

SPODEN & WILSON
CONSULTING ENGINEERS

338 EAST CENTER STREET, SUITE 2
KINGSPORT, TENNESSEE 37660
Phone (423) 245-1181 Fax (423) 245-0852
email sweng@spodenwilson.com

COLUMN SCHEDULE

MK. DESCRIPTION

6x6 Pressure Treated Wood Posts w/
"Stand-off" Post Bases equal to SIMPSON
Strong Tie Model No. LCB66, 12 Ga on
Concrete Pier Mk. P1.

Coo COLUMN BASE DETAILS on Short S 11-

CONCRETE PIER SCHEDULE					
Conc. f'c = 3,000 psi Reinf.: ASTM A615, Grade 60					
MARK	PIER SIZE	PIER REINFORCING			
P1	12" × 12"	4 - #6 Vert. and dowel to Footing #3 Ties @ 12"			
P2	12" × 26"	8 - #7 Vert. and dowel to Footing 2 - #3 Ties each 12			
See CONCRETE PIER DETA		on sheet S-11			

	UMN FOOTING 9	
MK.	SIZE	REINF. E.W., B.F.
61-011	6'-0" × 6'-0" × 1'-0"	8-#5
51-811	5'-8" × 5'-8" × 1'-0"	6-#5
4'-0"	4'-0" × 4'-0" × 1'-0"	5-#4
31-811	3'-8" × 3'-8" × 1'-0"	4-#4

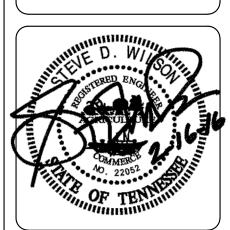
_	CONCRETE SLAB SCHEDULE Concrete flc = 4,000 psi			
М	K.	DESCRIPTION		
6	6)	6" Concrete Slab-on-grade w/ 2 Layers 6x6xW2.9/W2.9 WWF on 4" Compacted Crushed Stone w/ 6 mil. Polyethylene Vapor Barrier		
	्रे	6" Exterior Concrete (f'c = 4,000 psi Air-entrained) Slab-on-grade w/ 2 Layers 6x6xW2.9/W2.9 WWF on 4" Compacted Crushed Stone		

A New Agricultural Facility For Dounty Tennesse

Architectural Services



130 Regional Park Dr. Kingsport, TN 37660 Phn (423) 349-7760 Fax (423) 349-7413 www.grcinc.com

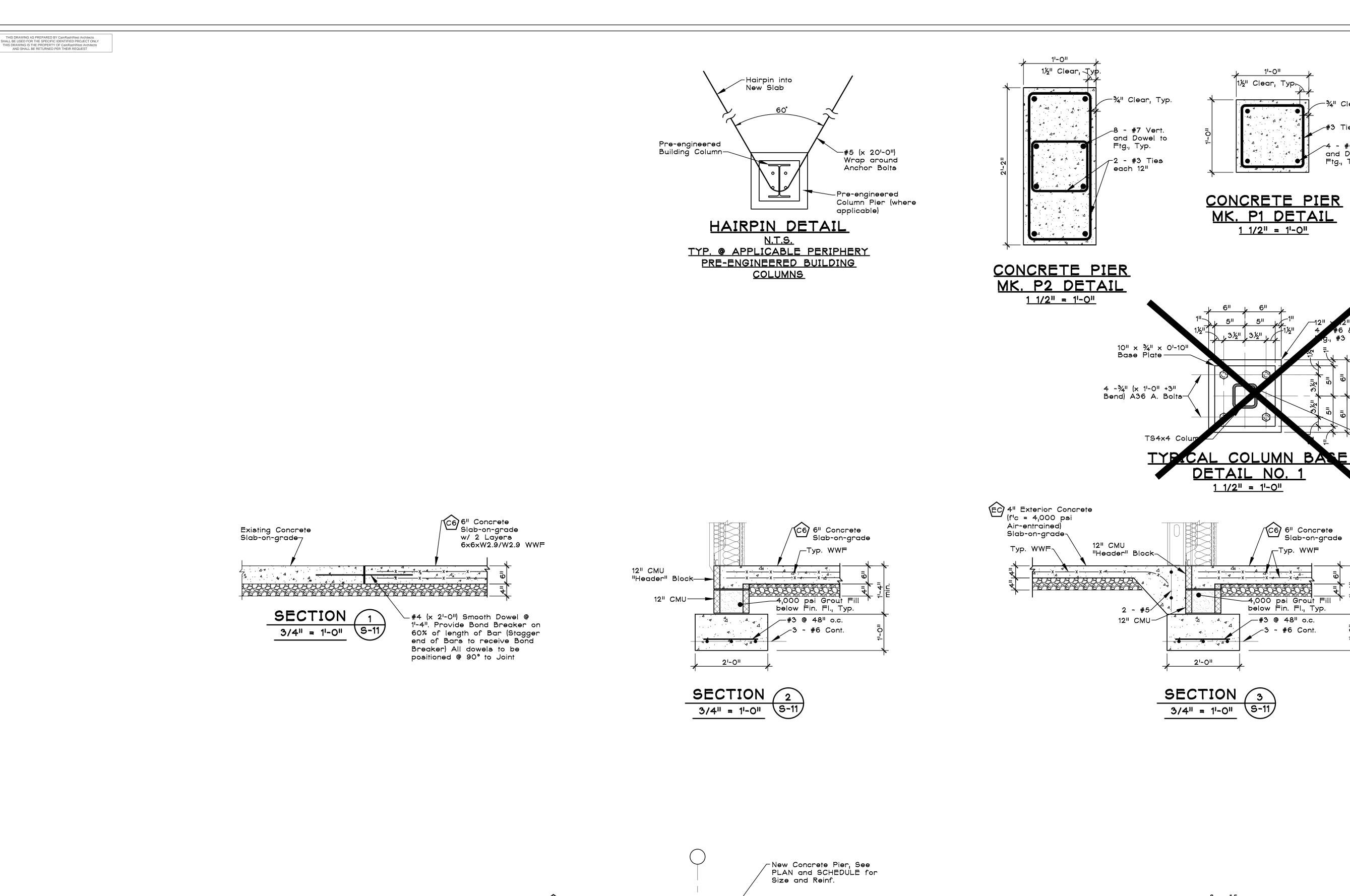


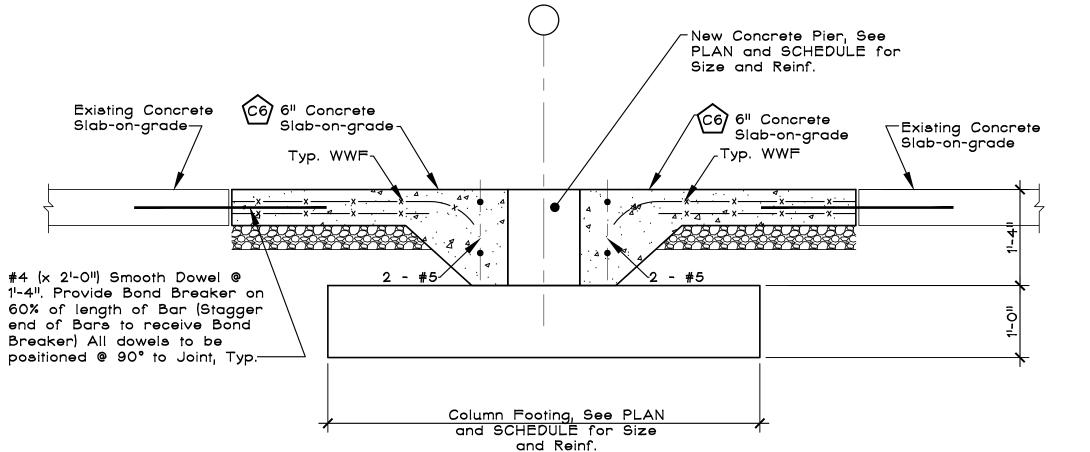
issued	01-29-2016		
checked	SDW		
approved	SDW		
drawn	DRH		
project no. 201538-SW15149			
drawing nai	me		
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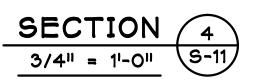
ADDENDUM NO. 1 02-16-2016

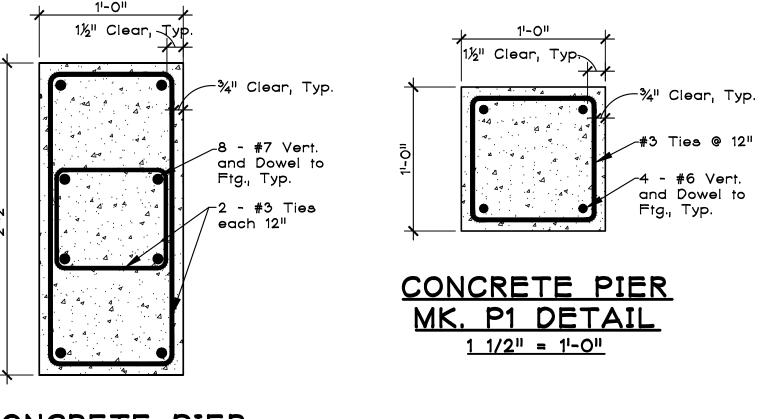
S-10

FOOTING PLAN

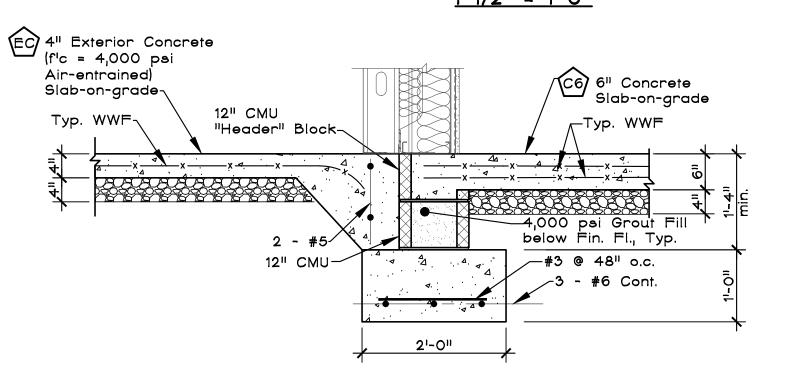


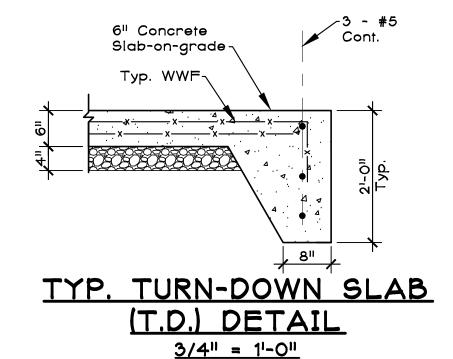






#3 Ties @ 12" DELETE TYP. **COLUMN BASE** DETAIL





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## **GENERAL NOTES:**

### **FOUNDATIONS:**

- 1. FOUNDATIONS FOR PROJECT ARE TO BE SPREAD FOOTINGS AS SHOWN ON THE DRAWINGS.
- 2. SPREAD FOOTINGS DESIGN IS BASED ON ALLOWABLE BEARING VALUE OF 2,000 P.S.F. SPREAD FOOTING SIZES MUST BE REVISED
- IF CONDITIONS INDICATE LOWER VALUES. 3. ALL FILL UNDER SLABS SHALL BE COMPACTED TO MINIMUM SOIL BEARING CAPACITY OF 2,000 P.S.F. MODULUS OF SUBGRADE 100 PCI.
- 4. WALL FOOTINGS TO BE 2'-0" X 1'-0", 3 #6 CONTINUOUS UNLESS OTHERWISE SHOWN, DETAILED OR SCHEDULED. 5. FOOTINGS TO BE STEPPED AT CHANGES IN ELEVATION STEPPED WALL FOOTINGS SHALL BE LIMITED TO SLOPE OF STEPS OF TWO HORIZONTAL TO ONE VERTICAL. LONGITUDINAL REINFORCING TO

### **CONCRETE:**

STEP WITH STEP.

1. ALL CONCRETE WORK SHALL COMPLY WITH A.C.I. 318-05 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

3. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE

- 2. CONCRETE SHALL BE STANDARD WEIGHT AND SHALL ATTAIN 28 DAY COMPRESSIVE STRENGTH OF 3,000 P.S.I. EXCEPT AS NOTED.
- 4. LAP BAR SPLICES 36 DIAMETERS (12" MIN.).

## STRUCTURAL STEEL:

- 1. STRUCTURAL STEEL SHALL BE ASTM A992/A572 (50 KSI). PIPE COLUMNS: ASTM A53, TYPE E OR S. TUBE STEEL: ASTM A500,
- GRADE B. MISC. STEEL SHAPES, ANGLES, ETC. TO BE ASTM A36. 2. ALL BEAM CONNECTIONS ARE TO CONFORM TO A.I.S.C. STANDARD SINGLE SHEAR "TAB" PLATE TO WEB CONNECTIONS CAPABLE OF SUPPORTING 75% OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES FOR "ALLOWABLE LOADS ON BEAMS" UNLESS OTHERWISE NOTED.
- 3. SHOP CONNECTIONS WELDED, A.W.S. SPECIFICATIONS. 4. FIELD CONNECTIONS - HIGH STRENGTH BOLTED WITH ASTM A325 BOLTS (3/4" DIA. EXCEPT AS NOTED). . ANCHOR BOLTS ARE TO BE ASTM A36 AND ARE TO BE FURNISHED BY THE STRUCTURAL STEEL FABRICATOR AND SET BY THE

GENERAL CONTRACTOR. BASE PLATES ARE TO BE GROUTED WITH

# NON-SHRINK GROUT, EMBECO OR APPROVED EQUAL.

1. HORIZONTAL MASONRY JOINT REINFORCING TO BE 9 GAUGE "LADDER TYPE" AT 16" O.C. VERT. EXCEPT AS NOTED AT 8" O.C.

VERTICAL DOVETAIL ANCHOR SLOTS IN CONCRETE.

2. CONCRETE MASONRY UNITS TO BE AS INDICATED ON THESE DWGS. 3. AT ALL AREAS WHERE MASONRY ABUTS CONCRETE, PROVIDE

## WOOD:

MASONRY:

- 1. ALL WOOD FRAMING TO BE NO. 2 SOUTHERN PINE, Fb= 1,200 P.S.I., 19% MOISTURE, VISUAL GRADING RULES OR EQUAL EXCEPT AS
- 2. ALL FASTENERS TO BE GALVANIZED.

#### MASONRY:

- 1. HORIZONTAL MASONRY JOINT REINFORCING TO BE 9 GAUGE "LADDER TYPE" AT 16" O.C. VERT. EXCEPT AS NOTED, AT 8" O.C.
- 2. CONCRETE MASONRY UNITS TO BE AS INDICATED ON THESE DWGS

### LIGHT GAGE METAL:

1. ALL LIGHT GAGE METAL FRAMING INCLUDING JOISTS, STUDS, TRACK, BRIDGING, END CLOSURES, AND ACCESSORIES TO BE ASTM A446, GRADE A (33 KSI), GALVANIZED (G-60) MEETING ASTM A525 AND

## DESIGN LIVE LOADING - OFFICE BUILDING [ARENA BUILDING - ASSEMBLY] (IBC 2012)

= 1.00 [1.15]

= 0.162

Roof Snow Snow Ground Load Snow Exposure Factor, Ce Snow Load Importance Factor, Is Thermal Factor, Ct	= 20 lbs./s.f. = 15 lbs./s.f. = 15 lbs./s.f. = 0.90 = 1.0 [1.10] = 1.0
Floors: Slab-on-Grade	= 250 lbs./s.f.
Wind Basic Wind Speed (3 Second Gust)	= 20 lbs./s.f. = 115 [120] m.p.h.

Wind Exposure Category: Internal Pressure Coefficient: = (+)0.18, (-)0.18= (+)13.3, (-)17.0Components and Cladding:

Seismic: Seismic Importance Factor, IE

Wind Importance Factor, Iw

= 1.0 [1.25] II [III] Occupancy Category Mapped Spectral Response Acceleration, S. = 0.291 Mapped Spectral Response Acceleration, S1 = 0.101 Site Class D Spectral Response Coefficient, Sps = 0.304

Spectral Response Coefficient, S1 Seismic Design Category Basic Seismic-Force-Resisting System(s): Building Frame System: Ordinary Steel

Concentrically Braced Frames Design Base Shear, V Seismic Response Coefficient, Cs

= 50K [140] = 0.10 [0.154] Response Modification Factor, R = 3 Analysis Procedure: Equivalent Lateral Force Procedure; V = CsW

Pre-engineered Building Note: The above listed superstructure live loading criteria to be coordinated with Pre-engineered Building loading criteria as provided by Varco Pruden Buildings. Foundations for this project have been designed based on reaction(s) derivation

from the above Loading Criteria and from Column

Reactions provided by Varco Pruden Buildings.

## New Guardrails and Handrails:

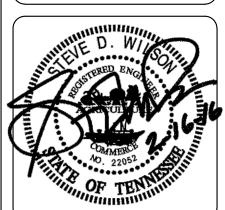
200 lb. Concentrated Load at any point, any direction at top of Rail System or 50 lbs./l.f. horizontally, any direction at top of Rail System all transferred thru supports to the building structure.

> ADDENDUM NO. 1 02-16-2016

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01-29-2016 issued checked approved

project no. 201538-SW15149 drawing name SECTIONS, DETAILS AND GENERAL NOTES

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