p	=	Passive resistance per vertical wall element
P_a	=	Active earth pressure per vertical wall element
β	=	Ground surface slope behind wall
β'	=	Ground surface slope in front of wall
K_a	=	Active earth pressure coefficient; Refer to Figure 5.5.2A
K_p	=	Passive earth pressure coefficient; Refer to Figures 5.5.2C and 5.5.2D
ϕ'	=	Effective angle of soil friction

NOTES:

- (1) For temporary walls embedded in granular soil or rock, refer to Figure 5.6.2A to determine passive resistance and use diagrams on Figure 5.6.2C to determine active earth pressure of retained soil.
- (2) Surcharge and water pressures must be added to the indicated earth pressures.
- (3) Forces shown are per vertical wall element.
- (4) Pressure distributions below the exposed portion of the wall are based on an effective element width of $3b$, which is valid for $\geq 5b$. For $< 5b$, refer to Figures 5.6.2B and 5.6.2D for continuous wall elements to determine pressure distributions on embedded portions of the wall.

Reference AASHTO 17th edition, 2002.