

Purchasing Department

209 Water Street Johnson City, TN 37601 (423) 975-2716

ADDENDUM

TO:	All Prospective Vendors
FROM:	Valerie Harless, Assistant Director of Purchasing
SUBJECT:	Addendum No. 1 ITB #6498 J C Schools- Interior Renovations and Classroom Addition-Lake Ridge
DATE:	May 19, 2021

Consider this addendum an integral part of the above referenced Invitation to Bid:

See attached 160 page addendum from Thomas Weems, Architect

Note: Bid Opening Date changed to June 2, 2021 at 3:00 PM

Questions due by 12:00 PM local time on Friday May 28, 2021

All other specifications/requirements remain the same. Failure to acknowledge this addendum could be cause for rejection of your submittal. Your un-opened response envelope can be returned to you for re-submittal upon request. Any questions regarding addendum submittal please contact this office.



ADDENDUM #01

INTERIOR RENOVATIONS & CLASSROOM ADDITION FOR: LAKE RIDGE ELEMENTARY SCHOOL

1001 Lake Ridge Square | Johnson City, Tennessee 37601 TFM # 09665 TFM Project No. 2021-03-24-03



May 18, 2021

This Addendum forms a part of the Contract Documents and modifies the original Procurement Documents dated March 25, 2021. Bidders shall acknowledge receipt of this Addendum in the space provided on the Bid Form. Failure to do so may subject Bidder to disqualification.

This **160** page Addendum is issued *VIA Email* to the Owner, All Known Plan Holders of Record, and All Known Plan Rooms of Record on May 18, 2021.

CHANGES TO PROJECT MANUAL:

- 1. Refer to 00 1113 Advertisements for Bids
 - a. The bid date found in the first paragraph should be modified as follows: *"Wednesday, June 2, 2021 at 3:00 PM local time"*
- 2. Refer to 00 2113 Instructions to Bidders
 - Modify paragraph 3.2.2 as follows:
 "Submit questions in electronic format via email to: katie@thomasweemsarchitect no later than Friday, May 28, 2021 at 12:00 PM local time"

3. Refer to 00 2113.1 Supplementary Instructions to Bidders

- a. Modify paragraph 1.6 Questions as follows: "DATE: **Friday, May 28, 2021** at 12:00 PM local time"
- 4. Refer to 01 1100 Summary of Work
 - a. Modify paragraph 1.6 Phase Construction to *remove all references to completion dates*. Contractor to propose construction phasing order and schedule.
- 5. Refer to 01 2100 Allowances
 - Modify paragraph 3.1.H Allowance No. 8 as to add items noted below:
 2. [\$460,000.00] Four Hundred Sixty Thousand Dollars and Zero Cents of the total Trane HVAC Equipment Allowance is for controls labor subcontract and is not subject to sales taxes and is Non-Taxable. The remaining [\$728,129.00] Seven Hundred Twenty Eight Thousand One Hundred Twenty Nine Dollars and Zero Cents is subject to Sales Tax.



3. Sales Tax is not included in the Trane HVAC Equipment Allowance and is to be calculated by the Contactor and included in the Base Bid cost.

- 6. Refer to 04 0516 Masonry Grouting
 - a. Modify paragraph 2.3.B.2 as follows: "Compressive strength: Minimum strength **[4,000]** psi at [28] days."
- 7. Refer to 08 1113, Section 2.1 Manufacturers
 - a. Add CertainTeed as an acceptable manufacturer of a comparable product.
 - b. Substitution Request approved
- 8. Refer to 09 5100 Acoustical Ceilings, Section 2.1 Acceptable Manufacturers
 - a. Add Metal Products, Inc. as an acceptable manufacturer of a comparable product.
 - b. Substitution Request approved
- 9. Insert 12 2113 Horizontal Louver Blinds
- 10. Insert 22 0500 Common Work for Plumbing

CHANGES TO CONSTRUCTION DRAWINGS:

- 1. Refer to **G004 Phasing Plan**
 - a. Remove all references to completion dates. Contractor to propose construction phasing order and schedule.
- 2. Insert revised C1.1 Notes & Legends
- 3. Insert revised C2.1 Existing Condition and Demolition Plan
- 4. Insert revised C3.1 Layout And Utility Plan
- 5. Insert revised C4.1 Grading Plan
- 6. Insert revised C4.2 Storm Sewer Profiles
- 7. Insert revised C5.1 Erosion & Sediment Control Plan Phase 1
- 8. Insert revised C5.2 Erosion & Sediment Control Plan Phase 2
- 9. Insert revised C5.3 Erosion & Sediment Control Details
- 10. Insert revised C6.1 Miscellaneous Details
- 11. Insert revised C6.2 Miscellaneous Details



12. Insert revised AS011 – Architectural Site Plan

a. Graphics updated to coordinate with Civil drawings and indicate new paving.

13. Insert revised A601 – Door Schedule and Details

a. Revised door ratings

14. Refer to A800 – Finish Schedule

- a. Revise all references to "CT-1" to "RB-2"
- b. Revise base in Stair 195, Stair 255, Roof Stair 500A to "RB-2"

15. Refer to Electrical Drawings E101B, E101C, E101D, E103A, E201B and E203A

a. Refer to attached Electrical Addendum dated May 13, 2021.

GENERAL ITEMS:

- 1. Insert attached Pre-Bid Conference Agenda
- 2. Insert attached Pre-Bid Sign In Sheet
- 3. Insert revised Stormwater Management Plan dated 4/21/2021
- 4. Insert attached Electrical Addendum dated May 13, 2021
- 5. Insert **PTA Request for Payment Form dated 9/6/19**. Cost of Owner-provided, Contractor-Installed Sunshade listed.
- 6. Insert Rejected Substitution Request Quaker Windows. Not an equal product.

RESPONSE TO QUESTIONS:

Refer to the following responses to questions received by the Architect:

- 1. Given that the board will not meet to approve the project/contractor until the third week of June, I'd like to request the bid date be moved back to June 8th.
 - a. Bid Date revised to June 2, 2021. This is the latest date possible in order to prepare the agenda for the June 17, 2021 meeting.
- 2. The bid form included in specification section 00 4100 list the project name as Addition and Alterations to Existing Building Indian Trail Intermediate School.
 - a. Corrected bid form will be provided prior to bid date.
- Structural Drawings call for grout to be 4000 psi, project manual calls for 3000 psi. Which is correct?
 a. Refer to 04 0516 Masonry Grouting modification above.
- 4. Drawing A800 calls for the CT-1 base and none listed on finish description. No RB-2 shown on finish schedule.
 - a. Refer to modifications noted above.
- 5. The project manual calls out spec section 12 2113 Horizontal Blinds but there is no spec section under this heading or number.



a. 12 2113 Horizontal Blinds attached

- 6. With no Geotechnical Report available for this project will there be consideration for unit pricing for unsuitable soils or rock excavation since no information provided.
 - a. Yes. Refer to revised 00 4100 Bid Form and 01 2200 Unit Prices
- 7. With Spec 06-4600 Wood Trim: Is this specification section applicable to this project. No finish wood trim shown on drawings
 - a. Wood trim used at the junction of GWB and CMU. Refer to section details.
- 8. Detail 10&11/A414, 8/A612, 3/A613: Double 2x blocking shown on these details will fastened to a face block with an expansion anchor. This will not carry any load.
 - a. Detail 10 and 11 / A414: Double 2x blocking is not shown on either detail.
 - b. Detail 8 / A612: The detail shows that the double 2x's create a shelf for the bull nose sill. Not structural.
 - c. Detail 3 / A613: Standard head detail. The double 2x's create a shelf for the thru-wall flashing so any moisture doesn't pond. Not intended to be structural.
- 9. Provide the cost of the Owner-provided, Contractor-installed sunshade.
 - a. Refer to attached PTW Request for Payment Form.
- 10. Is the fire alarm new throughout the school or for the new addition only?
 - a. Refer to attached Electrical Addendum.

ATTACHMENTS:

- 1. Project Manual (2 sections 9 pages 8 ¹/₂" x 11")
- 2. Construction Drawings (12 sheets 30" x 42")
- 3. Stormwater Management Plan Report (112 pages 8 1/2" x 11")
- 4. Pre-Bid Agenda (6 page 8 ¹/₂" x 11")
- 5. Pre-Bid Sign-In (5 pages 8 ½" x 11")
- 6. PTA Form (1 page 8 ½" x 11")
- 7. Substitution Requests (3 requests 9 pages 8 1/2" x 11")

END OF ADDENDUM NO. 1

Cc: Randy Trivette – City of Johnson City Dennis Rhodes, Plans Examiner 2, TN State Fire Marshal's Office

SECTION 12 2113

HORIZONTAL LOUVER BLINDS

PART 1 GENERAL

1.1 SUMMARY

- A. Section Includes:
 - 1. Horizontal slat louver blinds.
 - 1. Operating hardware.

B. Related Sections:

- 1. Division 01: Administrative, procedural, and temporary work requirements.
- 2. Section 04 2000 Unit Masonry.
- 3. Section 06 1000 Rough Carpentry.
- 4. Section 09 9100 Painting.

1.2 REFERENCES

- A. National Fire Protection Association (NFPA):
 - 1. 701 Fire Tests for Flame-Resistant Textiles and Films.

1.3 SUBMITTALS

- A. Product Data: Submit Manufacturer's product data and installation information per type of blind specified.
- B. Sample: Submit [3] inch long slat samples in each color.
- C. Warranty: Submit Manufacturer's standard warranty form.

1.4 QUALITY ASSURANCE

A. Installer Qualifications: Minimum Five [5] years documented experience in work of this Section.

1.5 DELIVERY, STORAGE AND HANDLING

- A. Deliver to job site in manufactures original packaging.
- B. Packaging shall include labels with designation if required.
- C. Shades shall be stored horizontally in a way to prevent damages from accidents and water.
- D. Area of installation shall be enclosed and dry. All work operations above area of installation on ceilings, must be completed before installing.

1.6 **PROJECT CONDITIONS**

- A. Verify dimensions at site prior to fabrication of blinds.
- B. Do not install blinds until painting and finishing work is complete.

1.7 WARRANTY

A. Warranty: Provide manufacturer's standard warranty against defects in materials and manufacturing.

PART 2 PRODUCTS

2.1 MANUFACTURERS

- A. Design Basis Horizontal Louver Blinds: Contract Documents are based on products by: 1. Caco. Inc.
 - 119 Perma R Road Johnson City, Tennessee 37604 Telephone: 800-552-5278 Website: (<u>www.cacoinc.com</u>)
- B. Substitutions: Under provisions of Division 01.

2.2 HORIZONTAL LOUVER BLINDS

- A. Custom Signature Series [2] inch Aluminum Maxi Horizontal Maxi Blinds:
 - 1. Louver Slats: [2] inches wide, prefinished spring tempered aluminum, horizontal slats with radiused corners.
 - 2. Slat Support: Woven polypropylene ladders.
 - 3. Head Rail: Prefinished, formed aluminum or steel box, internally fitted for hardware, pulleys, and bearings for blind operation.
 - 4. Cord: Braided nylon or polypropylene.
 - 5. Control Wand: Hollow extruded plastic, height of window opening less [12] inches.
 - 6. Support Brackets: Suitable for wall or soffit mounting, formed metal to match head rail, allowing removal of head rail for maintenance without removing bracket.
 - 7. Operation: Full range lift locking.

2.3 FABRICATION

- A. Fabricate blinds to fit openings with uniform edge clearance of [1/4] inch.
- B. At openings requiring multiple blind units, provide separate blind assemblies with space of [1/4] inch between assemblies, occurring at window mullion centers.

2.4 FINISH

- A. Slats: Baked enamel, Color: To be selected from Manufacturer's standard colors.
- B. Head Rails and Brackets: Baked enamel, color to match slats.
- C. Ladders and cords: Dyed to closely match slats.
- D. Control Wands: Clear.

PART 3 EXECUTION

3.1 EXAMINATION

- A. Work area in which blinds will be installed should be free of conditions that interfere with blind installations and operations.
- B. Begin blind installation only when unsatisfactory conditions have been corrected.

3.2 INSTALLATION

- A. Install blinds in accordance with manufacturer's instructions.
- B. Secure with concealed fasteners.

- C. Place intermediate head supports at maximum [48] inches on center.
- D. Install intermediate support brackets and extension brackets as needed to prevent deflection in head rail.
- E. Installation Tolerances: Install blinds with adequate clearance to permit smooth operation of blinds and any sash operations.
 - 1. Maximum gap at window opening perimeter: [1/4] inch.
 - 2. Maximum offset from level: [1/8] inch.

3.3 ADJUSTING

A. Adjust shades for smooth, quiet operation.

END OF SECTION

SECTION 22 0500 COMMON WORK FOR PLUMBING

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. Drawings and general provisions of Contract, including General and Supplementary Conditions and Division 1 Specification sections, apply to work of this section.

1.2 WORK INCLUDED

A. The work required under this section of the specifications consists of basic materials and methods and is applicable to all work under Division 22.

B. The work of this section is subject to the requirements of the 22 05 01 General Work for Plumbing section of the specifications.

PART 2 - PRODUCTS

2.1 MOTORS

A. Furnish and install (or arrange for installation) all electric motors for all equipment specified under this section requiring same in accordance with the following:

1. All motors shall be NEMA standard designed for ample size to operate at their proper load and full speed continuously without causing noise or vibration or temperature rise in excess of their rating.

2. Motors 1/2 HP and less shall be designed & nameplated for 120 volt, 1 phase, 60 cycle operation; shall be permanent split capacitor type, 40 degrees Celsius continuous rise, open dripproof type; and shall be equipped with ball bearings.

3. All motors 3/4 HP and larger (unless specified otherwise) shall be designed and nameplated for 3 phase, 60 cycle operation, shall be single speed squirrel-cage type, NEMA Design B, normal starting torque, open dripproof type, quiet operating, 40 degrees Celsius continuous rise and shall be equipped with ball bearings.

4. All three phase motors one horsepower and larger shall comply with NEMA MG-1 Table 12-11 Energy Efficient standards or Table 12-12 Premium Efficient Standards as applicable to meet the Energy Independent Security Act of 2007.

B. The above shall apply to all motors unless otherwise specified with equipment.

2.2 STARTERS

A. Provide motor starters for all equipment under this division of the specifications. Installation shall be as specified in Division 26 of these specifications. Unless built-in as an integral part of the equipment or of custom design for specific application, all starters shall be the product of a single manufacturer. Starters shall meet requirements of current National Electric Codes.

B. All starters shall have overload protection. Starters shall have phase failure and undervoltage relay similar to Square D Type MPS, with built-in adjustable time delay response (3 second minimum). Undervoltage setting is adjustable from 75% to 100%. Starters shall have all necessary auxiliary interlocks required for operation of the respective systems, plus one spare auxiliary interlock. Starters shall have NEMA 1 general purpose enclosures.

C. All starters shall be manufactured by Cutler-Hammer; equal by General Electric, Square D, or Westinghouse are acceptable.

1. Single pole, 120 volt, 1 phase, 60 cycle manual (unless noted otherwise) starter for motors 1/2 HP and less. Note: Where motors have built-in thermal overload protection and starter is not required to accomplish control scheme, manual starter may be omitted.

2. Full voltage, three-pole, combination magnetic starter with fused disconnect with Bussman Low-Peak or Fusetron dual element fuses for all 3 phase motors. Fuses shall be sized in accordance with N.E.C. for all 3 phase motors.

3. For motors larger than 25 HP, the starting equipment of the resistance type, increment start, induction type or a combination of resistance and induction starting shall be used to limit the first step of the starting voltage, to not more than 65% of the line voltage or as required by the local electrical utility.

D. In addition to the features described above, the starters furnished shall include the following features:

1. All starters for 3 phase motors shall have 3 phase thermal overload protection. Size the heater overload elements to properly protect the motor being served. Heaters shall not be sized to be any larger than 115% of full-load amps, heater element furnished, and rating range of heater element in tabulate form.

2. Starters on all, 3-phase, 60 cycle electrical service shall have a 120 volt control circuit obtained from a fused control transformer built into the starter. Transformer shall be fused on each of the two lines. Fuses and transformers shall be sized to carry the holding coil circuits and any miscellaneous devices included plus 50-VA.

E. All starters shall have maintained contact hand-off-automatic switch & reset button in cover. All motors shall automatically restart after power loss is restored when set in automatic setting.

2.3 VIBRATION ISOLATION EQUIPMENT

A. Isolation shall conform to seismic requirements of Section 22 05 29 Hangers and Supports for Plumbing." Unless otherwise noted, equipment over 1 horsepower shall be isolated from the structure with resilient vibration and noise isolators supplied by Kinetics or Mason Industries to the Mechanical Installer. Where isolator type and required deflection are not shown, equipment shall be isolated in accordance with the ASHRAE systems book. Submittal shall include the complete design for the supplementary bases; a tabulation of the design data on the isolators including O.D., free operating and solid heights of the springs, free and operating heights of the neoprene or fiberglass isolators. Mounts and bases shall be manufactured by Peabody Noise Control or Mason Industries.

B. Model KIP-Q shall be precompressed molded fiberglass isolation pads, neoprene-jacketed and stabilized during manufacture. Pads shall be sized for 40 to 60 psi loading and shall be made of glass fibers produced by a multiple flame attenuation process which generates nominal fiber diameters not to exceed .00018 inches. Where the equipment base does not provide a uniform load surface, steel plates shall be bonded to the top of the pads. Model RD neoprene mounts shall incorporate completely enclosed metal inserts to permit bolting to the supported unit.

C. Model FDS shall be freestanding, unhoused, laterally stable spring mounts, incorporating leveling bolts and 1/4" thick noise isolation pads. To assure stability, the outside spring diameter shall be equal to or greater than the designed spring operating heights, and the horizontal stiffness. Springs shall have a minimum additional travel of 50% between the designed operating height and the solid height.

D. Model SFH shall be combination spring and fiberglass hangers, incorporating 2" thick neoprenejacketed precompressed molded fiberglass inserts in series with springs, all encased in welded steel bracket. The outside spring diameter shall be a minimum of 0.8 times the designed spring operating height, and shall have a minimum additional travel of 50% between the design height and solid height. E. Model FLS shall be freestanding, stable spring mounts, similar to Type FDS. They shall incorporate vertical limit stops to assure a constant height if the supported weight is removed, and to reduce movement due to wind load. The limit stops shall be isolated.

F. Model FYS spring isolators shall be seismic control restrained spring isolators, shall incorporate a single vibration isolator, having all of the characteristics of Model FDS springs as previously specified. Springs shall be assembled into a welded steel housing assembly engineered to limit movement of supported equipment during an earthquake without degrading the vibration isolation of the spring during normal equipment operating conditions. Vibration isolators shall incorporate a steel angle and plate motion limit assembly, and steel spring isolator, engineered as a system to accept a force in any direction equal to a minimum of 1.0 times the rated load capacity of the spring isolator without yield or failure, and shall limit movement of the point of level bolt connection to supported equipment to less than 1/2" in any direction, relative to any fixed point on the isolator assembly, while subjected to the rated force specified. The motion limit assembly shall be welded to a steel base plate having a 1/4" thick ribbed neoprene noise stop pad, and drilled holes for bolting to the supporting structures. A spring isolator, drilled and tapped load plate and leveling bolt assembly shall be positioned on the base plate, and shall carry all normal equipment operating loads.

G. All piping and electrical conduit in the mechanical equipment room and piping three supports away from other mechanical equipment shall be isolated from the structure by means of vibration and noise isolators. Suspended piping shall be isolated with Model SFH Hangers as described above. Floor mounted piping shall be isolated with FDS Spring Mounts as described above.

H. Flexible pipe connectors shall be incorporated in all piping connections to chillers, pumps and air handling units. Flexible pipe connectors shall be equal to Mason Industries, Inc. Type MFTNC, Neoprene-twin-sphere with floating flanges and control cables. Installation of the flexible connector and anchoring of the piping shall be in strict accordance with the manufacturer's directions.

I. Flexible connections shall be incorporated in the ductwork adjacent to all air-moving units. The connections shall be neoprene or canvas of approved construction.

2.4 ACCESS PANELS

A. Provide access panels not less than 24" x 24" for access to all concealed valves, unions, dampers, etc., where no other means of access is provided. Access panels shall be all steel construction with a 16 gauge ceiling frame and 16 gauge panel door. Doors shall be secured with concealed hinge and flush locks of either the cylinder type of screwdriver-operated type. Outside of door and frame shall be flush with finished wall or ceiling. Panels shall be painted with a rust-inhibitive primer at the factory. Panels shall be installed in openings provided under the construction sections of the specifications, and the work of the trades involved shall be coordinated as necessary. Access panels shall be so located and of sufficient size to permit service of components. Panels located in fire rated walls or ceilings shall be U.L. listed for rating equal to or greater than where they are installed.

PART 3 - EXECUTION

3.1 EXCAVATION AND BACKFILL FOR PIPE

A. Do all excavating and backfilling required for installation of underground work required by the mechanical work.

B. Excavating and backfilling shall comply with all applicable provisions of Section for Earthwork, including the provisions therein concerning classification of excavated materials. Any backfill in the area of the building shall conform to the requirements for engineered fill as specified in Section for Earthwork.

C. Unless otherwise shown or required, by the State Department of Health, provide separate trenches for sewer and water lines, respectively, with a minimum of 5 feet between lines and a minimum of 3 feet of

undisturbed earth between trenches. In locations, such as close to a building where separate trenches for sewers and water lines are not practical, lay the water pipe on a solid shelf at least 12 inches above the top of the sewer.

D. Sheeting, Bracing, Water Removal

1. Sheet and brace trenches, and remove water, as necessary to fully protect workmen and adjacent structures and permit proper installation of the work. Comply with local regulations or, in the absence thereof, with the provisions of the "Manual of Accident Prevention in Construction", of the AGC. Under no circumstances lay pipe or install appurtenances in water; keep the trench free from water until pipe joint material has hardened. The presence of ground water in the soil or the necessity of sheeting or bracing trenches shall not constitute a condition for which an increase may be made in the Contract Price.

2. Sheeting left in place shall be cut off not less than 2 ft. below finished grade. Sheeting shall not be removed until the trench is substantially backfilled.

E. Grading Trench Bottoms

Grade the bottom of trenches evenly to ensure uniform bearing for the full length of all pipes. Cut holes as necessary for joints and joint making. Excavate all the rock, cemented gravel, old masonry, or other material to at least 6" below the pipe at all points. Refill all cuts below grade with sand or fine gravel firmly compacted; the necessity of refill material shall not constitute a condition for which an increase may be made in the Contract Price.

F. Piping Inverts

All piping outside of building footprint shall have a minimum cover of 18" unless noted otherwise.

G. Bedding of Pipe

1. All pipe shall be installed on a minimum bedding of 6" of Class 1 embedment materials (1/4" to 1 1/2" graded stone).

2. Embedment material shall be placed in the trench to a sufficient height so that upon completion of compaction as required in the specifications that entire upper surface of the gravel shall be no lower than the bottom of the barrel of the pipe. Bell holes shall be made in the embedment so that the pipe shall be supported on its barrel portion only and the pipe laid to line and grade in the manner described in the specifications.

H. Special Supports

Should latent soil conditions, other than hard material as referred to above necessitate special supports for piping and appurtenances, including the removal of unsuitable material and refilling with gravel or other material, perform such work as directed by the Architect.

I. Backfilling

Notify architect and local inspecting authority before backfilling trenches. Tests and locations of pipe and appurtenances shall be recorded. Backfill by hand around pipe and for a depth of 12" above the pipe. Use Class 1 angular 1/4" to 1 1/2" graded stone and tamp firmly in layers not exceeding 6" in thickness, taking care not to disturb the pipe or injure the pipe coating. Compact to 95% density under building, sidewalks, and paved areas.

J. Tracer Wire

A insulated copper tracer wire or other approved conductor shall be installed adjacent to any underground nonmetallic piping. Access shall be provided to the tracer wire or the wire shall terminate above the ground

at each end of the nonmetallic piping. The tracer wire size shall not be less than 18 AWG and the insulation type shall be suitable for direct burial.

3.2 PIPING INSTALLATION

A. In general, install all piping as neatly as practicable as indicated and detailed on the drawings. Arrange and install piping straight, level, plumb, and as direct as possible. Form right angles and parallel lines with the structures. Keep pipes close to walls, partitions, ceilings, and slabs where possible. Where two or more pipes are located together, run parallel to each other and space at distances which will permit application of full insulation and access for servicing.

Β. Unless noted otherwise, connect all apparatus and equipment in accordance with the manufacturer's standard details as approved. Provide necessary piping, such as vent, relief, etc., wherever equipment is provided with connections for such piping. Unions or flanged connection shall be placed where necessary to permit easy dismantling of piping and apparatus and in connections to all equipment between shutoffs and the equipment. Each control valve shall have union or flanged connection immediately adjacent or be flare connected. All piping and apparatus connections shall be so installed as to avoid interference with tube or electrode removal from hot-water boilers, air-handling units, and domestic water heater, etc., and to allow for removal of an item of equipment without disturbing other items of equipment. Ream all pipe ends after cutting. All blow-off piping shall be permanently installed to indirect wastes. All pipe size changes shall be made with pipe reducer fittings or, if applicable, with reducing fittings. Piping shall be carefully installed to provide for expansion and for proper alignment. Pipe lines shall be guided and pipe shall be supported in such a manner that it will not creep, sway, or buckle. Anchors and supports shall be provided wherever necessary to prevent misalignment. Wherever possible, long radius elbows shall be used and not short radius. Eccentric reducers shall be used wherever necessary or indicated; concentric reducers and reducing fittings shall not be used where air trapping may occur. All pipe fittings shall be factory fittings.

C. Joints:

1. Sweat joints in copper tubing shall be with approved alloys. Lead free solders and fluxes that contain not more than 0.2 percent lead (per the Safe Water Act Amendments of 1986, Public Law 99-339) shall be used when joining copper to copper. Silver solders (95 tin - 5 silver) shall be used when joining copper with bronze or steel, and when joining Type K copper to copper, and for any copper joint below floor slab. The filler metal shall conform to AWS A5.8.

2. Dielectric brass adapters, brass unions, or brass bushing shall be used wherever dissimilar metals subject to galvanic activity are joined together, such as equipment connections, tank connection, etc.

3. Piping installer shall use neoprene gasketed compression joints on cast iron pipe.

D. Nipples:

1. All steel pipe nipples shall be threaded steel nipples, galvanized or black to match pipe.

- 2. All nipples used in conjunction with copper pipe shall be brass.
- E. Pipe Sleeves:

1. Fabricate from steel pipe having internal diameter not less than 1" larger than outside diameter of pipe. Length of sleeve shall extend full depth of construction pierced, and in the case of floor slabs, additionally extend 2" above top of slab.

2. Insert sleeves in forms before pour of floor & roof slabs, install sleeves as wall goes up for concrete block walls. Securely fasten sleeves to structure.

F. Protection of Floor or Wall Penetrations by Piping:

1. Provide ProSet Systems U.L. fire rated sleeve coupling Penetrators for each pipe penetration or fixture opening passing through fire rated floors, walls, partitions or floor ceiling assemblies. All Penetrators shall comply with ASTM E-814 or U.L. 1479 fire test standards.

2. Sleeve Penetrators shall have a built in anchor ring for waterproofing and anchoring into concrete pours or use the special fit Cored hole Penetrator for cored holes.

3. Copper and Steel piping shall have ProSeal Plugs on both sides of the penetrator to reduce noise and waterproof.

a. Copper and Steel insulated pipe - Use Systems "A".

b. PVC waste and vent piping - Use System "C".

4. All above systems to be installed in strict accordance with the manufacturer's instructions.

5. Alternate Firestopping Systems are acceptable if approved as "an approved substitute." However, any deviation from the above specification requires the Contractor to be responsible for determining the suitability of the proposed products and their intended use, and the Contractor shall assume all risks and liabilities whatsoever in connection therewith.

G. Valves: Install valves and hosecocks as shown on the drawings, and specifically in the cold water main entering the building, at runouts from mains to risers and or all branch lines feeding from mains on domestic cold water, and at entering and leaving sides of all equipment as necessary to isolate and service this equipment.

3.3 PROTECTION

A. Do not install any water piping over electrical switchgear. Provide galvanized sheet metal gutter, having 1-1/2" pipe drain to floor away from affected areas, for any water or drain piping having to cross the switchgear.

B. Electrical Ground: Notify electrical installer regarding location of any valves whose future removal for service will break the electrical grounding system.

3.4 CLEANING

A. Exercise care to keep all piping clear and free from foreign matter at all times.

B. After reaming, if cutting is required, clean each piece of all loose scale, dirt, etc.

C. Keep installed piping free from dirt and scale and protect open ends to prevent foreign matter entering. Use temporary plugs, caps, or other approved method of open and closure.

D. Defective, leaking, or otherwise unsatisfactory joints or material shall be remade or replaced. Peening, caulking, doping, etc., will not be permitted.

3.5 PAINTING OF MECHANICAL PIPING & METAL SURFACES

A. All finish will be performed under Division 9.

B. The equipment installer shall touch up all scratches, abrasions, etc., in either the prime or finish coats of all equipment and material furnished and installed by him. All rust and corrosion shall be removed from pipe, fittings, and other metal surfaces. All surfaces shall be left in a clean "factory-new" condition.

END OF SECTION

NOTIFICATION NOTES 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INFORMING THE OWNER IN WRITING TWO WEEKS PRIOR TO THE START OF CONSTRUCTION AND PRIOR TO REQUESTING FINAL INSPECTION AND PAYMENT.	12.PROOF ROLLING WITH A FULLY LOADED TANDEM DUMP TRUCK SHALL BE PERFORMED AS FOLLOWS: a. IN AREAS TO RECEIVE FILL, PROOF ROLLING SHALL BE PERFORMED AFTER TOPSOIL AND ORG
2. THE CONTRACTOR MUST GIVE ADEQUATE NOTICE TO ALL UTILITY OWNERS PRIOR TO EXCAVATION. THE CONTRACTOR SHALL NOTIFY EACH INDIVIDUAL UTILITY OWNER OF HIS PLAN OF OPERATION IN THE AREA OF THE UTILITIES. PRIOR TO COMMENCING WORK, THE CONTRACTOR SHALL CONTACT THE	MATERIAL HAS BEEN STRIPPED AND BEFORE FILLING OPERATIONS ARE INITIATED. b. IN CUT AREAS, PROOF ROLLING SHALL BE PERFORMED ON SUBGRADE AFTER FINISHED GRADES OBTAINED. c. IN ALL AREAS PRIOR TO PLACEMENT OF STONE BASE MATERIALS. ANY SOFT AREAS SHALL BE
UTILITY OWNERS AND REQUEST THEM TO PROPERLY LOCATE THEIR RESPECTIVE UTILITY ON THE GROUND. THE CONTRACTOR SHALL NOTIFY TENNESSEE ONE CALL SERVICE AT LEAST 3 WORKING DAYS PRIOR TO EXCAVATING. THE TENNESSEE ONE CALL SERVICE CAN BE CONTACTED BY CALLING 811 OR	SCARIFIED AND RECOMPACTED TO THE REQUIRED DENSITIES. 15.ALL STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF NO MORE THAN 8 INCHES. STRUCTURAL F SHALL BE COMPACTED TO 98% MAXIMUM DRY DENSITY (ASTM D698) WITH A MOISTURE CONTENT W
1-800-351-1111. 3. THE CONTRACTOR SHALL MAINTAIN A LIST OF EMERGENCY CONTACTS WITH EACH AFFECTED UTILITY FOR CASES OF ACCIDENTAL UTILITY DAMAGE.	2% OF OPTIMUM, OR AS DIRECTED BY A GEOTECHNICAL ENGINEERING FIRM. 16.ALL SIGNS, PAVEMENT MARKINGS, AND OTHER TRAFFIC CONTROL DEVICES SHALL CONFORM TO TH
4. THE CONTRACTOR WILL BE SOLELY RESPONSIBLE FOR CONTACTING ALL AFFECTED UTILITIES PRIOR TO SUBMITTING A BID TO DETERMINE THE EXTENT TO WHICH UTILITY RELOCATIONS AND/OR ADJUSTMENTS WILL HAVE UPON THE SCHEDULE OF WORK FOR THE PROJECT.	MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND "THE AMERICANS WITH DISABILIT ACT", AS APPLICABLE. 17.ALL STRIPING SHALL BE 4" WIDE WHITE TRAFFIC PAINT. PAINT SHALL BE NON-BLEEDING, QUI
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INFORMING THE UTILITY INSPECTION DIVISION IN WRITING THREE WEEKS PRIOR TO THE START OF UTILITY CONSTRUCTION, MAKING SERVICE CONNECTIONS, AND REQUESTING FINAL INSPECTION FOR APPROVAL BY THE UTILITY.	DRYING, ALKYD PETROLEUM BASE PAINT SUITABLE FOR TRAFFIC-BEARING SURFACE AND SHALL M FS TTP-85E AND BE MIXED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS BEFORE APPLICATION.
6. IF AN EXISTING UTILITY IS DAMAGED DURING CONSTRUCTION, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE UTILITY INSPECTOR AND RECORD THE LOCATION OF THE INCIDENT ON THE AS-BUILT	18.UNLESS OTHERWISE INDICATED, ALL EXCAVATION SHALL BE UNCLASSIFIED. 19.IF TEMPORARY FENCING IS REQUIRED, IT SHALL BE SUPPLIED BY THE CONTRACTOR
RECORD. 7. EXISTING UTILITY SERVICES SHALL BE MAINTAINED. WHERE SERVICE INTERRUPTION IS UNAVOIDABLE, COORDINATE WITH THE UTILITY OWNER AT LEAST 3 DAYS (72 HOURS) IN ADVANCE REGARDING THE	20.STORM, SURFACE, NUISANCE, OR OTHER WATERS MAY BE ENCOUNTERED AT VARIOUS TIMES DURIN CONSTRUCTION OF THE PROJECT. THE CONTRACTOR SHALL ASSUME ANY AND ALL RISKS AND LIABILITIES ARISING THEREFROM.
DURATION OF THE INTERRUPTION AND THE PROPER RESTORATION OF SERVICE. CONNECTION TO EXISTING PUBLIC FACILITIES SHALL BE MADE ONLY UNDER THE SUPERVISION OF A REPRESENTATIVE OF THE UTILITY. LIMIT INTERRUPTIONS TO OCCUR AND BE COMPLETED ON MONDAY THRU THURSDAY, 9:00 AM TO 4:00 PM. NO INTERRUPTIONS SHALL OCCUR ON FRIDAYS, WEEKENDS, OR HOLIDAYS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFICATION TO AFFECTED CUSTOMERS (CONTACT THE UTILITY FOR NOTIFICATION REQUIREMENTS). FOR SHUTDOWNS LONGER THAN 4 HOURS, CONTRACTOR SHALL SUBMIT A TEMPORARY PLAN TO KEEP ALL CUSTOMERS IN SERVICE. ALL COSTS ASSOCIATED	21.THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING ALL CONDITIONS SPECIFIED IN THE PER FOR THIS PROJECT. ANY FINES ISSUED FOR NON-COMPLIANCE WITH THE PERMIT CONDITIONS SH BE PAID BY THE CONTRACTOR STORMWATER QUALITY NOTES
WITH MAINTAINING SERVICE TO AFFECTED CUSTOMERS SHALL BE BORNE BY THE CONTRACTOR. THE CONTRACTOR SHALL HAVE ALL PERTINENT PARTS AND MATERIALS ON SITE PRIOR TO SHUTDOWN OF THE UTILITY SYSTEM. CONSTRUCTION EXCAVATION MUST BE PREPARED AND THE UTILITY EXPOSED PRIOR TO SHUTDOWN OF THE SYSTEM.	 THE APPROVED STORM WATER MANAGEMENT PLAN (SWMP) SHALL BE ADHERED TO DURING GRADING CONSTRUCTION ACTIVITIES. UNDER NO CIRCUMSTANCE IS THE OWNER OR OPERATOR OF LAND DEVELOPMENT ACTIVITIES ALLOWED TO DEVIATE FROM THE APPROVED SWMP WITHOUT PRIOR APPR OF A PLAN AMENDMENT BY THE GOVERNING AUTHORITY.
DEMOLITION NOTES	2. THE APPROVED SWMP SHALL BE AMENDED IF THE PROPOSED SITE CONDITIONS CHANGE AFTER PLA APPROVAL IS OBTAINED, OR IF IT IS DETERMINED BY THE GOVERNING AUTHORITY DURING THE COURSE OF GRADING OR CONSTRUCTION THAT THE APPROVED PLAN IS INADEQUATE.
 THE CONTRACTOR SHALL INDEMNIFY AND HOLD HARMLESS THE OWNER AND/OR PERSONNEL, EQUIPMENT AND/OR EXISTING FACILITIES IN THE DEMOLITION AND CONSTRUCTION DESCRIBED IN THE PLANS AND SPECIFICATIONS. 	3. BEFORE RUNOFF CAN BE ACCEPTED INTO A BMP, A DENSE AND VIGOROUS VEGETATIVE COVER SHA ESTABLISHED OVER ALL PERVIOUS PORTIONS OF THE CONTRIBUTING DRAINAGE WATERSHED AREA.
2. THE CONTRACTOR SHALL OBTAIN THE NECESSARY PERMITS AND NOTIFY ALL AFFECTED UTILITY COMPANIES PRIOR TO THE DEMOLITION OF ANY EXISTING STRUCTURES. ALL EXISTING UTILITIES INDICATED TO BE ABANDONED SHALL BE CAPPED OFF OR REMOVED.	SEDIMENT-LADEN RUNOFF SHALL NOT BE ALLOWED TO REACH ANY STORMWATER QUALITY BMP AT A TIME. IF SEDIMENT-LADEN RUNOFF ENTERS A STRUCTURE, THE STRUCTURE SHALL BE CLEANED, REPAIRED AND/OR RECONSTRUCTED IN ACCORDANCE WITH THE CONSTRUCTION PLANS.
3. EXISTING CONDITIONS AS DEPICTED ON THE PLANS ARE GENERAL AND ILLUSTRATIVE IN NATURE, AND DO NOT INCLUDE MECHANICAL, ELECTRICAL, OR MISCELLANEOUS STRUCTURES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO EXAMINE THE SITE AND BECOME FAMILIAR WITH EXISTING	UTILITY LAYOUT NOTES 1. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LAYOUT ALL THE UTILITIES BEING
CONDITIONS PRIOR TO BIDDING ON THE WORK FOR THIS PROJECT. IF CONDITIONS ENCOUNTERED DURING CONSTRUCTION ARE SIGNIFICANTLY DIFFERENT THAN THOSE ANTICIPATED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY.	CONSTRUCTED AND/OR RELOCATED WITHIN THE CONTRACT. 2. LOCATION OF ALL UTILITIES AND UNDERGROUND STRUCTURES SHOWN ARE APPROXIMATE AND THOS SHOWN ARE NOT NECESSARILY ALL OF THE EXISTING UTILITIES AND STRUCTURES. IT IS THE
 4. EXISTING ITEMS BOTH ABOVE AND BELOW GRADE SHALL BE REMOVED OR RELOCATED AS SPECIFIED ON THE PLANS, EXCEPT AS NOTED. 5. ANY OFF-SITE DEMOLITION SHALL BE PERFORMED BY LETTER OF PERMISSION ONLY. 	CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE EXACT LOCATION AND EXISTENCE OF ALL UTILITIES AND UNDERGROUND STRUCTURES AND TO COORDINATE EXCAVATION REQUIRED AROUND T UTILITIES WITH THE RESPECTIVE OWNERS.
 6. ALL DEMOLITION WASTE AND CONSTRUCTION DEBRIS SHALL BE REMOVED BY THE CONTRACTOR AND DISPOSED OF IN ACCORDANCE WITH LOCAL AND STATE REQUIREMENTS. 	3. THE CONTRACTOR SHALL CONFIRM FIELD LOCATIONS AND ELEVATIONS OF EXISTING UTILITIES, ABOVE AND BELOW GROUND, PRIOR TO THE BEGINNING OF WORK AND SHALL NOTIFY THE ENGINEE ANY DISCREPANCIES.
7. ALL OFF-SITE DISPOSAL AREAS SHALL BE PERMITTED IN ACCORDANCE WITH LOCAL AND TENNESSEE DEPARTMENT OF ENVIRONMENT AND CONSERVATION (TDEC) REQUIREMENTS.	4. ANY ADJUSTMENTS TO THE HORIZONTAL OR VERTICAL ALIGNMENTS REQUIRED TO AVOID EXISTING UTILITIES SHALL BE APPROVED BY THE OWNER PRIOR TO INSTALLATION.
 WATER, SANITARY SEWER, STORM SEWER, GAS, POWER, AND TELECOMMUNICATION FACILITY REMOVAL SHALL BE COORDINATED WITH THE RESPECTIVE UTILITY. THE CONTRACTOR SHALL MAINTAIN EXISTING UTILITY SERVICES DURING DEMOLITION AND CONSTRUCTION PHASES. 	5. THE MINIMUM HORIZONTAL SEPARATION BETWEEN THE CLOSEST TWO POINTS OF A WATER LINE AN SEWER LINE IS 10'. THE MINIMUM VERTICAL SEPARATION BETWEEN THE CLOSEST TWO POINTS O WATER AND SEWER LINE IS 18" WHEN THE MINIMUM HORIZONTAL SEPARATION OF 10' CANNOT BE ATTAINED. REFER TO THE PIPE SEPARATION DETAIL FOR ADDITIONAL CLEARANCE REQUIREMENT
ASBESTOS DEMOLITION NOTES	6. THE HORIZONTAL AND VERTICAL BEND FITTINGS AND DEFLECTIONS SHOWN ON THE DRAWINGS ARE NECESSARILY ALL OF THE BEND FITTINGS OR DEFLECTIONS REQUIRED FOR THE INSTALLATION O UTILITIES.
 THE CONTRACTOR SHALL PERFORM DEMOLITION OF STRUCTURES IN ACCORDANCE WITH OSHA REGULATIONS AND THE USEPA NATIONAL EMISSION STANDARDS FOR HAZARDOUS AIR POLLUTANTS (NESHAP). 	UTILITY CONSTRUCTION NOTES 1. ALL MATERIALS, METHODS, AND/OR INTEGRAL MATERIALS OUTLINED IN THE UTILITY SPECIFICA OR DRAWINGS NECESSARY TO PROVIDE A COMPLETE AND FUNCTIONAL INSTALLATION MUST BE INC
2. TEN DAYS PRIOR TO DEMOLITION, THE CONTRACTOR SHALL PROVIDE NOTIFICATION TO THE NESHAP ADMINISTRATOR:	IN THE CONTRACTOR'S BID. 2. THE CONTRACTOR SHALL HAVE APPROVAL OF ALL GOVERNING AGENCIES HAVING JURISDICTION OV
TENNESSEE DEPARTMENT OF ENVIRONMENT & CONSERVATION DIVISION OF AIR POLLUTION CONTROL 9TH FLOOR, L & C ANNEX 401 CHURCH STREET	THE PROPOSED UTILITY SYSTEM PRIOR TO INSTALLATION. 3. CONNECTION TO EXISTING UTILITIES SHALL BE MADE ONLY UNDER THE SUPERVISION OF A REPRESENTATIVE OF THE UTILITY.
NASHVILLE, TENNESSEE 37243 (615) 532-0572 3. THE CONTRACTOR SHALL SAMPLE AND ANALYZE MATERIALS DISCOVERED DURING DEMOLITION THAT ARE	4. THE CONTRACTOR SHALL PROVIDE SUPPORT AND PROTECTION OF EXISTING UTILITIES EXPOSED D EXCAVATION AS NEEDED TO MAINTAIN SERVICE AND THE INTEGRITY OF THE UTILITY.
SUSPECTED TO CONTAIN ASBESTOS MATERIALS. 4. ANY ASBESTOS CONTAMINATED DEMOLITION DEBRIS SHALL BE DISPOSED OF IN ACCORDANCE WITH LOCAL, STATE, AND FEDERAL REGULATIONS. CLEARING & GRUBBING NOTES	5. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY PROTECTIVE MEASURES TO SAFEGUARD EXISTIN UTILITIES FROM DAMAGE DURING CONSTRUCTION OF THIS PROJECT. IN THE EVENT THAT SPECIA EQUIPMENT IS REQUIRED TO WORK OVER AND AROUND EXISTING UTILITIES, THE CONTRACTOR WI REQUIRED TO FURNISH SUCH EQUIPMENT. THE COST OF PROTECTING UTILITIES FROM DAMAGE AN FURNISHING SPECIAL EQUIPMENT WILL BE INCLUDED IN THE PRICE BID FOR OTHER ITEMS OF CONSTRUCTION. THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL REPAIRS AND RELATED EXPENS
1. THE CONTRACTOR SHALL STRIP TOPSOIL, VEGETATION, AND PAVEMENT AS NEEDED TO COMPLETE THE WORK AS SHOWN ON THE CONSTRUCTION PLANS. CLEAR AND GRUB REMAINING AREAS IN CONSTRUCTION LIMITS.	EXISTING UTILITIES ARE DAMAGED. 6. ALL UTILITY INSTALLATION SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT, INCLUDI COORDINATION, TRENCHING, BACKFILL, COMPACTION, AND OVERALL CONSTRUCTION OPERATIONS.
2. ALL WASTE AND CONSTRUCTION DEBRIS SHALL BE REMOVED BY THE CONTRACTOR AND DISPOSED OF IN ACCORDANCE WITH LOCAL AND STATE REQUIREMENTS.	 ALL UTILITY EXCAVATION SHALL BE UNCLASSIFIED, UNLESS OTHERWISE INDICATED. THE CONTRACTOR SHALL INSTALL AND MAINTAIN SAFETY FENCE AROUND ALL OPEN TRENCHES AND
 ALL OFF-SITE BORROW OR DISPOSAL AREAS SHALL BE PERMITTED IN ACCORDANCE WITH LOCAL AND TENNESSEE DEPARTMENT OF ENVIRONMENT AND CONSERVATION (TDEC) REQUIREMENTS. UNLESS OTHERWISE INDICATED TO REMAIN, ALL EXISTING STRUCTURES, FENCING, TREES, ETC. 	AREAS TO PROTECT PREMISES FROM ENTRY BY UNAUTHORIZED PERSONS. LOCATION OF SAFETY FE SHALL BE COORDINATED WITH TRAFFIC CONTROL PLANS AS CONSTRUCTION PROCEEDS.
WITHIN CONSTRUCTION AREA SHALL BE REMOVED & DISPOSED OF OFFSITE. BURNING ON SITE WILL NOT BE PERMITTED.	9. IF A UTILITY TRENCH MUST BE LEFT OPEN IN A ROADWAY OR DRIVE ISLE, IT SHALL BE COVER WITH A HEAVY-DUTY STEEL PLATE (SUPPLIED BY CONTRACTOR), TEMPORARILY SECURED, AND PROPERLY SIGNED. TEMPORARY PAVEMENT SHALL BE SUPPLIED BY THE CONTRACTOR ON EACH SID PLATE, WHERE NECESSARY. THERE SHALL BE NO ADDITIONAL PAYMENT FOR TEMPORARY MEASURES
GENERAL LAYOUT NOTES 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXAMINING THE AREAS AND CONDITIONS UNDER WHICH	10.BACKFILL FOR UTILITY LINES SHALL BE PERFORMED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. UNLESS OTHERWISE INDICATED, BACKFILL FOR UTILITY LINES UNDER PAVED GRAVEL AREAS SHALL BE COMPACTED CRUSHED STONE AND FLOWABLE FILL UNDER OPEN-CUT ROAD
THE PROJECT IS TO BE CONSTRUCTED AND NOTIFY THE ARCHITECT OF CONDITIONS DETRIMENTAL TO THE PROPER AND TIMELY COMPLETION OF THE WORK. 2. THE CONTRACTOR SHALL IDENTIFY REQUIRED LINES, LEVELS, AND DATUM PRIOR TO CONSTRUCTION.	11.WHEN UNSUITABLE SOIL IS ENCOUNTERED AT THE TRENCH BOTTOM, REMOVE IT TO A DEPTH REQU TO ASSURE SUPPORT OF THE PIPELINE AND BACKFILL TO THE PROPER GRADE WITH COARSE AGGR
3. THE CONTRACTOR SHALL PROTECT BENCHMARKS, PROPERTY CORNERS, AND ALL OTHER SURVEY MARKERS FROM DAMAGE AND/OR DESTRUCTION.	AASHTO M-43, SIZE NO. 2 OR 3. 12.SOME UTILITY FACILITIES MAY NEED TO BE ADJUSTED CONCURRENTLY WITH THE CONTRACTOR'S OPERATIONS, WHILE SOME WORK MAY BE REQUIRED AROUND UTILITY FACILITIES THAT WILL REM
4. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LAYOUT ALL THE FACILITIES BEING CONSTRUCTED, RELOCATED, AND/OR ADJUSTED WITHIN THE CONTRACT.	IN PLACE. IT IS UNDERSTOOD AND AGREED THAT THE CONTRACTOR WILL RECEIVE NO ADDITION. COMPENSATION FOR ANY DELAYS OR INCONVENIENCE CAUSED BY THE UTILITY ADJUSTMENTS. PRIVATE UTILITIES
 5. ALL DIMENSIONS ARE REFERENCED TO THE FACE OF CURB UNLESS OTHERWISE INDICATED. 6. TYSINGER, HAMPTON AND PARTNERS, INC. WILL PROVIDE A DIGITAL FILE FOR USE IN SITE LAYOUT, IF REQUIRED. 	 ALL PRIVATE WATER AND SEWER LINES SHALL BE INSTALLED AND TESTED IN ACCORDANCE WITH APPLICABLE BUILDING/PLUMBING CODES, UNLESS OTHERWISE SPECIFIED.
GENERAL CONSTRUCTION NOTES	WATER LINE NOTES
 THE CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE APPROPRIATE GOVERNING AGENCY, UTILITY, AND THE OWNER, AS APPLICABLE. THE CONTRACTOR SHALL COORDINATE THE RELOCATION AND/OR ADJUSTMENT OF EXISTING UTILITIES 	WATER ACT (SDWA) SECTION 1417. 2. THE CONTRACTOR SHALL PROVIDE A POSITIVE/NEGATIVE GRADIENT BETWEEN AIR VALVES TO PRE
 (E.G., WATER, SANITARY SEWER, STORM SEWER, POWER, TELECOMMUNICATION, GAS, ETC.) WITH THE RESPECTIVE UTILITY OWNER. 3. NOTHING IN THE GENERAL NOTES SHALL RELIEVE THE CONTRACTOR FROM THE RESPONSIBILITY OF 	EXCESSIVE HIGH POINTS IN THE WATER LINE PROFILE. THE WATER LINE PROFILE MAY BE ADJU BY THE CONTRACTOR IN THE FIELD BY USING BENDS BUT SHALL NOT RESULT IN ADDITIONAL HI POINTS IN THE PIPELINE OR REQUIRE ADDITIONAL AIR RELEASE VALVES, UNLESS APPROVED BY UTILITY OWNER PRIOR TO CONSTRUCTION.
MAINTAINING THE SAFETY AND CONVENIENCE OF THE GENERAL PUBLIC WITHIN THE PROPOSED CONSTRUCTION AREA. 4. ALL TRENCHING, PIPE-LAYING, AND BACKFILLING SHALL BE IN ACCORDANCE WITH FEDERAL OSHA	3. THERE WILL BE NO CLASSIFICATION OF CUTS FOR WATER LINES. 4. UNLESS OTHERWISE SPECIFIED, PIPE AND FITTINGS FOR MAINS 6" IN DIAMETER AND LARGER S
REGULATIONS. 5. ALL SLOPES NOT OTHERWISE INDICATED SHALL BE 2:1 OR FLATTER. SLOPES GREATER THAN 3:1	MEET THE FOLLOWING STANDARDS (LATEST REVISION): <u>DUCTILE IRON</u> • PRESSURE CLASS 350
 SHALL (AFTER TOPSOIL, SEEDING AND MULCHING) BE COVERED WITH CURLEX BLANKETS BY AMERICAN EXCELSIOR OR APPROVED EQUAL. 6. ALL DISTURBED AREAS NOT PAVED SHALL BE SEEDED, FERTILIZED, MULCHED, WATERED, AND 	 ANSI/AWWA C104/A21.4 - CEMENT-MORTAR LINING ANSI/AWWA C105/A21.5 - POLYETHYLENE ENCASEMENT (WHERE SPECIFIED) ANSI/AWWA C110/A21.10 - FITTINGS
MAINTAINED UNTIL AN ADEQUATE STAND OF VEGETATÍON IS OBTAINED. A MINIMUM OF Á" OF TOPSOIL SHALL BE PLACED IN ALL SUCH AREAS. 7. SITE GRADING SHALL NOT PROCEED UNTIL EROSION CONTROL MEASURES HAVE BEEN INSTALLED.	 ANSI/AWWA C111/A21.11 - RUBBER-GASKET JOINTS ANSI/AWWA C115/A21.15 - FLANGED DUCTILE-IRON PIPE ANSI/AWWA C116/A21.16 - EPOXY COATINGS (WHERE SPECIFIED) ANSI/AWWA C150/A21.50 - THICKNESS
8. THIS PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE TENNESSEE DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST	 ANSI/AWWA C151/A21.51 - DUCTILE-IRON PIPE ANSI/AWWA C153/A21.53 - COMPACT FITTINGS ANSI/AWWA C600INSTALLATION FURNISH IN STANDARD LAYING LENGTHS OF 20'.
EDITION, AS APPLICABLE. 9. THIS PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE ADA ACCESSIBILITY GUIDELINES, AS APPLICABLE.	 PROVIDE A MINIMUM OF 1 MIL THICK BITUMINOUS COATING ON THE OUTSIDE SURFACE. THE CLASS OR NOMINAL THICKNESS, NET WEIGHT WITHOUT LINING, AND CASTING PERIOD SHAL CLEARLY MARKED ON EACH LENGTH OF PIPE. ADDITIONALLY, THE MANUFACTURER'S MARK, COUNTIONALLY, THE MANUFACTURER'S MARK, COUNTING, MARKE, COUNTING, COUNT
 — 10.ALL DRAINAGE STRUCTURES AND STORM SEWER PIPES SHALL MEET HEAVY DUTY TRAFFIC (H-20) LOADING AND BE INSTALLED ACCORDINGLY. 	WHERE CAST, YEAR IN WHICH THE PIPE WAS PRODUCED, AND THE LETTERS "DI" OR "DUCTILE' SHALL BE CAST OR STAMPED ON THE PIPE. PIPE SHALL BE FURNISHED COMPLETE WITH ALL NECESSARY ACCESSORIES.
11.SICP - SMOOTH INTERIOR CORRUGATED PLASTIC PIPES AND FITTINGS SHALL BE: A.D.SN12 MFRD. BY ADVANCED DRAINAGE SYSTEMS, INC.	5. ALL WATER LINE PIPE, FITTINGS, AND ACCESSORIES SHALL BE HYDROSTATICALLY TESTED IN ACCORDANCE WITH AWWA C600, DISINFECTED IN ACCORDANCE WITH AWWA C651, AND PASS BACTERIOLOGICAL TESTING BEFORE BEING PLACED IN SERVICE AND PRIOR TO ACCEPTANCE. TES SHALL BE CONDUCTED ON PIPE LENGTHS OF LESS THAN 1,000'. WHERE THIS IS NOT POSSIBLE,
3300 RIVERSIDE DRIVE, COLUMBUS OHIO 43221 (614) 457-3051 "HI-Q SURE-LOK" MFRD. BY HANCOR, INC.	TESTING SHALL BE CONDUCTED FROM ONE VALVE TO THE NEXT CLOSEST VALVE. 6. ALL WATER LINE FITTINGS (BENDS, TEES, VALVES, HYDRANTS, PLUGS, CAPS, ETC.) SHALL BE
901 OLIVE STREET FINDLAY, OHIO 95840 1-800-FOR-PIPE LANE ENTERPRISES, INC.	PROPERLY RESTRAINED TO RESIST THRUST FORCES IN ACCORDANCE WITH THE THRUST RESTRAINT DETAIL. UNLESS OTHERWISE INDICATED OR DIRECTED, THRUST RESTRAINT SHALL BE PROVIDED RESTRAINED JOINTS, RETAINER GLANDS, OR RESTRAINED GASKETS. JOINT RESTRAINT SHALL BE PROVIDED AT ALL PIPE JOINTS FALLING WITHIN THE RESTRAINED LENGTHS. RESTRAINING GASK
34 STROHM ROAD SHIPPENSBURG, PA 17257 (717) 532-5959 SICP SHALL BE INSTALLED PER MANUFACTURER'S GUIDELINE FOR HEAVY DUTY DRAINAGE	SHALL BE INSTALLED AS PIPE IS LAID. 7. ALL PIPE AND FITTINGS IN CONTACT WITH CONCRETE SHALL BE WRAPPED IN PLASTIC, MINIMUM 6-MIL THICKNESS.

APPLICATIONS. ALL FITTINGS SHALL BE PRE-MANUFACTURED FITTINGS AS PROVIDED BY THE ABOVE

LISTED SUPPLIERS.

DEM DUMP TRUCK SHALL BE PERFORMED AS FOLLOWS: LING SHALL BE PERFORMED AFTER TOPSOIL AND ORGANIC ORE FILLING OPERATIONS ARE INITIATED. BE PERFORMED ON SUBGRADE AFTER FINISHED GRADES ARE

EQUIRED DENSITIES. LIFTS OF NO MORE THAN 8 INCHES. STRUCTURAL FILL DENSITY (ASTM D698) WITH A MOISTURE CONTENT WITHIN DTECHNICAL ENGINEERING FIRM.

ER TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE ICES (MUTCD) AND "THE AMERICANS WITH DISABILITIES

RAFFIC PAINT. PAINT SHALL BE NON-BLEEDING, QUICK ITABLE FOR TRAFFIC-BEARING SURFACE AND SHALL MEET WITH MANUFACTURER'S INSTRUCTIONS BEFORE

OR MEETING ALL CONDITIONS SPECIFIED IN THE PERMITS NON-COMPLIANCE WITH THE PERMIT CONDITIONS SHALL

_AN (SWMP) SHALL BE ADHERED TO DURING GRADING AND CUMSTANCE IS THE OWNER OR OPERATOR OF LAND TATE FROM THE APPROVED SWMP WITHOUT PRIOR APPROVAL UTHORITY

THE PROPOSED SITE CONDITIONS CHANGE AFTER PLAN ERMINED BY THE GOVERNING AUTHORITY DURING THE THE APPROVED PLAN IS INADEQUATE. 3MP. A DENSE AND VIGOROUS VEGETATIVE COVER SHALL BE

OF THE CONTRIBUTING DRAINAGE WATERSHED AREA. _OWED TO REACH ANY STORMWATER QUALITY BMP AT ANY A STRUCTURE, THE STRUCTURE SHALL BE CLEANED, RDANCE WITH THE CONSTRUCTION PLANS.

OUND STRUCTURES SHOWN ARE APPROXIMATE AND THOSE EXISTING UTILITIES AND STRUCTURES. IT IS THE INE THE EXACT LOCATION AND EXISTENCE OF ALL AND TO COORDINATE EXCAVATION REQUIRED AROUND THESE

CATIONS AND ELEVATIONS OF EXISTING UTILITIES, BOTH BEGINNING OF WORK AND SHALL NOTIFY THE ENGINEER OF

VERTICAL ALIGNMENTS REQUIRED TO AVOID EXISTING NER PRIOR TO INSTALLATION. WEEN THE CLOSEST TWO POINTS OF A WATER LINE AND ANY L SEPARATION BETWEEN THE CLOSEST TWO POINTS OF A

MINIMUM HORIZONTAL SEPARATION OF 10' CANNOT BE ON DETAIL FOR ADDITIONAL CLEARANCE REQUIREMENTS. INGS AND DEFLECTIONS SHOWN ON THE DRAWINGS ARE NOT OR DEFLECTIONS REQUIRED FOR THE INSTALLATION OF THE

AL MATERIALS OUTLINED IN THE UTILITY SPECIFICATIONS MPLETE AND FUNCTIONAL INSTALLATION MUST BE INCLUDED

ALL GOVERNING AGENCIES HAVING JURISDICTION OVER INSTALLATION.

AND PROTECTION OF EXISTING UTILITIES EXPOSED DURING ICE AND THE INTEGRITY OF THE UTILITY.

SSARY PROTECTIVE MEASURES TO SAFEGUARD EXISTING FION OF THIS PROJECT. IN THE EVENT THAT SPECIAL AROUND EXISTING UTILITIES. THE CONTRACTOR WILL BE HE COST OF PROTECTING UTILITIES FROM DAMAGE AND INCLUDED IN THE PRICE BID FOR OTHER ITEMS OF RESPONSIBLE FOR ALL REPAIRS AND RELATED EXPENSES IF

1 TO THE REQUIREMENTS OF THE CONTRACT, INCLUDING PACTION, AND OVERALL CONSTRUCTION OPERATIONS. SSIFIED, UNLESS OTHERWISE INDICATED.

TAIN SAFETY FENCE AROUND ALL OPEN TRENCHES AND WORK BY UNAUTHORIZED PERSONS. LOCATION OF SAFETY FENCE FROL PLANS AS CONSTRUCTION PROCEEDS.

IN A ROADWAY OR DRIVE ISLE, IT SHALL BE COVERED ED BY CONTRACTOR), TEMPORARILY SECURED, AND HALL BE SUPPLIED BY THE CONTRACTOR ON EACH SIDE OF E NO ADDITIONAL PAYMENT FOR TEMPORARY MEASURES.

ERFORMED IN ACCORDANCE WITH THE PLANS AND ICATED, BACKFILL FOR UTILITY LINES UNDER PAVED AND ED STONE AND FLOWABLE FILL UNDER OPEN-CUT ROADWAYS. THE TRENCH BOTTOM, REMOVE IT TO A DEPTH REQUIRED BACKFILL TO THE PROPER GRADE WITH COARSE AGGREGATE

ADJUSTED CONCURRENTLY WITH THE CONTRACTOR'S QUIRED AROUND UTILITY FACILITIES THAT WILL REMAIN THAT THE CONTRACTOR WILL RECEIVE NO ADDITIONAL ENIENCE CAUSED BY THE UTILITY ADJUSTMENTS.

/E/NEGATIVE GRADIENT BETWEEN AIR VALVES TO PREVENT VE PROFILE. THE WATER LINE PROFILE MAY BE ADJUSTED NG BENDS BUT SHALL NOT RESULT IN ADDITIONAL HIGH ITIONAL AIR RELEASE VALVES, UNLESS APPROVED BY THE

TS FOR WATER LINES. FITTINGS FOR MAINS 6" IN DIAMETER AND LARGER SHALL REVISION):

UMINOUS COATING ON THE OUTSIDE SURFACE. WEIGHT WITHOUT LINING. AND CASTING PERIOD SHALL BE ADDITIONALLY, THE MANUFACTURER'S MARK, COUNTRY AS PRODUCED, AND THE LETTERS "DI" OR "DUCTILE" PIPE SHALL BE FURNISHED COMPLETE WITH ALL

CESSORIES SHALL BE HYDROSTATICALLY TESTED IN D IN ACCORDANCE WITH AWWA C651, AND PASS PLACED IN SERVICE AND PRIOR TO ACCEPTANCE. TESTING LESS THAN 1,000'. WHERE THIS IS NOT POSSIBLE, ALVE TO THE NEXT CLOSEST VALVE.

VALVES, HYDRANTS, PLUGS, CAPS, ETC.) SHALL BE FORCES IN ACCORDANCE WITH THE THRUST RESTRAINT DIRECTED, THRUST RESTRAINT SHALL BE PROVIDED USING RESTRAINED GASKETS. JOINT RESTRAINT SHALL BE THIN THE RESTRAINED LENGTHS. RESTRAINING GASKETS

- 8. WATER SERVICE LINES: ALL WATER SERVICE LINES SMALLER THAN 2" IN DIAMETER SHALL BE SEAMLESS TYPE K COPPER PIPE MEETING ASTM B-88 MANUFACTURED IN THE USA, UNLESS OTHERWISE INDICATED. ALL WATER SERVICE LINES 2" THROUGH 3" IN DIAMETER SHALL BE PVC SDR 13.5 OR HDPE DR 7. ALL WATER SERVICE LINES CROSSING EXISTING ROADWAYS SHALL BE BORED.
- 9. ALL CONNECTIONS TO THE WATER MAIN 2" AND LARGER SHALL BE MADE WITH A TEE OR TAPPING SLEEVE AND GATE VALVE. ALL CONNECTIONS SMALLER THAN 2" SHALL BE MADE WITH A CORPORATION STOP.
- 10. THE CONTRACTOR SHALL RECONNECT ALL EXISTING WATER LINES AS INDICATED ON THE PLANS OR AS DIRECTED. RELOCATED WATER METERS SHALL BE INSTALLED AT THE PROPERTY LINE, PERMANENT EASEMENT LINE, OR AS DIRECTED BY THE UTILITY. THE CONTRACTOR SHALL FIELD VERIFY EXISTING WATER METER SIZES TO BE RELOCATED/ADJUSTED, AND REPLACE IN-KIND WHERE SPECIFIED.
- SANITARY SEWER NOTES
- 1. ALL NEW DOWNSTREAM PIPES, MANHOLES, AND APPURTENANCES MUST BE TESTED AND ACCEPTABLE TO THE OWNER AND THE ENGINEER PRIOR TO RECEIVING SEWAGE FLOW. 2. SEWER DISCHARGE FROM EXISTING FACILITIES MUST BE MAINTAINED BY THE CONTRACTOR AT ALL TIMES DURING CONSTRUCTION. AT NO TIME SHALL SEWAGE BE DISCHARGED ONTO THE GROUND OR ANY TO ANY WATERCOURSE. TEMPORARY TIE-OVERS AND/OR PUMPING SHALL BE REQUIRED TO MAINTAIN SERVICE DURING CONSTRUCTION. ALL MATERIALS AND MEANS NECESSARY TO MAINTAIN SEWAGE FLOWS
- SHALL BE INCLUDED IN THE CONTRACT PRICE BID FOR OTHER ITEMS OF THE CONTRACT. 3. UNLESS OTHERWISE INDICATED, PIPE FOR GRAVITY SEWERS SHALL BE PVC. PIPE AND FITTINGS SHALL CONFORM TO ASTM D-3034 AND HAVE A STANDARD DIMENSION RATIO (SDR) OF 35 OR CONFORM TO ASTM F-679.
- 4. UNLESS OTHERWISE INDICATED, PIPE FOR PRESSURE SEWER LINES SHALL BE POLYVINYL CHLORIDE (PVC), ASTM D2241 SDR 21 OR AWWA C900 DR 18. POLYVINYL CHLORIDE (PVC) PIPE SHALL MEET THE FOLLOWING ASTM STANDARDS: D-1784 (PVC COMPOUND), D-2241 (PVC PIPE), D-3139 (JOINT), AND F-477 (GASKET).
- 5. WHERE INDICATED, DUCTILE IRON PIPE AND FITTINGS (PUSH-ON TYPE OR MECHANICAL JOINT) SHALL BE PRESSURE CLASS 350, MANUFACTURED IN ACCORDANCE WITH ANSI A21.5 (AWWA C150), ANSI A21.51 (AWWA C151), AND ANSI A21.10 (AWWA C110), LATEST REVISIONS. DUCTILE IRON FITTINGS SHALL MEET ANSI A21.11 (AWWA C111) AND ANSI A21.15 (AWWA C115). ALL DUCTILE IRON PIPE AND FITTINGS SHALL HAVE AN INTERIOR CERAMIC EPOXY LINING OF PROTECTO 401 IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS.
- 6. ALL PIPE AND FITTINGS IN CONTACT WITH CONCRETE SHALL BE WRAPPED IN PLASTIC, MINIMUM 6-MIL THICKNESS.

TEMPORARY UTILITY FACILITIES NOTES

- 1. THE CONTRACTOR MUST MAINTAIN ALL SERVICES DURING THE CONSTRUCTION. ANY COSTS ASSOCIATED WITH INSTALLATION OF REQUIRED TEMPORARY SERVICE LINES DUE TO THE CONSTRUCTION SEQUENCE OF WORK SHALL BE INCLUDED IN THE COST OF THE PERMANENT UTILITY ITEMS. 2. TEMPORARY TIE-OVERS MAY BE REQUIRED TO MAINTAIN SERVICE DURING CONSTRUCTION. ALL
- CONTRACT PRICE BID FOR OTHER ITEMS OF CONSTRUCTION.

CITY OF JOHNSON CITY WATER AND SEWER NOTES

- 1. ALL WATER AND SEWER CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS OF THE CITY OF JOHNSON CITY AND TDEC. 2. THE CONTRACTOR IS TO BE RESPONSIBLE FOR INFORMING THE CITY OF JOHNSON CITY WATER AND
- SEWER INSPECTION DIVISION IN WRITING TWO WEEKS PRIOR TO THE START OF CONSTRUCTION AND THREE WEEKS PRIOR TO WATER AND SEWER SERVICE CONNECTIONS TO THE CITY'S SYSTEM, CONFORMING TO THE REQUIREMENTS OF THE CITY'S INSPECTOR DURING CONSTRUCTION, AND REQUESTING FINAL INSPECTION AND APPROVAL FROM THE CITY.
- 3. ALL WATER AND SEWER PLANS MUST BE SUBMITTED TO AND APPROVED BY TDEC PRIOR TO CONSTRUCTION. A COPY OF THE APPROVAL LETTER MUST BE SUBMITTED TO THE JOHNSON CITY WATER AND SEWER ENGINEERING DEPARTMENT PRIOR TO CONSTRUCTION.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND VERIFYING THE ELEVATIONS OF EXISTING UTILITIES PRIOR TO CONSTRUCTION.
- 5. ALL EXCAVATION FOR TIE-INS AND TAPS AS WELL AS ALL BACKFILLING OPERATIONS WILL BE THE RESPONSIBILITY OF THE DEVELOPER.
- 6. BACKFILL FOR WATER AND SEWER LINES IN THE STREET SHALL BE COMPACTED CRUSHED STONE.
- 7. CONNECTION TO EXISTING CITY FACILITIES SHALL BE MADE ONLY UNDER THE SUPERVISION OF A REPRESENTATIVE OF THE CITY.
- 8. AS-BUILT PLANS MUST BE COMPLETED AND SHALL BE SUBMITTED ON PAPER AND IN DIGITAL FORM PRIOR TO ACCEPTANCE OF WATER AND SEWER LINES BY THE CITY. **EROSION AND SEDIMENT CONTROL NOTES**
- 1. THE CONTRACTOR SHALL INSTALL EROSION AND SEDIMENT CONTROLS AS SPECIFIED IN THE EROSION
- AND SEDIMENT CONTROL PLAN AND THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP). 2. THIS PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE TENNESSEE
- EROSION & SEDIMENT CONTROL HANDBOOK, AS APPLICABLE. 3. FOR DETAILS NOT PROVIDED, REFER TO THE LATEST EDITION OF THE TENNESSEE EROSION &
- SEDIMENT CONTROL HANDBOOK. 4. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT EROSION CONTROL MEASURES ARE IN PLACE PRIOR TO THE START OF ALL CONSTRUCTION. EROSION AND SEDIMENT CONTROLS SHALL BE PROVIDED AND MAINTAINED BY THE CONTRACTOR IN ACCORDANCE WITH THE TENNESSEE DEPARTMENT OF CONSERVATION (TDEC) RULES AND REGULATIONS. ALL WORK NEAR OR AROUND WATERWAYS SHALL CONFORM TO THE TDEC WATER QUALITY CONTROL BOARD DIVISION OF WATER POLLUTION CONTROL RULES AND REGULATIONS. VIOLATIONS AND FINES LEVIED DUE TO CONTRACTOR NEGLIGENCE SHALL BE
- RECOVERED AT THE CONTRACTOR'S EXPENSE. 5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT EXPOSED EARTH FROM EROSION RESULTING FROM CONSTRUCTION ACTIVITIES AND TO CONTAIN SEDIMENT THAT MAY RESULT FROM THE WORK. PRIOR TO BEGINNING WORK, ADEQUATE MEASURES MUST BE IN PLACE TO TRAP ANY SEDIMENT THAT MAY TRAVEL OFF-SITE IN THE EVENT OF RAIN. DURING THE PROGRESSION OF THE WORK,
- EXPOSED EARTH AREAS SHALL BE STABILIZED AS SOON AS POSSIBLE TO PREVENT EROSION. AT NO TIME SHALL EXPOSED EARTH FROM THE WORK SITE FLOW OFF-SITE TO WATERS OF THE STATE. 6. SILT FENCE TO BE INSTALLED ON THE DOWNSTREAM SIDE OF ALL STOCKPILED SOIL.
- 7. ALL DISTURBED AREAS NOT INDICATED TO BE PAVED OR RIP-RAPPED SHALL BE TOPSOILED, SEEDED, FERTILIZED, MULCHED, WATERED, AND MAINTAINED UNTIL AN ADEQUATE STAND OF VEGETATION IS ESTABLISHED. A MINIMUM OF 4" OF TOPSOIL SHALL BE PLACED IN ALL AREAS TO BE VEGETATED.
- 8. RAIN WATER THAT COLLECTS IN THE UTILITY TRENCH SHALL BE PUMPED INTO A TEMPORARY DEWATERING STRUCTURE OR FILTER BAG.
- 9. TRENCHES SHALL BE BACKFILLED DAILY AS CONSTRUCTION PROCEEDS. BACKFILLED TRENCHES SHALL BE SEEDED AND MULCHED OR SODDED DAILY, IF POSSIBLE, BUT NO LATER THAN 7 DAYS AFTER TRENCHING. ANY TEMPORARY SPOIL PILE OF EXCAVATED EARTH SHALL BE LOCATED WITHIN THE RIGHT-OF-WAY OR EASEMENT, STORED IN AN UPLAND LOCATION, AND STABILIZED WITHIN 7 DAYS. IF TRENCHES ARE NOT BACKFILLED OVERNIGHT, APPROPRIATE EROSION CONTROL MEASURES WILL BE INSTALLED UNTIL SUCH TIME THE TRENCH IS BACKFILLED.
- SPILLS AND NON-STORMWATER CONTINGENCIES
- 1. CONSTRUCTION VEHICLES SHALL CLEAN MUD FROM THEIR TIRES AND BODY ON-SITE SO THAT THE SEDIMENT WILL FLOW INTO SEDIMENT CONTROL DEVICES. ANY SEDIMENT THAT ENDS UP IN THE STREET OR OTHER PLACES OFF-SITE SHALL BE CLEANED UP WITH A SHOVEL AND BROOM OR OTHER MEANS BEFORE THE NEXT RAINFALL BUT SHALL NOT BE WASHED AWAY USING WATER. THE CLEANED UP SEDIMENT SHALL BE PLACED BACK ON-SITE OR TAKEN TO ANOTHER SITE WITH AN APPROVED AND FUNCTIONING SEDIMENT CONTROL PLAN.
- 2. VEHICLES AND EQUIPMENT SHALL BE FUELED ON-SITE NEAR THE CONSTRUCTION EXIT IN A DESIGNATED CONTAINMENT AREA. CLEAN UP ANY FUEL SPILL IMMEDIATELY. CONTAMINATED SOILS WILL BE PLACED ON HEAVY PLASTIC AND COVERED OR PLACED IN APPROVED CONTAINERS TO PREVENT CONTACT WITH STORMWATER. ALL FUEL TANKS SHALL BE STORED IN THE CONTAINMENT AREA. ALL OIL, OTHER VEHICLE FLUIDS, SOLVENTS, PAINT, ETC. SHALL BE STORED IN A CONSTRUCTION TRAILER OR OTHER APPROVED CONTAINER.
- 3. ABSORBENT MATERIAL (FOR LAND BASED SPILLS), BOOMS (FOR SPILLS INTO WATERWAYS), AND OTHER HAZARDOUS MATERIAL CLEANUP TOOLS AS NECESSARY SHALL BE AVAILABLE FOR IMMEDIATE USE IF AN ON-SITE SPILL OCCURS. IF A SPILL OF HAZARDOUS MATERIALS OCCURS, THE SPILL SHALL BE CONTAINED IMMEDIATELY AND THEN COMPLETELY CLEANED UP. IF THE SPILL HAS ENTERED A WATER SOURCE, SINKHOLE, STORM DRAIN, OR OTHER STORMWATER CONVEYANCE, THE LOCAL GOVERNMENT AUTHORITY SHALL BE CONTACTED IMMEDIATELY. ANY CONTAMINATED MATERIAL FROM THE CLEANUP SHALL BE DISPOSED OF IN ACCORDANCE WITH ALL STATE LAWS.
- 4. READY-MIX CONCRETE TRUCKS SHALL WASH OUT THEIR EQUIPMENT IN A DESIGNATED WASH PIT. THIS WASH PIT IS TO TRAP THE CONCRETE AND ITS WASH. THE CONTRACTOR SHALL MAINTAIN THIS PIT(S) AS NECESSARY TO ALWAYS HAVE AT LEAST 50% VOLUME. ANY MATERIAL REMOVED FROM THE WASH PIT SHALL BE USED FOR FILL MATERIAL ON-SITE OR DISPOSED OF IN ACCORDANCE WITH ALL STATE AND FEDERAL REGULATIONS. WASH FROM THE CONCRETE TRUCKS AND ANY OVERFLOW FROM THE WASH PIT SHALL NOT BE ALLOWED TO DISCHARGE TO A SEDIMENT BASIN, TRAP, POND STORM DRAIN, DITCH, STREAM, OTHER STORMWATER CONVEYANCE, OR TO WATERS OF THE STATE INCLUDING BOTH SURFACE AND GROUNDWATER.
- 5. ALL HAZARDOUS MATERIALS SUCH AS EMPTY OR PARTIALLY EMPTY PAINT CANS, OIL CANS, FILTERS, CLEANING FLUIDS, ETC. SHALL BE DISPOSED OF BY TAKING THEM TO A PERMITTED HAZARDOUS MATERIAL DISPOSAL SITE IN ACCORDANCE WITH STATE LAWS
- 6. THE WASHING OF PAINT TOOLS OR OTHER HAZARDOUS MATERIAL EQUIPMENT MUST BE PERFORMED AND DISPOSED OF IN ACCORDANCE WITH ALL STATE AND FEDERAL REGULATIONS. THE CLEANING RESIDUE FROM SUCH EQUIPMENT IS HAZARDOUS AND CANNOT BE DISCHARGED ONTO THE GROUND OR INTO A SEDIMENT BASIN, TRAP, POND, STORM DRAIN, DITCH, STREAM, OTHER STORMWATER CONVEYANCE, OR TO WATERS OF THE STATE INCLUDING BOTH SURFACE AND GROUNDWATER AND SHALL BE DISPOSED OF IN ACCORDANCE WITH STATE LAWS.
- 7. LITTER, CONSTRUCTION MATERIALS, CONSTRUCTION DEBRIS, CONSTRUCTION CHEMICALS, AND OTHER HAZARDOUS MATERIALS EXPOSED TO STORM WATER SHALL BE PICKED UP PRIOR TO ANTICIPATED STORM EVENTS OR BEFORE BEING CARRIED OFF OF THE SITE BY WIND (E.G., FORECASTED BY LOCAL WEATHER REPORTS), OR OTHERWISE PREVENTED FROM BECOMING A POLLUTANT SOURCE FOR STORM WATER DISCHARGES. LITTER, CONSTRUCTION MATERIALS, CONSTRUCTION DEBRIS, CONSTRUCTION CHEMICALS, AND OTHER HAZARDOUS MATERIALS SHALL NOT BE ALLOWED TO ENTER A SEDIMENT BASIN. TRAP, POND, STORM DRAINS, DITCH, STREAM, OTHER STORMWATER CONVEYANCE, OR TO WATERS OF THE STATE. THIS CAN BE ACCOMPLISHED BY SCREENING OUTFALLS, DAILY PICKUP OR CLEANUP, OR OTHER METHODS.
- 8. AFTER THEIR USE, MATERIALS USED FOR EROSION PREVENTION AND SEDIMENT CONTROL SHOULD BE REMOVED OR OTHERWISE PREVENTED FROM BECOMING A POLLUTANT SOURCE FOR STORM WATER DISCHARGES.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR LITTER CONTROL AND CLEANUP.
- 10.SEDIMENT CONTROLS SHALL BE PROVIDED FOR ANY WATER DISTRIBUTION OR WASTE DISPOSAL SYSTEM ON-SITE INCLUDING SANITARY SEWER OR SEPTIC SYSTEMS.

- PHASE 1

- PHASE 2

 - - OR PERMANENTLY CEASED.
 - EACH RAINFALL. THE CONTRACTOR SHALL REPAIR OR REPLACE ANY DAMAGED MEASURES.

 - OR AN EQUIVALENT COURSE

CONSTRUCTION SEQUENCE

THE CONTRACTOR SHALL ATTEND A PRECONSTRUCTION MEETING WITH THE CITY OF JOHNSON CITY'S STORMWATER MANAGEMENT STAFF. THE CONTRACTOR SHALL CONTACT DAVID ROCK AT (423) 975-2725 TO SCHEDULE THE MEETING.

2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING A GRADING PERMIT AND ANY OTHER PERMITS REQUIRED PRIOR TO BEGINNING CONSTRUCTION.

3. THE CONTRACTOR SHALL INSTALL CONSTRUCTION EXIT, SILT FENCE AND INLET PROTECTION AS SHOWN THE CONTRACTOR(S), OR ANYONE USING THESE DOCUMENTS IS ADVISED TO LAYOUT HIS WORK ON THE EROSION AND SEDIMENT CONTROL PLAN PHASE 1.

4. THE CONTRACTOR SHALL INSTALL DETENTION BASIN AND THEN STORM LINE D1 AND D2. (THE CONTRACTOR CAN THEN REMOVE THE EXISTING 30-INCH CMP WHERE INDICATED.)

5. THE CONTRACTOR SHALL INSTALL STORM LINE D5.1 THROUGH D5.5. (THE CONTRACTOR CAN THEN REMOVE THE EXISTING 15-INCH RCP FROM BENEATH NEW ADDITION AS INDICATED.) 6. THE CONTRACTOR SHALL INSTALL STORM PIPE AS AN INITIAL STEP WHEN GRADING IS OCCUR.

7. THE CONTRACTOR SHALL INSTALL INLET PROTECTION ON ALL NEW STORM LINES AS THEY ARE CONSTRUCTED TO TRAP AND PREVENT THE TRANSPORT OF SEDIMENT.

8. SEEDING AND MULCH SHALL BE APPLIED TO DENUDED AREAS WHERE CONSTRUCTION ACTIVITIES WILL NOT OCCUR WITHIN THE NEXT 14 DAYS.

9. THE CONTRACTOR SHALL CONSTRUCT PROJECT FACILITIES AS DEPICTED ON THE EROSION AND SEDIMENT CONTROL PLAN PHASE 1. ADDITIONAL MEASURES SUCH AS INLET PROTECTION, OUTLET PROTECTION, SEEDING, SILT FENCE, TUBES AND WADDLES, ETC. SHALL BE INSTALLED AS NEEDED TO MINIMIZE EROSION AND PREVENT THE TRANSPORT OF SEDIMENT.

1. SEEDING WITH MULCH SHALL BE APPLIED TO ANY REMAINING DISTURBED AREAS.

2. UPON SITE STABILIZATION. THE STORM SYSTEM (STORM PIPES AND STRUCTURES) SHALL BE FLUSHED WITH WATER AS NEEDED TO REMOVE ANY ACCUMULATION OF SILT OR MUD.

3. THE WATER QUALITY UNIT SHALL BE CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

ALL PHASES OF CONSTRUCTION

1. AT NO TIME SHALL THE CONTRACTOR SHALL THE CONTRACTOR PERFORM WATER OFF-SITE WITHOUT PROPERLY DOCUMENTED APPROVAL FROM THE LAND OWNER.

MATERIALS AND MEANS NECESSARY FOR A COMPLETE WORKING CONNECTION SHALL BE INCLUDED IN THE 2. THE CONTRACTOR SHALL CLEAR AND GRUB AREAS WITHIN CONSTRUCTION LIMITS. CLEARING SHALL BE LIMITED TO ONLY THOSE AREAS WHERE WORK WILL PROCEED IMMEDIATELY. 3. THE CONTRACTOR SHALL ESTABLISH VEGETATION IN ACCORDANCE WITH THE SEEDING SCHEDULE IN

> AREAS WHERE WORK IS COMPLETE TO PREVENT EROSION. 4. RESEEDING OR SODDING ANY DENUDED AREA SHALL BE REQUIRED WHERE NO CONSTRUCTION ACTIVITY HAS OCCURRED WITHIN 14 DAYS. STEEP SLOPES (35% GRADE OR GREATER) SHALL BE TEMPORARILY STABILIZED NO LATER THAN 7 DAYS AFTER CONSTRUCTION ACTIVITY ON THE SLOPE HAS TEMPORARILY

5. THE CONTRACTOR SHALL CHECK AND MAINTAIN ALL EROSION CONTROL MEASURES DAILY AND AFTER

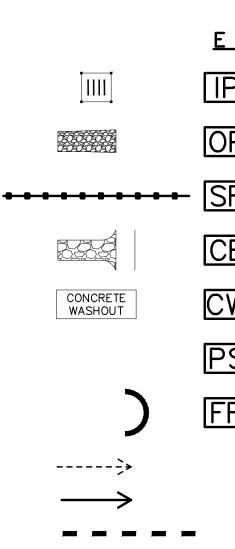
6. ALL OFF-SITE OR STREET SEDIMENT DEPOSITS OCCURRING AS A RESULT OF CONSTRUCTION ACTIVITIES OR A STORM EVENT SHALL BE CLEANED UP AFTER EACH WORK DAY AND WITHIN NO LATER THAN 24 HOURS OF THE OCCURRENCE.

7. THE CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL MEASURES AFTER CONSTRUCTION IS COMPLETED AND A HEALTHY STAND OF VEGETATIVE COVER HAS STABILIZED ALL SLOPES. 8. THE CONTRACTOR SHALL PERFORM INSPECTIONS AS R EQUIRED BY TDEC AT LEAST TWICE A WEEK, 72 HOURS OR MORE APART. INSPECTION REPORT FORMS SHALL BE MAINTAINED ON SITE. INSPECTORS MUST HAVE COMPLETED THE "FUNDAMENTALS OF EROSION PREVENTION AND SEDIMENT CONTROL" COURSE

TRAFFIC CONTROL NOTES

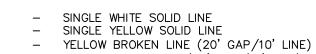
1. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL LOCAL PROPERTY OWNERS & BUSINESSES.

2. THE CONTRACTOR SHALL BE REQUIRED TO PROVIDE TEMPORARY TRAFFIC CONTROL MEASURES TO INSURE SAFETY FOR MOTORISTS, RESIDENTS, BUSINESSES, ETC. AT ALL TIMES DURING CONSTRUCTION.



<u>&</u>	<u>S LEGEND</u>
2	INLET PROTECTION
D	OUTLET PROTECTION
-	SILT FENCE
Ξ	CONSTRUCTION EXIT
N	CONCRETE WASHOUT
S	PERMANENT SEEDING
2	FILTER RING
	EXISTING FLOW LINE

	<u>LEGEND</u>	
XISTING	NEW	DESCRIPTION
—1579——	-1579.00-	CONTOUR
	+1578.44	SPOT ELEVATION
\bullet		BENCHMARK
↓		PROPERTY LINES
		CURB/EDGE OF PAVEMENT
= = =		STORM SEWER
D	\square	STORM MANHOLE
		CATCH BASIN/ CURB INLET
© _{CO(E)}	•	CLEANOUT (STORM/SANITA
—8" SAN—		SANITARY SEWER
S		SANITARY MANHOLE
\bigcirc		WATER METER
\bowtie	M	GATE VALVE
ж	$\overline{\mathbf{x}}$	FIRE HYDRANT
	— 8" WL —	WATER LINE
کر PP(E)		POWER POLE
ф _{LP(E)}	*	LIGHT POLE
		GUY WIRE
—OHE(E)—		OVERHEAD ELECTRIC
——NG(E)——		NATURAL GAS
	X X X	FENCE
	<u> </u>	CONCRETE AREAS
		ASPHALT PAVEMENT
		ASPHALT PAVEMENT
ΡΔ\/FMF	NT MARKING	ABBREVIATIONS



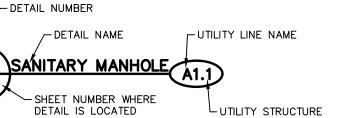
-	WHITE BROKEN LINE (20' GAP/10' LINE)
-	WHITE DOTTED LINE (4' GAP/2' LINE)

- WHITE STOP BAR _ DOUBLE YELLOW SOLID LINE
- XHATCH CROSSHATCHING TLUA – TURN LANE-USE ARROW
 - **OTHER ABBREVIATIONS**

PVC – POLYVINYL CHLORIDE DUCTILE IRON PIPE

- CENTRIFUGALLY CAST FIBERGLASS CCFRPM -REINFORCED POLYMER MORTAR REINFORCED CONCRETE PIPE _
 - SMOOTH INTERIOR CORRUGATED PLASTIC

DETAIL CALLOUT IDENTIFICATION



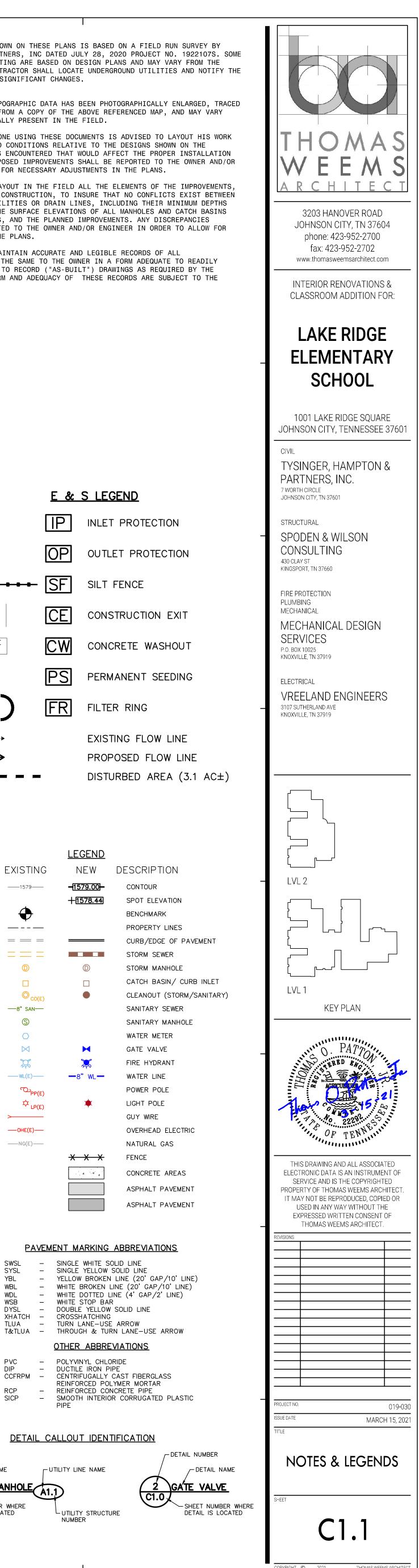
SYS

WDL

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TH&P PROJECT #: 19221

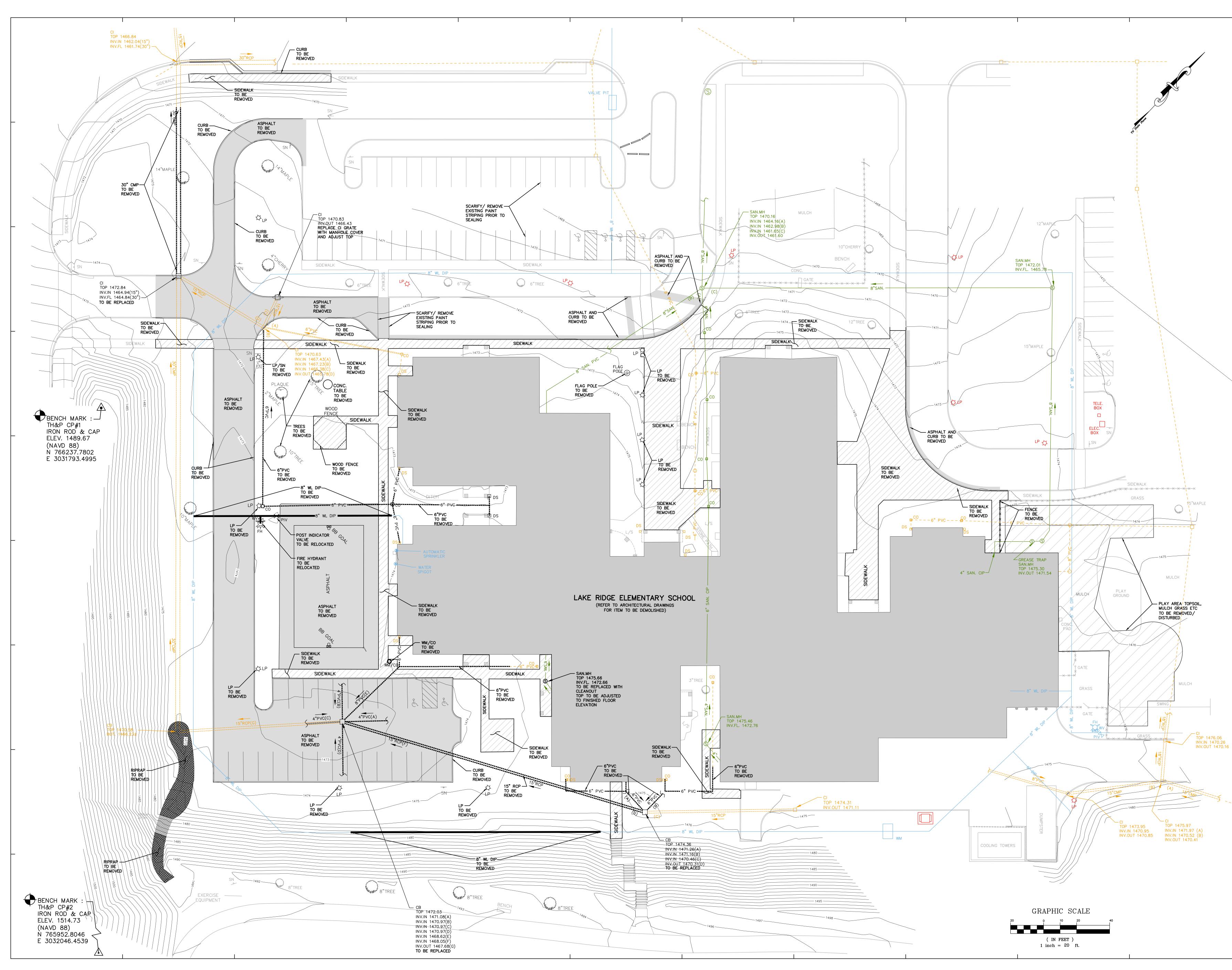
THE TOPOGRAPHIC DATA SHOWN ON THESE PLANS IS BASED ON A FIELD RUN SURVEY BY TYSINGER, HAMPTON & PARTNERS, INC DATED JULY 28, 2020 PROJECT NO. 1922107S. SOME UTILITIES SHOWN AS EXISTING ARE BASED ON DESIGN PLANS AND MAY VARY FROM THE LOCATION SHOWN. THE CONTRACTOR SHALL LOCATE UNDERGROUND UTILITIES AND NOTIFY THE ARCHITECT IF THERE ARE SIGNIFICANT CHANGES.

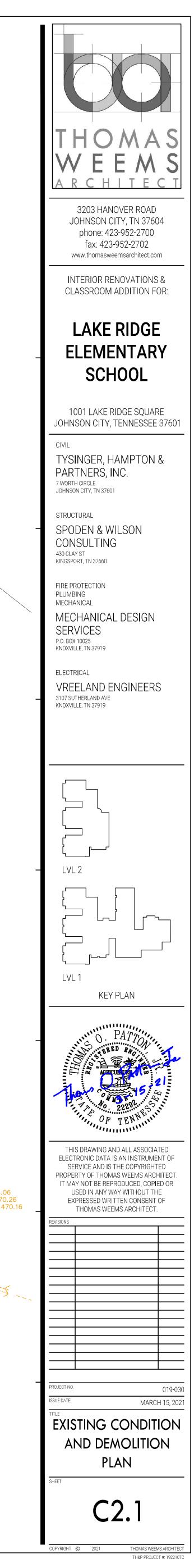
THE CONTOURS AND OTHER TOPOGRAPHIC DATA HAS BEEN PHOTOGRAPHICALLY ENLARGED, TRACED AND OTHERWISE TRANSFERRED FROM A COPY OF THE ABOVE REFERENCED MAP, AND MAY VARY SLIGHTLY FROM WHAT IS ACTUALLY PRESENT IN THE FIELD.

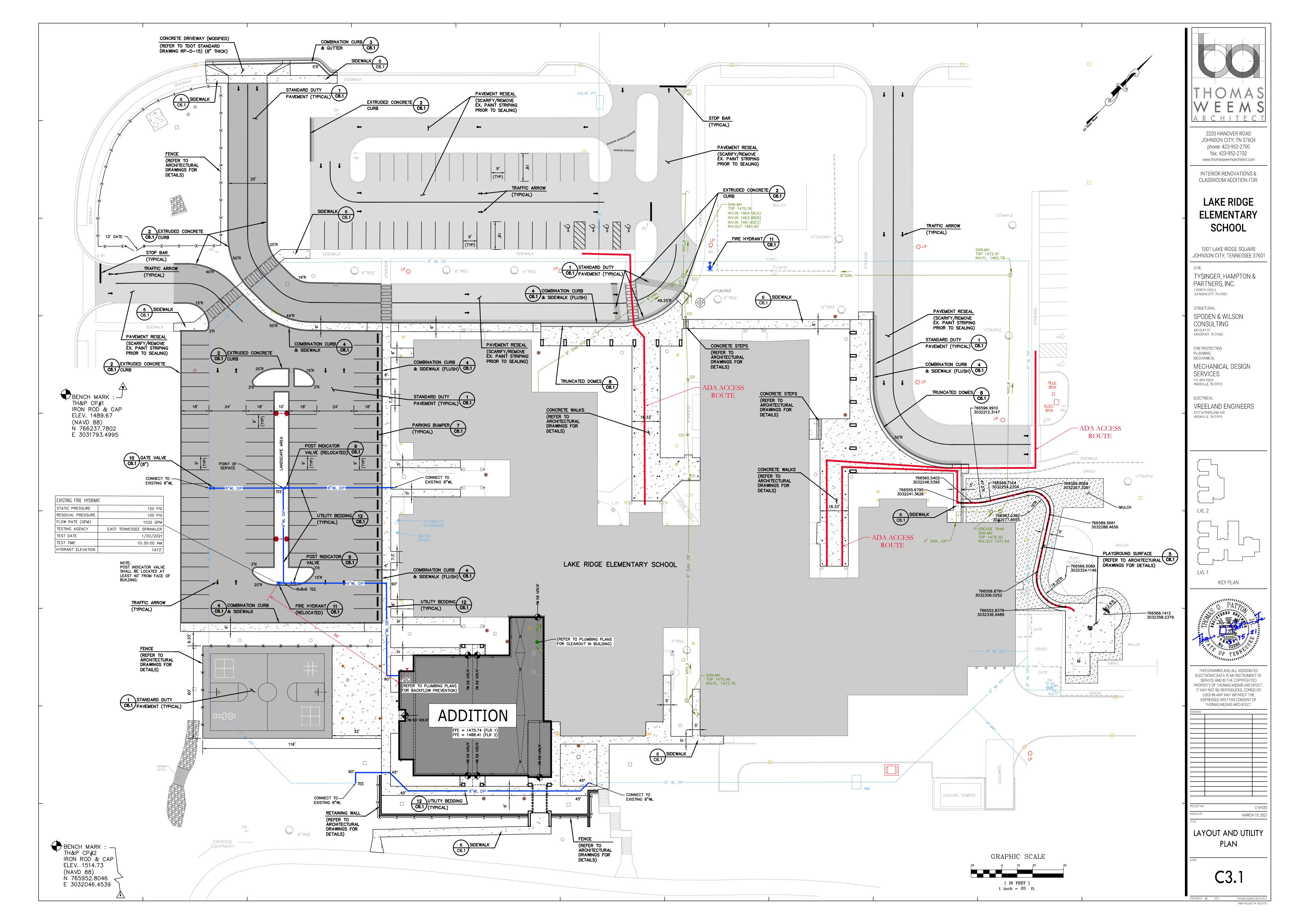
AND VERIFY THE ACTUAL FIELD CONDITIONS RELATIVE TO THE DESIGNS SHOWN ON THE DRAWINGS. ANY DISCREPANCIES ENCOUNTERED THAT WOULD AFFECT THE PROPER INSTALLATION OR CONSTRUCTION OF THE PROPOSED IMPROVEMENTS SHALL BE REPORTED TO THE OWNER AND/OR ENGINEER IN ORDER TO ALLOW FOR NECESSARY ADJUSTMENTS IN THE PLANS.

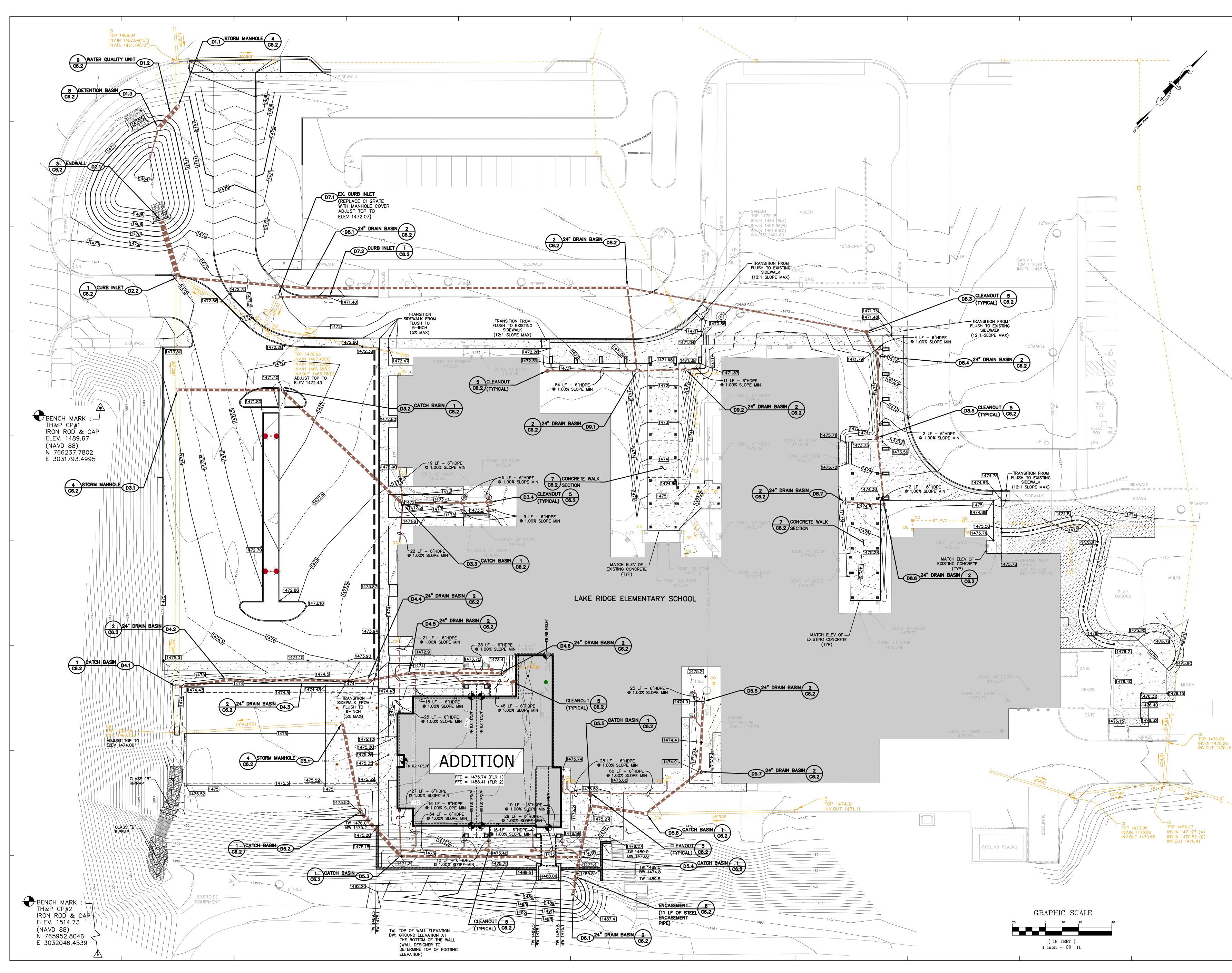
THE CONTRACTOR(S) SHALL LAYOUT IN THE FIELD ALL THE ELEMENTS OF THE IMPROVEMENTS, PRIOR TO AND WELL AHEAD OF CONSTRUCTION, TO INSURE THAT NO CONFLICTS EXIST BETWEEN ANY UNDERGROUND SERVICE UTILITIES OR DRAIN LINES, INCLUDING THEIR MINIMUM DEPTHS BELOW FINISHED SURFACES, THE SURFACE ELEVATIONS OF ALL MANHOLES AND CATCH BASINS RELATIVE TO FINISHED GRADES, AND THE PLANNED IMPROVEMENTS. ANY DISCREPANCIES ENCOUNTERED SHALL BE REPORTED TO THE OWNER AND/OR ENGINEER IN ORDER TO ALLOW FOR NECESSARY ADJUSTMENTS IN THE PLANS

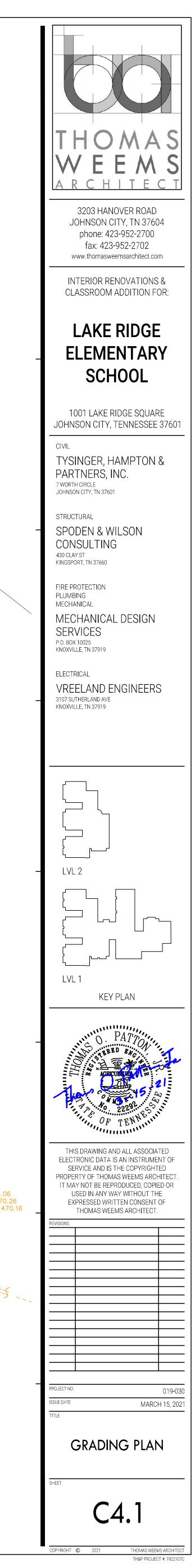
THE CONTRACTOR(S) SHALL MAINTAIN ACCURATE AND LEGIBLE RECORDS OF ALL INSTALLATIONS, AND DELIVER THE SAME TO THE OWNER IN A FORM ADEQUATE TO READILY TRANSFER THE DATA DIRECTLY TO RECORD ("AS-BUILT") DRAWINGS AS REQUIRED BY THE REVIEWING AGENCIES. THE FORM AND ADEQUACY OF THESE RECORDS ARE SUBJECT TO THE APPROVAL OF RECIPIENT.

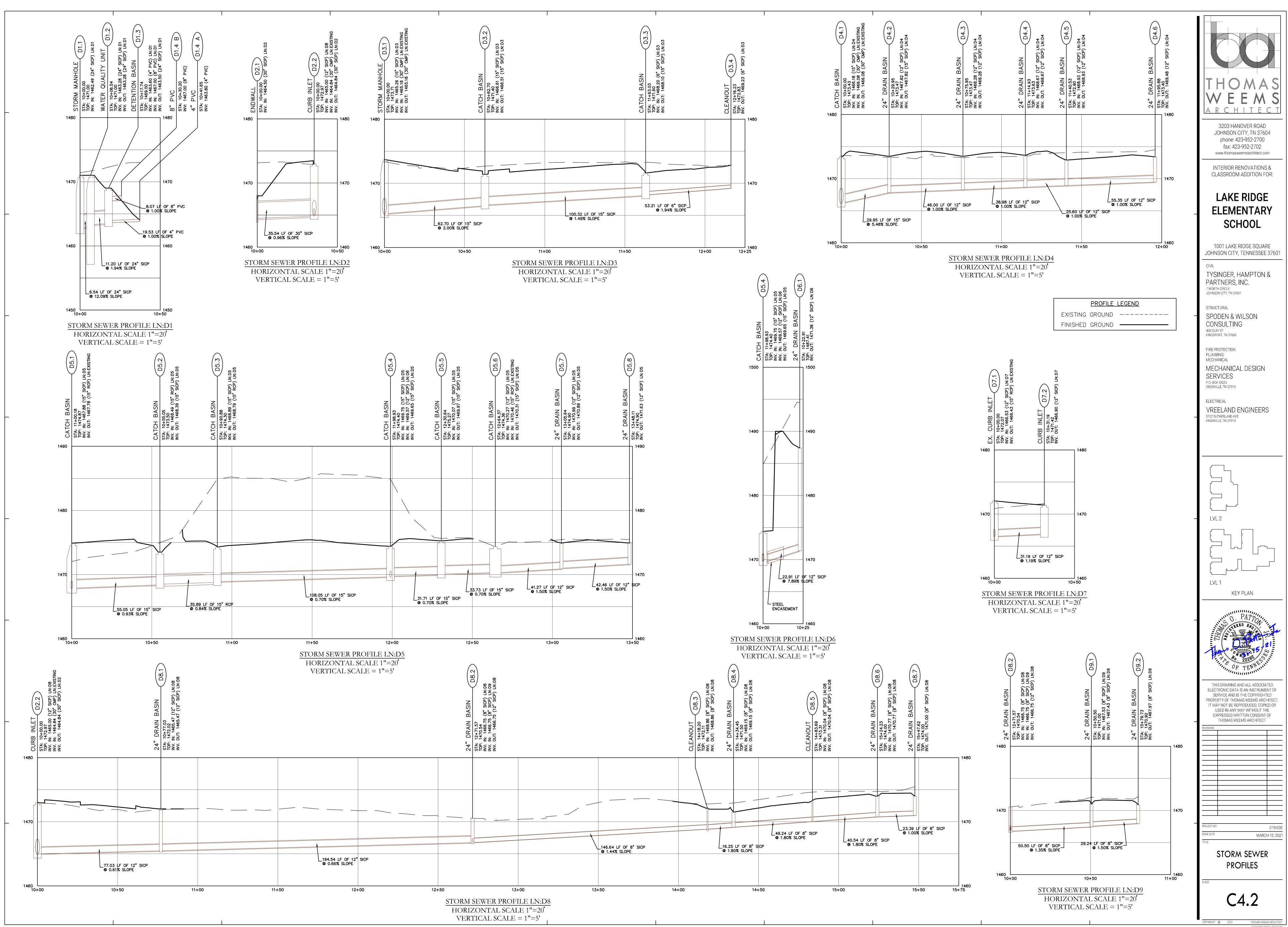




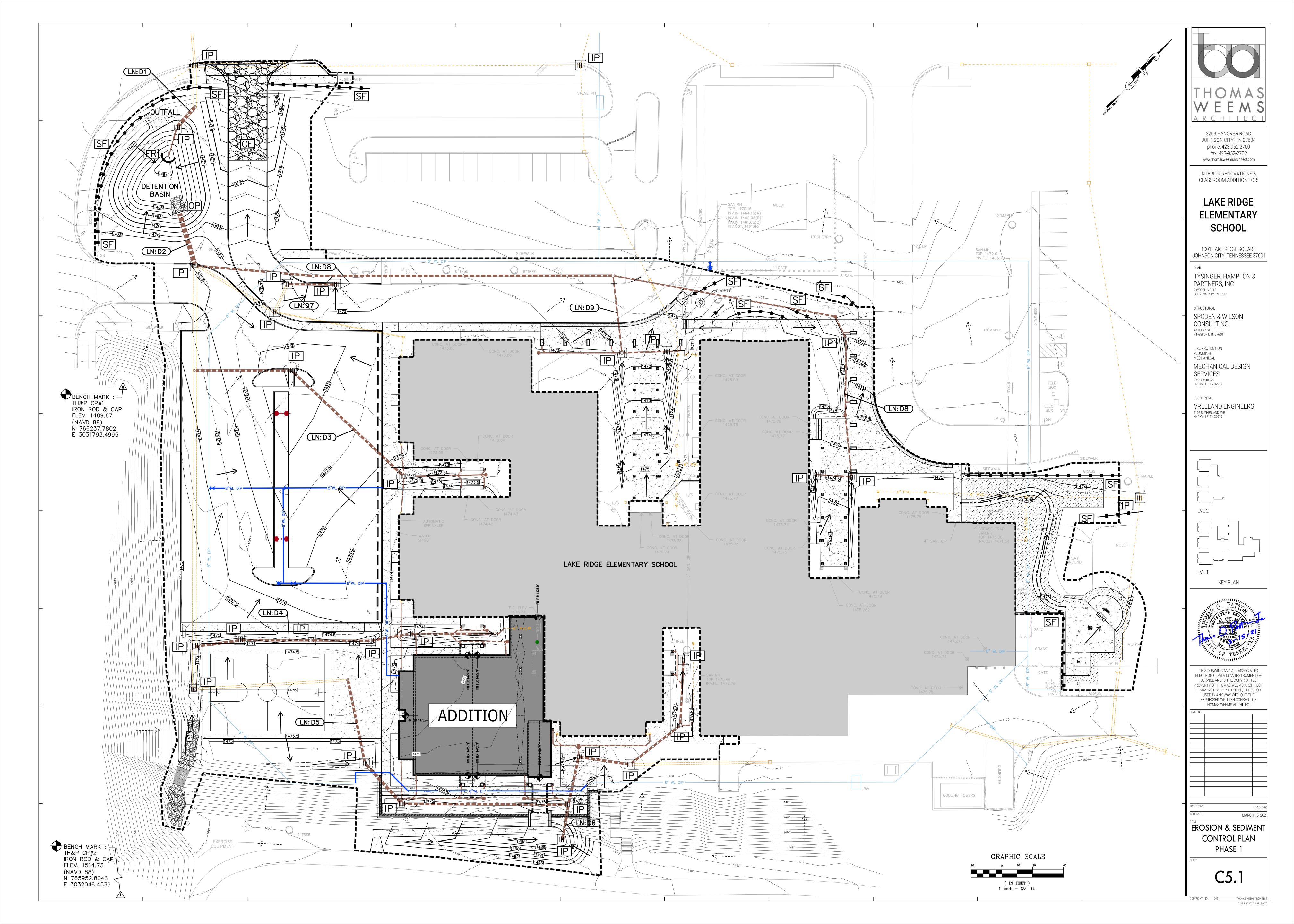


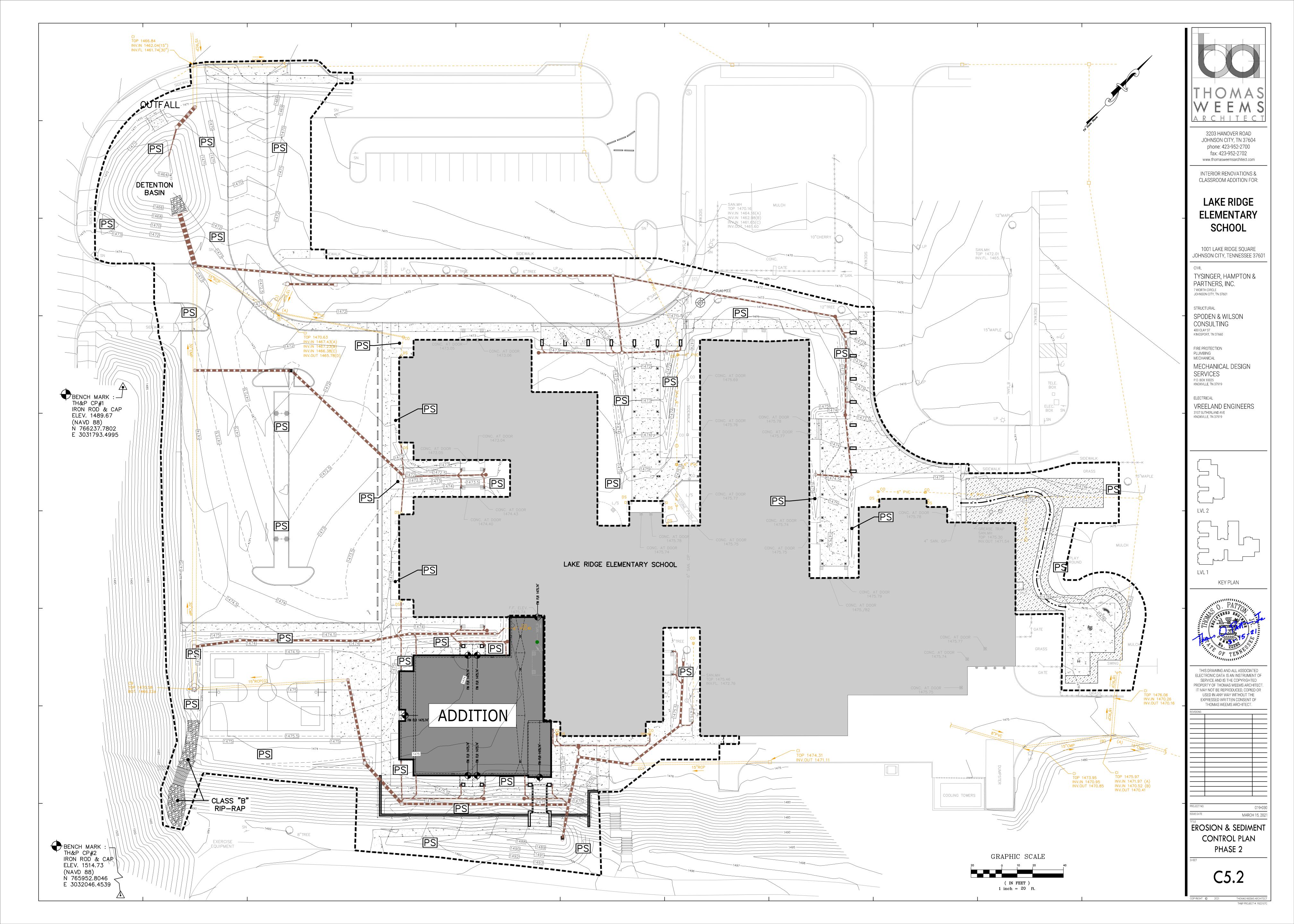


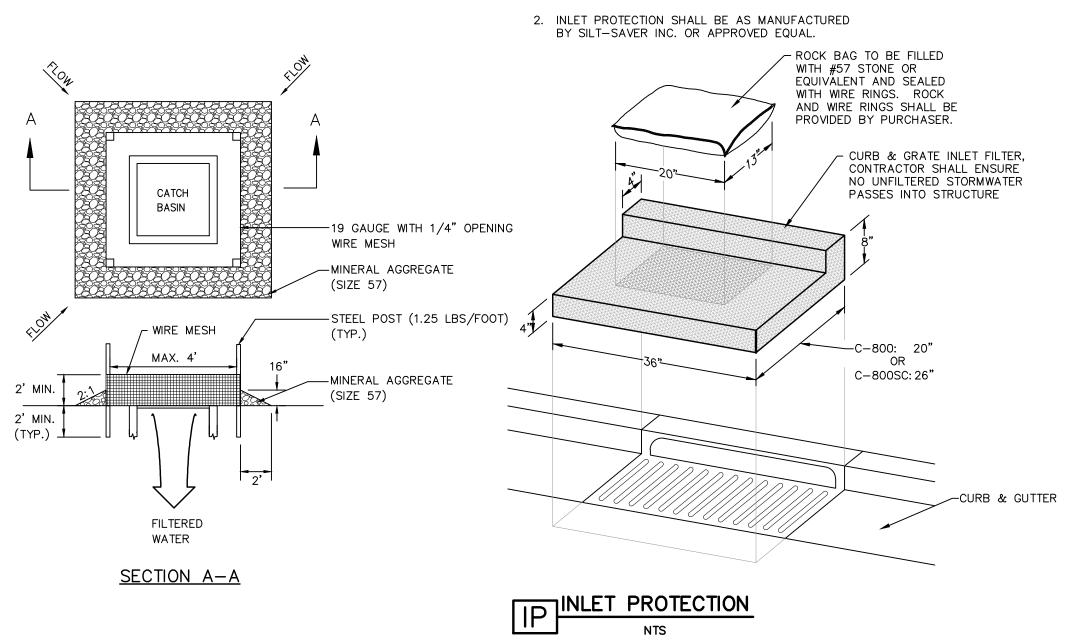


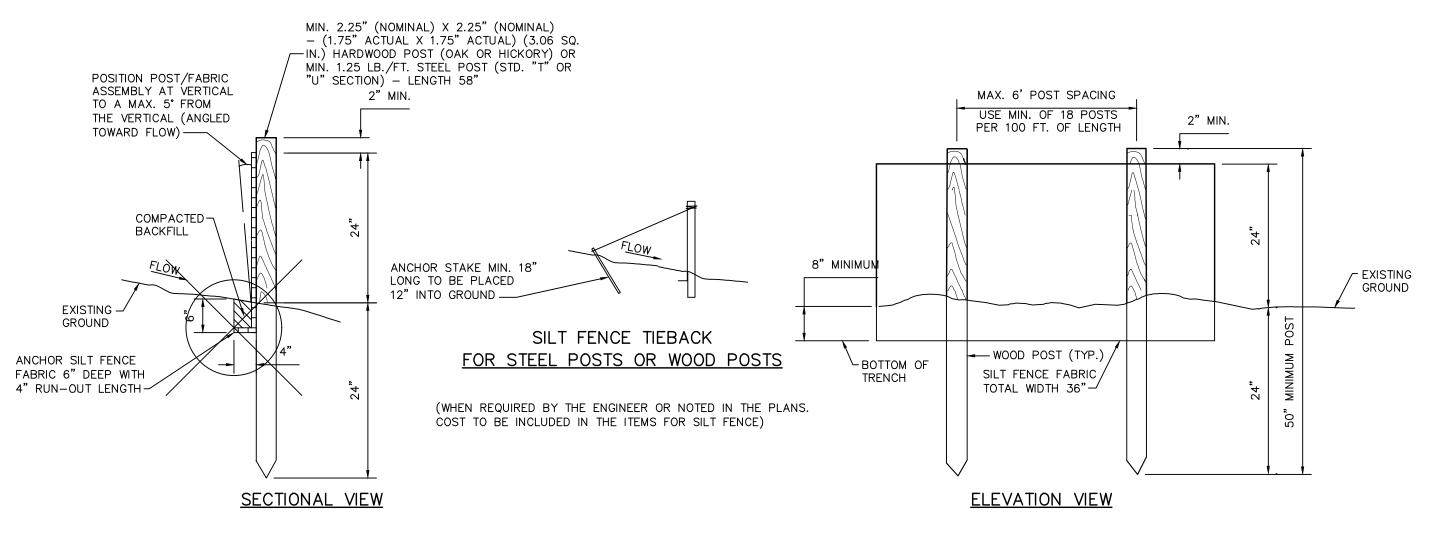


TH&P PROJECT #: 1922107









	SILT	FEN
SL		NTS

			SEEDING MIXES			
ZONE		PREFERREI)		ALLOWABLE	
ZONE	BEST	MARGINAL	RATE/MIX (LB/AC PLS)	BEST	MARGINAL	RATE/MIX (LB/AC PLS)
>2500 FT ELEVATION; STEEP SLOPES	MAR 20 – APR 30	AUG 15 – AUG 30 MAR 1 – MAR 20 APR 20 – JUNE 15	15 BROWNTOP MILLET* (NURSE CROP) 5 PURPLETOP 10 LITTLE BLUESTEM	JULY 25 – AUG 15 MAR 20 – APR 30	JULY 15 - JULY 25 AUG 15 - AUG 30 MAR 1 - MAR 20 APR 20 - MAY 15	100 KY 31 FESCUE** 20 KOBE LESPEDZA**
<2500 FT ELEVATION; STEEP SLOPES	AUG 15 — SEPT 1 MAR 1 — APR 1	SEPT 1 – SEPT 15 APR 1 – JUNE 10	10 INDIAN GRASS 0.5 MONARDA (BERGAMOT) 4 MARYLAND SENNA	AUG 15 – SEPT 1 MAR 1 – APR 1	JULY 25 – AUG 15 SEPT 1 – SEPT 15 APR 1 – MAY 10	10 KOREAN LESPEDZA** 5 REDTOP
>2500 FT ELEVATION; SHALLOW SOILS	MAR 20 – APR 20	AUG 15 – AUG 30 MAR 5 – MAR 20 APR 20 – JUNE 15	15 BROWNTOP MILLET* (NURSE CROP) 4 PURPLETOP 10 LITTLE BLUESTEM 10 BROOMSEDGE	JULY 25 – AUG 15 MAR 20 – APR 20	JULY 15 - JULY 25 AUG 15 - AUG 30 MAR 5 - MAR 20 APR 20 - MAY 15	40 KY 31 FESCUE** 10 KOREAN LESPEDZA**
<2500 FT ELEVATION; SHALLOW SOILS	AUG 15 — SEPT 1 MAR 1 — APR 1	SEPT 1 – SEPT 15 APR 1 – JUNE 10	2 PARTRIDGE PEA 2 BLACK-EYED SUSAN 0.5 MONARDA (BERGAMOT)	AUG 15 – SEPT 1 MAR 1 – APR 1	JULY 25 – AUG15 SEPT 1 – SEPT 15 APR 1 – MAY 10	10 REDTOP 10 CROWN VETCH
>2500 FT ELEVATION; MODERATE SLOPES	MAR 20 – APR 20	AUG 15 – AUG 30 MAR 5 – MAR 20 APR 20 – JUNE 15	15 BROWNTOP MILLET* (NURSE CROP) 4 PURPLETOP 10 LITTLE BLUESTEM 10 INDIAN GRASS	JULY 25 – AUG 15 MAR 20 – APR 20	JULY 15 - JULY 25 AUG 15 - AUG 30 MAR 5 - MAR 20 APR 20 - MAY 15	60 KY 31 FESCUE** 15 KOREAN LESPEDZA**
<2500 FT ELEVATION; MODERATE SLOPES	AUG 15 — SEPT 1 MAR 1 — APR 1	SEPT 1 – SEPT 15 APR 1 – JUNE 10	2 BLACK-EYED SUSAN 0.5 MONARDA (BERGAMOT) 4 MARYLAND SENNA	AUG 15 – SEPT 1 MAR 1 – APR 1	JULY 25 – AUG 15 SEPT 1 – SEPT 15 APR 1 – MAY 10	15 KOBE LESPEDZA**
>2500 FT ELEVATION; HIGH MAINTENANCE	MAR 20 – APR 20	AUG 15 – AUG 30 MAR 5 – MAR 20 APR 20 – JUNE 15	15 BROWNTOP MILLET* (NURSE CROP) 45 RED FESCUE*	JULY 25 – AUG15 MAR 20 – APR 20	JULY 15 - JULY 25 AUG 15 - AUG 30 MAR 5 - MAR 20 APR 20 - MAY 15	200 KY 31 FESCUE**
<2500 FT ELEVATION; HIGH MAINTENANCE	AUG 15 — SEPT 1 MAR 1 — APR 1	SEPT 1 – SEPT 15 APR 1 – JUNE 10	45 HARD FESCUE* 25 CHEWING FESCUE*	AUG 15 – SEPT 1 MAR 1 – APR 1	JULY 25 – AUG 15 SEPT 1 – SEPT 15 APR 1 – MAY 10	200 KT 3T TESCOE

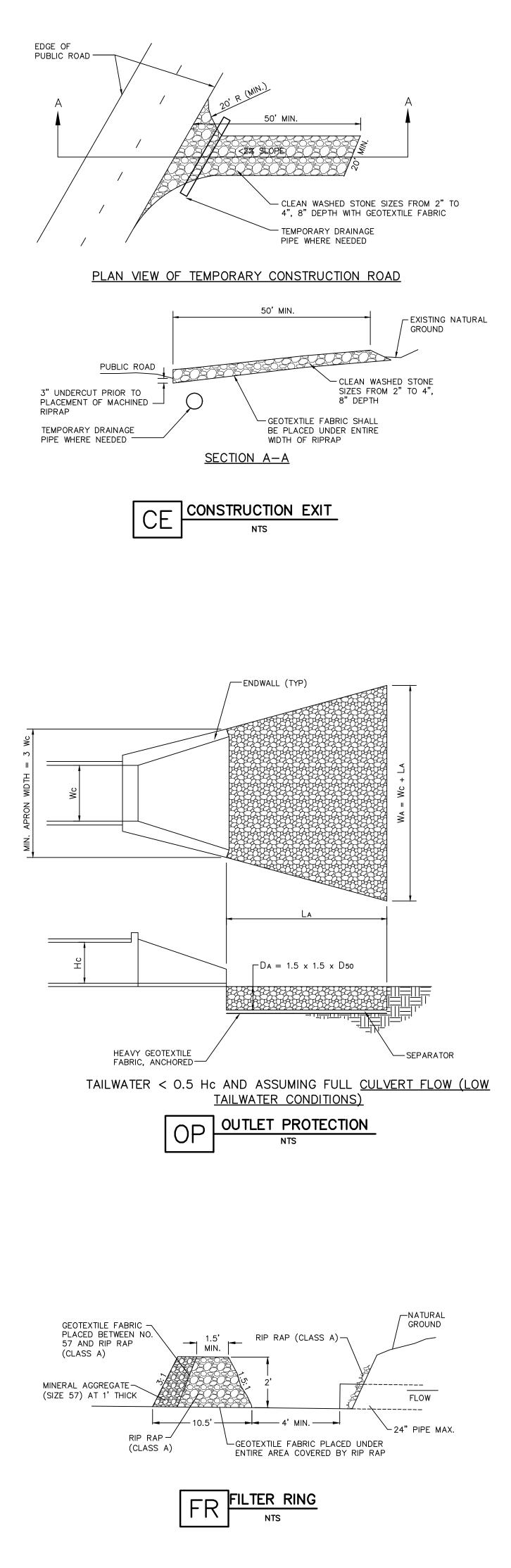
NOTE : 1. FOR SEED BED PREPARATION, MULCHING AND FERTILIZING REFER TO THE TENNESSEE EROSION AND SEDIMENT CONTROL HANDBOOK, LATEST EDITION.

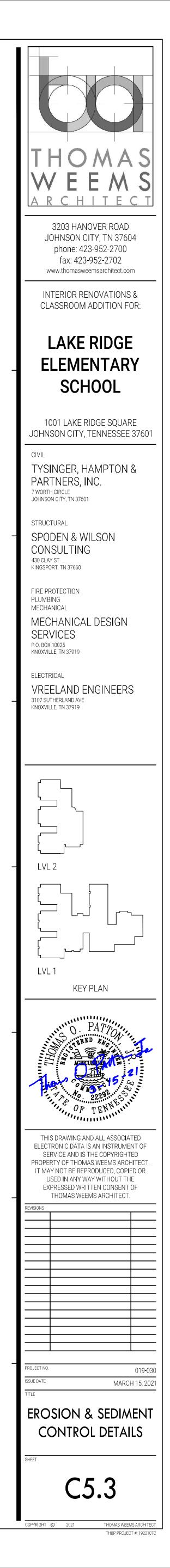
PS PERMANENT VEGETATION

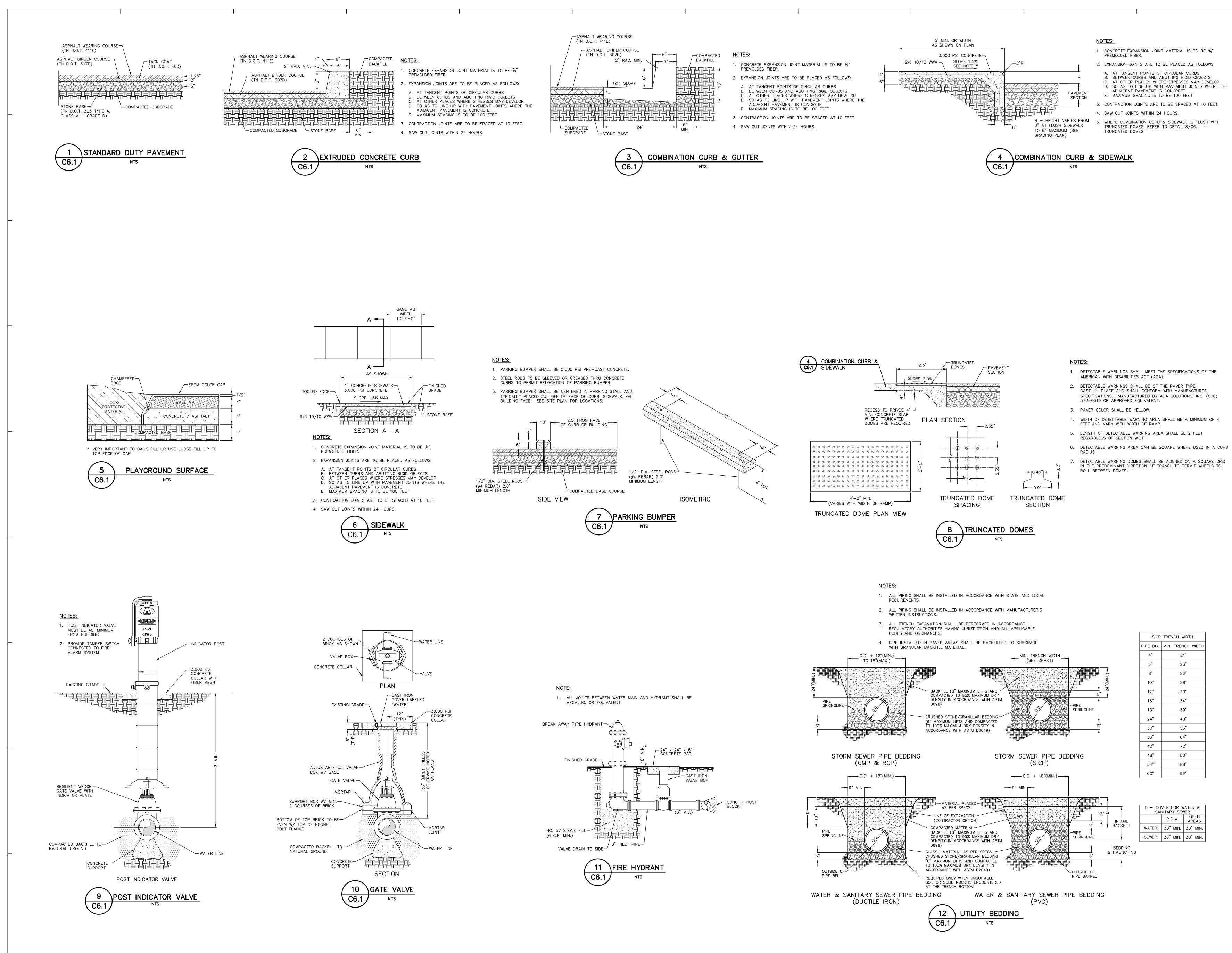
NOTES:

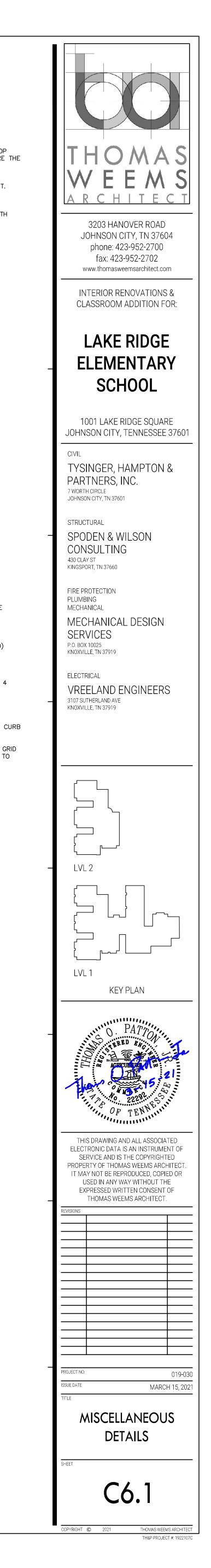
1. INLET PROTECTION SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

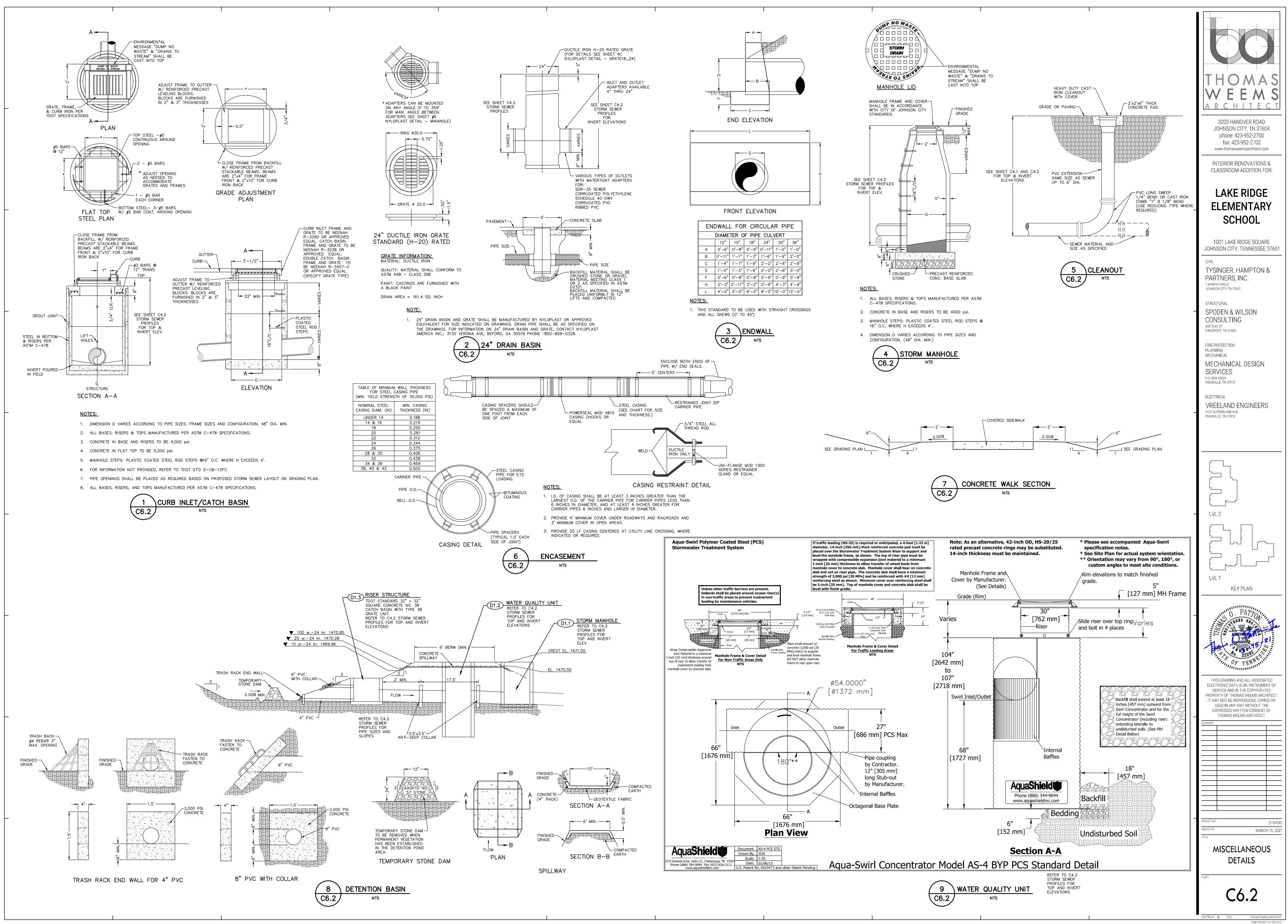
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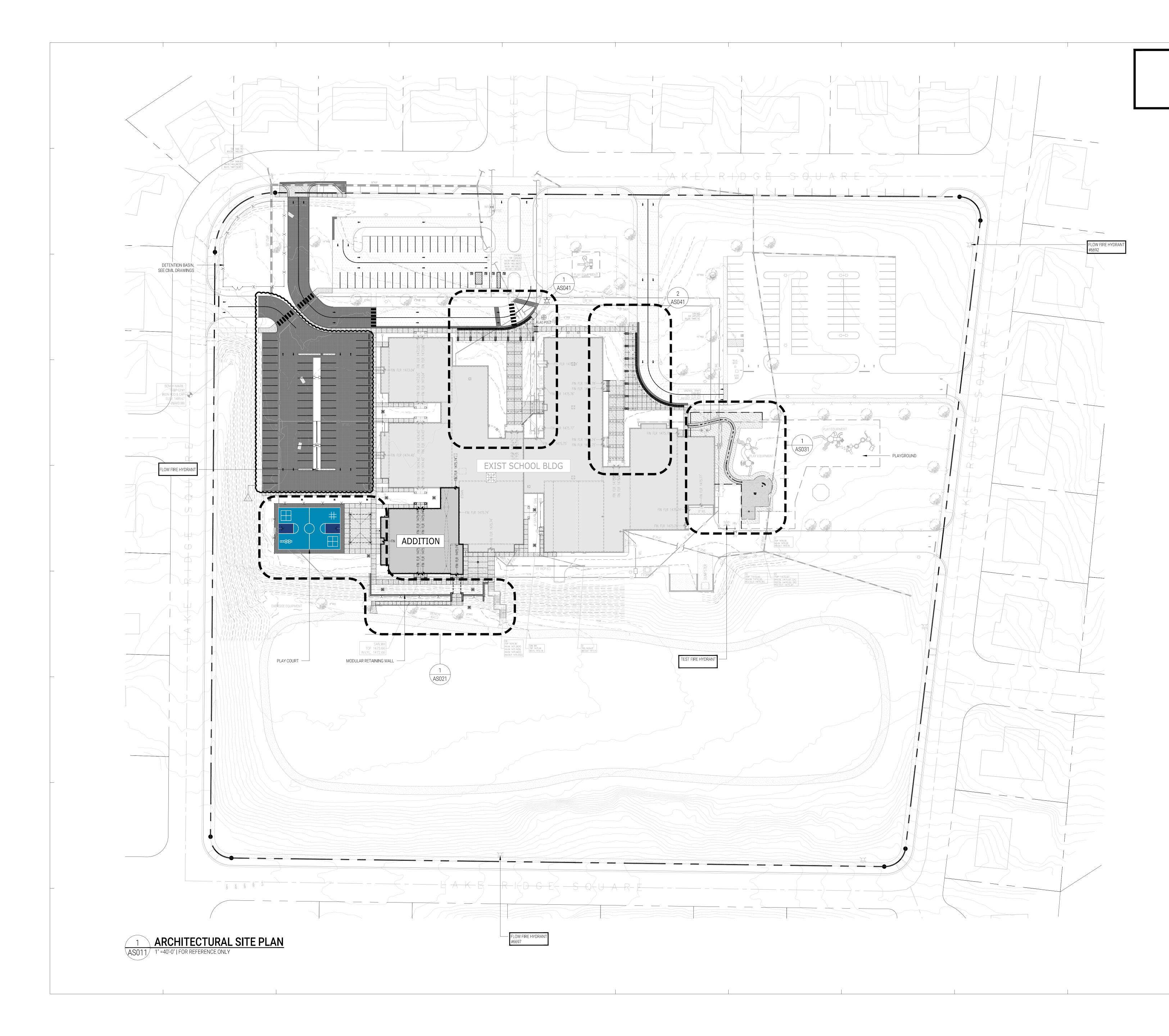




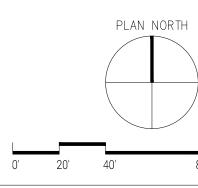




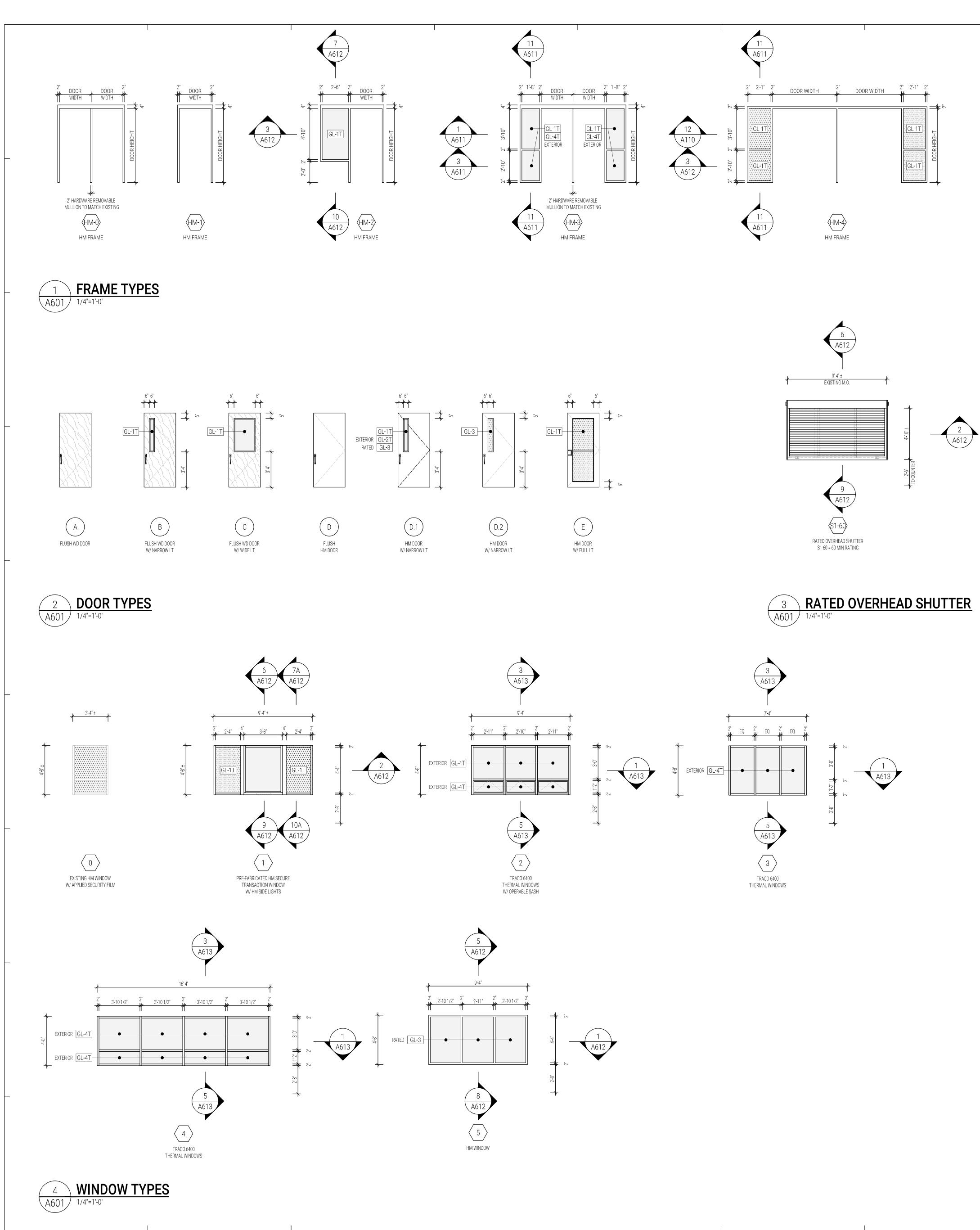




REFER TO CIVIL DRAWINGS FOR FULL SITE PACKAGE







						D	OOR S	SCHE	DULE						
MARK	PANEL QUANTITY	PANEL HEIGHT	PANEL WIDTH	PANEL THICKNESS	PANEL MATERIAL	DOOR TYPE	FRAME MATERIAL	FRAME TYPE	HEAD DETAIL	SILL DETAIL	JAMB DETAIL	FIRE RATING	HARDWARE	NOTES	MARK
X100	1	ETR	3'-0"±	ETR	ETR	ETR	ETR	ETR	-	-	_	ETR	-	4	X100
X101	1	ETR	3'-0"±	ETR	ETR	ETR	ETR	ETR	-	3 / AS047	-	ETR	18		X101
X102	1	ETR	3'-0"±	ETR	ETR	ETR	ETR	ETR	-	3 / AS047	-	ETR	18		X102
X103 X104	1	ETR ETR	3'-0"± 3'-0"±	ETR	ETR ETR	ETR	ETR	ETR ETR	- -	-	-	ETR ETR	- -	4	X103 X104
										SIM TO				4	
X105	2	ETR	3'-0" ±	ETR	ETR	ETR	ETR	ETR	-	3 / AS047	-	ETR	18		X105
X106 X107	1	ETR ETR	3'-0"± 3'-0"±	ETR ETR	ETR ETR	ETR ETR	ETR ETR	ETR ETR	-	-	-	ETR ETR	-	4	X106 X107
X200	1	ETR	3'-0"±	ETR	ETR	ETR	ETR	ETR	-	-	-	ETR	-	4	X200
102A	2	7'-0"	3'-0"	0'-1 3/4"	HM	E	НМ	HM-4	7 / A611	11 / A611	3 / A611		16	3	102A
102/(102B	2	7'-0"	3'-0"	0'-1 3/4"	HM	E	HM	HM-4	7 / A611	11 / A611	3 / A611		15	3	102/X
103	1	7'-0"	3'-0"	0'-1 3/4"	HM	D.2	HM	HM-1	SIM. TO 8 / A611	-	SIM. TO 4 / A611	45 MIN	13	2, 3	103
190A	2	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-3	5 / A611	9 / A611	1 / A611		3	1	190A
190B	2	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	НМ	HM-3	8 / A611	_	4 / A611		14		190B
190C	1	7'-0"±	3'-0"±	0'-1 3/4"±	HM	D.1	НМ	HM-1	6 / A611	10 / A611	2 / A611	45 MIN	12	2	190C
191A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611	-	4 / A611	45 MIN	10	2	191A
191B	1	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-1	5 / A611	9 / A611	1 / A611		2	1	191B
191C 191D	1	7'-0" 7'-0"	3'-0" 3'-0"	0'-1 3/4"	WD WD	C	HM HM	HM-2 HM-1	8 / A611 8 / A611	-	4 / A611 4 / A611		6	4	191C 191D
191D 192A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611		4 / A611	45 MIN	10	2	191D 192A
192B	1	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-1	5 / A611	9 / A611	1 / A611		2	1	192B
192C	1	7'-0"	3'-0"	0'-1 3/4"	WD	С	HM	HM-2	8 / A611	-	4 / A611		6	4	192C
192D	1	7'-0"	3'-0"	0'-1 3/4"	WD	А	НМ	HM-1	8 / A611	-	4 / A611		4		192D
193A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611	-	4 / A611	45 MIN	10	2	193A
193B	1	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-1	5 / A611	9 / A611	1 / A611		2	1	193B
193C	1	7'-0"	3'-0"	0'-1 3/4"	WD	C	HM	HM-2	8 / A611	-	4 / A611		6	4	193C
193D	1	7'-0"	3'-0"	0'-1 3/4"	WD	A	HM	HM-1	8 / A611	-	4 / A611 4 / A611		4	0	193D
194A 194B	1	7'-0" 7'-0"	3'-0" 3'-0"	0'-1 3/4"	WD HM	B D.1	HM HM	HM-1 HM-1	8 / A611 5 / A611	- 9 / A611	4 / A611 1 / A611	45 MIN	10 2	2	194A 194B
194C	1	7'-0"	3'-0"	0'-1 3/4"	WD	C	HM	HM-2	8 / A611		4 / A611		6	4	194D
194D	1	7'-0"	3'-0"	0'-1 3/4"	WD	A	HM	HM-1	8 / A611		4 / A611		4	· · ·	194D
195A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611	-	4 / A611	1 HOUR	11	2	195A
195B	1	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	НМ	HM-1	5 / A611	9 / A611	1 / A611		2	1	195B
196A	1	7'-0"	3'-0"	0'-1 3/4"	WD	А	НМ	HM-1	8 / A611	-	4 / A611	45 MIN	7	2	196A
196B	2	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-0	5 / A611	9 / A611	1 / A611		3	1	196B
197 198	1	7'-0" 7'-0"	3'-0" 3'-0"	0'-1 3/4"	WD WD	A	HM HM	HM-1 HM-1	8 / A611 8 / A611	-	4 / A611 4 / A611	45 MIN 45 MIN	5	2	197 198
190		7-0	3-0		WD	A			1			45 10111	5	Z	190
250A	2	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-3	5 / A611	8 / A413	1 / A611		3	1	250A
250B	2	7'-0"	3'-0"	0'-1 3/4"	HM	D.1	HM	HM-3	8 / A611	-	4 / A611		14		250B
251A 251C	1	7'-0" 7'-0"	3'-0" 3'-0"	0'-1 3/4"	WD WD	B C	HM HM	HM-1 HM-2	8 / A611 8 / A611	-	4 / A611 4 / A611	45 MIN	10 6	2	251A 251C
251D	1	7'-0"	3'-0"	0'-1 3/4"	WD		HM	HM-1	8 / A611		4 / A611		4	4	2510 251D
252A	1	7'-0"	3'-0"	0'-1 3/4"	WD	B	HM	HM-1	8 / A611	_	4 / A611	45 MIN	10	2	251D
252C	1	7'-0"	3'-0"	0'-1 3/4"	WD	C	HM	HM-2	8 / A611	-	4 / A611		6	4	252C
252D	1	7'-0"	3'-0"	0'-1 3/4"	WD	А	HM	HM-1	8 / A611	-	4 / A611		4		252D
253A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611	_	4 / A611	45 MIN	10	2	253A
253C	1	7'-0"	3'-0"	0'-1 3/4"	WD	С	HM	HM-2	8 / A611	-	4 / A611		6	4	253C
253D	1	7'-0"	3'-0"	0'-1 3/4"	WD	Α	HM	HM-1	8 / A611	-	4 / A611		4		253D
254A	1	7'-0"	3'-0"	0'-1 3/4"	WD	В	HM	HM-1	8 / A611	-	4 / A611	45 MIN	10	2	254A
254C 254D	1	7'-0" 7'-0"	3'-0" 3'-0"	0'-1 3/4"	WD WD	C	HM HM	HM-2 HM-1	8 / A611 8 / A611	-	4 / A611 4 / A611		6	4	254C 254D
254D 255	1	7-0" 7'-0"	3-0"	0'-1 3/4	WD WD	B	HM	HM-1 HM-1	8 / A611 8 / A611	-	4 / A611 4 / A611	1 HOUR	4	2	254D
255	1	7'-0"	3'-0"	0'-1 3/4"	WD	A	HM	HM-1	8 / A611	-	4 / A611	45 MIN	7	2	255
257	1	7'-0"	3'-0"	0'-1 3/4"	WD	A	HM	HM-1	8 / A611	_	4 / A611	45 MIN	5	2	257
258	1	7'-0"	3'-0"	0'-1 3/4"	WD	A	HM	HM-1	8 / A611	-	4 / A611	45 MIN	9	2	258
	1		1	1			1	1	1	1	1	[
500A	1	7'-0"	3'-0"	0'-1 3/4"	НМ	D	HM	HM-1	8 / A611	-	4 / A611	1 HOUR	8		500A

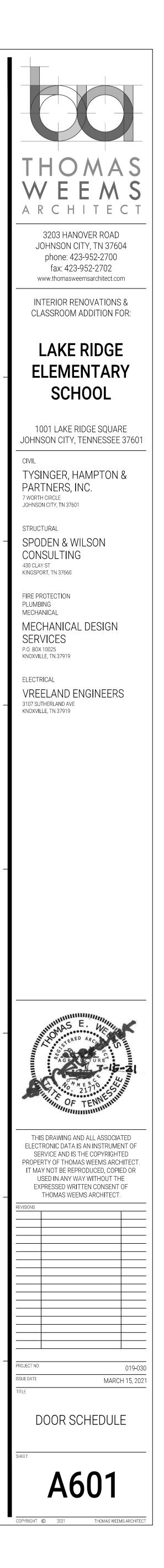
	GENERAL NOTES - DOOR SCHEDULE
1	NOTED DIMENSIONS ARE DOOR CLEAN OPENING, EXCLUSIVE OF STOPS.
2	DOORS MAY BE UNDERCUT A MAXIMUM DIMENSION ALLOWED BY FIRE LABEL.
3	DIMENSIONS INDICATED AS FEET AND INCHES; 3'-0" = 3 FEET, 0 INCHES
4	ALL DOORS ARE 1-3/4" THICK UNLESS NOTED OTHERWISE.
5	ALL DOOR AND FRAME ASSEMBLIES ARE MINIMUM LABEL NOTED.
6	ALL EXISTING AND NEW HOLLOW METAL DOORS / FRAMES ARE TO BE FACTORY PRIMED AND FIELD PAINTED.
8	VISION PANELS IN NON-RATED DOORS TO BE INSTALLED IN METAL FRAMES. FRAMES TO BE PAINTED, REFERENCE FINISH SCHEDULE.
9	VISION PANELS IN RATED DOORS TO BE INSTALLED IN METAL FRAMES. FRAMES TO BE PAINTED, REFERENCE FINISH SCHEDULE.
10	INTERIOR AND EXTERIOR PASSAGE DOORS WITH LATCHING HARDWARE MUST BE EQUIPPED WITH OPERATING HARDWARE DEVICES THAT COMPLY WITH NCHD 4.3.9 (A) (1-4).
11	PROVIDE TEXTURED SURFACES ON DOOR HANDLES LEADING TO HAZARDOUS AREAS.
12	INSTALL VISION PANELS IN DOORS WITH BOTTOM EDGE NO HIGHER THAN 40" A.F.F.
13	RATED DOORS TO HAVE RATED FRAMES, HARDWARE, CLOSERS, AND OTHER FIRE-RATED ACCESSORIES.
14	HARDWARE SCHEDULE INCLUDED IN PROJECT MANUAL - SPECIFICATION SECTION 08 7100 DOOR HARDWARE.
15	EXTERIOR HOLLOW METAL DOORS ARE TO BE INSULATED.
16	CONTRACTOR SHALL BE RESPONSIBLE FOR ENLISTING AN EXPERIENCED COMPANY IN THE PREPARATION OF FACILITY'S EXISTING DOOR(S) FOR UNDERCUTTING, AS SCHEDULED.
	WORK SHALL BE PERFORMED TO THE SATISFACTION OF THE ARCHITECT AND/OR OWNER. IF WORK IS FOUND TO BE UNSATISFACTORY, CONTRACTOR SHALL REPLACE DOOR(S) AS REQUIRED.
	KEYED NOTES - DOOR SCHEDULE
1	PROVIDE GL-2T TINTED SOLAR GLASS AT ALL EXTERIOR EGRESS DOORS, U.N.O.
2	PROVIDE <u>GL-21</u> TINTED SOLAR GLASS AT ALL EXTERIOR EGRESS DOORS, U.N.U. PROVIDE GL-3 FIRE-RATED GLASS AT ALL RATED DOORS, U.N.O.
۷	

PROVIDE ACCESS CONTROL HARDWARE AT DOORS IN LOBBY 102A, TYP.

UNDERCUT DOOR, COORDINATE WITH MECHANICAL DRAWINGS.

DO	OR, WINDOW, AND FRAME LEGEND
AL	ALUMINUM FRAMED/GLAZED
ETR	EXISTING TO REMAIN
НМ	HOLLOW METAL
LL	LEAD LINED
P LAM	PLASTIC LAMINATE FACED

	GLAZING SCHEDULE
	REFER TO SPECIFICATIONS
GLAZING TYPE	DESCRIPTION
GL-1T	1/4" THICK <u>TEMPERED</u> GLASS, CLEAR
GL-2T	1/4" THICK <u>TEMPERED</u> GLASS, SOLAR TINT
GL-3	FIRE-RATED GLASS
GL-4T	LOW-E, SOLAR TINT INSULATING GLASS, <u>TEMPERED</u>
	SECURITY FILM APPLIED TO GLASS





PRE-BID CONFERENCE AGENDA and MINUTES

ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

1001 Lake Ridge Square

Johnson City, Tennessee 37601

DATE: Tuesday, May 11, 2020 at 3:30 PM Local Time.

LOCATION: Project site, Lake Ridge Elementary School, 1001 Lake Ridge Square, Johnson City, Tennessee 37601

Bids will be accepted from Bidders of Record only (those obtaining Bid Documents from Thomas Weems Architect).

A. INTRODUCTION OF PROJECT TEAM:

- 1. The Owner is: City of Johnson City Tennessee, Municipal and Safety Building, 601 East Main Street, Johnson City, Tennessee 37601.
 - a. Owners Representative: Randy Trivette, Telephone: 423-434-5718.
- 2. The Project is: ITB #6498 Interior Renovations and Classroom Addition for Lake Ridge Elementary School, 1001 Lake Ridge Square, Johnson City, Tennessee 37601.
 - a. Principal: Renee Woods, Telephone: 423-610-6030.
 - b. Supervisor of Maintenance: Joe Barnes, Telephone: 423-791-0218.
- 3. The Architect is: Thomas Weems Architect, 3203 Hanover Road, Johnson City, Tennessee 37604.
 - a. Architect: Thomas Weems AIA, Telephone: 423-952-2700 X1.
 - b. Architect: Katie Hill, Telephone: 423-952-2700 X2. (Contact for ALL questions).
- 4. The Architect's Consultants: The Architect has retained the following design professionals who have prepared designated portions of the Contract Documents:
 - a. Civil Engineer: Tysinger Hampton and Partners, Inc., 7 Worth Circle, Johnson City, Tennessee 37601, Telephone: 423-282-2687.
 - b. Structural Engineer: Spoden and Wilson Consulting Engineers, 430 Clay Street, Kingsport, Tennessee 37660, Telephone: 423-245-1811.
 - c. Mechanical and Fire Protection Engineer: Mechanical Design Services, 310 Forest Park Boulevard, Knoxville, Tennessee 37919, Telephone: 865-617-3181.
 - d. Electrical Engineer: Vreeland Engineers, Inc., 3107 Sutherland Avenue, Knoxville, Tennesee37919, Telephone: 865-637-4451.

B. ADVERTISEMENT FOR BIDS:

 ITB #6498: Sealed bids for Johnson City Schools – Interior Renovations and Classroom Addition for Lake Ridge Elementary School, as described in the Contract Documents, will be received by the Johnson City, Purchasing Department, Debbie Dillon, Director, 209 Water Street (37601), P O Box 2150 (37605) Johnson City, Tennessee:

UNTIL: Thursday, May 27, 2021 at 3:00 PM local time.

As a response to COVID -19 Public Health Emergency solicitations will be opened publicly via a web conference only. Public
attendance not permitted. Information normally available in person can be obtained through other methods. Bids shall be hand
carried or mailed to Johnson City Purchasing Department, 209 Water Street, Johnson City, Tennessee 37601. Bids shall *not* be
submitted via email.



JOIN ZOOM MEETING: ITB# 6498 - LAKE RIDGE ELEMENTARY SCHOOL VIRTUAL BID OPENING

MEETING ID: 812 4483 9616

PASSCODE: 998735

- a. If you do not have access to a webcam, or you have no audio with your system, you may call (646) 518-9805 to join.
- b. Any issues accessing the zoom web meeting please call (423) 975-2715 for assistance.
- 3. A Pre-Bid Conference will be held at Lake Ridge Elementary School, 1001 Lake Ridge Square, Johnson City, Tennessee 37601:

DATE: Tuesday, May 11, 2021 at 3:30 PM local time.

- c. Following a general review, Contractors may tour the project site.
- d. Note that social distancing and wearing of masks will be required at the Pre-Bid Conference and any Site Visit.
- 4. Submit questions in electronic format via Email: Thomas Weems Architect, 3203 Hanover Road, Johnson City, Tennessee 37604, Telephone: 423-952-2700. Attention: <u>katie@thomasweemsarchitect.com</u> until:

DATE: Monday, May 24, 2021 at 12:00 PM local time.

5. Anticipated Start Date for Construction:

DATE: June 2021.

6. All bidders must be licensed contractors as required by Contractor's Licensing Act 1994 (TCA Title 62, Chapter 6) and all requirements therein. The project requires a [5] % Bid Bond, specific insurance and [100] % Payment and Performance Bond. Contractors must comply with all Drug Free Requirements.

D. DOCUMENTS:

- 1. Bidders may obtain electronic copies of the bidding documents [pdf files] at no cost from the Office of the Architect:
 - a. Thomas Weems Architect, 3203 Hanover Road, Johnson City, Tennessee 37604, Telephone: 423-952-2700. Email: tom@thomasweemsarchitect.com
- 2. Additional printed copies of the bidding documents, if required, may be purchased for a fee from:
 - a. Knoxville Blueprint and Supply Company, Inc., 622 Leroy Ave NW, Knoxville Tennessee 37921, Telephone: 865-525-0463. Email: knoxblue.com
- 3. Bidding Documents may be examined at the following locations:
 - a. Associated General Contractors Plan Room, 209 Neal Drive, Blountville, Tennessee 37617, Telephone: 423-323-7121. Email: planroom@tricitiestnagc.org
 - b. Knoxville Builders Exchange, 300 Clark Street, Knoxville, Tennessee 37921, Telephone: 865-525-0443. Email: reporter@bxtn.org
 - c. Fw Dodge McGraw Hill, 622 Leroy Avenue, Knoxville, Tennessee 37921, Telephone: 865-673-9042. Email: Dodge.Docs@construction.com

E. BIDDING PROCEDURES:

- 1. The project will be constructed under a Single Stipulated Sum Prime Contract: AIA A101-2017 Owner Contractor Agreement.
- 2. Bidders shall include all Labor, Materials, Equipment, Transportation, Construction Plant and Facilities necessary to complete Work in bid.
- 3. Observe Tennessee Public Law.
 - a. Successful bidders will be required to comply with applicable Equal Employment Opportunity laws and regulations.
 - b. Successful bidders will be required to furnish Performance bond and Labor and Material Payment Bond prior to beginning this work.
 - c. Successful bidders must comply with applicable federal, state and local codes, including the Americans with Disabilities Act (ADA) Federal Register.



- 4. Contract Time: Substantial completion following receipt of the Owner's written notice to proceed:
 - a. New Construction: Two level, [8] Classroom pod addition and site work. Work of this Phase shall commence June 2021 and be substantially complete and ready for occupancy by February 2022.
 - b. Mechanical Upgrades: Install curbs and roof top HVAC units. Work of this Phase shall commence June 2021 and be substantially complete by August 1, 2021.
 - c. Mechanical Upgrades: Interior mechanical work to connect new roof top HVAC and interior renovation work to provide above ceiling access. Work of this Phase shall commence February 1, 2022 and be complete and ready for occupancy by September 30, 2022.
- 5. Bid Forms: Submit Bid Form 00 4100.
 - a. Fill in ALL Relevant Blanks Handwritten or Typewritten.
 - b. Fill in ALL amounts for Unit Prices.
 - c. Include Owner's Contingency in Base Bid amount.
 - d. Initial ALL Erasures/Changes.
 - e. Acknowledge all Addenda.
 - f. Sign/Seal and submit Bid Form.
- 6. Bid shall be accompanied by City of Johnson City 'Front End' documents and 00 2455 Compliance with Public Acts 587 and 844 and 00 2456 Tennessee Criminal History Background Check, Tennessee Code Annotated Section 49-5-413.
 - a. Fill in ALL Relevant Blanks Handwritten or Typewritten.
 - b. Sign/Seal and submit with Bid Form.
- 7. Bid shall be accompanied by Bid Bond in the amount of five percent (5%) of Bid.

F. COVID-19 PROTOCOLS:

- Contractor required COVID-19 protocols to be utilized by the Contractor throughout the course of this project. A copy of the "Tennessee Pledge – Construction Worksites" document is attached hereto and is available at:
 a. https://www.tn.gov/governor/covid-19/economic-recovery/construction-worksite-guidelines.html.
- Contractor shall utilize the "Tennessee Pledge" guidelines for "Construction Worksites" as issued by the State of Tennessee. These safeguarding protocols are based on the recommendation of the CDC and OSHA and shall be in effect for the duration of this project unless otherwise instructed or modified.
- 3. All construction personnel will be required to wear a mask inside the existing school space during school operating hours.
- 4. Contractor shall maintain a log on site to screen all employees (GC and Sub-Contractors) reporting to work and visitors for COVID-19 symptoms as described on Page 1 of the Tennessee Pledge guidelines for Construction Worksites. Log shall contain the following information for all workers and visitors: date, name, time in/time out, company or affiliation, and a yes/no box to answer the following five questions:
 - a. Have you been in close contact with a confirmed case of COVID-19?
 - b. Are you experiencing a cough, shortness of breath or sore throat?
 - c. Have you had a fever in the last 48 hours?
 - d. Have you had new loss of taste or smell?
 - e. Have you had vomiting or diarrhea in the last 24 hours?

G. CONTRACT REQUIREMENTS:

- 1. General Conditions: AIA-007 General Conditions of the Contract.
- 2. Supplementary Conditions:
 - a. Article 7, Changes in the Work: Contractor's Fee for Changes in the Work:
 - i. For the Contractor, for work performed by the Contractor's own forces: [15] % of the cost.
 - ii. For the Contractor, for work performed by the Contractor's Subcontractor: [5] % of the amount due the Subcontractor.
 - iii. For each Subcontractor, for work performed by that Subcontractor or Sub-subcontractor's own forces: [15] % of the cost.
 - iv. For each Subcontractor, for work performed by the Subcontractor's Sub-subcontractor: [5] % of the cost.
 - b. Article 8, Time:
 - i. Liquidated damages: the sum of five hundred dollars and zero cents [\$500.00] as fixed and agreed liquidated damages



for each calendar day of delay until the project is substantially complete.

- c. Article 10, Protection of Persons and Property:
 - i. General Contractor is responsible for design and implementation of safety programs.
 - ii. Proper conduct of employees must be enforced.
 - iii. Use of drones on site requires aviation liability insurance coverage and FCC operator's license.
 - iv. Use of alcohol and tobacco on site is prohibited.
 - v. Alcohol free workplace.
 - vi. Gun free zone.
- d. Article 11, Insurance and Bonds: General Contractor to Provide:
 - i. General Contractor to provide insurance specified in Front End Document FE 1600.
 - ii. Bid Security: In the amount of five percent (5%) of Bid.
 - iii. Performance Bond and Payment Bond: In the amount of one hundred percent (100%) of the Contract Sum.
- 3. Existing Conditions:
 - a. Owner will occupy Building for administrative, custodial and school functions.
 - i. General Contractor must minimize disturbance of normal daily functions.
 - ii. General Contractor to coordinate with Owner for use of space.
 - iii. Do not block driveways.
 - iv. Do not park on yards or athletic fields.
 - v. General Contractor to coordinate use of parking lot and driveways with Owner.
 - vi. Construction access and staging areas to be reviewed at Pre-Bid Conference.
- 4. Scope of Work:
 - a. The Work of Project is defined by the Contract Documents and consists of the following: ITB 6498 Interior Renovations and Classroom Addition for Lake Ridge Elementary School. Work includes: Demolition, civil, general construction, structural, fire protection, mechanical and electrical work.
- 5. Preceding Work by the Owner:
 - a. Not Applicable.
- 6. Concurrent Work by the Owner:
 - a. Not Applicable.
- 7. Allowances:
 - a. Allowance No. 1: Owner's Contingency Allowance: Include the sum of [\$350,000.00] Three Hundred Fifty Thousand Dollars and Zero Cents for unforeseen conditions and related costs. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
 - b. Allowance No. 2: Lump Sum Cash Allowance: Include the sum of [\$25,000.00] Twenty Five Thousand Dollars and Zero Cents for testing and inspection services, including special inspections, as specified in Section 01 4523 Testing and Inspection Services. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
 - c. Allowance No. 3: Lump Sum Cash Allowance: Include the sum of [\$2,500.00] Two Thousand Five Hundred Dollars and Zero Cents for gypsum cement underlayment as specified in Section 03 5413 – Gypsum Cement Underlayment. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
 - d. Allowance No. 4: Lump Sum Cash Allowance: Include the sum of [\$2,500.00] Two Thousand Five Hundred Dollars and Zero Cents for wall and ceiling access panels not shown on Drawings and as specified in Section 08 3110 – Wall and Ceiling Access Panels. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
 - e. Allowance No. 5: Lump Sum Cash Allowance: Include the sum of [\$60,000.00] Sixty Thousand Dollars and Zero Cents for removal and replacement of existing acoustical ceiling tile / grid, light fixtures, sprinklers, diffusers, grilles and devices not shown on Drawings but required for installation of new HVAC upgrades. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit. Coordinate quantity allowance adjustment with corresponding unit-price requirements in Section 012200 Unit Prices.



- f. Allowance No. 6: Lump Sum Cash Allowance: Include the sum of: [\$100,000.00] One Hundred Thousand Dollars and Zero Cents for classroom furniture, fixtures and equipment. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
- g. Allowance No. 7: Lump Sum Cash Allowance: Include the sum of: [\$15,000.00] Fifteen Thousand Dollars and Zero Cents for IT equipment and devices not shown on Drawings. This allowance includes material cost, receiving, handling, installation and Contractor overhead and profit.
- h. Allowance No. 8: Lump Sum Cash Allowance: Include the sum of: [\$1,188,129.00] One Million One Hundred Eighty Eight Thousand One Hundred Twenty Nine Dollars and Zero Cents for purchase and delivery of Trane HVAC equipment as specified in Section 23 8114 Rooftop Dx Unit, Section 23 3616 Variable / Constant Volume Air Terminals, Section 23 8126 Ductless Split Unit, Gas Rooftop Units (RTU) Schedule On Drawing M001, Ductless Split Schedule Shown On Drawing M-001, Volume Box Schedule (VVB) Shown On Drawing M-002. This allowance also includes Trane controls equipment and labor as shown in HVAC Controls Drawings M401, M402, M403 and specified in Section 23 0923 Building Systems Controls. This allowance includes the material costs and freight from Trane but does not include receiving, handling, and Contractor mark-up and profit.
- 8. Unit Prices:
 - a. Unit Price No. 1 Acoustical Ceiling Removal and Replacement: Description: Removal and replacement of acoustical ceiling tile and grid not shown on Drawings but required to install specified HVAC upgrades and as specified in Section 09 5100 Acoustical Ceilings. Does not include removal and replacement of light fixtures, sprinkler heads, diffusers, grilles or other devices included in Allowance No. 5. Unit of Measure: Square foot. Quantity Allowance: Coordinate unit price with allowance adjustment requirements in Section 01 2100 Allowances.
- 9. Substitutions: Substitution Request Form 01 2500:
 - a. Fill in ALL Relevant Blanks Handwritten or Typewritten.
 - b. Sign and submit prior to Bid Date.
- 10. Contract Modification Procedures:
 - a. Architect's Supplemental Instructions: AIA Document G710 Architect's Supplemental Instructions.
 - b. Proposal Requests: AIA Document G709 Proposal Request.
 - c. Contractor Proposed Changes: Contractor's standard.
 - d. Construction Change Directive: AIA Document G713 Construction Change Directive.
 - e. Change Order: AIA Document G701 Change Order.
- 11. Request for Information: Request for Information Form 01 2613:
 - a. Fill in ALL Relevant Blanks Handwritten or Typewritten.
 - b. Sign and submit.
- 12. Construction Progress Schedule:
 - a. Submit initial Progress Schedule within [15] calendar days after receipt of Owner's written notice to proceed.
 - b. Submit revised Progress Schedule with each Application for Payment.
- 13. Submittal Procedures:
 - a. Submit proposed Products List within [15] calendar days after receipt of Owner's written notice to proceed.
 - b. Submit submittals electronically.
- 14. Quality Assurance, Testing and Inspection:
 - a. Construction Testing and Inspection Services: Included in specified Allowance.
 - b. Quality Assurance Services: Included in specified Allowance.
 - c. Special Testing Services: Provided by Owner.
- 15. Temporary Facilities:
 - a. Temporary utilities provided by General Contractor.
 - b. General Contractor to provide barriers to prevent unauthorized entry to construction areas and to protect existing facilities and adjacent properties from construction activities.



H. QUESTIONS:

	1.	
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	5.	
I.	NOTE	<u>:S:</u>
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J.	<u>SITE</u>	TOUR:
	1.	
	2.	
	3.	
	4.	
	5.	

Site Tour to be conducted at end of Pre-Bid Conference.
 Contact CoJC Facilities Director Randy Trivette (423-434-5718) to arrange additional site visits.



ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

1001 Lake Ridge Square

Johnson City, Tennessee 37601

DATE: Tuesday, May 11, 2020 at 3:30 PM Local Time.

LOCATION: Project site, Lake Ridge Elementary School, 1001 Lake Ridge Square, Johnson City, Tennessee 37601

Bids will be accepted from Bidders of Record only (those obtaining Bid Documents from Thomas Weems Architect).

Company Name	BURWIL CONSTRUCTION					
Address	620 LOUST ST.					
City	BRISTOL	State	TN	Zip	37620	
Contact Name	STENE JOHNSON					
Phone	423-968-4158	Fax 423-968-3199				
Email						
Linda	SJOHNSON C BURWIL. COM					
Company Name	COC Culat					
Address	GRC Construction					
	130 Regional Park Dr	State	TH	Zip	2166n	
City	Kingsport	Sidle	100	Δip	2.000	
0 1 111	0) 51 1					
Contact Name	Bob Edwisten	___	1			
Phone	349 7760	Fax				
Email	bobe GICINC Com					
	•					
Company Name	ARMSTRONG CONSTRUCTION Co. In	IC.				
Address	151 SHELBY ST.					
City	KINGSPORT	State	TN	Zip	3760	
Contact Name	BEN Malluray					
Phone	423 246 6183	Fax	123	246 7602		
Email	BEN Q. ARMSTRONG CONSTR.	DETION . O				
Company Name	JE Grun Company					
Address	303 B. Market St.					
City	Johnson City	State		Zip	37601	
	Contraction of the second seco	110	_			
Contact Name	in Careen					
Phone	423-926.5(6)	Fax	423	- 926-3512	,	
Email		1 401	1 1			
	Jingolun O jegulan GV. Com					

3203 hanover road johnson city = tennessee 37604 = phone 423-952-2700 = fax 423-952-2702 = www.thomasweemsarchitect.com



ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

0					
Company Name	J. E. GREEN (O.				
Address	303 E. MALKET ST.			7:	1
City	JOHNSON CITY	State	TN	Zip 3,2601	
					i
Contact Name	TAYLOR GREEN				
Phone	423-926-5161	Fax			
Email	taylorgreen. jeg @ginail.com				
Company Name	JA Street & Associates				
Address	Birch St.				
City	Blountville, TN	State TN		Zip 37620)
					······································
Contact Name	Lawton Sizemord				
Phone	423-323-8017	Fax			
Email	Isizemore @ justreet. Com				
	Crizemon (W) Sansfreet. Com				
Company Name	JASTRUET & ASSOCIATES				
Address	JA ANOLI & ADJULIAND				
City		State		Zip	
City		Oluio			<u></u>
Contact Name	Adam Killian				1
	423 - 267 - 0331	Fax			
Phone	90)-009-0331 Guilden Discussion	Гал			
Email	akillion @ Jastreet. Com				
	BRADER J.A.	C.t.	-1		1
Company Name		3/10			
Address	245 BIRCH			7:	200
City	BOUNTVILLE TN	State	1 ZV	Zip	37617
	\sim				
Contact Name	Brian 10E	1	C/200		
Phone	423 323 8017	Fax	423 3	323 10	567
Email	broce astreet.	cam			
Company Name	Acorn Electrical Specialist, Inc.				
Address	403 ROLK LANE				
City	Piner Flats	State	TN	Zip	37686
Contact Name	Scott DePriest				
Phone	423-538-6007	Fax	4123-5	38-595-	3
Email	Scotto Acom electrical, com		100		
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ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

Company Name	JODY HOOD ~ MASSEY ELEC	TRIC.			
Address	546 EASTERN STAR RD.				
City	KINGSPORT, TN.	State 🦽	TN	Zip	37663
Contact Name					
Phone		Fax			
Email					
Company Name	Trane				
Address	10384 Wallace Alley St.				1
City	kingsport	State	YN	Zip 📸	37620
Contact Name	Amy Goodyear				
Phone	717-497-8137	Fax			
Email	amelia.goodyear @ trane.c.	m			
Company Name	S.B. WHITECOJAC.				
Address	P.O. BOX 1734	Olata Ca		7:-	
City	JOHNSON CITY,	State 200	TN	Zip 🧫	37605
Contact Name	DAVID MEKINNEY]
Phone	423-926-B127	Fax	435-		
Email	dmckinney @ Sowhiteco.com				
Email	dimensionery & Southiteco.com				
	dmckinney @ Sowhiteco.com				
Email Company Name	dmckinney @ Sbwhiteco.com Thane 10384 WAllace Alley ST			Zip	37605
Email Company Name Address	dmckinney @ Sowhiteco.com			Zip	37605
Email Company Name Address	dmckinney @ Sbwhiteco.com Thane 10384 WAllace Alley ST Kingsport			Zip	37605
Email Company Name Address City	dmckinney C Sowhiteco.com TRANE 10384 WAllACE Alley ST Kingsport Toth Williams			Zip	37605
Email Company Name Address City Contact Name	dmckinney @ Sbwhiteco.com TRANE 10384 WAllACE Alley ST Kingsport Jorw Williams 423-794-6334	State 7		Zip	37605
Email Company Name Address City Contact Name Phone Email	dmckinney @ Sbwhiteco.com TRANE 10384 WAllace Alley ST Kingsport Joth Williams 423-794-6334 JWilliams 48 TRANE, Com	State 7		Zip	37605
Email Company Name Address City Contact Name Phone	dmckinney @ Sbwhiteco.com TRANE 10384 WAllACE Alley ST Kingsport JoHN Williams 423-794-6334 JWilliams 48 TRANE, Com Creative Magna/	State 7		Zip	37605
Email Company Name Address City Contact Name Phone Email Company Name Address	dmckinney @ Sbwhiteco.com TRANE 10384 WAllace Alley ST Kingsport Joth Williams 423-794-6334 JWilliams 48 TRANE, Com	State 7			37605
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Email Company Name Address City Contact Name Phone Email Company Name Address City	dmckinney @ Sbwhiteco.com TRANE 10384 WAllAR Alley ST Kingsport Joth Williams 423-794-6334 JWilliams 4& TRANE.com Creative Magna/ 301 Bohannon Ave Greavelle	State 7			37605
Email Company Name Address City Contact Name Phone Email Company Name Address City Contact Name	dmckinney @ Sbwhiteco.com TRANE 10384 WAllAR Alley ST Kingsport John Williams 423-794-6334 JWilliams 48 TRANE.com Creative Magna/ 301 Bohangon Ave Grazy.lle Devon Jallar	State TN Fax State			37605
Email Company Name Address City Contact Name Phone Email Company Name Address City	dmckinney @ Sbwhiteco.com TRANE 10384 WAllAR Alley ST Kingsport Joth Williams 423-794-6334 JWilliams 4& TRANE.com Creative Magna/ 301 Bohannon Ave Greavelle	State 7			3760.5

3203 hanover road johnson city tennessee 37604 phone 423-952-2700 fax 423-952-2702 www.thomasweemsarchitect.com



ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

Company Many	Dhe last is				1
Company Name	Path Construction				
Address	125 E Algongun Rd.	Ctata		Zin	6
City	Arlington Heights	State	TL	Zip	60005
					1
Contact Name	Paul Stacey				
Phone	872-400-1803	Fax			
Email	pstacey@pathcc.com				
Company Name	H & M CONSTFUCTORS				
Address	187 DEAVERVIEW RG		1		
City	A Sheville	State ,	NG	Zip	2623 78303
Contact Name	Effic Jones				
Phone	828.716.9543	Fax			
Email	evenes to h-M Constructors.	COM			
	the second				
Company Name	HUAC, no				
Address	BJ Third St		1	T	127100
City	Bristol	State VI		Zip	31620
Contact Name	Andy Savage		1		
Phone	423-939-5800	Fax			
Email	asavage @ hvpc-inc.com				
	3				
Company Name	PRESTON CONST. CO.				
Address	1503 NATERON LN				
City	JUHNSON CITY	State	TN	Zip	37607
Contact Name	RICHARD PROSTON				
Phone	423-924-0172	Fax	423.43	4-2457	
Email	richard eprestancenstuction comp	my.con			
Company Name	City of Johnson City				
Address			1		
City		State		Zip	
Contact Name	I landy Threete				
Phone	423-434-5718	Fax			
Email	Vtrivette CJOhnson CI ty Th.	org			

3203 hanover road - johnson city - tennessee 37604 - phone 423-952-2700 - fax 423-952-2702 - www.thomasweemsarchitect.com



PRE-BID CONFERENCE SIGN IN SHEET

ITB #6498 INTERIOR RENOVATIONS AND CLASSROOM ADDITION: LAKE RIDGE ELEMENTARY SCHOOL

Company Name	THOMAS WEEMS ARCHITECT				
Address	3203 HANOVER ROAD				
City	JAHNSON CITY	State	TN	Zip	37604
	VG MUTCH OILY				
Contact Name	TO AL MEGNAC				
Phone	TOM WEEMS	Fax			
	423-952-2700				
Email	tomethomaswcomsarchite	rt. com			
Company Name	THONKS WEENS ARCHITEO	1			
Address	3203 HANDVER ROAD				
City	JOHNSON CITY	State	TN	Zip •	37604
	Dallanio				
Contact Name	LATE HUL				
Phone	423-952-2700	Fax			
	423-0152-2100				
Email	katiecthomas weems ara	ALLECI	com		
	10				
Company Name					
Address					
City		State		Zip	
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Company Name					
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City		State		Zip	
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Email					

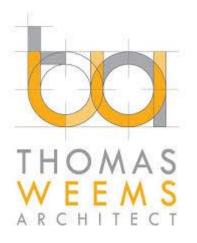


STORM WATER MANAGEMENT PLAN

LAKE RIDGE ELEMENTARY SCHOOL ADDITION

Johnson City, Tennessee

4/21/2021



Prepared For: Tom Weems Architect 3203 Hanover Road Johnson City, TN 37604

Date and Signature Page

Design Report

Prepared For: Tom Weems Architect 3203 Hanover Road Johnson City, TN 37604

This report is current as of 4/21/2021.

Thomas O. Patton, Jr., PE

Thoms O. PAOL- Ja

Revision Log

Revision Number	Date	Pages	Description
1	4/21/21	Appendix G	COJC Comments 4/9/21





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Post-Development	3
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Maintenance of Water Quality Management Facilities	5

APPENDICES

- Appendix A: Location Map
- Appendix B: Drainage Maps
- Appendix C: Hydrologic Analysis
- Appendix D: Hydraulic Analysis
- Appendix E: Water Quality Calculations
- Appendix F: BMP Operations and Maintenance
- Appendix G: Fire Truck Exhibit



PROJECT DESCRIPTION

Lake Ridge Elementary School is located on Lake Ridge Square in northern Johnson City, south of Boone Lake. An addition is proposed to be constructed on the southwestern side of the school that will consist of a new building, increased parking spaces, a new location for the ball court, new covered walkways to the main entrances, and a new playground structure. A location map of the project site is included in Appendix A.

Existing Site Conditions

The existing site of the school grounds consists of one continuous school building structure, three separate surface parking lots, a ball court, a playground area, and a grass field with a walking track. All storm water flows to the north into the existing storm water system and eventually discharges into Boone Lake. No detention basins or similar structures currently exist on site.

Proposed Site Improvements

The proposed improvements on the western side of the school include an addition to the main building structure, a revised parking lot layout to increase the amount of parking spaces, a new location for the ball court, and associated sidewalks and ancillary structures. Both of the main entrance ways on the northern side of the school will receive wider concrete walkways that will be covered by a canopy structure. The playground area to the east of the school will receive a new play structure system with a rubberized surface that will be impervious. Additional storm water sewers will accompany the improvements on the north and west sides of the building that will discharge to a new detention basin facility. Finally, a water quality device is being installed to meet water quality regulations. Refer to the civil construction drawings titled *"Lake Ridge Elementary School"* for a full representation of the proposed construction.



STORM WATER MANAGEMENT SYSTEM

Storm water management for the proposed addition consists of roof drains that will feed directly into the storm sewer system and the storm sewer system itself which will involve demolition of some existing lines and addition of new smooth interior corrugated plastic (SICP) lines. The system will flow to a proposed detention basin facility to manage storm water quantity. From the detention basin, storm water will flow back into the existing storm drainage system and on to Boone Lake. A brief discussion of the design of each of the proposed storm water measures follows.

Storm Water Conveyance

The proposed addition will use roof drains that will collect water from the proposed building addition and direct it to the existing storm sewer system via additional storm sewers that will consist of approximately 1,390 linear feet of new piping. For the improvements to the north and west of the school, new drain basins and catch basins will be installed to collect surface water and discharge it into the storm sewer system. All additional storm sewers will discharge to structure D2.2 in the northwestern corner of the school grounds which will discharge into the detention basin. From the detention basin, storm water will discharge into an existing storm sewer structure located on Lake Ridge Square just north of the detention basin, shown on the drainage maps in Appendix B. All storm water will eventually discharge to Boone Lake, which is approximately 2,500 feet north of the school.

See Appendix D for a summary of proposed storm sewer piping and associated calculations.

Storm Water Detention

A detention basin facility has been designed to control the increased runoff from the site improvements. The basin was designed to limit post-development peak discharge to pre-development rates for the 1-year 24-hour storm through the 100-year 24-hour storm. To accomplish this, the detention basin has a storage volume of approximately 12,000 cubic feet and an outlet structure with two orifices to control storm events as



water rises in the basin. The outlet structure is designed to be fitted with a typical storm grate over the principal spillway to filter large debris. Finally, a concrete-lined emergency spillway has been designed to safely discharge larger storm events.

Pre-development and post-development storm water calculations for the site have been completed using the software program PondPack V8i by Bentley.

Pre-Development

The pre-development scenario includes the existing drainage area for the school grounds calculated at the confluence point in the existing storm sewer system that occurs just north of the school property (see the drainage maps in Appendix B). For calculation purposes the drainage area has been split into two sub-areas, one that drains to where the detention basin is proposed and one that drains "offsite" which will bypass the proposed detention basin and drain to the same point of confluence in the existing storm drainage system.

Post-Development

The post-development scenario includes the building addition, parking lot redesign, ball court, new walkways, and the new play structure. Storm water runoff is increased in the post-development scenario due to the addition of impervious surfaces over existing pervious surfaces. Therefore, the detention basin is proposed to store the excess runoff and release it at peak flow rates that are less than the pre-development peak flow rates. Existing flow patterns will not be changed except for the covered walkway areas to the north of the school that will be collected and conveyed to the proposed detention basin. See the table below for a comparison of pre- and post-development peak flow rates.



	Peak Flow (cfs)				
Storm Event	Pre-Development	Post-Development			
1-year	13.67	9.54			
2-year	16.55	12.34			
5-year	29.48	26.11			
10-year	42.41	42.18			
25-year	50.37	49.23			
100-year	66.99	65.26			

Table 1 – Peak Flow Comparison

Based on the results of this hydrologic analysis, the addition of the proposed improvements along with the proposed storm water infrastructure is not expected to increase storm water runoff from the site. Therefore, downstream properties are not expected to be impacted by the proposed construction. Calculations for the hydrologic analysis are included in Appendix C.

STORM WATER QUALITY

Water quality for the site has been managed according to the *Northeast Tennessee Water Quality BMP Manual, February 2008.* One Aqua-Swirl BMP treatment device manufactured by Aquashield will be utilized as the permanent storm water management measure to achieve the required 80% total suspended solids (TSS) pollutant removal for the site. According to the list of water quality devices accepted by the City of Johnson City, Aqua-Swirl Concentrator devices have been tested to remove 87.3% of TSS. Therefore, the proposed Aqua-Swirl Concentrator Model AS-4 unit will be adequate to remove the required amount of TSS from the new impervious surfaces. See the civil construction drawings entitled *"Lake Ridge Elementary School"*, provided under separate cover, for a detail of the proposed Aqua-Swirl unit

Channel protection for the receiving stream has been considered by controlling the peak discharge from the 1-year storm. A 4-inch orifice at the base of the dry detention basin outlet structure is calculated to control peak discharge from the 1-year storm from the



entire site to 9.54 cubic feet per second (cfs) which is a 30% decrease from the predevelopment peak discharge at the same point of outfall.

See Appendix E for water quality calculations.

Maintenance of Water Quality Management Facilities

Proper and regular maintenance of the storm water control measures is necessary in order for the facilities to function properly. A Maintenance Checklist has been prepared specific to the storm water control measures proposed for the Lake Ridge Elementary School site, and is included in Appendix F. The storm water control measures and relevant details are depicted on the civil construction drawings entitled "*Lake Ridge Elementary School*".

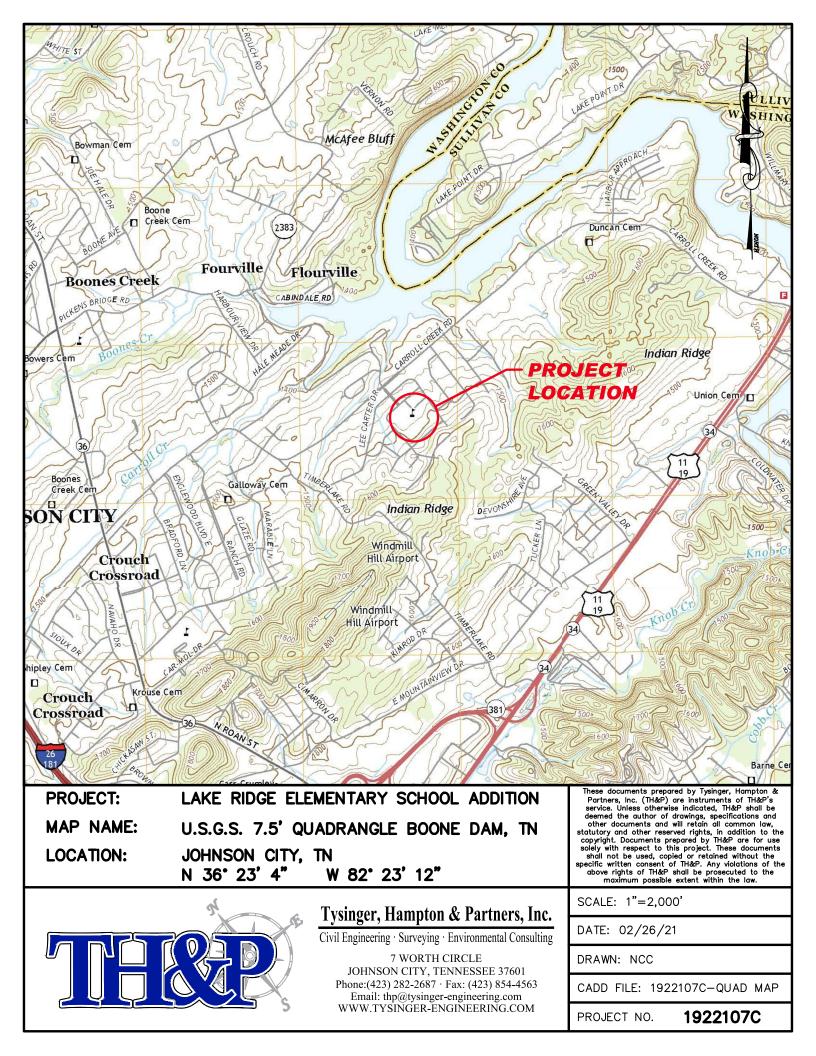


APPENDIX A

Location Map



APPENDIX

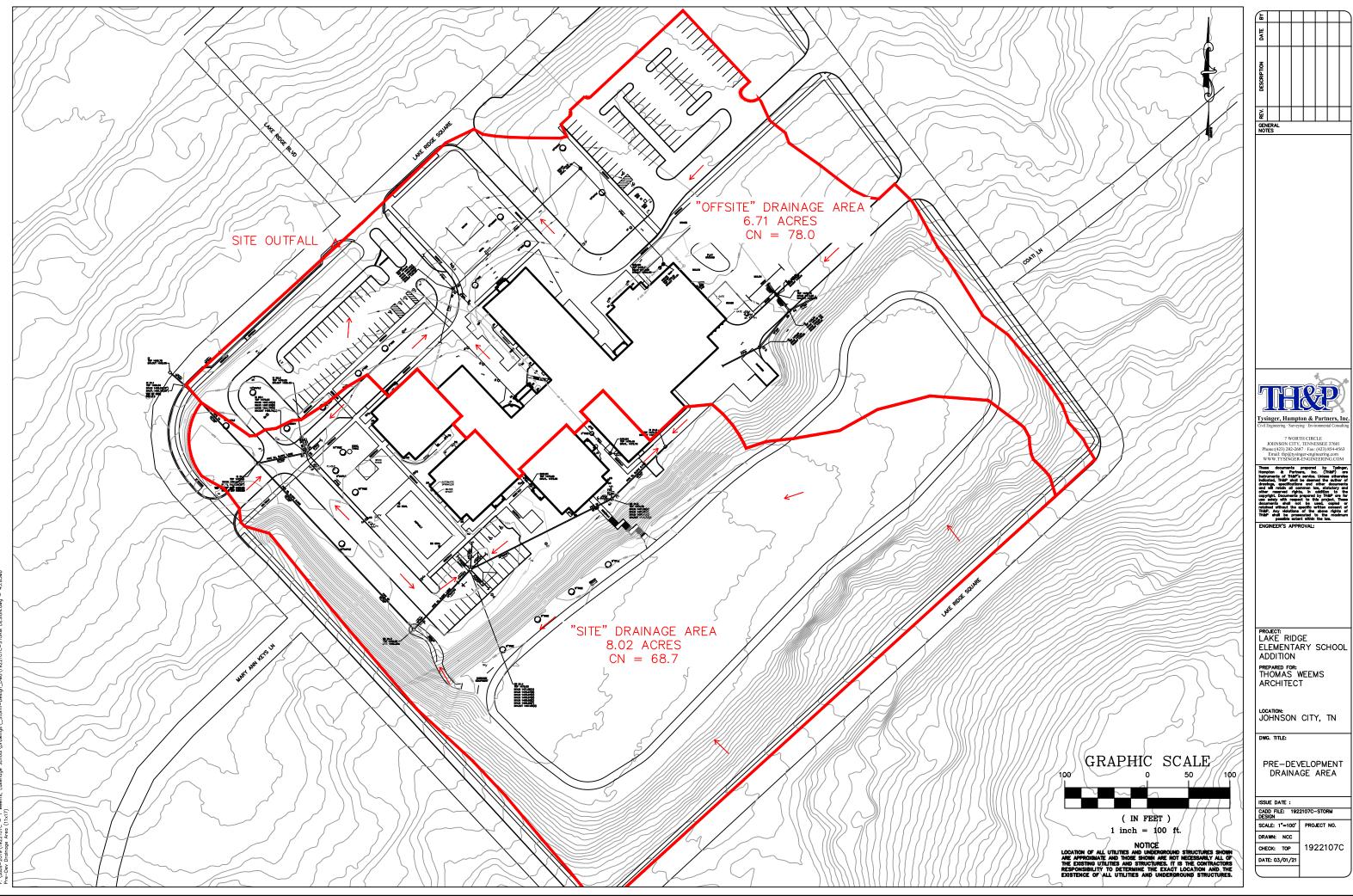


APPENDIX B

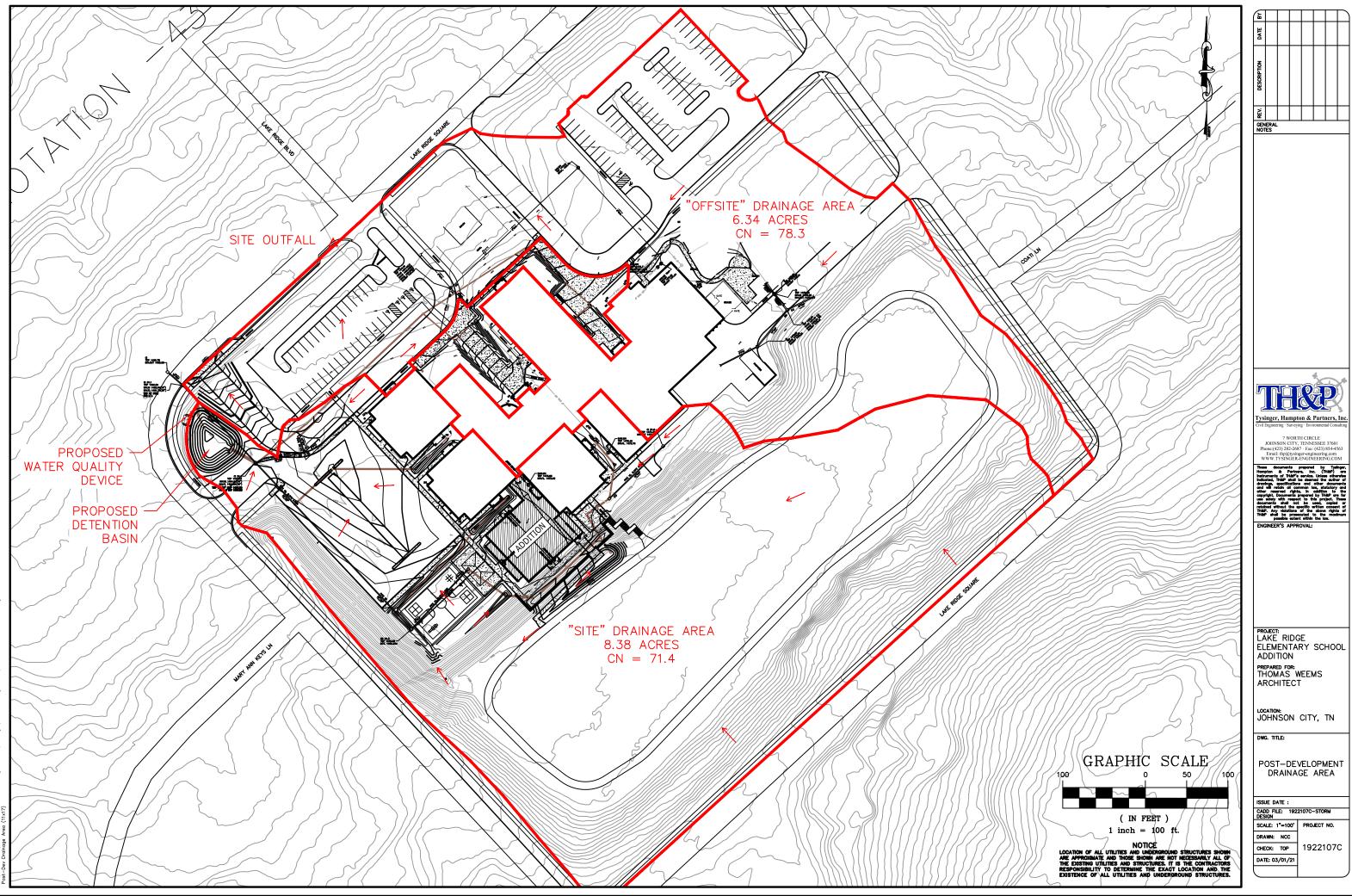
Drainage Maps



APPENDIX



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APPENDIX C

Hydrologic Analysis



APPENDIX

Curve Number Calculations

TH&P Project No. Date:

3/1/2021

Pervious Area

Impervious Area

262975

102251

6.0371

2.3474

61

98

1922107C

Pre-Development			Pre-Development			
Area (sq. ft.)	Area (ac)	Curve Number		Area (sq. ft.)	Area (ac)	Curve Numbe
349377	8.0206	68.679	Total Catchment Area	292126	6.7063	77.997
276870.7	6.3561	61	Pervious Area	157929	3.6256	61
72506.3	1.6645	98	Impervious Area	134197	3.0807	98
	Area (sq. ft.) 349377 276870.7	Area (sq. ft.) Area (ac) 349377 8.0206 276870.7 6.3561	Area (sq. ft.) Area (ac) Curve Number 349377 8.0206 68.679 276870.7 6.3561 61	Area (sq. ft.) Area (ac) Curve Number 349377 8.0206 68.679 276870.7 6.3561 61	Area (sq. ft.) Area (ac) Curve Number Area (sq. ft.) Area (sq. ft.) 349377 8.0206 68.679 Total Catchment Area 292126 276870.7 6.3561 61 Pervious Area 157929	Area (sq. ft.) Area (ac) Curve Number 349377 8.0206 68.679 276870.7 6.3561 61

Pervious Area

Impervious Area

147368

128909

3.3831

2.9593

61

98

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Johnson City		
	Time-Depth Curve, 1 years (Pre-Development 1)	3
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	Time-Depth Curve, 10 years (Pre-Development 10)	9
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	Time of Concentration Calculations, 1 years (Post-Development 1)	15
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	Pond Routed Hydrograph (total out), 1 years (Post- Development 1)	31
	Pond Routed Hydrograph (total out), 2 years (Post- Development 2)	38
	Pond Routed Hydrograph (total out), 5 years (Post- Development 5)	45
	Pond Routed Hydrograph (total out), 10 years (Post- Development 10)	52
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	Pond Routed Hydrograph (total out), 100 years (Post- Development 100)	68

Subsection: Master Network Summary

Catchments Summary

Label Scenario		Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Site	Pre-Development 1	1	0.353	12.030	5.01
Site	Post-Development 1	1	0.444	12.030	6.66
Site	Pre-Development 2	2	0.431	12.030	6.36
Site	Post-Development 2	2	0.535	12.030	8.19
Site	Pre-Development 5	5	0.799	12.010	12.57
Site	Post-Development 5	5	0.952	12.010	15.23
Site	Pre-Development 10	10	1.182	12.010	19.02
Site	Post-Development 10	10	1.378	12.010	22.33
Site	Pre-Development 25	25	1.423	12.010	23.04
Site	Post-Development 25	25	1.644	12.010	26.71
Site	Pre-Development 100	100	1.940	12.010	31.53
Site	Post-Development 100	100	2.209	12.000	35.93
Offsite	Pre-Development 1	1	0.533	11.990	8.84
Offsite	Post-Development 1	1	0.512	11.990	8.50
Offsite	Pre-Development 2	2	0.623	11.990	10.39
Offsite	Post-Development 2	2	0.597	11.990	9.98
Offsite	Pre-Development 5	5	1.019	11.980	17.21
Offsite	Post-Development 5	5	0.974	11.980	16.45
Offsite	Pre-Development 10	10	1.407	11.970	23.79
Offsite	Post-Development 10	10	1.343	11.970	22.71
Offsite	Pre-Development 25	25	1.644	11.970	27.77
Offsite	Post-Development 25	25	1.568	11.970	26.49
Offsite	Pre-Development 100	100	2.139	11.960	35.98
Offsite	Post-Development 100	100	2.037	11.970	34.25

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Site Outfall	Pre-Development 1	1	0.886	12.010	13.67
Site Outfall	Post-Development 1	1	0.956	12.000	9.54
Site Outfall	Pre-Development 2	2	1.054	12.010	16.55
Site Outfall	Post-Development 2	2	1.132	12.030	12.34
Site Outfall	Pre-Development 5	5	1.818	12.000	29.48
Site Outfall	Post-Development 5	5	1.926	12.050	26.11
Site Outfall	Pre-Development 10	10	2.588	11.990	42.41
Site Outfall	Post-Development 10	10	2.721	12.020	42.18
Site Outfall	Pre-Development 25	25	3.067	11.990	50.37
Site Outfall	Post-Development 25	25	3.212	12.010	49.23
Site Outfall	Pre-Development 100	100	4.078	11.980	66.99

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Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft ³ /s)
Site Outfall	Post-Development 100	100	4.246	12.020	65.26

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft ³ /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Detention Basin (IN)	Post- Development 1	1	0.444	12.030	6.66	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 1	1	0.444	12.170	3.14	1,467.94	0.093
Detention Basin (IN)	Post- Development	2	0.535	12.030	8.19	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 2	2	0.535	12.170	3.87	1,468.41	0.115
Detention Basin (IN)	Post- Development 5	5	0.952	12.010	15.23	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 5	5	0.952	12.080	12.78	1,469.57	0.180
Detention Basin (IN)	Post- Development 10	10	1.378	12.010	22.33	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 10	10	1.378	12.050	21.13	1,469.93	0.204
Detention Basin (IN)	Post- Development 25	25	1.644	12.010	26.71	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 25	25	1.644	12.060	24.42	1,470.20	0.223
Detention Basin (IN)	Post- Development 100	100	2.209	12.000	35.93	(N/A)	(N/A)
Detention Basin (OUT)	Post- Development 100	100	2.209	12.040	33.97	1,470.79	0.269

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Time-Depth Curve:TypeII24hr: 1
(2.8 in)LabelTypeII24hr: 1
(2.8 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event1 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

T:			Douth	Donth	
Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth
. ,	. ,		. ,	. ,	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.0	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.1	0.1	0.1
4.500	0.2	0.2	0.2	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.2	0.2
6.500	0.2	0.3	0.3	0.3	0.3
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.3
8.000	0.3	0.3	0.3	0.3	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.4	0.4	0.4
9.500	0.4	0.5	0.5	0.5	0.5
10.000	0.5	0.5	0.5	0.5	0.5
10.500	0.6	0.6	0.6	0.6	0.6
11.000	0.6	0.7	0.7	0.7	0.7
11.500	0.8	0.8	1.0	1.2	1.6
12.000	1.8	1.9	1.9	2.0	2.0
12.500	2.0	2.0	2.1	2.1	2.1
13.000	2.1	2.1	2.2	2.2	2.2
13.500	2.2	2.2	2.2	2.2	2.2
14.000	2.3	2.3	2.3	2.3	2.3
14.500	2.3	2.3	2.3	2.3	2.3
15.000	2.3	2.4	2.4	2.4	2.4
15.500	2.4	2.4	2.4	2.4	2.4
16.000	2.4	2.4	2.4	2.4	2.4

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	2.5	2.5	2.5	2.5	2.5
17.000	2.5	2.5	2.5	2.5	2.5
17.500	2.5	2.5	2.5	2.5	2.5
18.000	2.5	2.5	2.5	2.5	2.6
18.500	2.6	2.6	2.6	2.6	2.6
19.000	2.6	2.6	2.6	2.6	2.6
19.500	2.6	2.6	2.6	2.6	2.6
20.000	2.6	2.6	2.6	2.6	2.6
20.500	2.6	2.6	2.6	2.6	2.6
21.000	2.7	2.7	2.7	2.7	2.7
21.500	2.7	2.7	2.7	2.7	2.7
22.000	2.7	2.7	2.7	2.7	2.7
22.500	2.7	2.7	2.7	2.7	2.7
23.000	2.7	2.7	2.7	2.7	2.7
23.500	2.7	2.7	2.7	2.7	2.7
24.000	2.8	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

Time-Depth Curve:TypeII24hr: 1 (3.0 in)LabelTypeII24hr: 1 (3.0 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event2 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.2	0.2	0.2
4.500	0.2	0.2	0.2	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.3	0.3
6.500	0.3	0.3	0.3	0.3	0.3
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.4
8.000	0.4	0.4	0.4	0.4	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.5	0.5	0.5
9.500	0.5	0.5	0.5	0.5	0.5
10.000	0.5	0.6	0.6	0.6	0.6
10.500	0.6	0.6	0.6	0.7	0.7
11.000	0.7	0.7	0.7	0.8	0.8
11.500	0.8	0.9	1.1	1.3	1.7
12.000	2.0	2.0	2.1	2.1	2.2
12.500	2.2	2.2	2.2	2.3	2.3
13.000	2.3	2.3	2.3	2.4	2.4
13.500	2.4	2.4	2.4	2.4	2.4
14.000	2.4	2.5	2.5	2.5	2.5
14.500	2.5	2.5	2.5	2.5	2.5
15.000	2.5	2.6	2.6	2.6	2.6
15.500	2.6	2.6	2.6	2.6	2.6
16.000	2.6	2.6	2.6	2.6	2.6

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	2.7	2.7	2.7	2.7	2.7
17.000	2.7	2.7	2.7	2.7	2.7
17.500	2.7	2.7	2.7	2.7	2.7
18.000	2.7	2.7	2.8	2.8	2.8
18.500	2.8	2.8	2.8	2.8	2.8
19.000	2.8	2.8	2.8	2.8	2.8
19.500	2.8	2.8	2.8	2.8	2.8
20.000	2.8	2.8	2.8	2.8	2.9
20.500	2.9	2.9	2.9	2.9	2.9
21.000	2.9	2.9	2.9	2.9	2.9
21.500	2.9	2.9	2.9	2.9	2.9
22.000	2.9	2.9	2.9	2.9	2.9
22.500	2.9	2.9	2.9	2.9	2.9
23.000	2.9	2.9	3.0	3.0	3.0
23.500	3.0	3.0	3.0	3.0	3.0
24.000	3.0	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

Time-Depth Curve:TypeII24hr: 1 (3.9 in)LabelTypeII24hr: 1 (3.9 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event5 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.2	0.2	0.2	0.2	0.2
4.500	0.2	0.2	0.2	0.2	0.2
5.000	0.2	0.3	0.3	0.3	0.3
5.500	0.3	0.3	0.3	0.3	0.3
6.000	0.3	0.3	0.3	0.3	0.3
6.500	0.3	0.4	0.4	0.4	0.4
7.000	0.4	0.4	0.4	0.4	0.4
7.500	0.4	0.4	0.4	0.5	0.5
8.000	0.5	0.5	0.5	0.5	0.5
8.500	0.5	0.5	0.5	0.6	0.6
9.000	0.6	0.6	0.6	0.6	0.6
9.500	0.6	0.7	0.7	0.7	0.7
10.000	0.7	0.7	0.7	0.8	0.8
10.500	0.8	0.8	0.8	0.9	0.9
11.000	0.9	1.0	1.0	1.0	1.1
11.500	1.1	1.2	1.4	1.7	2.2
12.000	2.6	2.7	2.7	2.8	2.8
12.500	2.9	2.9	2.9	3.0	3.0
13.000	3.0	3.0	3.1	3.1	3.1
13.500	3.1	3.2	3.2	3.2	3.2
14.000	3.2	3.2	3.2	3.3	3.3
14.500	3.3	3.3	3.3	3.3	3.3
15.000	3.3	3.4	3.4	3.4	3.4
15.500	3.4	3.4	3.4	3.4	3.4
16.000	3.4	3.5	3.5	3.5	3.5

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	3.5	3.5	3.5	3.5	3.5
17.000	3.5	3.5	3.6	3.6	3.6
17.500	3.6	3.6	3.6	3.6	3.6
18.000	3.6	3.6	3.6	3.6	3.6
18.500	3.6	3.7	3.7	3.7	3.7
19.000	3.7	3.7	3.7	3.7	3.7
19.500	3.7	3.7	3.7	3.7	3.7
20.000	3.7	3.7	3.7	3.7	3.8
20.500	3.8	3.8	3.8	3.8	3.8
21.000	3.8	3.8	3.8	3.8	3.8
21.500	3.8	3.8	3.8	3.8	3.8
22.000	3.8	3.8	3.8	3.8	3.8
22.500	3.9	3.9	3.9	3.9	3.9
23.000	3.9	3.9	3.9	3.9	3.9
23.500	3.9	3.9	3.9	3.9	3.9
24.000	3.9	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

Time-Depth Curve:TypeII24hr: 1 (4.8 in)LabelTypeII24hr: 1 (4.8 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event10 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time		Depth	Depth	Depth	Depth	Depth
(hours)		(in)	(in)	(in)	(in)	(in)
0.0	00	0.0	0.0	0.0	0.0	0.0
0.5		0.0	0.0	0.0	0.0	0.0
1.0	00	0.1	0.1	0.1	0.1	0.1
1.5	00	0.1	0.1	0.1	0.1	0.1
2.0	00	0.1	0.1	0.1	0.1	0.1
2.5	00	0.1	0.1	0.1	0.2	0.2
3.0	00	0.2	0.2	0.2	0.2	0.2
3.5	00	0.2	0.2	0.2	0.2	0.2
4.0		0.2	0.2	0.2	0.2	0.3
4.5		0.3	0.3	0.3	0.3	0.3
5.0		0.3	0.3	0.3	0.3	0.3
5.5		0.3	0.3	0.4	0.4	0.4
6.0		0.4	0.4	0.4	0.4	0.4
6.5		0.4	0.4	0.4	0.5	0.5
7.0		0.5	0.5	0.5	0.5	0.5
7.5		0.5	0.5	0.5	0.6	0.6
8.0		0.6	0.6	0.6	0.6	0.6
8.5		0.6	0.6	0.7	0.7	0.7
9.0		0.7	0.7	0.7	0.7	0.8
9.5		0.8	0.8	0.8	0.8	0.8
10.0		0.9	0.9	0.9	0.9	0.9
10.5		1.0	1.0	1.0	1.1	1.1
11.0		1.1	1.2	1.2	1.2	1.3
11.5		1.3	1.5	1.7	2.1	2.7
12.0		3.2	3.3	3.3	3.4	3.5
12.5		3.5 3.7	3.5	3.6	3.6	3.7
13.0		3.7 3.8	3.7 3.8	3.7	3.8 3.9	3.8
13.5 14.0		3.8 3.9	3.8 3.9	3.9 3.9	3.9	3.9 4.0
14.0		3.9 4.0	3.9 4.0	3.9 4.0	4.0 4.0	4.0 4.1
14.5		4.0 4.1	4.0 4.1	4.0 4.1	4.0	4.1 4.1
15.0		4.1 4.1	4.1 4.2	4.1	4.1	4.1 4.2
16.0		4.1				4.2
16.0	00	4.2	4.2	4.2	4.2	4.2

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	4.3	4.3	4.3	4.3	4.3
17.000	4.3	4.3	4.3	4.3	4.3
17.500	4.3	4.4	4.4	4.4	4.4
18.000	4.4	4.4	4.4	4.4	4.4
18.500	4.4	4.4	4.5	4.5	4.5
19.000	4.5	4.5	4.5	4.5	4.5
19.500	4.5	4.5	4.5	4.5	4.5
20.000	4.5	4.5	4.6	4.6	4.6
20.500	4.6	4.6	4.6	4.6	4.6
21.000	4.6	4.6	4.6	4.6	4.6
21.500	4.6	4.6	4.6	4.6	4.7
22.000	4.7	4.7	4.7	4.7	4.7
22.500	4.7	4.7	4.7	4.7	4.7
23.000	4.7	4.7	4.7	4.7	4.7
23.500	4.7	4.7	4.8	4.8	4.8
24.000	4.8	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

Time-Depth Curve:TypeII24hr: 1
(5.3 in)LabelTypeII24hr: 1
(5.3 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event25 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.3	0.3	0.3	0.3
5.000	0.3	0.3	0.3	0.4	0.4
5.500	0.4	0.4	0.4	0.4	0.4
6.000	0.4	0.4	0.4	0.5	0.5
6.500	0.5	0.5	0.5	0.5	0.5
7.000	0.5	0.5	0.5	0.6	0.6
7.500	0.6	0.6	0.6	0.6	0.6
8.000	0.6	0.6	0.7	0.7	0.7
8.500	0.7	0.7	0.7	0.7	0.8
9.000	0.8	0.8	0.8	0.8	0.8
9.500	0.9	0.9	0.9	0.9	0.9
10.000	1.0	1.0	1.0	1.0	1.0
10.500	1.1	1.1	1.1	1.2	1.2
11.000	1.2	1.3	1.3	1.4	1.4
11.500	1.5 3.5	1.6 3.6	1.9 3.7	2.3 3.8	3.0
12.000 12.500	3.5 3.9	3.6 3.9	3.7 4.0	3.8 4.0	3.8
13.000	3.9 4.1	3.9 4.1	4.0 4.1	4.0	4.0 4.2
13.500	4.1	4.1	4.1	4.2	4.2
13.500	4.2	4.2	4.3	4.3	4.3
14.500	4.4	4.4	4.4	4.5	4.5
15.000	4.5	4.5	4.5	4.5	4.6
15.500	4.6	4.6	4.6	4.6	4.6
16.000	4.6	4.6	4.7	4.7	4.7

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	4.7	4.7	4.7	4.7	4.7
17.000	4.8	4.8	4.8	4.8	4.8
17.500	4.8	4.8	4.8	4.8	4.8
18.000	4.9	4.9	4.9	4.9	4.9
18.500	4.9	4.9	4.9	4.9	4.9
19.000	4.9	5.0	5.0	5.0	5.0
19.500	5.0	5.0	5.0	5.0	5.0
20.000	5.0	5.0	5.0	5.0	5.0
20.500	5.1	5.1	5.1	5.1	5.1
21.000	5.1	5.1	5.1	5.1	5.1
21.500	5.1	5.1	5.1	5.1	5.1
22.000	5.1	5.2	5.2	5.2	5.2
22.500	5.2	5.2	5.2	5.2	5.2
23.000	5.2	5.2	5.2	5.2	5.2
23.500	5.2	5.2	5.3	5.3	5.3
24.000	5.3	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

Time-Depth Curve:TypeII24hr: 1 (6.3 in)LabelTypeII24hr: 1 (6.3 in)Start Time0.000 hoursIncrement0.100 hoursEnd Time24.000 hoursReturn Event100 years

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.2	0.2	0.2
2.500	0.2	0.2	0.2	0.2	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.3	0.3	0.3	0.3	0.3
4.000	0.3	0.3	0.3	0.3	0.3
4.500	0.3	0.4	0.4	0.4	0.4
5.000	0.4	0.4	0.4	0.4	0.4
5.500	0.4	0.5	0.5	0.5	0.5
6.000	0.5	0.5	0.5	0.5	0.5
6.500	0.6	0.6	0.6	0.6	0.6
7.000	0.6	0.6	0.6	0.7	0.7
7.500	0.7	0.7	0.7	0.7	0.7
8.000	0.8	0.8	0.8	0.8	0.8
8.500	0.8	0.8	0.9	0.9	0.9
9.000	0.9	0.9	1.0	1.0	1.0
9.500	1.0	1.0	1.1	1.1	1.1
10.000	1.1	1.2	1.2	1.2	1.2
10.500	1.3	1.3	1.4	1.4	1.4
11.000	1.5	1.5	1.6	1.6	1.7
11.500	1.8	1.9	2.2	2.7	3.6
12.000	4.2	4.3	4.4	4.5	4.6
12.500	4.6	4.7	4.7	4.8	4.8
13.000	4.8	4.9	4.9	5.0	5.0
13.500	5.0	5.0	5.1	5.1	5.1
14.000	5.1	5.2	5.2	5.2	5.2
14.500	5.3	5.3	5.3	5.3	5.3
15.000	5.4	5.4	5.4	5.4	5.4
15.500	5.4	5.5	5.5	5.5	5.5
16.000	5.5	5.5	5.6	5.6	5.6

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Subsection: Time-Depth Curve Label: Johnson City Scenario: Pre-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
16.500	5.6	5.6	5.6	5.6	5.7
17.000	5.7	5.7	5.7	5.7	5.7
17.500	5.7	5.7	5.7	5.8	5.8
18.000	5.8	5.8	5.8	5.8	5.8
18.500	5.8	5.8	5.9	5.9	5.9
19.000	5.9	5.9	5.9	5.9	5.9
19.500	5.9	5.9	6.0	6.0	6.0
20.000	6.0	6.0	6.0	6.0	6.0
20.500	6.0	6.0	6.0	6.0	6.1
21.000	6.1	6.1	6.1	6.1	6.1
21.500	6.1	6.1	6.1	6.1	6.1
22.000	6.1	6.1	6.2	6.2	6.2
22.500	6.2	6.2	6.2	6.2	6.2
23.000	6.2	6.2	6.2	6.2	6.2
23.500	6.2	6.3	6.3	6.3	6.3
24.000	6.3	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Time of Concentration Calculations Label: Offsite

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Post-Development 1

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	119.50 ft
Manning's n	0.130
Slope	0.258 ft/ft
2 Year 24 Hour Depth	3.0 in
Average Velocity	0.53 ft/s
Segment Time of Concentration	0.063 hours
Segment #2: TR-55 Shallow Con	centrated Flow
Hydraulic Length	224.50 ft
Is Paved?	False
Slope	0.054 ft/ft
Average Velocity	3.76 ft/s
Segment Time of Concentration	0.017 hours
Segment #3: Length and Velocity	/
Hydraulic Length	945.70 ft
Velocity	4.00 ft/s
Segment Time of Concentration	0.066 hours
Time of Concentration (Composite	e)
Time of Concentration (Composite)	0.145 hours

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Subsection: Time of Concentration Calculations Label: Offsite Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

==== User Defined Length & Velocity

Tc =	(Lf / V) / 3600
Where:	Tc= Time of concentration, hours
	Lf= Flow length, feet
	V= Velocity, ft/sec

==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Aq= Flow area, square feet Wp= Wetted perimeter, feet V= Velocity, ft/sec Sf= Slope, ft/ft n= Manning's n Tc= Time of concentration, hours Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc =	Unpaved surface: V = 16.1345 * (Sf**0.5)
	Paved Surface: V = 20.3282 * (Sf**0.5)
Where:	(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

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Subsection: Time of Concentration Calculations Label: Offsite

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Pre-Development 1

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	119.50 ft
Manning's n	0.130
Slope	0.258 ft/ft
2 Year 24 Hour Depth	3.0 in
Average Velocity	0.53 ft/s
Segment Time of Concentration	0.063 hours
Segment #2: TR-55 Shallow Cor	ncentrated Flow
Hydraulic Length	224.50 ft
Is Paved?	False
Slope	0.054 ft/ft
Average Velocity	3.76 ft/s
Segment Time of Concentration	0.017 hours
Segment #3: Length and Velocit	у
Hydraulic Length	945.70 ft
Velocity	4.00 ft/s
Segment Time of Concentration	0.066 hours
Time of Concentration (Composit	ie)
Time of Concentration (Composite)	0.145 hours

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Subsection: Time of Concentration Calculations Label: Offsite Scenario: Pre-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

==== User Defined Length & Velocity

Tc =	(Lf / V) / 3600
Where:	Tc= Time of concentration, hours
	Lf= Flow length, feet
	V= Velocity, ft/sec

==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Aq= Flow area, square feet Wp= Wetted perimeter, feet V= Velocity, ft/sec Sf= Slope, ft/ft n= Manning's n Tc= Time of concentration, hours Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Tc =	Unpaved surface: V = 16.1345 * (Sf**0.5)
	Paved Surface: V = 20.3282 * (Sf**0.5)
Where:	(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

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Subsection: Time of Concentration Calculations Label: Site

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Post-Development 1

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	114.50 ft
Manning's n	0.130
Slope	0.269 ft/ft
2 Year 24 Hour Depth	3.0 in
Average Velocity	0.53 ft/s
Segment Time of	0.059 hours
Concentration	
Segment #2: TR-55 Shallow Concer	ntrated Flow
Hydraulic Length	751.50 ft
Is Paved?	False
Slope	0.021 ft/ft
Average Velocity	2.36 ft/s
Segment Time of	0.088 hours
Concentration	
Segment #3: TR-55 Channel Flow	
Flow Area	0.5 ft ²
Hydraulic Length	95.80 ft
Manning's n	0.032
Slope	0.184 ft/ft
Wetted Perimeter	4.20 ft
Average Velocity	4.83 ft/s
Segment Time of Concentration	0.006 hours
Segment #4: Length and Velocity	
Hydraulic Length	275.50 ft
Velocity	4.20 ft/s
Segment Time of	0.018 hours
Concentration	
Segment #5: Length and Velocity	
Hydraulic Length	35.10 ft
Hydraulic Length Velocity	35.10 ft 8.00 ft/s

Time of Concentration (Composite)

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Subsection: Time of Concentration Calculations Label: Site

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Post-Development 1

Time of Concentration (Composite)

Time of Concentration (Composite) 0.173 hours

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Subsection: Time of Concentration Calculations Label: Site Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

==== User Defined Length & Velocity

Tc =	(Lf / V) / 3600
Where:	Tc= Time of concentration, hours
	Lf= Flow length, feet
	V= Velocity, ft/sec

==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Aq= Flow area, square feet Wp= Wetted perimeter, feet V= Velocity, ft/sec Sf= Slope, ft/ft n= Manning's n Tc= Time of concentration, hours Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

Тс	=
----	---

Unpaved surface: V = 16.1345 * (Sf**0.5)

Paved Surface: V = 20.3282 * (Sf**0.5)

Where:	(Lf / V) / 3600 V= Velocity, ft/sec
where.	Sf= Slope, ft/ft
	Tc= Time of concentration, hours
	Lf= Flow length, feet

==== SCS TR-55 Sheet Flow

Tc =	(0.007 * ((n * Lf)**0.8)) / ((P**0.5) * (Sf**0.4))
Where:	Tc= Time of concentration, hours
	n= Manning's n
	Lf= Flow length, feet
	P= 2yr, 24hr Rain depth, inches
	Sf= Slope, %

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Subsection: Time of Concentration Calculations Label: Site

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Pre-Development 1

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	114.50 ft
Manning's n	0.130
Slope	0.269 ft/ft
2 Year 24 Hour Depth	3.0 in
Average Velocity	0.53 ft/s
Segment Time of	0.059 hours
Concentration	
Segment #2: TR-55 Shallow Concen	trated Flow
Hydraulic Length	751.50 ft
Is Paved?	False
Slope	0.021 ft/ft
Average Velocity	2.36 ft/s
Segment Time of	0.088 hours
Concentration	
Segment #3: TR-55 Channel Flow	
Flow Area	0.5 ft ²
Hydraulic Length	95.80 ft
Manning's n	0.032
Slope	0.184 ft/ft
Wetted Perimeter	4.20 ft
Average Velocity	4.83 ft/s
Segment Time of Concentration	0.006 hours
Segment #4: Length and Velocity	
Hydraulic Length	275.50 ft
Velocity	4.20 ft/s
Segment Time of	0.018 hours
Concentration	
Segment #5: Length and Velocity	
Segment #5: Length and Velocity Hydraulic Length	35.10 ft
<u> </u>	35.10 ft 8.00 ft/s
Hydraulic Length	

Time of Concentration (Composite)

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Subsection: Time of Concentration Calculations Label: Site

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Pre-Development 1

Time of Concentration (Composite)

Time of Concentration (Composite) 0.173 hours

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Subsection: Time of Concentration Calculations Label: Site Scenario: Pre-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

==== User Defined Length & Velocity

Tc =	(Lf / V) / 3600
Where:	Tc= Time of concentration, hours
	Lf= Flow length, feet
	V= Velocity, ft/sec

==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Ag= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

==== SCS TR-55 Shallow Concentration Flow

IC =

Unpaved surface: V = 16.1345 * (Sf**0.5)

Paved Surface: V = 20.3282 * (Sf**0.5)

Where:	(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft	
	Tc= Time of concentration, hours Lf= Flow length, feet	

==== SCS TR-55 Sheet Flow

Tc =	(0.007 * ((n * Lf)**0.8)) / ((P**0.5) * (Sf**0.4))
Where:	Tc= Time of concentration, hours
	n= Manning's n
	Lf= Flow length, feet
	P= 2yr, 24hr Rain depth, inches
	Sf= Slope, %

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Subsection: Elevation-Area Volume Curve

Label: Detention Basin

Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

Scenario: Post-Development 100

Elevation (ft)	Planimeter (ft ²)	Area (acres)	A1+A2+sqr(A1*A 2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
1,463.80	0.0	0.000	0.000	0.000	0.000
1,464.00	0.0	0.002	0.004	0.000	0.000
1,465.00	0.0	0.017	0.025	0.008	0.008
1,466.00	0.0	0.024	0.061	0.020	0.029
1,467.00	0.0	0.033	0.086	0.029	0.058
1,468.00	0.0	0.044	0.115	0.038	0.096
1,469.00	0.0	0.056	0.150	0.050	0.146
1,470.00	0.0	0.070	0.189	0.063	0.209
1,471.00	0.0	0.085	0.233	0.078	0.286

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Subsection: Volume Equations Label: Detention Basin Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

Pond Volume Equations

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = (1/3) * (EL2 - El1) * (Area1 + Area2 + sqr(Area1 * Area2))

where:	EL1, EL2	Lower and upper elevations of the increment
	Area1, Area2	Areas computed for EL1, EL2, respectively
	Volume	Incremental volume between EL1 and EL2

Subsection: Outlet Input Data Label: DB Outlet Structure Scenario: Post-Development 1

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Requested Pond Water Surface Elevations					
Minimum (Headwater) 1,463.80 ft					
Increment (Headwater)	0.10 ft				
Maximum (Headwater) 1,471.00 ft					

Outlet Connectivity

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Orifice-Circular	Orifice - 2	Forward	Culvert - 1	1,467.00	1,471.00
Inlet Box	Riser - 1	Forward	Culvert - 1	1,469.00	1,471.00
Orifice-Circular	Orifice - 1	Forward	Culvert - 1	1,463.80	1,471.00
Culvert-Circular	Culvert - 1	Forward	TW	1,463.50	1,471.00
Irregular Weir	Weir - 1	Forward	TW	1,470.50	1,471.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

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Subsection: Outlet Input Data Label: DB Outlet Structure Scenario: Post-Development 1

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Structure ID: Riser - 1 Structure Type: Inlet Box	
Number of Openings	1
Elevation	1,469.00 ft
Orifice Area	3.6 ft ²
Orifice Coefficient	0.600
Weir Length	6.00 ft
Weir Coefficient	3.00 (ft^0.5)/s
K Reverse	1.000
Manning's n	0.000
Kev, Charged Riser	0.000
Weir Submergence	False
Orifice H to crest	False

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Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	10.80 ft
Length (Computed Barrel)	10.80 ft
Slope (Computed)	0.020 ft/ft
Dutlet Control Data	
Manning's n	0.013
Ке	0.200
Kb	0.012
Kr	0.200
Convergence Tolerance	0.00 ft
nlet Control Data	
Equation Form	Form 1
К	0.0045
Μ	2.0000
C	0.0317
Y	0.6900
T1 ratio (HW/D)	0.000
T2 ratio (HW/D)	1.187
Slope Correction Factor	-0.500

Subsection: Outlet Input Data Label: DB Outlet Structure Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Use unsubmerged inlet control 0 equation below T1 elevation. Use submerged inlet control 0 equation above T2

elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	1,463.50 ft	T1 Flow	15.55 ft³/s
T2 Elevation	1,465.87 ft	T2 Flow	17.77 ft³/s

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Subsection: Outlet Input Data Label: DB Outlet Structure Scenario: Post-Development 1

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

opment 1						
Structure ID: Weir - 1 Structure Type: Irregular Weir						
Station	Elevation					
(ft)	(ft)					
0.00	0.50 0.00					
11.50	0.00					
13.00	0.50					
Lowest Elevation	1,470.50 ft					
Weir Coefficient	3.00 (ft^0.5)/s					
Weir Coemcient	3.00 (11-0.3)/5					
Structure ID: Orifice - 1 Structure Type: Orifice-Circula	r					
Number of Openings	1					
Elevation	1,463.80 ft					
Orifice Diameter	4.0 in					
Orifice Coefficient	0.900					
Structure ID: Orifice - 2 Structure Type: Orifice-Circula	r					
Number of Openings	1					
Elevation	1,467.00 ft					
Orifice Diameter	8.0 in					
Orifice Coefficient	0.900					
Structure ID: TW Structure Type: TW Setup, DS	Channel					
Tailwater Type	Free Outfall					
Convergence Tolerances						
Maximum Iterations	30					
Tailwater Tolerance (Minimum)	0.01 ft					
Tailwater Tolerance (Maximum)	0.50 ft					
Headwater Tolerance (Minimum)	0.01 ft					
Headwater Tolerance (Maximum)	0.50 ft					
Flow Tolerance (Minimum)	0.001 ft ³ /s					
Flow Tolerance (Maximum)	10.000 ft ³ /s					

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT)

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

Scenario: Post-Development 1

Peak Discharge	3.14 ft ³ /s
Time to Peak	12.170 hours
Hydrograph Volume	0.444 ac-ft

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft³/s)
11.590	0.00	0.00	0.00	0.00	0.01
11.640	0.01	0.02	0.02	0.04	0.05
11.690	0.07	0.09	0.11	0.14	0.20
11.740	0.25	0.31	0.33	0.36	0.39
11.790	0.42	0.44	0.47	0.50	0.53
11.840	0.56	0.58	0.61	0.64	0.67
11.890	0.70	0.73	0.77	0.80	0.84
11.940	0.87	0.90	0.94	0.98	1.00
11.990	1.03	1.06	1.07	1.13	1.22
12.040	1.36	1.56	1.77	2.03	2.61
12.090	2.80	2.89	2.97	3.03	3.08
12.140	3.11	3.13	3.14	3.14	3.14
12.190	3.13	3.11	3.09	3.07	3.04
12.240	3.00	2.97	2.93	2.89	2.86
12.290	2.82	2.78	2.74	2.66	2.50
12.340	2.36	2.24	2.13	2.03	1.95
12.390	1.92	1.90	1.87	1.84	1.82
12.440	1.79	1.77	1.74	1.72	1.69
12.490	1.67	1.65	1.62	1.60	1.58
12.540	1.55	1.53	1.51	1.49	1.47
12.590	1.45	1.43	1.41	1.40	1.38
12.640	1.36	1.35	1.33	1.32	1.31
12.690	1.30	1.28	1.27	1.26	1.25
12.740	1.24	1.23	1.22	1.21	1.20
12.790	1.19	1.18	1.17	1.16	1.15
12.840	1.14	1.14	1.14	1.13	1.13
12.890	1.13	1.12	1.12	1.11	1.11
12.940	1.11	1.10	1.10	1.10	1.09
12.990	1.09	1.08	1.08	1.08	1.07
13.040	1.07	1.07	1.07	1.07	1.07
13.090	1.07	1.07	1.07	1.07	1.07
13.140	1.06	1.06	1.05	1.05	1.05
13.190	1.04	1.04	1.04	1.04	1.04
13.240	1.03	1.03	1.03	1.03	1.03
13.290	1.03	1.03	1.03	1.02	1.02
13.340	1.01	1.01	1.01	1.00	1.00
13.390	1.00	1.00	0.99	0.99	0.99

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
13.440	0.99	0.99	0.98	0.98	0.98
13.490	0.98	0.98	0.97	0.97	0.97
13.540	0.97	0.96	0.96	0.96	0.95
13.590	0.95	0.94	0.94	0.94	0.93
13.640	0.93	0.92	0.92	0.92	0.91
13.690	0.91	0.91	0.90	0.90	0.90
13.740	0.90	0.89	0.89	0.89	0.88
13.790	0.88	0.88	0.88	0.87	0.87
13.840	0.87	0.86	0.86	0.85	0.85
13.890	0.85	0.84	0.84	0.84	0.83
13.940	0.83	0.83	0.82	0.82	0.82
13.990	0.81	0.81	0.80	0.80	0.80
14.040	0.79	0.79	0.79	0.78	0.78
14.090	0.77	0.77	0.77	0.76	0.76
14.140	0.76	0.75	0.75	0.74	0.74
14.190	0.74	0.73	0.73	0.72	0.72
14.240	0.72	0.71	0.71	0.70	0.70
14.290	0.70	0.69	0.69	0.68	0.68
14.340	0.68	0.67	0.67	0.66	0.66
14.390	0.65	0.65	0.65	0.64	0.64
14.440	0.63	0.63	0.62	0.62	0.62
14.490	0.61	0.61	0.60	0.60	0.59
14.540	0.59	0.59	0.58	0.58	0.57
14.590	0.57	0.56	0.56	0.55	0.55
14.640	0.54	0.54	0.53	0.53	0.52
14.690	0.52	0.51	0.51	0.50	0.50
14.740	0.49	0.49	0.48	0.48	0.47
14.790	0.47	0.46	0.46	0.46	0.45
14.840	0.45	0.44	0.44	0.44	0.43
14.890	0.43	0.43	0.42	0.42	0.41
14.940	0.41	0.40	0.40	0.39	0.39
14.990	0.38	0.38	0.38	0.38	0.37
15.040	0.37	0.37	0.36	0.36	0.36
15.090	0.36	0.35	0.35	0.35	0.35
15.140	0.35	0.35	0.34	0.34	0.34
15.190	0.34	0.34	0.34	0.34	0.34
15.240	0.34	0.33	0.33	0.33	0.33
15.290	0.33	0.33	0.33	0.33	0.33
15.340	0.33	0.33	0.33	0.32	0.32
15.390	0.32	0.32	0.32	0.32	0.32
15.440	0.32	0.32	0.32	0.32	0.32
15.490	0.32	0.32	0.31	0.31	0.31

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft³/s)
15.540	0.31	0.31	0.31	0.31	0.31
15.590	0.31	0.30	0.30	0.30	0.30
15.640	0.30	0.30	0.30	0.30	0.30
15.690	0.30	0.30	0.30	0.30	0.29
15.740	0.29	0.29	0.29	0.29	0.29
15.790	0.29	0.29	0.29	0.29	0.29
15.840	0.29	0.29	0.28	0.28	0.28
15.890	0.28	0.28	0.28	0.28	0.28
15.940	0.28	0.28	0.28	0.28	0.28
15.990	0.28	0.28	0.27	0.27	0.27
16.040	0.27	0.27	0.27	0.27	0.27
16.090	0.27	0.27	0.27	0.27	0.27
16.140	0.27	0.27	0.26	0.26	0.26
16.190	0.26	0.26	0.26	0.26	0.26
16.240	0.26	0.26	0.26	0.26	0.26
16.290	0.26	0.26	0.26	0.26	0.26
16.340	0.26	0.26	0.26	0.26	0.26
16.390	0.26	0.26	0.26	0.26	0.26
16.440	0.26	0.26	0.26	0.26	0.26
16.490	0.26	0.26	0.26	0.25	0.25
16.540	0.25	0.25	0.25	0.25	0.25
16.590	0.25	0.25	0.25	0.25	0.25
16.640	0.25	0.25	0.25	0.25	0.25
16.690	0.25	0.25	0.25	0.25	0.25
16.740	0.25	0.25	0.25	0.25	0.25
16.790	0.25	0.25	0.25	0.25	0.25
16.840	0.25	0.25	0.25	0.25	0.25
16.890	0.25	0.25	0.25	0.25	0.25
16.940	0.24	0.24	0.24	0.24	0.24
16.990	0.24	0.24	0.24	0.24	0.24
17.040	0.24	0.24	0.24	0.24	0.24
17.090	0.24	0.24	0.24	0.24	0.24
17.140	0.24	0.24	0.24	0.24	0.24
17.190	0.24	0.24	0.24	0.24	0.24
17.240	0.24	0.24	0.24	0.24	0.24
17.290	0.24	0.24	0.24	0.24	0.24
17.340	0.24	0.23	0.23	0.23	0.23
17.390	0.23	0.23	0.23	0.23	0.23
17.440	0.23	0.23	0.23	0.23	0.23
17.490	0.23	0.23	0.23	0.23	0.23
17.540	0.23	0.23	0.23	0.23	0.23
17.590	0.23	0.23	0.23	0.23	0.23

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
17.640	0.23	0.23	0.23	0.23	0.23
17.690	0.23	0.23	0.23	0.23	0.23
17.740	0.23	0.22	0.22	0.22	0.22
17.790	0.22	0.22	0.22	0.22	0.22
17.840	0.22	0.22	0.22	0.22	0.22
17.890	0.22	0.22	0.22	0.22	0.22
17.940	0.22	0.22	0.22	0.22	0.22
17.990	0.22	0.22	0.22	0.22	0.22
18.040	0.22	0.22	0.22	0.22	0.22
18.090	0.22	0.22	0.22	0.22	0.21
18.140	0.21	0.21	0.21	0.21	0.21
18.190	0.21	0.21	0.21	0.21	0.21
18.240	0.21	0.21	0.21	0.21	0.21
18.290	0.21	0.21	0.21	0.21	0.21
18.340	0.21	0.21	0.21	0.21	0.21
18.390	0.21	0.21	0.21	0.21	0.21
18.440	0.21	0.21	0.21	0.21	0.21
18.490	0.21	0.21	0.20	0.20	0.20
18.540	0.20	0.20	0.20	0.20	0.20
18.590	0.20	0.20	0.20	0.20	0.20
18.640	0.20	0.20	0.20	0.20	0.20
18.690	0.20	0.20	0.20	0.20	0.20
18.740	0.20	0.20	0.20	0.20	0.20
18.790	0.20	0.20	0.20	0.20	0.20
18.840	0.20	0.20	0.20	0.20	0.19
18.890	0.19	0.19	0.19	0.19	0.19
18.940	0.19	0.19	0.19	0.19	0.19
18.990	0.19	0.19	0.19	0.19	0.19
19.040	0.19	0.19	0.19	0.19	0.19
19.090	0.19	0.19	0.19	0.19	0.19
19.140	0.19	0.19	0.19	0.19	0.19
19.190	0.19	0.19	0.19	0.19	0.19
19.240	0.19	0.18	0.18	0.18	0.18
19.290	0.18	0.18	0.18	0.18	0.18
19.340	0.18	0.18	0.18	0.18	0.18
19.390	0.18	0.18	0.18	0.18	0.18
19.440	0.18	0.18	0.18	0.18	0.18
19.490	0.18	0.18	0.18	0.18	0.18
19.540	0.18	0.18	0.18	0.18	0.18
19.590	0.18	0.17	0.17	0.17	0.17
19.640	0.17	0.17	0.17	0.17	0.17
19.690	0.17	0.17	0.17	0.17	0.17

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
19.740	0.17	0.17	0.17	0.17	0.17
19.790	0.17	0.17	0.17	0.17	0.17
19.840	0.17	0.17	0.17	0.17	0.17
19.890	0.17	0.17	0.17	0.17	0.17
19.940	0.17	0.17	0.16	0.16	0.16
19.990	0.16	0.16	0.16	0.16	0.16
20.040	0.16	0.16	0.16	0.16	0.16
20.090	0.16	0.16	0.16	0.16	0.16
20.140	0.16	0.16	0.16	0.16	0.16
20.190	0.16	0.16	0.16	0.16	0.16
20.240	0.16	0.16	0.16	0.16	0.16
20.290	0.16	0.16	0.16	0.16	0.16
20.340	0.16	0.16	0.16	0.16	0.16
20.390	0.16	0.16	0.16	0.16	0.16
20.440	0.16	0.16	0.16	0.16	0.16
20.490	0.16	0.16	0.16	0.16	0.16
20.540	0.16	0.16	0.16	0.16	0.16
20.590	0.16	0.16	0.16	0.16	0.16
20.640	0.16	0.16	0.16	0.16	0.16
20.690	0.16	0.16	0.16	0.16	0.16
20.740	0.16	0.16	0.16	0.16	0.16
20.790	0.16	0.16	0.16	0.16	0.16
20.840	0.16	0.16	0.16	0.16	0.16
20.890	0.16	0.16	0.16	0.16	0.16
20.940	0.16	0.16	0.16	0.16	0.16
20.990	0.16	0.16	0.16	0.16	0.15
21.040	0.15	0.15	0.15	0.15	0.15
21.090	0.15	0.15	0.15	0.15	0.15
21.140	0.15	0.15	0.15	0.15	0.15
21.190	0.15	0.15	0.15	0.15	0.15
21.240	0.15	0.15	0.15	0.15	0.15
21.290	0.15	0.15	0.15	0.15	0.15
21.340	0.15	0.15	0.15	0.15	0.15
21.390	0.15	0.15	0.15	0.15	0.15
21.440	0.15	0.15	0.15	0.15	0.15
21.490	0.15	0.15 0.15	0.15	0.15 0.15	0.15
21.540	0.15 0.15	0.15	0.15 0.15	0.15	0.15
21.590					0.15
21.640	0.15	0.15	0.15	0.15	0.15
21.690	0.15	0.15	0.15	0.15 0.15	0.15
21.740	0.15	0.15	0.15		0.15
21.790	0.15	0.15	0.15	0.15	0.15

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1

Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
21.840	0.15	0.15	0.15	0.15	0.15
21.890	0.15	0.15	0.15	0.15	0.15
21.940	0.15	0.15	0.15	0.15	0.15
21.990	0.15	0.15	0.15	0.15	0.15
22.040	0.15	0.15	0.15	0.15	0.15
22.090	0.15	0.15	0.15	0.15	0.15
22.140	0.15	0.15	0.15	0.15	0.15
22.190	0.15	0.15	0.15	0.15	0.15
22.240	0.15	0.15	0.15	0.15	0.15
22.290	0.15	0.15	0.15	0.15	0.15
22.340	0.15	0.15	0.15	0.15	0.15
22.390	0.15	0.15	0.15	0.15	0.15
22.440	0.15	0.15	0.15	0.15	0.15
22.490	0.15	0.15	0.15	0.15	0.15
22.540	0.15	0.15	0.15	0.15	0.15
22.590	0.15	0.15	0.15	0.15	0.15
22.640	0.15	0.15	0.15	0.15	0.15
22.690	0.15	0.15	0.15	0.15	0.15
22.740	0.15	0.15	0.15	0.15	0.15
22.790	0.15	0.15	0.15	0.15	0.15
22.840	0.15	0.15	0.15	0.15	0.15
22.890	0.15	0.15	0.15	0.15	0.15
22.940	0.15	0.15	0.15	0.15	0.15
22.990	0.15	0.15	0.15	0.15	0.15
23.040	0.15	0.15	0.15	0.15	0.15
23.090	0.15	0.15	0.15	0.14	0.14
23.140	0.14	0.14	0.14	0.14	0.14
23.190	0.14	0.14	0.14	0.14	0.14
23.240	0.14	0.14	0.14	0.14	0.14
23.290	0.14	0.14	0.14	0.14	0.14
23.340	0.14	0.14	0.14	0.14	0.14
23.390	0.14	0.14	0.14	0.14	0.14
23.440	0.14	0.14	0.14	0.14	0.14
23.490	0.14	0.14	0.14	0.14	0.14
23.540	0.14	0.14	0.14	0.14	0.14
23.590	0.14	0.14	0.14	0.14	0.14
23.640	0.14	0.14	0.14	0.14	0.14
23.690	0.14	0.14	0.14	0.14	0.14
23.740	0.14	0.14	0.14	0.14	0.14
23.790	0.14	0.14	0.14	0.14	0.14
23.840	0.14	0.14	0.14	0.14	0.14
23.890	0.14	0.14	0.14	0.14	0.14

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 1 Return Event: 1 years Storm Event: TypeII 24hr: 1 (2.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
23.940	0.14	0.14	0.14	0.14	0.14
23.990	0.14	0.14	0.14	0.14	0.14
24.040	0.14	0.14	0.13	0.13	0.13
24.090	0.12	0.12	0.11	0.10	0.10
24.140	0.09	0.08	0.07	0.07	0.06
24.190	0.05	0.05	0.04	0.04	0.03
24.240	0.03	0.02	0.02	0.02	0.02
24.290	0.02	0.01	0.01	0.01	0.01
24.340	0.01	0.01	0.01	0.01	0.00
24.390	0.00	0.00	0.00	0.00	0.00
24.440	0.00	0.00	0.00	0.00	0.00
24.490	0.00	(N/A)	(N/A)	(N/A)	(N/A)

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT)

Scenario: Post-Development 2

Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

Peak Discharge 3.87 ft³/s Time to Peak 12.170 hours 0.535 ac-ft Hydrograph Volume

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.460	0.00	0.00	0.00	0.00	0.00
11.460	0.00	0.00	0.00	0.00	0.00
11.510	0.01	0.01	0.01	0.01	0.01
11.500	0.02	0.02	0.03	0.10	0.03
11.660	0.08	0.18	0.08	0.10	0.12
11.000	0.14	0.18	0.22	0.20	0.31
11.760	0.33	0.35	0.50	0.40	0.43
11.760	0.45	0.47	0.50	0.52	0.55
11.810	0.58	0.80	0.83	0.80	0.88
11.000	0.71	0.90	0.77	0.80	1.01
11.910	1.04	1.07	1.11	1.22	1.01
12.010	1.68	2.01	2.76	2.95	3.13
12.010	3.28	3.41	3.51	3.60	3.68
12.000	3.74	3.78	3.82	3.84	3.86
12.110	3.87	3.78	3.86	3.85	3.83
12.100	3.81	3.79	3.76	3.74	3.71
12.210	3.67	3.64	3.60	3.56	3.52
12.200	3.48	3.44	3.39	3.35	3.30
12.310	3.25	3.20	3.15	3.11	3.05
12.500	3.00	2.95	2.90	2.85	2.80
12.460	2.75	2.67	2.48	2.32	2.00
12.510	2.05	1.95	1.92	1.88	1.85
12.560	1.82	1.79	1.76	1.73	1.71
12.610	1.68	1.65	1.63	1.60	1.58
12.660	1.56	1.53	1.51	1.49	1.47
12.710	1.45	1.44	1.42	1.40	1.38
12.760	1.37	1.35	1.34	1.33	1.32
12.810	1.31	1.30	1.29	1.28	1.27
12.860	1.25	1.24	1.24	1.23	1.22
12.910	1.21	1.20	1.19	1.19	1.18
12.960	1.17	1.16	1.15	1.15	1.14
13.010	1.14	1.14	1.13	1.13	1.13
13.060	1.12	1.12	1.12	1.11	1.11
13.110	1.10	1.10	1.10	1.09	1.09
13.160	1.09	1.09	1.08	1.08	1.08
13.210	1.07	1.07	1.07	1.07	1.07
13.260	1.07	1.07	1.07	1.07	1.07

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2

Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
13.310	1.07	1.06	1.06	1.06	1.05
13.360	1.05	1.05	1.05	1.04	1.04
13.410	1.04	1.04	1.03	1.03	1.03
13.460	1.03	1.03	1.03	1.03	1.03
13.510	1.03	1.02	1.02	1.02	1.01
13.560	1.01	1.01	1.00	1.00	1.00
13.610	1.00	0.99	0.99	0.99	0.99
13.660	0.99	0.99	0.98	0.98	0.98
13.710	0.98	0.98	0.97	0.97	0.97
13.760	0.97	0.96	0.96	0.96	0.95
13.810	0.95	0.95	0.94	0.94	0.94
13.860	0.93	0.93	0.92	0.92	0.92
13.910	0.91	0.91	0.91	0.91	0.90
13.960	0.90	0.90	0.90	0.89	0.89
14.010	0.89	0.88	0.88	0.88	0.88
14.060	0.87	0.87	0.87	0.86	0.86
14.110	0.86	0.85	0.85	0.85	0.84
14.160	0.84	0.84	0.83	0.83	0.83
14.210	0.82	0.82	0.82	0.81	0.81
14.260	0.81	0.80	0.80	0.80	0.79
14.310	0.79	0.79	0.78	0.78	0.78
14.360	0.77	0.77	0.77	0.76	0.76
14.410	0.76	0.75	0.75	0.75	0.74
14.460	0.74	0.73	0.73	0.73	0.72
14.510	0.72	0.72	0.71	0.71	0.71
14.560	0.70	0.70	0.70	0.69	0.69
14.610	0.69	0.68	0.68	0.68	0.67
14.660	0.67	0.67	0.66	0.66	0.65
14.710	0.65	0.65	0.64	0.64	0.64
14.760	0.63	0.63	0.63	0.62	0.62
14.810	0.61	0.61	0.61	0.60	0.60
14.860	0.60	0.59	0.59	0.59	0.58
14.910	0.58	0.58	0.57	0.57	0.56
14.960	0.56	0.56	0.55	0.55	0.54
15.010	0.54	0.54	0.53	0.53	0.52
15.060	0.52	0.52	0.51	0.51	0.50
15.110	0.50	0.50	0.49	0.49	0.48
15.160	0.48	0.48	0.47	0.47	0.47
15.210	0.46	0.46	0.46	0.45	0.45
15.260	0.45	0.44	0.44	0.44	0.44
15.310	0.43	0.43	0.43	0.43	0.42
15.360	0.42	0.42	0.41	0.41	0.40

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
15.410	0.40	0.40	0.40	0.39	0.39
15.460	0.39	0.39	0.38	0.38	0.38
15.510	0.38	0.38	0.38	0.37	0.37
15.560	0.37	0.37	0.37	0.37	0.36
15.610	0.36	0.36	0.36	0.36	0.36
15.660	0.36	0.36	0.35	0.35	0.35
15.710	0.35	0.35	0.35	0.35	0.35
15.760	0.35	0.34	0.34	0.34	0.34
15.810	0.34	0.34	0.34	0.34	0.34
15.860	0.34	0.34	0.33	0.33	0.33
15.910	0.33	0.33	0.33	0.33	0.33
15.960	0.33	0.33	0.33	0.33	0.32
16.010	0.32	0.32	0.32	0.32	0.32
16.060	0.32	0.32	0.32	0.32	0.32
16.110	0.32	0.31	0.31	0.31	0.31
16.160	0.31	0.31	0.31	0.31	0.31
16.210	0.31	0.31	0.30	0.30	0.30
16.260	0.30	0.30	0.30	0.30	0.30
16.310	0.30	0.30	0.30	0.30	0.30
16.360	0.30	0.30	0.30	0.30	0.30
16.410	0.30	0.30	0.30	0.30	0.30
16.460	0.30	0.30	0.30	0.30	0.30
16.510	0.30	0.30	0.30	0.30	0.29
16.560	0.29	0.29	0.29	0.29	0.29
16.610	0.29	0.29	0.29	0.29	0.29
16.660	0.29	0.29	0.29	0.29	0.29
16.710	0.29	0.29	0.29	0.29	0.29
16.760	0.29	0.29	0.29	0.29	0.29
16.810	0.29	0.29	0.29	0.29	0.29
16.860	0.29	0.29	0.29	0.29	0.28
16.910	0.28	0.28	0.28	0.28	0.28
16.960	0.28	0.28	0.28	0.28	0.28
17.010	0.28	0.28	0.28	0.28	0.28
17.060	0.28	0.28	0.28	0.28	0.28
17.110	0.28	0.28	0.28	0.28	0.28
17.160	0.28	0.28	0.28	0.28	0.28
17.210	0.28	0.28	0.28	0.27	0.27
17.260	0.27	0.27	0.27	0.27	0.27
17.310	0.27	0.27	0.27	0.27	0.27
17.360	0.27	0.27	0.27	0.27	0.27
17.410	0.27	0.27	0.27	0.27	0.27
17.460	0.27	0.27	0.27	0.27	0.27

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
17.510	0.27	0.27	0.27	0.27	0.27
17.560	0.27	0.27	0.27	0.26	0.26
17.610	0.26	0.26	0.26	0.26	0.26
17.660	0.26	0.26	0.26	0.26	0.26
17.710	0.26	0.26	0.26	0.26	0.26
17.760	0.26	0.26	0.26	0.26	0.26
17.810	0.26	0.26	0.26	0.26	0.26
17.860	0.26	0.26	0.26	0.26	0.26
17.910	0.26	0.25	0.25	0.25	0.25
17.960	0.25	0.25	0.25	0.25	0.25
18.010	0.25	0.25	0.25	0.25	0.25
18.060	0.25	0.25	0.25	0.25	0.25
18.110	0.25	0.25	0.25	0.25	0.25
18.160	0.25	0.25	0.25	0.25	0.25
18.210	0.25	0.25	0.25	0.25	0.24
18.260	0.24	0.24	0.24	0.24	0.24
18.310	0.24	0.24	0.24	0.24	0.24
18.360	0.24	0.24	0.24	0.24	0.24
18.410	0.24	0.24	0.24	0.24	0.24
18.460	0.24	0.24	0.24	0.24	0.24
18.510	0.24	0.24	0.24	0.24	0.24
18.560	0.24	0.24	0.23	0.23	0.23
18.610	0.23	0.23	0.23	0.23	0.23
18.660	0.23	0.23	0.23	0.23	0.23
18.710	0.23	0.23	0.23	0.23	0.23
18.760	0.23	0.23	0.23	0.23	0.23
18.810	0.23	0.23	0.23	0.23	0.23
18.860	0.23	0.23	0.23	0.22	0.22
18.910	0.22	0.22	0.22	0.22	0.22
18.960	0.22	0.22	0.22	0.22	0.22
19.010	0.22	0.22	0.22	0.22	0.22
19.060	0.22	0.22	0.22	0.22	0.22
19.110	0.22	0.22	0.22	0.22	0.22
19.160	0.22	0.22	0.22	0.21	0.21
19.210	0.21	0.21	0.21	0.21	0.21
19.260	0.21	0.21	0.21	0.21	0.21
19.310	0.21	0.21	0.21	0.21	0.21
19.360	0.21	0.21	0.21	0.21	0.21
19.410	0.21	0.21	0.21	0.21	0.21
19.460	0.21	0.21	0.21	0.21	0.21
19.510	0.20	0.20	0.20	0.20	0.20
19.560	0.20	0.20	0.20	0.20	0.20

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

				-	
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
19.610	0.20	0.20	0.20	0.20	0.20
19.660	0.20	0.20	0.20	0.20	0.20
19.710	0.20	0.20	0.20	0.20	0.20
19.760	0.20	0.20	0.20	0.20	0.20
19.810	0.19	0.19	0.19	0.19	0.19
19.860	0.19	0.19	0.19	0.19	0.19
19.910	0.19	0.19	0.19	0.19	0.19
19.960	0.19	0.19	0.19	0.19	0.19
20.010	0.19	0.19	0.19	0.19	0.19
20.060	0.19	0.19	0.19	0.19	0.19
20.110	0.19	0.19	0.18	0.18	0.18
20.160	0.18	0.18	0.18	0.18	0.18
20.210	0.18	0.18	0.18	0.18	0.18
20.260	0.18	0.18	0.18	0.18	0.18
20.310	0.18	0.18	0.18	0.18	0.18
20.360	0.18	0.18	0.18	0.18	0.18
20.410	0.18	0.18	0.18	0.18	0.18
20.460	0.18	0.18	0.18	0.18	0.18
20.510	0.18	0.18	0.18	0.18	0.18
20.560	0.18	0.18	0.18	0.18	0.18
20.610	0.18	0.18	0.18	0.18	0.18
20.660	0.18	0.18	0.18	0.18	0.18
20.710	0.18	0.18	0.18	0.18	0.18
20.760	0.18	0.18	0.18	0.18	0.18
20.810	0.18	0.18	0.18	0.18	0.18
20.860	0.18	0.18	0.18	0.18	0.18
20.910	0.18	0.18	0.18	0.18	0.18
20.960	0.18	0.18	0.18	0.18	0.18
21.010	0.18	0.18	0.18	0.18	0.18
21.060	0.18	0.18	0.18	0.18	0.18
21.110	0.18	0.18	0.18	0.18	0.18
21.160	0.18	0.18	0.18	0.18	0.18
21.210	0.18	0.18	0.18	0.18	0.18
21.260	0.18	0.18	0.18	0.18	0.18
21.310	0.18	0.18	0.18	0.18	0.18
21.360	0.18	0.18	0.18	0.18	0.18
21.410	0.18	0.18	0.18	0.18	0.18
21.460	0.18	0.18	0.18	0.18	0.18
21.510	0.18	0.18	0.18	0.18	0.18
21.560	0.18	0.18	0.18	0.18	0.18
21.610	0.18	0.18	0.18	0.18	0.18
21.660	0.18	0.18	0.18	0.17	0.17

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time					 Flow
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
21.710	0.17	0.17	0.17	0.17	0.17
21.760	0.17	0.17	0.17	0.17	0.17
21.810	0.17	0.17	0.17	0.17	0.17
21.860	0.17	0.17	0.17	0.17	0.17
21.910	0.17	0.17	0.17	0.17	0.17
21.960	0.17	0.17	0.17	0.17	0.17
22.010	0.17	0.17	0.17	0.17	0.17
22.060	0.17	0.17	0.17	0.17	0.17
22.110	0.17	0.17	0.17	0.17	0.17
22.160	0.17	0.17	0.17	0.17	0.17
22.210	0.17	0.17	0.17	0.17	0.17
22.260	0.17	0.17	0.17	0.17	0.17
22.310	0.17	0.17	0.17	0.17	0.17
22.360	0.17	0.17	0.17	0.17	0.17
22.410	0.17	0.17	0.17	0.17	0.17
22.460	0.17	0.17	0.17	0.17	0.17
22.510	0.17	0.17	0.17	0.17	0.17
22.560	0.17	0.17	0.17	0.17	0.17
22.610	0.17	0.17	0.17	0.17	0.17
22.660	0.17	0.17	0.17	0.17	0.17
22.710	0.17	0.17	0.17	0.17	0.17
22.760	0.17	0.17	0.17	0.17	0.17
22.810	0.17	0.17	0.17	0.17	0.17
22.860	0.17	0.17	0.17	0.17	0.17
22.910	0.17	0.17	0.17	0.17	0.17
22.960	0.17	0.17	0.17	0.17	0.17
23.010	0.17	0.17	0.17	0.17	0.17
23.060	0.17	0.17	0.17	0.17	0.17
23.110	0.17	0.17	0.17	0.17	0.17
23.160	0.17	0.17	0.17	0.17	0.17
23.210	0.17	0.17	0.17	0.17	0.17
23.260	0.17	0.17	0.17	0.17	0.17
23.310	0.17	0.17	0.17	0.17	0.17
23.360	0.17	0.17	0.17	0.17	0.17
23.410	0.17	0.17	0.16	0.16	0.16
23.460	0.16	0.16	0.16	0.16	0.16
23.510	0.16	0.16	0.16	0.16	0.16
23.560 23.610	0.16 0.16	0.16	0.16 0.16	0.16 0.16	0.16 0.16
		0.16			
23.660 23.710	0.16 0.16	0.16 0.16	0.16 0.16	0.16 0.16	0.16 0.16
23.710					
23.760	0.16	0.16	0.16	0.16	0.16

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 2 Return Event: 2 years Storm Event: TypeII 24hr: 1 (3.0 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
23.810	0.16	0.16	0.16	0.16	0.16
23.860	0.16	0.16	0.16	0.16	0.16
23.910	0.16	0.16	0.16	0.16	0.16
23.960	0.16	0.16	0.16	0.16	0.16
24.010	0.16	0.16	0.16	0.16	0.16
24.060	0.15	0.15	0.14	0.14	0.13
24.110	0.12	0.12	0.11	0.10	0.09
24.160	0.08	0.08	0.07	0.06	0.05
24.210	0.05	0.04	0.04	0.03	0.03
24.260	0.02	0.02	0.02	0.02	0.02
24.310	0.01	0.01	0.01	0.01	0.01
24.360	0.01	0.01	0.01	0.00	0.00
24.410	0.00	0.00	0.00	0.00	0.00
24.460	0.00	0.00	0.00	0.00	0.00

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT)

Scenario: Post-Development 5

Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

Peak Discharge12.78 ft³/sTime to Peak12.080 hoursHydrograph Volume0.952 ac-ft

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
10.620	0.00	0.00	0.00	0.00	0.00
10.670	0.00	0.00	0.01	0.01	0.01
10.720	0.01	0.01	0.01	0.01	0.02
10.770	0.02	0.02	0.02	0.02	0.03
10.820	0.03	0.03	0.03	0.04	0.04
10.870	0.04	0.04	0.05	0.05	0.05
10.920	0.05	0.06	0.06	0.06	0.06
10.970	0.07	0.07	0.07	0.08	0.08
11.020	0.08	0.08	0.09	0.09	0.09
11.070	0.10	0.10	0.10	0.11	0.11
11.120	0.12	0.12	0.12	0.13	0.13
11.170	0.14	0.14	0.15	0.16	0.16
11.220	0.17	0.18	0.18	0.19	0.19
11.270	0.20	0.21	0.21	0.22	0.23
11.320	0.23	0.24	0.25	0.25	0.26
11.370	0.27	0.28	0.28	0.29	0.30
11.420	0.31	0.31	0.32	0.32	0.32
11.470	0.33	0.33	0.34	0.35	0.35
11.520	0.36	0.37	0.38	0.38	0.39
11.570	0.41	0.42	0.43	0.44	0.45
11.620	0.47	0.48	0.50	0.52	0.54
11.670	0.56	0.58	0.60	0.62	0.64
11.720	0.66	0.68	0.71	0.73	0.75
11.770	0.78	0.80	0.83	0.86	0.89
11.820	0.91	0.95	0.98	1.00	1.03
11.870	1.07	1.11	1.23	1.49	1.88
11.920	2.80	3.12	3.41	3.67	3.91
11.970	4.13	4.33	4.51	5.18	6.67
12.020	8.27	9.69	10.86	11.73	12.33
12.070	12.66	12.78	12.70	12.46	12.10
12.120	11.64	11.13	10.59	10.01	9.43
12.170	8.90	8.37	7.87	7.43	7.03
12.220	6.64	6.30	6.02	5.76	5.50
12.270	5.26	5.12	4.97	4.83	4.70
12.320	4.57	4.55	4.53	4.51	4.48
12.370	4.45	4.43	4.40	4.37	4.34
12.420	4.31	4.28	4.25	4.21	4.17

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
12.470	4.13	4.10	4.06	4.02	3.98
12.520	3.94	3.90	3.86	3.82	3.77
12.570	3.73	3.69	3.64	3.60	3.55
12.620	3.50	3.45	3.41	3.36	3.31
12.670	3.26	3.21	3.16	3.11	3.06
12.720	3.01	2.96	2.91	2.86	2.81
12.770	2.77	2.72	2.58	2.43	2.29
12.820	2.17	2.07	1.98	1.94	1.92
12.870	1.89	1.87	1.85	1.83	1.81
12.920	1.79	1.77	1.75	1.74	1.72
12.970	1.70	1.69	1.67	1.66	1.64
13.020	1.63	1.61	1.60	1.58	1.57
13.070	1.56	1.54	1.53	1.52	1.51
13.120	1.50	1.49	1.48	1.47	1.46
13.170	1.44	1.43	1.42	1.42	1.41
13.220	1.40	1.39	1.38	1.37	1.36
13.270	1.35	1.35	1.34	1.33	1.33
13.320	1.32	1.32	1.31	1.30	1.30
13.370	1.29	1.29	1.28	1.27	1.27
13.420	1.26	1.26	1.25	1.24	1.24
13.470	1.23	1.23	1.22	1.22	1.22
13.520	1.21	1.21	1.20	1.20	1.19
13.570	1.19	1.18	1.18	1.17	1.17
13.620	1.16	1.16	1.15	1.15	1.14
13.670	1.14	1.14	1.14	1.14	1.13
13.720	1.13	1.13	1.13	1.12	1.12
13.770	1.12	1.12	1.12	1.11	1.11
13.820	1.11	1.11	1.10	1.10	1.10
13.870	1.10	1.10	1.09	1.09	1.09
13.920	1.09	1.08	1.08	1.08	1.08
13.970	1.07	1.07	1.07	1.07	1.07
14.020	1.07	1.07	1.07	1.07	1.07
14.070	1.07	1.07	1.07	1.07	1.07
14.120	1.07	1.06	1.06	1.06	1.06
14.170	1.05	1.05	1.05	1.05	1.04
14.220	1.04	1.04	1.04	1.04	1.04
14.270	1.03	1.03	1.03	1.03	1.03
14.320	1.03	1.03	1.03	1.03	1.03
14.370	1.03	1.03	1.02	1.02	1.02
14.420	1.02	1.01	1.01	1.01	1.01
14.470	1.00	1.00	1.00	1.00	1.00
14.520	1.00	0.99	0.99	0.99	0.99

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
14.570	0.99	0.99	0.99	0.99	0.98
14.620	0.98	0.98	0.98	0.98	0.98
14.670	0.98	0.97	0.97	0.97	0.97
14.720	0.97	0.97	0.96	0.96	0.96
14.770	0.96	0.95	0.95	0.95	0.95
14.820	0.95	0.94	0.94	0.94	0.94
14.870	0.93	0.93	0.93	0.92	0.92
14.920	0.92	0.92	0.92	0.91	0.91
14.970	0.91	0.91	0.91	0.90	0.90
15.020	0.90	0.90	0.90	0.90	0.89
15.070	0.89	0.89	0.89	0.89	0.89
15.120	0.88	0.88	0.88	0.88	0.88
15.170	0.87	0.87	0.87	0.87	0.87
15.220	0.86	0.86	0.86	0.86	0.85
15.270	0.85	0.85	0.85	0.85	0.84
15.320	0.84	0.84	0.84	0.84	0.83
15.370	0.83	0.83	0.83	0.82	0.82
15.420	0.82	0.82	0.82	0.81	0.81
15.470	0.81	0.81	0.80	0.80	0.80
15.520	0.80	0.80	0.79	0.79	0.79
15.570	0.79	0.78	0.78	0.78	0.78
15.620	0.78	0.77	0.77	0.77	0.77
15.670	0.76	0.76	0.76	0.76	0.75
15.720	0.75	0.75	0.75	0.75	0.74
15.770	0.74	0.74	0.74	0.73	0.73
15.820	0.73	0.73	0.72	0.72	0.72
15.870	0.72	0.71	0.71	0.71	0.71
15.920	0.71	0.70	0.70	0.70	0.70
15.970	0.69	0.69	0.69	0.69	0.68
16.020	0.68	0.68	0.68	0.67	0.67
16.070	0.67	0.67	0.66	0.66	0.66
16.120	0.66	0.65	0.65	0.65	0.65
16.170 16.220	0.64 0.63	0.64 0.63	0.64 0.62	0.64 0.62	0.63 0.62
16.220	0.63	0.63	0.62	0.62	0.62
16.270	0.62	0.60	0.60	0.60	0.61
16.320	0.60	0.60	0.60	0.60	0.60
16.420	0.59	0.59	0.58	0.59	0.59
16.470	0.58	0.57	0.57	0.56	0.56
16.520	0.56	0.56	0.55	0.55	0.55
16.570	0.55	0.55	0.54	0.53	0.53
16.620	0.55	0.55	0.53	0.53	0.54
10.020	0.54		0.00	0.00	0.00

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft³/s)
16.670	0.53	0.52	0.52	0.52	0.52
16.720	0.52	0.51	0.51	0.51	0.51
16.770	0.51	0.51	0.50	0.50	0.50
16.820	0.50	0.50	0.50	0.49	0.49
16.870	0.49	0.49	0.49	0.49	0.49
16.920	0.48	0.48	0.48	0.48	0.48
16.970	0.48	0.48	0.47	0.47	0.47
17.020	0.47	0.47	0.47	0.47	0.47
17.070	0.47	0.47	0.46	0.46	0.46
17.120	0.46	0.46	0.46	0.46	0.46
17.170	0.46	0.46	0.46	0.46	0.45
17.220	0.45	0.45	0.45	0.45	0.45
17.270	0.45	0.45	0.45	0.45	0.45
17.320	0.45	0.45	0.45	0.44	0.44
17.370	0.44	0.44	0.44	0.44	0.44
17.420	0.44	0.44	0.44	0.44	0.44
17.470	0.44	0.44	0.44	0.44	0.43
17.520	0.43	0.43	0.43	0.43	0.43
17.570	0.43	0.43	0.43	0.43	0.43
17.620	0.43	0.43	0.43	0.43	0.43
17.670	0.43	0.42	0.42	0.42	0.42
17.720	0.42	0.42	0.42	0.42	0.42
17.770	0.42	0.42	0.42	0.41	0.41
17.820	0.41	0.41	0.41	0.41	0.41
17.870	0.41	0.41	0.41	0.41	0.41
17.920	0.41	0.41	0.41	0.41	0.41
17.970	0.40	0.40	0.40	0.40	0.40
18.020	0.40	0.40	0.40	0.40	0.40
18.070	0.40	0.40	0.40	0.40	0.40
18.120	0.40	0.40	0.40	0.40	0.40
18.170	0.39	0.39	0.39	0.39	0.39
18.220	0.39	0.39	0.39	0.39	0.39
18.270	0.39	0.39	0.39	0.39	0.39
18.320	0.39	0.39	0.39	0.39	0.38
18.370	0.38	0.38	0.38	0.38	0.38
18.420	0.38	0.38	0.38	0.38	0.38
18.470	0.38	0.38	0.38	0.38	0.38
18.520	0.38	0.38	0.38	0.38	0.37
18.570	0.37	0.37	0.37	0.37	0.37
18.620	0.37	0.37	0.37	0.37	0.37
18.670	0.37	0.37	0.37	0.37	0.37
18.720	0.37	0.37	0.36	0.36	0.36

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft³/s)
18.770	0.36	0.36	0.36	0.36	0.36
18.820	0.36	0.36	0.36	0.36	0.36
18.870	0.36	0.36	0.36	0.36	0.36
18.920	0.35	0.35	0.35	0.35	0.35
18.970	0.35	0.35	0.35	0.35	0.35
19.020	0.35	0.35	0.35	0.35	0.35
19.070	0.35	0.35	0.35	0.35	0.35
19.120	0.34	0.34	0.34	0.34	0.34
19.170	0.34	0.34	0.34	0.34	0.34
19.220	0.34	0.34	0.34	0.34	0.34
19.270	0.34	0.34	0.34	0.34	0.33
19.320	0.33	0.33	0.33	0.33	0.33
19.370	0.33	0.33	0.33	0.33	0.33
19.420	0.33	0.33	0.33	0.33	0.33
19.470	0.33	0.33	0.33	0.32	0.32
19.520	0.32	0.32	0.32	0.32	0.32
19.570	0.32	0.32	0.32	0.32	0.32
19.620	0.32	0.32	0.32	0.32	0.32
19.670	0.32	0.32	0.32	0.31	0.31
19.720	0.31	0.31	0.31	0.31	0.31
19.770	0.31	0.31	0.31	0.31	0.31
19.820	0.31	0.30	0.30	0.30	0.30
19.870	0.30	0.30	0.30	0.30	0.30
19.920	0.30	0.30	0.30	0.30	0.30
19.970	0.30	0.30	0.30	0.30	0.29
20.020	0.29	0.29	0.29	0.29	0.29
20.070	0.29	0.29	0.29	0.29	0.29
20.120	0.29	0.29	0.29	0.29	0.29
20.170	0.29	0.29	0.29	0.29	0.29
20.220	0.29	0.29	0.29	0.29	0.29
20.270	0.29	0.29	0.29	0.29	0.29
20.320	0.29	0.29	0.29	0.29	0.29
20.370	0.29	0.29	0.28	0.28	0.28
20.420	0.28	0.28	0.28	0.28	0.28
20.470	0.28	0.28	0.28	0.28	0.28
20.520	0.28	0.28	0.28	0.28	0.28
20.570	0.28	0.28	0.28	0.28	0.28
20.620	0.28	0.28	0.28	0.28	0.28
20.670	0.28	0.28	0.28	0.28	0.28
20.720	0.28	0.28	0.28	0.28	0.28
20.770	0.28	0.28	0.28	0.28	0.28
20.820	0.28	0.28	0.28	0.28	0.28

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)
20.870	0.28	0.28	0.28	0.28	0.28
20.920	0.28	0.28	0.28	0.28	0.28
20.970	0.28	0.28	0.28	0.28	0.28
21.020	0.28	0.28	0.28	0.28	0.28
21.070	0.28	0.28	0.28	0.28	0.28
21.120	0.28	0.28	0.28	0.28	0.28
21.170	0.28	0.28	0.28	0.28	0.28
21.220	0.28	0.28	0.28	0.28	0.28
21.270	0.28	0.28	0.28	0.28	0.28
21.320	0.28	0.28	0.28	0.28	0.28
21.370	0.28	0.28	0.28	0.28	0.28
21.420	0.28	0.28	0.28	0.27	0.27
21.470	0.27	0.27	0.27	0.27	0.27
21.520	0.27	0.27	0.27	0.27	0.27
21.570	0.27	0.27	0.27	0.27	0.27
21.620	0.27	0.27	0.27	0.27	0.27
21.670	0.27	0.27	0.27	0.27	0.27
21.720	0.27	0.27	0.27	0.27	0.27
21.770	0.27	0.27	0.27	0.27	0.27
21.820	0.27	0.27	0.27	0.27	0.27
21.870	0.27	0.27	0.27	0.27	0.27
21.920	0.27	0.27	0.27	0.27	0.27
21.970	0.27	0.27	0.27	0.27	0.27
22.020	0.27	0.27	0.27	0.27	0.27
22.070	0.27	0.27	0.27	0.27	0.27
22.120	0.27	0.27	0.27	0.27	0.27
22.170	0.27	0.27	0.27	0.27	0.27
22.220	0.27	0.27	0.27	0.27	0.27
22.270	0.27	0.27	0.27	0.27	0.27
22.320	0.27	0.27	0.27	0.27	0.27
22.370	0.27	0.27	0.27	0.27	0.27
22.420	0.27	0.27	0.27	0.27	0.27
22.470	0.27	0.27	0.27	0.27	0.27
22.520	0.27	0.26	0.26	0.26	0.26
22.570	0.26	0.26	0.26	0.26	0.26
22.620	0.26	0.26	0.26	0.26	0.26
22.670	0.26	0.26	0.26	0.26	0.26
22.720	0.26	0.26	0.26	0.26	0.26
22.770	0.26	0.26	0.26	0.26	0.26
22.820	0.26	0.26	0.26	0.26	0.26
22.870	0.26	0.26	0.26	0.26	0.26
22.920	0.26	0.26	0.26	0.26	0.26

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 5 Return Event: 5 years Storm Event: TypeII 24hr: 1 (3.9 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft³/s)
22.970	0.26	0.26	0.26	0.26	0.26
23.020	0.26	0.26	0.26	0.26	0.26
23.070	0.26	0.26	0.26	0.26	0.26
23.120	0.26	0.26	0.26	0.26	0.26
23.170	0.26	0.26	0.26	0.26	0.26
23.220	0.26	0.26	0.26	0.26	0.26
23.270	0.26	0.26	0.26	0.26	0.26
23.320	0.26	0.26	0.26	0.26	0.26
23.370	0.26	0.26	0.26	0.26	0.26
23.420	0.26	0.26	0.26	0.26	0.26
23.470	0.26	0.26	0.26	0.26	0.26
23.520	0.26	0.26	0.26	0.25	0.25
23.570	0.25	0.25	0.25	0.25	0.25
23.620	0.25	0.25	0.25	0.25	0.25
23.670	0.25	0.25	0.25	0.25	0.25
23.720	0.25	0.25	0.25	0.25	0.25
23.770	0.25	0.25	0.25	0.25	0.25
23.820	0.25	0.25	0.25	0.25	0.25
23.870	0.25	0.25	0.25	0.25	0.25
23.920	0.25	0.25	0.25	0.25	0.25
23.970	0.25	0.25	0.25	0.25	0.25
24.020	0.25	0.25	0.24	0.24	0.23
24.070	0.23	0.22	0.21	0.19	0.18
24.120	0.17	0.15	0.14	0.13	0.12
24.170	0.11	0.10	0.09	0.08	0.07
24.220	0.06	0.06	0.05	0.04	0.04
24.270	0.03	0.03	0.03	0.02	0.02
24.320	0.02	0.02	0.01	0.01	0.01
24.370	0.01	0.01	0.01	0.01	0.01
24.420	0.00	0.00	0.00	0.00	0.00
24.470	0.00	0.00	0.00	0.00	0.00
24.520	0.00	0.00	(N/A)	(N/A)	(N/A)

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

Scenario: Post-Development 10

Peak Discharge	21.13 ft ³ /s
Time to Peak	12.050 hours
Hydrograph Volume	1.378 ac-ft

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
9.780	0.00	0.00	0.00	0.00	0.00
9.830	0.00	0.00	0.00	0.01	0.01
9.880	0.01	0.01	0.01	0.01	0.01
9.930	0.01	0.01	0.01	0.02	0.02
9.980	0.02	0.02	0.02	0.02	0.02
10.030	0.03	0.03	0.03	0.03	0.03
10.080	0.03	0.03	0.04	0.04	0.04
10.130	0.04	0.04	0.04	0.05	0.05
10.180	0.05	0.05	0.05	0.05	0.06
10.230	0.06	0.06	0.06	0.06	0.07
10.280	0.07	0.07	0.07	0.07	0.08
10.330	0.08	0.08	0.08	0.08	0.09
10.380	0.09	0.09	0.09	0.10	0.10
10.430	0.10	0.10	0.11	0.11	0.11
10.480	0.11	0.12	0.12	0.12	0.12
10.530	0.13	0.13	0.13	0.13	0.14
10.580	0.14	0.14	0.15	0.15	0.16
10.630	0.16	0.16	0.17	0.17	0.17
10.680	0.18	0.18	0.18	0.19	0.19
10.730	0.20	0.20	0.20	0.21	0.21
10.780	0.22	0.22	0.22	0.23	0.23
10.830	0.24	0.24	0.25	0.25	0.25
10.880	0.26	0.26	0.27	0.27	0.28
10.930	0.28	0.29	0.29	0.30	0.30
10.980	0.31	0.31	0.31	0.32	0.32
11.030	0.32	0.32	0.33	0.33	0.34
11.080	0.34	0.35	0.35	0.36	0.36
11.130	0.37	0.37	0.38	0.38	0.39
11.180	0.39	0.40	0.41	0.41	0.42
11.230	0.43	0.43	0.43	0.44	0.44
11.280	0.45	0.45	0.46	0.46	0.47
11.330	0.47	0.48	0.49	0.49	0.50
11.380	0.51	0.51	0.52	0.53	0.53
11.430	0.54	0.55	0.55	0.56	0.56
11.480	0.57	0.58	0.58	0.59	0.59
11.530	0.60	0.61	0.61	0.62	0.63
11.580	0.64	0.65	0.66	0.67	0.69

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.630	0.70	0.71	0.73	0.75	0.77
11.680	0.78	0.81	0.83	0.85	0.87
11.730	0.89	0.92	0.95	0.98	1.00
11.780	1.03	1.05	1.07	1.13	1.27
11.830	1.50	1.83	2.71	3.00	3.28
11.880	3.55	3.80	4.04	4.29	4.50
11.930	5.54	7.81	10.37	12.76	14.86
11.980	16.61	18.01	19.14	19.99	20.59
12.030	20.91	21.09	21.13	21.02	20.77
12.080	20.32	19.67	18.90	18.04	17.14
12.130	16.19	15.23	14.28	13.35	12.49
12.180	11.66	10.91	10.22	9.58	9.03
12.230	8.53	8.07	7.65	7.30	6.97
12.280	6.65	6.37	6.15	5.94	5.74
12.330	5.56	5.38	5.22	5.12	5.02
12.380	4.92	4.82	4.72	4.63	4.57
12.430	4.55	4.53	4.52	4.50	4.48
12.480	4.46	4.43	4.41	4.38	4.36
12.530	4.34	4.31	4.29	4.26	4.23
12.580	4.20	4.16	4.13	4.09	4.06
12.630	4.03	3.99	3.96	3.92	3.89
12.680	3.85	3.82	3.78	3.74	3.71
12.730	3.67	3.63	3.59	3.56	3.52
12.780	3.48	3.44	3.40	3.36	3.33
12.830	3.29	3.25	3.21	3.17	3.13
12.880	3.10	3.06	3.02	2.98	2.94
12.930	2.90	2.86	2.83	2.79	2.76
12.980	2.73	2.65	2.52	2.42	2.33
13.030 13.080	2.25	2.18	2.11 1.93	2.06 1.92	2.01 1.91
13.080	1.97 1.90	1.95 1.89	1.93	1.92	1.91
13.130	1.90	1.89	1.88	1.80	1.85
13.180	1.84	1.83	1.82	1.81	1.80
13.230	1.79	1.78	1.77	1.76 1.71	1.75
13.280	1.74	1.73	1.72	1.71	1.70
13.330	1.65	1.68	1.68	1.67	1.60
13.430	1.65	1.60	1.59	1.58	1.58
13.480	1.57	1.56	1.55	1.55	1.58
13.530	1.57	1.50	1.52	1.55	1.54
13.580	1.55	1.32	1.48	1.51	1.30
13.630	1.30	1.49	1.45	1.46	1.47
13.680	1.43	1.42	1.42	1.41	1.40
15.000	1.75		utama ina ila-t-	1. ⁴ 1	1.70

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft ³ /s)	Flow (ft³/s)
13.730	1.40	1.39	1.39	1.38	1.37
13.780	1.37	1.36	1.36	1.35	1.35
13.830	1.34	1.34	1.33	1.33	1.32
13.880	1.32	1.32	1.31	1.31	1.30
13.930	1.30	1.29	1.29	1.28	1.28
13.980	1.27	1.27	1.26	1.26	1.25
14.030	1.25	1.25	1.24	1.24	1.23
14.080	1.23	1.22	1.22	1.22	1.21
14.130	1.21	1.20	1.20	1.20	1.19
14.180	1.19	1.19	1.18	1.18	1.17
14.230	1.17	1.17	1.16	1.16	1.16
14.280	1.15	1.15	1.15	1.14	1.14
14.330	1.14	1.14	1.14	1.14	1.14
14.380	1.13	1.13	1.13	1.13	1.13
14.430	1.13	1.12	1.12	1.12	1.12
14.480	1.12	1.12	1.11	1.11	1.11
14.530	1.11	1.11	1.11	1.11	1.10
14.580	1.10	1.10	1.10	1.10	1.10
14.630	1.10	1.09	1.09	1.09	1.09
14.680	1.09	1.09	1.09	1.08	1.08
14.730	1.08	1.08	1.08	1.08	1.08
14.780	1.07	1.07	1.07	1.07	1.07
14.830	1.07	1.07	1.07	1.07	1.07
14.880	1.07	1.07	1.07	1.07	1.07
14.930	1.07	1.07	1.07	1.07	1.07
14.980	1.07	1.07	1.07	1.07	1.06
15.030	1.06	1.06	1.06	1.06	1.06
15.080	1.05	1.05	1.05	1.05	1.05
15.130	1.05	1.04	1.04	1.04	1.04
15.180	1.04	1.04	1.04	1.04	1.04
15.230	1.03	1.03	1.03	1.03	1.03
15.280	1.03	1.03	1.03	1.03	1.03
15.330	1.03	1.03	1.03	1.03	1.03
15.380	1.03	1.03	1.02	1.02	1.02
15.430	1.02	1.02	1.01	1.01	1.01
15.480	1.01	1.01	1.00	1.00	1.00
15.530	1.00	1.00	1.00	1.00	1.00
15.580	0.99	0.99	0.99	0.99	0.99
15.630	0.99	0.99	0.99	0.99	0.99
15.680	0.98	0.98	0.98	0.98	0.98
15.730	0.98	0.98	0.98	0.98	0.97
15.780	0.97	0.97	0.97	0.97	0.97

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)Flow (ft ³ /s)Flow (ft ³ /s)Flow (ft ³ /s)Flow (ft ³ /s)15.8300.960.960.960.960.960.9615.8800.950.950.950.950.9515.9300.940.940.940.940.9415.9800.930.930.930.930.9216.0300.920.920.920.910.9116.1300.900.900.900.900.9016.1800.900.890.890.890.8916.2300.880.880.880.880.8816.3300.870.870.870.8716.3300.870.870.870.860.8816.3300.870.870.870.860.8816.3300.870.870.870.860.8816.3300.840.840.840.840.8316.5300.810.810.810.810.8116.5300.810.800.790.790.7916.7800.790.780.780.7716.8300.760.760.760.760.7616.8300.780.780.780.770.7716.8300.770.770.770.770.7716.8300.780.780.780.780.7816.9300.750.750.750.750.7517.0300.750.7				-		
15.830 0.96 0.96 0.96 0.96 0.96 15.880 0.95 0.95 0.95 0.95 0.95 15.930 0.93 0.93 0.93 0.93 0.93 0.93 16.030 0.92 0.92 0.92 0.91 0.91 0.91 16.130 0.90 0.90 0.90 0.90 0.90 0.90 16.180 0.90 0.89 0.88 0.88 0.88 0.88 16.20 0.88 0.88 0.88 0.88 0.88 0.88 16.330 0.87 0.87 0.87 0.86 0.86 16.330 0.87 0.87 0.86 0.86 0.88 16.430 0.84 0.84 0.84 0.84 0.84 0.84 16.430 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>						
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17.830 0.62 <						
	17.880	0.62	0.61	0.61	0.61	0.61

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Bentley Systems, Inc. Haestad Methods Solution

Center

27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 55 of 76

Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

			-		
Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
17.930	0.61	0.61	0.61	0.61	0.60
17.980	0.60	0.60	0.60	0.60	0.60
18.030	0.60	0.60	0.59	0.59	0.59
18.080	0.59	0.59	0.59	0.59	0.59
18.130	0.59	0.58	0.58	0.58	0.58
18.180	0.58	0.58	0.58	0.58	0.58
18.230	0.57	0.57	0.57	0.57	0.57
18.280	0.57	0.57	0.56	0.56	0.56
18.330	0.56	0.56	0.56	0.56	0.55
18.380	0.55	0.55	0.55	0.55	0.55
18.430	0.55	0.55	0.55	0.54	0.54
18.480	0.54	0.54	0.54	0.54	0.54
18.530	0.54	0.53	0.53	0.53	0.53
18.580	0.53	0.53	0.53	0.53	0.53
18.630	0.52	0.52	0.52	0.52	0.52
18.680	0.52	0.52	0.52	0.52	0.51
18.730	0.51	0.51	0.51	0.51	0.51
18.780	0.51	0.51	0.51	0.51	0.51
18.830	0.50	0.50	0.50	0.50	0.50
18.880	0.50	0.50	0.50	0.50	0.50
18.930	0.49	0.49	0.49	0.49	0.49
18.980	0.49	0.49	0.49	0.49	0.49
19.030	0.48	0.48	0.48	0.48	0.48
19.080	0.48	0.48	0.48	0.48	0.48
19.130	0.48	0.48	0.47	0.47	0.47
19.180	0.47	0.47	0.47	0.47	0.47
19.230	0.47	0.47	0.47	0.47	0.46
19.280	0.46	0.46	0.46	0.46	0.46
19.330	0.46	0.46	0.46	0.46	0.46
19.380	0.46	0.46	0.45	0.45	0.45
19.430	0.45	0.45	0.45	0.45	0.45
19.480	0.45	0.45	0.45	0.45	0.45
19.530	0.45	0.44	0.44	0.44	0.44
19.580	0.44	0.44	0.44	0.44	0.44
19.630	0.44	0.44	0.44	0.44	0.43
19.680	0.43	0.43	0.43	0.43	0.43
19.730	0.43	0.43	0.43	0.43	0.43
19.780	0.43	0.43	0.42	0.42	0.42
19.830	0.42	0.42	0.42	0.42	0.42
19.880	0.41	0.41	0.41	0.41	0.41
19.930	0.41	0.41	0.41	0.41	0.41
19.980	0.41	0.40	0.40	0.40	0.40
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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
20.030	0.40	0.40	0.40	0.40	0.40
20.080	0.40	0.40	0.40	0.40	0.39
20.130	0.39	0.39	0.39	0.39	0.39
20.180	0.39	0.39	0.39	0.39	0.39
20.230	0.39	0.39	0.39	0.39	0.39
20.280	0.39	0.39	0.39	0.39	0.39
20.330	0.38	0.38	0.38	0.38	0.38
20.380	0.38	0.38	0.38	0.38	0.38
20.430	0.38	0.38	0.38	0.38	0.38
20.480	0.38	0.38	0.38	0.38	0.38
20.530	0.38	0.38	0.38	0.38	0.38
20.580	0.38	0.38	0.38	0.38	0.38
20.630	0.38	0.38	0.38	0.38	0.38
20.680	0.38	0.38	0.38	0.38	0.38
20.730	0.38	0.38	0.38	0.38	0.38
20.780	0.38	0.38	0.38	0.38	0.38
20.830	0.38	0.38	0.38	0.38	0.38
20.880	0.38	0.38	0.38	0.38	0.38
20.930	0.38	0.38	0.37	0.37	0.37
20.980	0.37	0.37	0.37	0.37	0.37
21.030	0.37	0.37	0.37	0.37	0.37
21.080	0.37	0.37	0.37	0.37	0.37
21.130	0.37	0.37	0.37	0.37	0.37
21.180	0.37	0.37	0.37	0.37	0.37
21.230	0.37	0.37	0.37	0.37	0.37
21.280	0.37	0.37	0.37	0.37	0.37
21.330	0.37	0.37	0.37	0.37	0.37
21.380	0.37	0.37	0.37	0.37	0.37
21.430	0.37	0.37	0.37	0.37	0.37
21.480	0.37	0.37	0.37	0.37	0.37
21.530	0.37	0.37	0.37	0.37	0.37
21.580	0.37	0.37	0.37	0.37	0.37
21.630	0.37	0.37	0.37	0.37	0.37
21.680	0.37	0.36	0.36	0.36	0.36
21.730	0.36	0.36	0.36	0.36	0.36
21.780	0.36	0.36	0.36	0.36	0.36
21.830	0.36	0.36	0.36	0.36	0.36
21.880	0.36	0.36	0.36	0.36	0.36
21.930	0.36	0.36	0.36	0.36	0.36
21.980	0.36	0.36	0.36	0.36	0.36
22.030	0.36	0.36	0.36	0.36	0.36
22.080	0.36	0.36	0.36	0.36	0.36

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
22.130	0.36	0.36	0.36	0.36	0.36
22.180	0.36	0.36	0.36	0.36	0.36
22.230	0.36	0.36	0.36	0.36	0.36
22.280	0.36	0.36	0.36	0.36	0.36
22.330	0.36	0.36	0.36	0.36	0.36
22.380	0.36	0.36	0.36	0.36	0.36
22.430	0.36	0.36	0.36	0.36	0.35
22.480	0.35	0.35	0.35	0.35	0.35
22.530	0.35	0.35	0.35	0.35	0.35
22.580	0.35	0.35	0.35	0.35	0.35
22.630	0.35	0.35	0.35	0.35	0.35
22.680	0.35	0.35	0.35	0.35	0.35
22.730	0.35	0.35	0.35	0.35	0.35
22.780	0.35	0.35	0.35	0.35	0.35
22.830	0.35	0.35	0.35	0.35	0.35
22.880	0.35	0.35	0.35	0.35	0.35
22.930	0.35	0.35	0.35	0.35	0.35
22.980	0.35	0.35	0.35	0.35	0.35
23.030	0.35	0.35	0.35	0.35	0.35
23.080	0.35	0.35	0.35	0.35	0.35
23.130	0.35	0.35	0.35	0.35	0.35
23.180	0.35	0.35	0.35	0.35	0.34
23.230	0.34	0.34	0.34	0.34	0.34
23.280	0.34	0.34	0.34	0.34	0.34
23.330	0.34	0.34	0.34	0.34	0.34
23.380	0.34	0.34	0.34	0.34	0.34
23.430	0.34	0.34	0.34	0.34	0.34
23.480	0.34	0.34	0.34	0.34	0.34
23.530	0.34	0.34	0.34	0.34	0.34
23.580	0.34	0.34	0.34	0.34	0.34
23.630	0.34	0.34	0.34	0.34	0.34
23.680	0.34	0.34	0.34	0.34	0.34
23.730	0.34	0.34	0.34	0.34	0.34
23.780	0.34	0.34	0.34	0.34	0.34
23.830	0.34	0.34	0.34	0.34	0.34
23.880	0.34	0.34	0.34	0.34	0.34
23.930	0.34	0.34	0.34	0.34	0.33
23.980	0.33	0.33	0.33	0.33	0.33
24.030	0.33	0.33	0.33	0.33	0.32
24.080	0.32	0.30	0.28	0.25	0.23
24.130	0.21	0.18	0.16	0.15	0.13
24.180	0.12	0.11	0.10	0.09	0.08

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 10 Return Event: 10 years Storm Event: TypeII 24hr: 1 (4.8 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
24.230	0.07	0.06	0.06	0.05	0.04
24.280	0.04	0.03	0.03	0.03	0.02
24.330	0.02	0.02	0.02	0.01	0.01
24.380	0.01	0.01	0.01	0.01	0.01
24.430	0.01	0.00	0.00	0.00	0.00
24.480	0.00	0.00	0.00	0.00	0.00
24.530	0.00	0.00	(N/A)	(N/A)	(N/A)

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT)

Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

Scenario: Post-Development 25

Peak Discharge	24.42 ft ³ /s
Time to Peak	12.060 hours
Hydrograph Volume	1.644 ac-ft

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft ³ /s)	Flow (ft³/s)
9.280	0.00	0.00	0.00	0.00	0.00
9.330	0.00	0.00	0.00	0.00	0.01
9.380	0.01	0.01	0.01	0.01	0.01
9.430	0.01	0.01	0.01	0.01	0.01
9.480	0.02	0.02	0.02	0.02	0.02
9.530	0.02	0.02	0.02	0.03	0.03
9.580	0.03	0.03	0.03	0.03	0.03
9.630	0.03	0.04	0.04	0.04	0.04
9.680	0.04	0.04	0.04	0.04	0.05
9.730	0.05	0.05	0.05	0.05	0.05
9.780	0.05	0.06	0.06	0.06	0.06
9.830	0.06	0.06	0.07	0.07	0.07
9.880	0.07	0.07	0.07	0.08	0.08
9.930	0.08	0.08	0.08	0.08	0.09
9.980	0.09	0.09	0.09	0.09	0.09
10.030	0.10	0.10	0.10	0.10	0.10
10.080	0.11	0.11	0.11	0.11	0.12
10.130	0.12	0.12	0.12	0.12	0.13
10.180	0.13	0.13	0.13	0.14	0.14
10.230	0.14	0.15	0.15	0.15	0.16
10.280	0.16	0.16	0.16	0.17	0.17
10.330	0.17	0.18	0.18	0.18	0.19
10.380	0.19	0.19	0.19	0.20	0.20
10.430	0.20	0.21	0.21	0.21	0.22
10.480	0.22	0.22	0.23	0.23	0.23
10.530	0.24	0.24	0.24	0.25	0.25
10.580	0.26	0.26	0.26	0.27	0.27
10.630	0.28	0.28	0.28	0.29	0.29
10.680	0.30	0.30	0.31	0.31	0.31
10.730	0.31	0.32	0.32	0.32	0.33
10.780	0.33	0.33	0.34	0.34	0.35
10.830	0.35	0.35	0.36	0.36	0.37
10.880	0.37	0.38	0.38	0.39	0.39
10.930	0.39	0.40	0.40	0.41	0.41
10.980	0.42	0.43	0.43	0.43	0.43
11.030	0.44	0.44	0.45	0.45	0.45
11.080	0.46	0.46	0.47	0.47	0.48

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time			Flow		
Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.130	0.48	0.49	0.49	0.50	0.50
11.130	0.48	0.49	0.49	0.50	0.53
11.130	0.51	0.51	0.55	0.55	0.55
	0.54	0.54		0.55	0.50
11.280	0.56	0.57	0.58 0.60	0.58	
11.330					0.61
11.380	0.62	0.63	0.63	0.64	0.65
11.430	0.65	0.66	0.66	0.67	0.68
11.480	0.68	0.69	0.69	0.70	0.71
11.530	0.71	0.72	0.73	0.74	0.75
11.580	0.76	0.77	0.78	0.79	0.80
11.630	0.82	0.83	0.85	0.86	0.88
11.680	0.90	0.92	0.95	0.97	0.99
11.730	1.01	1.03	1.07	1.08	1.14
11.780	1.28	1.49	1.79	2.52	2.93
11.830	3.19	3.43	3.67	3.89	4.11
11.880	4.33	4.54	5.92	8.35	11.03
11.930	13.63	16.03	18.14	19.98	21.32
11.980	22.35	22.90	23.28	23.62	23.91
12.030	24.14	24.32	24.41	24.42	24.34
12.080	24.16	23.88	23.50	23.00	22.22
12.130	20.88	19.30	17.76	16.39	15.13
12.180	14.00	13.00	12.11	11.30	10.60
12.230	9.96	9.39	8.90	8.46	8.04
12.280	7.68	7.36	7.07	6.80	6.54
12.330	6.32	6.14	5.97	5.80	5.64
12.380	5.49	5.35	5.22	5.14	5.05
12.430	4.97	4.88	4.80	4.72	4.63
12.480	4.57	4.56	4.54	4.53	4.51
12.530	4.49	4.47	4.45	4.43	4.41
12.580	4.38	4.36	4.34	4.31	4.29
12.630	4.27	4.24	4.21	4.18	4.15
12.680	4.12	4.09	4.05	4.02	3.99
12.730	3.96	3.93	3.90	3.87	3.84
12.780	3.81	3.77	3.74	3.71	3.68
12.830	3.64	3.61	3.58	3.55	3.51
12.880	3.48	3.45	3.41	3.38	3.35
12.930	3.31	3.28	3.25	3.21	3.18
12.980	3.15	3.11	3.08	3.04	3.01
13.030	2.97	2.94	2.91	2.88	2.85
13.080	2.82	2.79	2.76	2.73	2.67
13.130	2.56	2.75	2.70	2.75	2.25
13.180	2.30	2.15	2.59	2.07	2.23
13.100	2.20	2.15	2.11	2.07	2.03

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
13.230	2.00	1.98	1.95	1.95	1.94
13.280	1.93	1.93	1.92	1.91	1.90
13.330	1.90	1.89	1.88	1.87	1.87
13.380	1.86	1.85	1.84	1.83	1.83
13.430	1.82	1.81	1.80	1.80	1.79
13.480	1.78	1.77	1.76	1.76	1.75
13.530	1.74	1.73	1.72	1.72	1.71
13.580	1.70	1.69	1.69	1.68	1.67
13.630	1.67	1.66	1.65	1.64	1.64
13.680	1.63	1.62	1.61	1.61	1.60
13.730	1.59	1.58	1.58	1.57	1.56
13.780	1.56	1.55	1.54	1.54	1.53
13.830	1.52	1.52	1.51	1.51	1.50
13.880	1.49	1.49	1.48	1.48	1.47
13.930	1.46	1.46	1.45	1.45	1.44
13.980	1.43	1.43	1.42	1.42	1.41
14.030	1.40	1.40	1.39	1.39	1.38
14.080	1.37	1.37	1.36	1.36	1.35
14.130	1.35	1.35	1.34	1.34	1.33
14.180	1.33	1.33	1.32	1.32	1.31
14.230	1.31	1.31	1.30	1.30	1.29
14.280	1.29	1.29	1.28	1.28	1.28
14.330	1.27	1.27	1.27	1.26	1.26
14.380	1.26	1.25	1.25	1.25	1.24
14.430	1.24	1.24	1.23	1.23	1.23
14.480	1.23	1.22	1.22	1.22	1.22
14.530	1.21	1.21	1.21	1.21	1.20
14.580	1.20	1.20	1.20	1.19	1.19
14.630	1.19	1.19	1.18	1.18	1.18
14.680	1.18	1.17	1.17	1.17	1.17
14.730	1.16	1.16	1.16	1.16	1.15
14.780	1.15	1.15	1.15	1.15	1.14
14.830	1.14	1.14	1.14	1.14	1.14
14.880	1.14	1.14	1.14	1.14	1.13
14.930	1.13	1.13	1.13	1.13	1.13
14.980	1.13	1.13	1.12	1.12	1.12
15.030	1.12	1.12	1.12	1.12	1.12
15.080	1.12	1.11	1.11	1.11	1.11
15.130	1.11	1.11	1.11	1.10	1.10
15.180	1.10	1.10	1.10 1.09	1.10 1.09	1.10 1.09
15.230 15.280	1.10 1.09	1.10 1.09	1.09	1.09	1.09
15.280	1.09	1.09	1.09	1.09	1.08

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25

Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
15.330	1.08	1.08	1.08	1.08	1.08
15.380	1.08	1.08	1.07	1.07	1.07
15.430	1.07	1.07	1.07	1.07	1.07
15.480	1.07	1.07	1.07	1.07	1.07
15.530	1.07	1.07	1.07	1.07	1.07
15.580	1.07	1.07	1.07	1.07	1.07
15.630	1.07	1.07	1.06	1.06	1.06
15.680	1.06	1.06	1.06	1.05	1.05
15.730	1.05	1.05	1.05	1.05	1.04
15.780	1.04	1.04	1.04	1.04	1.04
15.830	1.04	1.04	1.04	1.03	1.03
15.880	1.03	1.03	1.03	1.03	1.03
15.930	1.03	1.03	1.03	1.03	1.03
15.980	1.03	1.03	1.03	1.03	1.02
16.030	1.02	1.02	1.02	1.01	1.01
16.080	1.01	1.01	1.01	1.00	1.00
16.130	1.00	1.00	1.00	1.00	1.00
16.180	1.00	0.99	0.99	0.99	0.99
16.230	0.99	0.99	0.99	0.99	0.99
16.280	0.99	0.98	0.98	0.98	0.98
16.330	0.98	0.98	0.98	0.98	0.98
16.380	0.97	0.97	0.97	0.97	0.97
16.430	0.97	0.97	0.96	0.96	0.96
16.480	0.96	0.96	0.96	0.95	0.95
16.530	0.95	0.95	0.95	0.94	0.94
16.580	0.94	0.94	0.94	0.93	0.93
16.630	0.93	0.93	0.93	0.92	0.92
16.680	0.92	0.92	0.92	0.92	0.91
16.730	0.91	0.91	0.91	0.91	0.91
16.780	0.91	0.90	0.90	0.90	0.90
16.830	0.90	0.90	0.90	0.90	0.89
16.880	0.89	0.89	0.89	0.89	0.89
16.930	0.89	0.89	0.88	0.88	0.88
16.980	0.88	0.88	0.88	0.88	0.87
17.030	0.87	0.87	0.87	0.87	0.87
17.080	0.87	0.86	0.86	0.86	0.86
17.130	0.86	0.86	0.85	0.85	0.85
17.180	0.85	0.85	0.85	0.85	0.84
17.230	0.84	0.84	0.84	0.84	0.84
17.280	0.84	0.83	0.83	0.83	0.83
17.330	0.83	0.83	0.83	0.82	0.82
17.380	0.82	0.82	0.82	0.82	0.81

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25

Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) **Output Time Increment = 0.010 hours** Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
17.430	0.81	0.81	0.81	0.81	0.81
17.480	0.81	0.80	0.80	0.80	0.80
17.530	0.80	0.80	0.80	0.79	0.79
17.580	0.79	0.79	0.79	0.79	0.79
17.630	0.78	0.78	0.78	0.78	0.78
17.680	0.78	0.78	0.77	0.77	0.77
17.730	0.77	0.77	0.77	0.77	0.76
17.780	0.76	0.76	0.76	0.76	0.76
17.830	0.76	0.75	0.75	0.75	0.75
17.880	0.75	0.75	0.75	0.74	0.74
17.930	0.74	0.74	0.74	0.74	0.74
17.980	0.73	0.73	0.73	0.73	0.73
18.030	0.73	0.73	0.73	0.72	0.72
18.080	0.72	0.72	0.72	0.72	0.72
18.130	0.71	0.71	0.71	0.71	0.71
18.180	0.71	0.71	0.71	0.70	0.70
18.230	0.70	0.70	0.70	0.70	0.70
18.280	0.69	0.69	0.69	0.69	0.69
18.330	0.69	0.69	0.68	0.68	0.68
18.380	0.68	0.68	0.68	0.68	0.68
18.430	0.67	0.67	0.67	0.67	0.67
18.480	0.67	0.67	0.66	0.66	0.66
18.530	0.66	0.66	0.66	0.66	0.66
18.580	0.65	0.65	0.65	0.65	0.65
18.630	0.65	0.65	0.65	0.64	0.64
18.680	0.64	0.64	0.64	0.64	0.64
18.730	0.63	0.63	0.63	0.63	0.63
18.780	0.63	0.63	0.62	0.62	0.62
18.830	0.62	0.62	0.62	0.62	0.62
18.880	0.61	0.61	0.61	0.61	0.61
18.930	0.61	0.61	0.61	0.60	0.60
18.980	0.60	0.60	0.60	0.60	0.60
19.030	0.60	0.59	0.59	0.59	0.59
19.080	0.59	0.59	0.59	0.59	0.59
19.130	0.58	0.58	0.58	0.58	0.58
19.180	0.58	0.58	0.58	0.58	0.57
19.230	0.57	0.57	0.57	0.57	0.57
19.280	0.57	0.56	0.56	0.56	0.56
19.330	0.56	0.56	0.56	0.55	0.55
19.380	0.55	0.55	0.55	0.55	0.55
19.430	0.55	0.54	0.54	0.54	0.54
19.480	0.54	0.54	0.54	0.54	0.53

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

			-		
Time	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
(hours)					
19.530	0.53	0.53	0.53	0.53	0.53
19.580	0.53	0.53	0.52	0.52	0.52
19.630	0.52	0.52	0.52	0.52	0.52
19.680	0.51	0.51	0.51	0.51	0.51
19.730	0.51	0.51	0.51	0.51	0.50
19.780	0.50	0.50	0.50	0.50	0.50
19.830	0.50	0.50	0.50	0.49	0.49
19.880	0.49	0.49	0.49	0.49	0.49
19.930	0.49	0.48	0.48	0.48	0.48
19.980	0.48	0.48	0.48	0.48	0.48
20.030	0.48	0.47	0.47	0.47	0.47
20.080	0.47	0.47	0.47	0.47	0.47
20.130	0.47	0.46	0.46	0.46	0.46
20.180	0.46	0.46	0.46	0.46	0.46
20.230	0.46	0.46	0.46	0.46	0.46
20.280	0.45	0.45	0.45	0.45	0.45
20.330	0.45	0.45	0.45	0.45	0.45
20.380	0.45	0.45	0.45	0.45	0.45
20.430	0.45	0.45	0.45	0.45	0.45
20.480	0.45	0.44	0.44	0.44	0.44
20.530	0.44	0.44	0.44	0.44	0.44
20.580	0.44	0.44	0.44	0.44	0.44
20.630	0.44	0.44	0.44	0.44	0.44
20.680	0.44	0.44	0.44	0.44	0.44
20.730	0.44	0.44	0.44	0.44	0.44
20.780	0.44	0.44	0.44	0.44	0.44
20.830	0.44	0.44	0.44	0.44	0.44
20.880	0.44	0.43	0.43	0.43	0.43
20.930	0.43	0.43	0.43	0.43	0.43
20.980	0.43	0.43	0.43	0.43	0.43
21.030	0.43	0.43	0.43	0.43	0.43
21.080	0.43	0.43	0.43	0.43	0.43
21.130	0.43	0.43	0.43	0.43	0.43
21.180	0.43	0.43	0.43	0.43	0.43
21.230	0.43	0.43	0.43	0.43	0.43
21.280	0.43	0.43	0.43	0.43	0.43
21.330	0.43	0.43	0.43	0.43	0.43
21.380	0.43	0.43	0.43	0.43	0.43
21.430	0.43	0.43	0.42	0.42	0.42
21.480	0.42	0.42	0.42	0.42	0.42
21.530	0.42	0.42	0.42	0.42	0.42
21.580	0.42	0.42	0.42	0.42	0.42

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

	-		-	-	
Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
21.630	0.42	0.42	0.42	0.42	0.42
21.680	0.42	0.42	0.42	0.42	0.42
21.730	0.42	0.42	0.42	0.42	0.42
21.780	0.42	0.42	0.42	0.42	0.42
21.830	0.42	0.42	0.42	0.42	0.42
21.880	0.42	0.42	0.42	0.42	0.42
21.930	0.42	0.42	0.42	0.42	0.42
21.980	0.42 0.41	0.42	0.42	0.42	0.42
22.030		0.41	0.41	0.41	0.41
22.080	0.41	0.41	0.41	0.41	0.41
22.130	0.41	0.41	0.41	0.41	0.41
22.180	0.41	0.41	0.41	0.41	0.41
22.230	0.41	0.41	0.41	0.41	0.41
22.280	0.41	0.41	0.41	0.41	0.41
22.330	0.41	0.41	0.41	0.41	0.41
22.380	0.41	0.41	0.41	0.41	0.41
22.430	0.41	0.41	0.41	0.41	0.41
22.480	0.41	0.41	0.41	0.41	0.41
22.530	0.41	0.41	0.41	0.41	0.41
22.580	0.41	0.41	0.41	0.41	0.41
22.630	0.41	0.41	0.41	0.41	0.40
22.680	0.40	0.40	0.40	0.40	0.40
22.730	0.40	0.40	0.40	0.40	0.40
22.780	0.40	0.40	0.40	0.40	0.40
22.830	0.40	0.40	0.40	0.40	0.40
22.880	0.40	0.40	0.40	0.40	0.40
22.930	0.40	0.40	0.40	0.40	0.40
22.980	0.40	0.40	0.40	0.40	0.40
23.030	0.40	0.40	0.40	0.40	0.40
23.080	0.40	0.40	0.40	0.40	0.40
23.130	0.40	0.40	0.40	0.40	0.40
23.180	0.40	0.40	0.40	0.40	0.40
23.230	0.40	0.40	0.40	0.40 0.40	0.40
23.280	0.40	0.40	0.40		0.40
23.330	0.39	0.39	0.39	0.39	0.39
23.380	0.39	0.39	0.39	0.39	0.39
23.430	0.39	0.39	0.39	0.39	0.39
23.480	0.39	0.39	0.39	0.39	0.39
23.530	0.39	0.39	0.39	0.39	0.39
23.580	0.39	0.39	0.39	0.39	0.39
23.630	0.39	0.39	0.39	0.39	0.39
23.680	0.39	0.39	0.39	0.39	0.39

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 25 Return Event: 25 years Storm Event: TypeII 24hr: 1 (5.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
23.730	0.39	0.39	0.39	0.39	0.39
23.780	0.39	0.39	0.39	0.39	0.39
23.830	0.39	0.39	0.39	0.39	0.39
23.880	0.39	0.39	0.39	0.39	0.39
23.930	0.39	0.39	0.39	0.39	0.38
23.980	0.38	0.38	0.38	0.38	0.38
24.030	0.38	0.38	0.38	0.38	0.37
24.080	0.37	0.36	0.35	0.33	0.32
24.130	0.30	0.25	0.22	0.18	0.16
24.180	0.14	0.13	0.11	0.10	0.09
24.230	0.08	0.07	0.07	0.06	0.05
24.280	0.04	0.04	0.03	0.03	0.03
24.330	0.02	0.02	0.02	0.02	0.01
24.380	0.01	0.01	0.01	0.01	0.01
24.430	0.01	0.01	0.00	0.00	0.00
24.480	0.00	0.00	0.00	0.00	0.00
24.530	0.00	0.00	0.00	(N/A)	(N/A)

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

Peak Discharge33.97 ft³/sTime to Peak12.040 hoursHydrograph Volume2.209 ac-ft

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
8.450	0.00	0.00	0.00	0.00	0.00
8.500	0.00	0.00	0.00	0.00	0.01
8.550	0.01	0.01	0.01	0.01	0.01
8.600	0.01	0.01	0.01	0.01	0.02
8.650	0.02	0.02	0.02	0.02	0.02
8.700	0.02	0.02	0.03	0.03	0.03
8.750	0.03	0.03	0.03	0.03	0.04
8.800	0.04	0.04	0.04	0.04	0.04
8.850	0.04	0.05	0.05	0.05	0.05
8.900	0.05	0.05	0.05	0.06	0.06
8.950	0.06	0.06	0.06	0.06	0.07
9.000	0.07	0.07	0.07	0.07	0.07
9.050	0.08	0.08	0.08	0.08	0.08
9.100	0.08	0.08	0.09	0.09	0.09
9.150	0.09	0.09	0.10	0.10	0.10
9.200	0.10	0.10	0.10	0.10	0.11
9.250	0.11	0.11	0.11	0.11	0.11
9.300	0.12	0.12	0.12	0.12	0.12
9.350	0.12	0.13	0.13	0.13	0.13
9.400	0.13	0.13	0.13	0.14	0.14
9.450	0.14	0.14	0.14	0.15	0.15
9.500	0.15	0.15	0.15	0.15	0.15
9.550	0.16	0.16	0.16	0.16	0.16
9.600	0.16	0.17	0.17	0.17	0.17
9.650	0.17	0.18	0.18	0.18	0.18
9.700	0.18	0.19	0.19	0.19	0.19
9.750	0.20	0.20	0.20	0.20	0.21
9.800	0.21	0.21	0.21	0.22	0.22
9.850	0.22	0.22	0.23	0.23	0.23
9.900	0.23	0.24	0.24	0.24	0.25
9.950	0.25	0.25	0.25	0.26	0.26
10.000	0.26	0.27	0.27	0.27	0.28
10.050	0.28	0.28	0.29	0.29	0.29
10.100	0.30	0.30	0.30	0.31	0.31
10.150	0.31	0.31	0.31	0.32	0.32
10.200	0.32	0.32	0.33	0.33	0.33
10.250	0.33	0.34	0.34	0.34	0.35

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft ³ /s)				
10.300	0.35	0.36	0.36	0.36	0.37
10.350	0.37	0.37	0.38	0.38	0.38
10.400	0.39	0.39	0.40	0.40	0.40
10.450	0.41	0.41	0.42	0.42	0.42
10.500	0.43	0.43	0.43	0.43	0.44
10.550	0.44	0.44	0.45	0.45	0.45
10.600	0.46	0.46	0.46	0.47	0.47
10.650	0.47	0.48	0.48	0.49	0.49
10.700	0.50	0.50	0.50	0.51	0.51
10.750	0.52	0.52	0.52	0.53	0.53
10.800	0.54	0.54	0.55	0.55	0.56
10.850	0.56	0.57	0.57	0.57	0.58
10.900	0.58	0.59	0.59	0.59	0.60
10.950	0.60	0.61	0.61	0.61	0.62
11.000	0.62	0.63	0.63	0.64	0.64
11.050	0.65	0.65	0.66	0.66	0.66
11.100	0.67	0.67	0.68	0.68	0.69
11.150	0.69	0.70	0.70	0.71	0.71
11.200	0.72	0.72	0.73	0.74	0.74
11.250	0.75	0.75	0.76	0.76	0.77
11.300	0.78	0.78	0.79	0.80	0.80
11.350	0.81	0.81	0.82	0.83	0.83
11.400	0.84	0.85	0.85	0.86	0.87
11.450	0.88	0.88	0.89	0.89	0.90
11.500	0.91	0.91	0.92	0.93	0.94
11.550	0.95	0.96	0.97	0.98	0.99
11.600	0.99	1.00	1.02	1.03	1.04
11.650	1.07	1.07	1.10	1.14	1.23
11.700	1.35	1.55	1.79	2.27	2.81
11.750	3.03	3.24	3.44	3.63	3.83
11.800	4.02	4.21	4.38	4.56	5.78
11.850	7.75	10.02	12.34	14.64	16.90
11.900	19.11	21.10	22.66	23.44	24.24
11.950	25.04	25.83	26.57	27.76	29.30
12.000	30.82	32.16	33.12	33.71	33.97
12.050	33.92	33.59	32.98	32.13	31.05
12.100	29.99	28.76	27.71	26.72	26.14
12.150	25.43	24.63	23.73	22.75	20.84
12.200	18.58	16.70	15.18	13.93	12.89
12.250	12.01	11.25	10.62	10.05	9.54
12.300	9.12	8.74	8.40	8.08	7.79
12.350	7.55	7.33	7.11	6.91	6.72

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

		-		-	
Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
12.400	6.55	6.38	6.25	6.12	5.99
12.450	5.87	5.75	5.64	5.52	5.41
12.500	5.30	5.21	5.14	5.07	5.00
12.550	4.93	4.86	4.78	4.71	4.64
12.600	4.57	4.56	4.55	4.54	4.52
12.650	4.51	4.49	4.48	4.46	4.44
12.700	4.42	4.40	4.38	4.36	4.35
12.750	4.33	4.31	4.29	4.27	4.26
12.800	4.24	4.21	4.19	4.16	4.14
12.850	4.11	4.09	4.07	4.04	4.02
12.900	4.00	3.97	3.95	3.93	3.90
12.950	3.88	3.85	3.83	3.80	3.78
13.000	3.75	3.73	3.70	3.68	3.65
13.050	3.63	3.60	3.57	3.55	3.52
13.100	3.49	3.47	3.44	3.41	3.39
13.150	3.36	3.33	3.31	3.28	3.25
13.200	3.23	3.20	3.17	3.15	3.12
13.250	3.09	3.07	3.04	3.01	2.99
13.300	2.96	2.94	2.91	2.89	2.86
13.350	2.84	2.82	2.80	2.77	2.75
13.400	2.73	2.71	2.63	2.55	2.49
13.450	2.44	2.39	2.34	2.30	2.27
13.500	2.24	2.21	2.19	2.16	2.14
13.550	2.12	2.10	2.08	2.07	2.05
13.600	2.04	2.02	2.01	2.00	1.99
13.650	1.97	1.96	1.95	1.95	1.95
13.700	1.94	1.94	1.93	1.93	1.92
13.750	1.92	1.92	1.91	1.90	1.90
13.800	1.89	1.89	1.88	1.88	1.87
13.850	1.87	1.86	1.85	1.85	1.84
13.900	1.83	1.83	1.82	1.81	1.81
13.950	1.80	1.79	1.79	1.78	1.77
14.000	1.77	1.76	1.75	1.75	1.74
14.050	1.73	1.73	1.72	1.71	1.71
14.100	1.70	1.69	1.69	1.68	1.67
14.150	1.67	1.66	1.65	1.65	1.64
14.200	1.64	1.63	1.63	1.62	1.61
14.250	1.61	1.60	1.60	1.59	1.59
14.300	1.58	1.58	1.57	1.57	1.56
14.350	1.56	1.55	1.55	1.55	1.54
14.400	1.54	1.53	1.53	1.53	1.52
14.450	1.52	1.52	1.51	1.51	1.51

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Bentley Systems, Inc. Haestad Methods Solution

Center

27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 70 of 76

Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

	Flow	Flow	Flow	Flow	Flow
Time (hours)	(ft ³ /s)				
14.500	1.50	1.50	1.50	1.49	1.49
14.550	1.49	1.48	1.48	1.48	1.47
14.600	1.47	1.47	1.46	1.46	1.46
14.650	1.46	1.45	1.45	1.45	1.44
14.700	1.44	1.44	1.43	1.43	1.43
14.750	1.43	1.42	1.42	1.42	1.41
14.800	1.41	1.41	1.41	1.40	1.40
14.850	1.40	1.39	1.39	1.39	1.39
14.900	1.38	1.38	1.38	1.37	1.37
14.950	1.37	1.37	1.36	1.36	1.36
15.000	1.36	1.35	1.35	1.35	1.35
15.050	1.35	1.34	1.34	1.34	1.34
15.100	1.33	1.33	1.33	1.33	1.33
15.150	1.32	1.32	1.32	1.32	1.31
15.200	1.31	1.31	1.31	1.31	1.30
15.250	1.30	1.30	1.30	1.29	1.29
15.300	1.29	1.29	1.28	1.28	1.28
15.350	1.28	1.27	1.27	1.27	1.27
15.400	1.26	1.26	1.26	1.26	1.25
15.450	1.25	1.25	1.25	1.24	1.24
15.500	1.24	1.24	1.23	1.23	1.23
15.550	1.23	1.23	1.22	1.22	1.22
15.600	1.22	1.21	1.21	1.21	1.21
15.650	1.20	1.20	1.20	1.20	1.20
15.700 15.750	1.19	1.19	1.19 1.18	1.19 1.17	1.18 1.17
15.800	1.18 1.17	1.18 1.17	1.18	1.17	1.17
15.800	1.17	1.17	1.16	1.16	1.16
15.830	1.16	1.15	1.15	1.15	1.15
15.900	1.14	1.14	1.14	1.14	1.14
16.000	1.13	1.13	1.14	1.13	1.13
16.050	1.13	1.12	1.13	1.13	1.13
16.100	1.13	1.12	1.12	1.12	1.12
16.150	1.12	1.12	1.12	1.12	1.11
16.200	1.11	1.10	1.10	1.10	1.10
16.250	1.10	1.10	1.10	1.10	1.09
16.300	1.09	1.09	1.09	1.09	1.09
16.350	1.09	1.08	1.08	1.08	1.08
16.400	1.08	1.08	1.08	1.08	1.07
16.450	1.07	1.07	1.07	1.07	1.07
16.500	1.07	1.07	1.07	1.07	1.07
16.550	1.07	1.07	1.07	1.07	1.07

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
16.600	1.07	1.07	1.07	1.07	1.07
16.650	1.07	1.07	1.07	1.07	1.07
16.700	1.07	1.07	1.07	1.07	1.06
16.750	1.06	1.06	1.06	1.06	1.06
16.800	1.06	1.06	1.05	1.05	1.05
16.850	1.05	1.05	1.05	1.05	1.05
16.900	1.04	1.04	1.04	1.04	1.04
16.950	1.04	1.04	1.04	1.04	1.04
17.000	1.04	1.04	1.03	1.03	1.03
17.050	1.03	1.03	1.03	1.03	1.03
17.100	1.03	1.03	1.03	1.03	1.03
17.150	1.03	1.03	1.03	1.03	1.03
17.200	1.03	1.03	1.03	1.03	1.02
17.250	1.02	1.02	1.02	1.02	1.02
17.300	1.02	1.01	1.01	1.01	1.01
17.350	1.01	1.01	1.01	1.00	1.00
17.400	1.00	1.00	1.00	1.00	1.00
17.450	1.00	1.00	1.00	1.00	0.99
17.500	0.99	0.99	0.99	0.99	0.99
17.550	0.99	0.99	0.99	0.99	0.99
17.600	0.99	0.99	0.99	0.98	0.98
17.650	0.98	0.98	0.98	0.98	0.98
17.700	0.98	0.98	0.98	0.98	0.98
17.750	0.98	0.97	0.97	0.97	0.97
17.800	0.97	0.97	0.97	0.97	0.97
17.850	0.97	0.96	0.96	0.96	0.96
17.900	0.96	0.96	0.96	0.95	0.95
17.950	0.95	0.95	0.95	0.95	0.95
18.000	0.95	0.94	0.94	0.94	0.94
18.050	0.94	0.94	0.93	0.93	0.93
18.100	0.93	0.93	0.93	0.93	0.92
18.150	0.92	0.92	0.92	0.92	0.92
18.200	0.92	0.91	0.91	0.91	0.91
18.250	0.91	0.91	0.91	0.91	0.91
18.300	0.91	0.90	0.90	0.90	0.90
18.350	0.90	0.90	0.90	0.90	0.90
18.400	0.89	0.89	0.89	0.89	0.89
18.450	0.89	0.89	0.89	0.89	0.89
18.500	0.88	0.88	0.88	0.88	0.88
18.550	0.88	0.88	0.88	0.88	0.87
18.600	0.87	0.87	0.87	0.87	0.87
18.650	0.87	0.86	0.86	0.86	0.86

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

(hours)(ft²/s)(ft²/s)(ft²/s)(ft²/s)18.7000.860.860.860.860.8518.7000.850.850.850.850.8518.8000.850.850.840.840.8418.8500.840.840.840.840.8318.9000.830.830.830.830.8318.9500.810.810.810.810.8119.0000.820.820.820.820.8219.0000.810.810.810.810.8119.1000.810.810.800.800.8019.1500.810.810.810.810.8119.1000.790.790.790.790.7919.2000.790.790.780.770.7719.3000.780.780.780.780.7719.3000.780.760.760.750.7519.5000.750.750.750.750.7519.5000.740.740.730.730.7319.6500.730.730.730.730.7219.7000.720.710.710.710.7119.8000.710.710.710.710.7019.9000.690.690.680.680.6819.9500.690.690.690.690.6919.9000.660.660.660.660.66<						
18.700 0.86 0.86 0.86 0.86 0.85 0.85 18.750 0.85 0.85 0.85 0.84 0.84 0.84 18.850 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.82 0.83 0.83 <td< th=""><th>Time</th><th>Flow</th><th>Flow</th><th>Flow</th><th>Flow</th><th>Flow</th></td<>	Time	Flow	Flow	Flow	Flow	Flow
18.750 0.85 0.85 0.85 0.84 0.84 0.84 18.800 0.83 0.84 0.84 0.84 0.84 18.800 0.83 0.83 0.83 0.83 0.83 18.900 0.83 0.83 0.82 0.82 0.82 19.000 0.82 0.82 0.82 0.82 0.82 19.000 0.81 0.81 0.81 0.81 0.81 19.000 0.81 0.81 0.81 0.81 0.81 19.100 0.81 0.81 0.80 0.80 0.80 19.100 0.81 0.81 0.80 0.80 0.80 19.100 0.79 0.79 0.79 0.79 0.79 19.200 0.77 0.77 0.77 0.77 0.77 19.300 0.78 0.78 0.78 0.78 0.75 19.50 0.74 0.74 0.74 0.74 0.74 19.50						
18.800 0.85 0.84 0.84 0.84 0.84 18.850 0.83 0.83 0.83 0.83 0.83 0.83 18.900 0.82 0.82 0.82 0.82 0.82 19.000 0.82 0.82 0.82 0.82 0.82 19.000 0.81 0.81 0.81 0.81 0.81 0.81 19.100 0.81 0.81 0.80 0.80 0.80 0.80 19.150 0.80 0.80 0.80 0.80 0.79 0.79 19.200 0.79 0.79 0.79 0.79 0.79 0.79 19.350 0.77 0.77 0.76 0.76 0.76 0.76 19.450 0.76 0.76 0.75 0.75 0.75 1.75 19.500 0.74 0.74 0.74 0.74 0.74 19.50 0.71 0.71 0.71 0.71 0.71 19.500 0.72						
18.850 0.84 0.84 0.84 0.83 18.900 0.83 0.83 0.83 0.83 0.83 18.950 0.83 0.82 0.82 0.82 0.82 19.000 0.82 0.82 0.82 0.82 0.82 19.050 0.81 0.81 0.81 0.81 0.81 19.100 0.81 0.81 0.80 0.80 0.80 19.150 0.80 0.80 0.80 0.80 0.80 19.200 0.79 0.79 0.79 0.79 0.79 19.300 0.78 0.78 0.78 0.77 0.77 19.300 0.78 0.76 0.76 0.76 0.76 19.400 0.77 0.77 0.77 0.77 0.77 19.400 0.74 0.74 0.74 0.74 0.74 19.400 0.77 0.76 0.75 0.75 0.75 19.500 0.77 0.77						
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19.6000.740.740.730.730.7319.6500.730.730.730.730.7219.7000.720.720.720.720.7219.7500.720.710.710.710.7119.8000.710.710.710.700.7019.8500.700.700.700.700.7019.9000.690.690.690.690.6919.9500.690.690.680.680.6820.0000.680.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.3000.640.640.640.640.6420.3000.640.640.630.630.6320.4000.620.620.620.620.6220.4000.620.620.620.620.6220.4000.620.620.610.610.6120.5000.610.610.610.610.6120.5000.610.610.600.600.6020.6000.600.600.600.600.6020.6000.600.600.600.600.6020.5000.610.600.600.600.6020.6500.600.600.600.600.60<	19.500	0.75		0.75	0.75	0.75
19.650 0.73 0.73 0.73 0.73 0.72 19.700 0.72 0.72 0.72 0.72 0.72 19.750 0.72 0.71 0.71 0.71 0.71 19.800 0.71 0.71 0.71 0.70 0.70 19.850 0.70 0.70 0.70 0.70 0.70 19.900 0.69 0.69 0.69 0.69 0.69 19.900 0.69 0.69 0.68 0.68 0.68 20.000 0.68 0.68 0.68 0.68 0.67 20.050 0.67 0.67 0.67 0.67 0.67 20.100 0.66 0.66 0.66 0.66 0.66 20.100 0.65 0.65 0.65 0.65 0.65 20.200 0.65 0.65 0.65 0.66 0.64 20.200 0.64 0.64 0.64 0.64 0.64 20.300 0.64						
19.7000.720.720.720.720.7219.7500.720.710.710.710.7119.8000.710.710.710.7019.8500.700.700.700.7019.9000.690.690.690.6919.9500.690.690.680.6820.0000.680.670.670.6720.1000.660.660.660.6620.1000.660.660.660.6620.1500.650.650.650.6520.2000.650.650.650.6520.2000.650.650.650.6520.2000.650.650.650.6420.3000.640.640.640.6420.3000.640.620.620.6220.4000.620.620.620.6220.4000.620.620.610.6120.5000.610.610.610.6120.5000.610.600.600.6020.6000.600.600.600.6020.6000.600.600.600.590.59	19.600	0.74	0.74	0.73	0.73	0.73
19.7500.720.710.710.710.7119.8000.710.710.710.7019.8500.700.700.700.7019.9000.690.690.690.6919.9500.690.690.680.6820.0000.680.680.680.6720.0500.670.670.670.6720.1000.660.660.660.6620.1500.650.650.650.6520.2000.650.650.650.6520.2000.650.650.650.6520.2000.650.650.650.6520.2000.640.640.640.6420.3000.640.640.630.6320.3500.630.630.630.6320.3500.610.610.610.6120.5000.610.610.610.6120.5000.610.600.600.6020.6000.600.600.600.6020.6000.600.600.600.6020.6500.600.600.590.590.59	19.650	0.73	0.73	0.73	0.73	0.72
19.8000.710.710.710.710.7019.8500.700.700.700.700.7019.9000.690.690.690.690.6919.9500.690.690.680.680.6820.0000.680.670.670.670.6720.0500.670.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.3000.640.640.640.640.6420.3000.640.640.630.630.6320.4000.620.620.620.620.6220.4000.620.620.610.610.6120.5000.610.610.610.610.6120.5000.610.600.600.600.6020.4500.620.620.620.620.6220.4500.610.610.610.610.6120.5000.610.600.600.600.600.6020.6000.600.600.600.600.600.6020.6500.600.600.600.600.600.60	19.700	0.72	0.72	0.72	0.72	0.72
19.8500.700.700.700.700.7019.9000.690.690.690.690.6919.9500.690.690.680.680.6820.0000.680.680.680.680.6720.0500.670.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.3000.640.640.640.640.6420.3000.640.640.630.630.6320.4000.620.620.620.620.6220.4000.620.620.610.610.6120.5000.610.610.610.610.6120.5000.610.600.600.600.6020.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5000.610.610.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	19.750	0.72	0.71	0.71	0.71	0.71
19.9000.690.690.690.690.6919.9500.690.690.680.680.6820.0000.680.680.680.680.6720.0500.670.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.2000.650.650.650.650.6420.2000.640.640.640.640.6420.2000.650.650.650.650.6520.2000.640.640.630.630.6320.3000.640.640.630.630.6320.3500.630.630.630.630.6220.4000.620.620.610.610.6120.5000.610.610.610.610.6120.5000.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	19.800	0.71	0.71	0.71	0.71	0.70
19.9500.690.680.680.680.6820.0000.680.680.680.680.6720.0500.670.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.2500.640.640.640.640.6420.3000.640.640.630.630.6320.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5500.610.610.610.610.6120.5500.610.600.600.600.6020.6500.600.600.600.600.60	19.850	0.70		0.70		
20.0000.680.680.680.680.6720.0500.670.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.3000.640.640.640.630.6320.3500.630.630.630.630.6220.4000.620.620.620.620.6220.4500.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	19.900	0.69	0.69		0.69	0.69
20.0500.670.670.670.6720.1000.660.660.660.660.6620.1500.660.660.650.650.6520.2000.650.650.650.650.6420.3000.640.640.640.640.6420.3000.640.640.630.630.6320.3500.630.630.630.630.6220.4000.620.620.620.620.6220.4500.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	19.950	0.69	0.69		0.68	
20.100 0.66 0.66 0.66 0.66 0.66 20.150 0.66 0.66 0.65 0.65 0.65 20.200 0.65 0.65 0.65 0.65 0.64 20.250 0.64 0.64 0.64 0.64 0.64 20.300 0.64 0.64 0.63 0.63 0.63 20.350 0.63 0.63 0.63 0.63 0.62 20.400 0.62 0.62 0.62 0.62 0.62 20.450 0.61 0.61 0.61 0.61 0.61 20.500 0.61 0.61 0.61 0.61 0.61 20.500 0.61 0.60 0.60 0.60 0.60 20.500 0.61 0.60 0.60 0.60 0.60 20.500 0.61 0.60 0.60 0.60 0.60 20.600 0.60 0.60 0.60 0.60 0.60 20.600 0.60	20.000	0.68	0.68	0.68	0.68	0.67
20.150 0.66 0.66 0.65 0.65 0.65 20.200 0.65 0.65 0.65 0.65 0.64 20.250 0.64 0.64 0.64 0.64 0.64 20.300 0.64 0.64 0.63 0.63 0.63 20.350 0.63 0.63 0.63 0.63 0.62 20.400 0.62 0.62 0.62 0.62 0.62 20.450 0.62 0.62 0.61 0.61 0.61 20.500 0.61 0.61 0.61 0.61 0.61 20.550 0.61 0.60 0.60 0.60 0.60 20.550 0.61 0.60 0.60 0.60 0.60 20.550 0.61 0.60 0.60 0.60 0.60 20.650 0.60 0.60 0.59 0.59 0.59	20.050	0.67	0.67	0.67	0.67	0.67
20.2000.650.650.650.6420.2500.640.640.640.640.6420.3000.640.640.630.630.6320.3500.630.630.630.630.6220.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5500.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.100	0.66	0.66	0.66	0.66	
20.2500.640.640.640.640.6420.3000.640.640.630.630.6320.3500.630.630.630.630.6220.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5000.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.150	0.66	0.66	0.65	0.65	0.65
20.3000.640.640.630.630.6320.3500.630.630.630.630.6220.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5000.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.200	0.65				0.64
20.3500.630.630.630.6220.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5000.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.250	0.64	0.64		0.64	0.64
20.4000.620.620.620.620.6220.4500.620.620.610.610.6120.5000.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.300	0.64	0.64	0.63	0.63	0.63
20.4500.620.620.610.610.6120.5000.610.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.350	0.63			0.63	0.62
20.5000.610.610.610.6120.5500.610.600.600.600.6020.6000.600.600.600.600.6020.6500.600.600.590.590.59	20.400	0.62	0.62	0.62	0.62	0.62
20.550 0.61 0.60 0.60 0.60 0.60 20.600 0.60 0.60 0.60 0.60 0.60 20.650 0.60 0.60 0.59 0.59 0.59	20.450	0.62	0.62	0.61	0.61	0.61
20.600 0.60 0.60 0.60 0.60 0.60 20.650 0.60 0.60 0.59 0.59 0.59	20.500	0.61	0.61	0.61		0.61
20.650 0.60 0.60 0.59 0.59 0.59	20.550	0.61	0.60	0.60	0.60	0.60
	20.600	0.60	0.60	0.60	0.60	0.60
	20.650	0.60	0.60	0.59	0.59	0.59
	20.700	0.59	0.59	0.59	0.59	0.59
20.750 0.59 0.59 0.59 0.59 0.59	20.750	0.59	0.59	0.59	0.59	0.59

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
20.800	0.58	0.58	0.58	0.58	0.58
20.850	0.58	0.58	0.58	0.58	0.58
20.900	0.58	0.58	0.58	0.58	0.58
20.950	0.57	0.57	0.57	0.57	0.57
21.000	0.57	0.57	0.57	0.57	0.57
21.050	0.57	0.57	0.57	0.56	0.56
21.100	0.56	0.56	0.56	0.56	0.56
21.150	0.56	0.56	0.56	0.56	0.56
21.200	0.56	0.56	0.56	0.56	0.56
21.250	0.56	0.55	0.55	0.55	0.55
21.300	0.55	0.55	0.55	0.55	0.55
21.350	0.55	0.55	0.55	0.55	0.55
21.400	0.55	0.55	0.55	0.55	0.55
21.450	0.55	0.55	0.55	0.55	0.55
21.500	0.55	0.54	0.54	0.54	0.54
21.550	0.54	0.54	0.54	0.54	0.54
21.600	0.54	0.54	0.54	0.54	0.54
21.650	0.54	0.54	0.54	0.54	0.54
21.700	0.54	0.54	0.54	0.54	0.54
21.750	0.54	0.54	0.54	0.54	0.54
21.800	0.54	0.54	0.54	0.53	0.53
21.850	0.53	0.53	0.53	0.53	0.53
21.900	0.53	0.53	0.53	0.53	0.53
21.950	0.53	0.53	0.53	0.53	0.53
22.000	0.53	0.53	0.53	0.53	0.53
22.050	0.53	0.53	0.53	0.53	0.53
22.100	0.53	0.53	0.53	0.53	0.53
22.150	0.53	0.53	0.53	0.53	0.53
22.200	0.53	0.53	0.53	0.52	0.52
22.250	0.52	0.52	0.52	0.52	0.52
22.300	0.52	0.52	0.52	0.52	0.52
22.350	0.52	0.52	0.52	0.52	0.52
22.400 22.450	0.52 0.52	0.52 0.52	0.52 0.52	0.52 0.52	0.52
					0.52
22.500 22.550	0.52 0.52	0.52 0.52	0.52 0.52	0.52 0.52	0.52 0.52
22.550	0.52			0.52	
22.600	0.52	0.52 0.52	0.52 0.52	0.52	0.52 0.52
22.650	0.52	0.52	0.52	0.52	0.52
22.700	0.51		0.51	0.51	0.51
22.750	0.51	0.51 0.51	0.51	0.51	0.51
22.800	0.51	0.51	0.51	0.51	0.51
22.850	0.51	0.51	0.51	0.51	0.51

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Subsection: Pond Routed Hydrograph (total out) Label: Detention Basin (OUT) Scenario: Post-Development 100 Return Event: 100 years Storm Event: TypeII 24hr: 1 (6.3 in)

HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.010 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
22.900	0.51	0.51	0.51	0.51	0.51
22.950	0.51	0.51	0.51	0.51	0.51
23.000	0.51	0.51	0.51	0.51	0.51
23.050	0.51	0.51	0.51	0.51	0.51
23.100	0.51	0.51	0.51	0.51	0.51
23.150	0.51	0.51	0.51	0.51	0.51
23.200	0.50	0.50	0.50	0.50	0.50
23.250	0.50	0.50	0.50	0.50	0.50
23.300	0.50	0.50	0.50	0.50	0.50
23.350	0.50	0.50	0.50	0.50	0.50
23.400	0.50	0.50	0.50	0.50	0.50
23.450	0.50	0.50	0.50	0.50	0.50
23.500	0.50	0.50	0.50	0.50	0.50
23.550	0.50	0.50	0.50	0.50	0.50
23.600	0.50	0.50	0.50	0.50	0.50
23.650	0.49	0.49	0.49	0.49	0.49
23.700	0.49	0.49	0.49	0.49	0.49
23.750	0.49	0.49	0.49	0.49	0.49
23.800	0.49	0.49	0.49	0.49	0.49
23.850	0.49	0.49	0.49	0.49	0.49
23.900	0.49	0.49	0.49	0.49	0.49
23.950	0.49	0.49	0.49	0.49	0.49
24.000	0.49	0.49	0.49	0.49	0.49
24.050	0.48	0.48	0.48	0.47	0.47
24.100	0.46	0.46	0.45	0.44	0.43
24.150	0.42	0.40	0.38	0.35	0.33
24.200	0.28	0.21	0.16	0.13	0.11
24.250	0.10	0.09	0.08	0.07	0.06
24.300	0.05	0.04	0.04	0.03	0.03
24.350	0.02	0.02	0.02	0.02	0.01
24.400	0.01	0.01	0.01	0.01	0.01
24.450	0.01	0.01	0.00	0.00	0.00
24.500	0.00	0.00	0.00	0.00	0.00
24.550	0.00	0.00	(N/A)	(N/A)	(N/A)

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APPENDIX D

Hydraulic Analysis



APPENDIX

STORM SEWER TABULATION

LAKE RIDGE ELEMENTARY SCHOOL ADDITION

TH&P PROJECT NO. 1922107C

												19221070	-								Invert F	levations
Line ID	Drainage Area (A)	Runoff Coeff. (C)	T _c	Intensity	Flow (Q)	Pipe Material	Manning's n	Line Slope	Fall	Calc'd Diameter	Design Diameter	Capacity Flow Full (Q _f)	Q/Q _f	V/V _f	Velocity	Line Length	Upstream Structure	Structure Top Elevation	Cover	M.H. Drop	Upstream	Downstream
504 50 2	acres	0.50	min.	in/hr	cfs	0.00	0.010	%	ft	in	in	cfs	0.04	0.7	ft/s	ft	50.0	ft msl	ft	ft	ft msl	ft msl
D9.1-D9.2	0.10	0.52	5	7.66	0.38	SICP	0.012	1.51%	0.44	5	8	1.607	0.24	0.7	3.22	29.2	D9.2	1470.90	2.14	2.93	1467.97	1467.53
D8.2-D9.1	0.08	0.71	5	7.66	0.80	SICP	0.012	1.50%	0.88	7	8	1.602	0.50	0.84	3.85	58.8	D9.1	1471.00	2.78	3.57	1467.43	1466.55
D8.6-D8.7	0.02	0.53	5	7.66	0.06	SICP	0.012	0.98%	0.23	3	8	1.298	0.05	0.44	1.64	23.4	D8.7	1474.00	2.19	2.98	1471.02	1470.79
D8.5-D8.6	0.04	0.53	5	7.66	0.24	SICP	0.012	1.80%	0.73	4	8	1.757	0.14	0.61	3.07	40.5	D8.6	1474.00	2.52	3.31	1470.69	1469.96
D8.4-D8.5	-	-	5	7.66	0.24	SICP	0.012	1.71%	0.84	4	8	1.710	0.14	0.61	2.99	49.2	D8.5	1473.31	2.56	3.35	1469.96	1469.12
D8.3-D8.4	0.06	0.51	5	7.66	0.48	SICP	0.012	1.03%	0.13	0	<u> </u>	1.328	0.37	0.78	2.97	12.6	D8.4	1471.50	1.69	2.48	1469.02	1468.89
D8.2-D8.3	-	-	5	7.66	0.48	SICP	0.012	1.39%	2.14	9	8	1.545	0.32	0.75	3.32	153.7	D8.3	1472.46	2.78	3.57	1468.89	1466.75
D8.1-D8.2 D2.2-D8.1	-	-	5	7.66 7.66	1.28 1.28	SICP	0.012	0.66% 0.61%	1.28 0.47	9	12	3.131 3.015	0.41	0.8	3.19	194.5 77.0	D8.2 D8.1	1470.29 1472.24	2.41 5.64	3.54 6.77	1466.75 1465.47	1465.47 1465.00
D2.2-D8.1 D7.1-D7.2	- 0.11	- 0.69	5	7.66	0.61	SICP	0.012	1.19%	0.47	6	12 12	4.204	0.43	0.81 0.62	3.11 3.32	77.0 31.2	D8.1 D7.2	1472.24	3.38	4.50	1465.47	1465.00
D7.1-D7.2 D2.2-D7.1 (Exist.)	0.11		5	7.66		RCP			1.49	8	12	4.204 7.993		0.62	4.04		D7.2 D7.1	1471.40	4.26	4.50 5.64	1466.90	1466.53
D2.2-D7.1 (EXISL.)	0.07	0.9	5	7.66	1.12 0.73	RCP	0.015	2.04% 0.74%	0.65	0 0	15	4.814	0.15 0.16	0.62	2.47	73.1 87.9	D7.1 D1.6(X)	1472.07	4.26	3.20	1406.43	1464.94
D1.6(X)-D5.5 (EXISL) D5.7-D5.8	0.24	0.40	5	7.66	0.73	SICP	0.015	1.51%	0.65	8 6	15	4.814	0.16	0.63	3.68	42.5	D1.6(X) D5.8	1474.31	2.14	3.20	1471.11	1470.46
D5.6-D5.7	0.10	0.87	5	7.66	1.20	SICP	0.012	1.50%	0.62	8	12	4.735	0.14	0.71	4.28	42.3	D5.8 D5.7	1474.90	2.14	4.01	1471.03	1470.33
D5.5-D5.6	0.15	0.54	5	7.66	2.53	SICP	0.012	0.71%	0.82	° 11	12	5.903	0.28	0.71	3.90	33.7	D5.6	1474.90	2.67	4.01	1470.89	1470.27
D5.4-D5.5	0.13	0.33	5	7.66	3.36	SICP	0.012	0.69%	0.24	11	15	5.829	0.43	0.81	4.23	31.7	D3.0 D5.5	1474.30	3.92	4.03 5.30	1470.31	1470.07
D5.4-D5.5	0.12	0.39	5	7.66	0.23	SICP	0.012	7.90%	1.81	3	13	10.849	0.03	0.36	4.23	22.9	D3.3 D6.1	1473.27	14.91	16.03	1409.97	1469.73
D5.3-D5.4	0.06	0.35	5	7.66	3.76	SICP	0.012	0.70%	0.76	13	15	5.869	0.65	0.92	4.40	108.1	D0.1 D5.4	1474.40	3.38	4.75	1469.65	1468.89
D5.2-D5.3	0.16	0.3	5	7.66	4.14	RCP	0.012	0.84%	0.30	13	15	5.119	0.81	0.92	4.13	35.9	D5.4	1474.30	4.13	5.51	1468.79	1468.49
D5.1-D5.2	0.10	0.9	5	7.66	4.77	SICP	0.013	0.93%	0.50	14	15	6.736	0.71	0.95	5.21	55.1	D5.2	1473.50	3.73	5.11	1468.39	1467.88
D1.3(X)-D5.1 (Exist.)	-	-	5	7.66	4.77	RCP	0.015	1.60%	1.55	13	15	7.081	0.68	0.93	5.37	96.9	D5.1	1474.87	5.71	7.09	1467.78	1466.23
D4.5-D4.6	0.19	0.88	5	7.66	1.29	SICP	0.013	0.99%	0.55	8	12	3.847	0.34	0.76	3.72	55.4	D4.6	1473.30	2.69	3.82	1469.48	1468.93
D4.4-D4.5	0.04	0.42	5	7.66	1.40	SICP	0.012	1.02%	0.26	9	12	3.890	0.36	0.77	3.81	25.6	D4.5	1473.30	3.24	4.37	1468.93	1468.67
D4.3-D4.4	0.05	0.87	5	7.66	1.73	SICP	0.012	1.00%	0.39	9	12	3.861	0.45	0.82	4.03	39.0	D4.4	1472.90	3.11	4.23	1468.67	1468.28
D4.2-D4.3	0.07	0.87	5	7.66	2.23	SICP	0.012	1.00%	0.46	10	12	3.860	0.58	0.89	4.37	46.0	D4.3	1473.90	4.50	5.62	1468.28	1467.82
D4.1-D4.2	0.25	0.87	5	7.66	3.90	SICP	0.012	5.48%	1.64	9	15	16.376	0.24	0.7	9.34	30.0	D4.2	1474.80	5.61	6.98	1467.82	1466.18
D4.1-D1.3(X) (Exist.)	5.26	0.30	10	6.35	14.80	CMP	0.017	0.60%	0.15	25	30	24.296	0.61	0.9	4.45	25.0	D1.3(X)	1470.58	1.72	4.35	1466.23	1466.08
D3.1-D4.1 (Exist.)	-	-	5	7.66	18.70	СМР	0.017	0.50%	0.90	29	30	22.179	0.85	1	4.52	180.4	D4.1	1470.58	1.88	4.50	1466.08	1465.18
D3.2-D3.3	0.19	0.87	5	7.66	1.27	SICP	0.012	1.49%	1.49	8	15	8.529	0.15	0.62	4.31	100.3	D3.3	1471.60	2.13	3.50	1468.10	1466.61
D3.1-D3.2	1.02	0.57	5	7.66	5.74	SICP	0.012	1.99%	1.25	13	15	9.881	0.59	0.89	7.17	62.7	D3.2	1471.40	3.52	4.89	1466.51	1465.26
D2.2-D3.1 (Exist.)	0.17	0.46	5	7.66	21.15	CMP	0.017	0.50%	0.34	30	30	22.212	0.96	1.03	4.66	67.4	D3.1	1473.71	5.91	8.53	1465.18	1464.84
D2.1-D2.2	0.07	0.90	5	7.66	24.07	SICP	0.012	0.96%	0.34	25	30	43.486	0.56	0.88	7.80	35.5	D2.2	1472.98	5.52	8.14	1464.84	1464.50
D1.2-D1.3	-	-	5	7.66	24.07	SICP	0.012	1.96%	0.22	22	24	34.348	0.71	0.95	10.39	11.2	D1.3	1471.00	5.38	7.50	1463.50	1463.28
D1.1-D1.2	-	-	5	7.66	24.07	SICP	0.012	12.08%	0.79	15	24	85.178	0.29	0.73	19.79	6.5	D1.2	1471.00	5.60	7.72	1463.28	1462.49
	•		•		IDF File:		Johnson City			25	year			•		Total Nun	nber of Lines:	34			Run Date:	03/10/21
															Total N	umber of Pr	oposed Lines:	29				

APPENDIX E

Water Quality Calculations



APPENDIX



WATER QUALITY CALCULATIONS

PROJECT: LAKE RIDGE ELEMENTARY SCHOOL ADDITION PROJECT NO: 1922107C DATE: 3/1/2021

1. COMPUTE RUNOFF COEFFICIENT

RV = 0.015 + 0.0092*(I)

WHERE:

- I = PERCENT IMPERVIOUS AREA OF THE SITE
- I = IA/A X 100%
- IA = CUMULATIVE AREA OF ALL IMPERVIOUS SURFACES ON THE SITE (ACRES)
- A = SITE AREA (ACRES)

AREA	Α
IMPERVIOUS	2.35
PERVIOUS	6.04
TOTAL	8.38

IA =	2.35	ACRES
A =	8.38	ACRES
I =	28.00	%

0.27

2. COMPUTE WATER QUALITY RUNOF DEPTH

DWQ = 1.04*RV

RV =

DWQ =	0.28	INCHES	
3. COMPUTE CURVE NUMBER)		
5. COMPOTE CORVE NOMBER	•		
SCS CURVE NUMBER = CN			
			CN
IMPERVIOUS =	2.35	ACRES	98
OPEN SPACE =	6.04	ACRES	61
WEIGHTED CN =	71		
4. COMPUTE IA/P			
INITIAL ABSTRACTION = IA			
IA =	0.817	FROM TABLE	3-4
P =	1.04	INCHES (RAIN	IFALL DEPTH FOR 85% STORM EVENT
IA/P =	0.79		
5. COMPUTE TIME OF CONCE	NIKATION		
TIME OF CONCENTRATION = T	С		
TC =	10.4	MINUTES	
TC =	0.17	HOURS	J
6. COMPUTE UNIT PEAK DISC	HARGE		
or commone onthin EAR Disc			



WATER QUALITY CALCULATIONS

PROJECT: LAKE RIDGE ELEMENTARY SCHOOL ADDITION PROJECT NO: 1922107C DATE: 3/1/2021

7. COMPUTE WATER QUALITY VOLUME

WQV = [P*R*V*(A-N)]/12

WHERE: WQV = WATER QUALITY VOLUME OF THE SITE (AC-FT) P = RAINFALL DEPTH FOR THE 85% STORM EVENT (1.04 INCHES) RV = RUNOFF COEFFICIENT A = SITE AREA (ACRES) N = NATURAL AREA PRESERVATION (ACRES) P = 1.04 INCHES RV = 0.27 A = 8.38 ACRES N = 0.00 ACRES WQV = 0.20 AC-FT 8,628 CF

8. COMPUTE WATER QUALITY PEAK DISCHARGE

QWQ = QU*A*DWQ

- QWQ = WATER QUALITY PEAK DISCHARGE (CFS)
 - QU = UNIT PEAK DISCHARGE (CFS/MI2/INCH), FIGURE 3-2
 - A = DRAINAGE AREA (MI2)
- DWQ = WATER QUALITY RUNOFF DEPTH (INCHES)

QWQ = 2.23 CFS

9. AQUA-SWIRL SIZING

WATER QUALIT	Y TREATMENT FLOW =	2.23	CFS
ļ	AQUA-SWIRL MODEL =	AS-4	
	TREATS =	3.2	CFS

APPENDIX F

BMP Operations and Maintenance



APPENDIX



Aqua-Swirl[®] Stormwater Treatment System

Inspection and Maintenance Manual



AquaShield[™], Inc. 2733 Kanasita Drive Suite 111 Chattanooga, TN 37343 Toll free (888) 344-9044 Phone: (423) 870-8888 Fax: (423) 826-2112 Email: info@aquashieldinc.com <u>www.aquashieldinc.com</u>

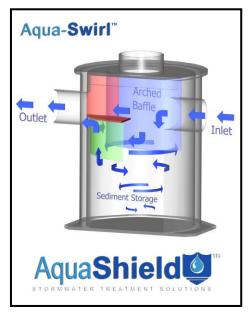
November 2016

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Aqua-Swirl[®] Stormwater Treatment System

The Aqua-Swirl[®] Stormwater Treatment System (Aqua-Swirl[®]) is a vortex-type hydrodynamic separator designed and supplied by AquaShieldTM, Inc. (AquaShieldTM). Aqua-Swirl[®] technology removes pollutants including suspended solids, debris, floatables and free-floating oil from stormwater runoff. Both treatment and storage are accomplished in the single swirl chamber without the use of multiple or hidden, blind access chambers.



Aqua-Swirl[®] Stormwater Treatment System



Floatable debris in the Aqua-Swirl®



The treatment operation begins when stormwater enters the Aqua-Swirl[®] through a tangential inlet pipe that produces a circular (or vortex) flow pattern that causes contaminates to settle to the base of the unit. Since stormwater flow is intermittent by nature, the Aqua-Swirl[®] retains water between storm events providing both dynamic and quiescent settling of solids. The dynamic settling occurs during each storm event while the quiescent settling takes place between successive storms. A combination of gravitational and hydrodynamic drag forces encourages the solids to drop out of the flow and migrate to the center of the chamber where velocities are the lowest.

The treated flow then exits the Aqua-Swirl[®] behind the arched outer baffle. The top of the baffle is sealed across the treatment channel, thereby eliminating floatable pollutants from escaping the system. A vent pipe is extended up the riser to expose the backside of the baffle to atmospheric conditions, preventing a siphon from forming at the bottom of the baffle.



The Aqua-Swirl[®] system can be modified to fit a variety of purposes in the field, and the angles for inlet and outlet lines can be modified to fit most applications. The photo below demonstrates the flexibility of Aqua-Swirl[®] installations using a "twin" configuration in order to double the water quality treatment capacity. Two Aqua-Swirl[®] units were placed side by side in order to treat a high volume of water while occupying a small amount of space.



Custom designed AS-9 Twin Aqua-Swirl®



The Aqua-Swirl[®] system is designed so that it can easily be used for retrofit applications. With the invert of the inlet and outlet pipe at the same elevation, the Aqua-Swirl[®] can easily be connected directly to the existing storm conveyance drainage system. Furthermore, because of the lightweight nature and small footprint of the Aqua-Swirl[®], existing infrastructure utilities (i.e., wires, poles, trees) would be unaffected by installation.



The long term performance of any stormwater treatment structure, including manufactured or land based systems, depends on a consistent maintenance plan. Inspection and maintenance functions are simple and easy for the Aqua-Swirl[®] allowing all inspections to be performed from the surface.

It is important that a routine inspection and maintenance program be established for each unit based on: (a) the volume or load of the contaminants of concern, (b) the frequency of releases of contaminants at the facility or location, and (c) the nature of the area being drained.

In order to ensure that our systems are being maintained properly, AquaShieldTM offers a maintenance solution to all of our customers. We will arrange to have maintenance performed.



Aqua-Swirl[®] manhole cover



The Aqua-Swirl[®] can be inspected from the surface, eliminating the need to enter the system to determine when cleanout should be performed. In most cases, AquaShieldTM recommends a quarterly inspection for the first year of operation to develop an appropriate schedule of maintenance. Based on experience of the system's first year in operation, we recommend that the inspection schedule be revised to reflect the site-specific conditions encountered. Typically, the inspection schedule for subsequent years is reduced to semi-annual inspection.



The Aqua-Swirl[®] has been designed to minimize and simplify the inspection and maintenance process. The single chamber system can be inspected and maintained entirely from the surface thereby eliminating the need for confined space entry. Furthermore, the entire structure (specifically, the floor) is accessible for visual inspection from the surface. There are no areas of the structure that are blocked from visual inspection or periodic cleaning. Inspection of any free-floating oil and floatable debris can be directly observed and maintained through the manhole access provided directly over the swirl chamber.

Aqua-Swirl[®] Inspection Procedure

To inspect the Aqua-Swirl[®], a hook is typically needed to remove the manhole cover. AquaShieldTM provides a customized manhole cover with our distinctive logo to make it easy for maintenance crews to locate the system in the field. We also provide a permanent metal information plate affixed inside the access riser which provides our contact information, the Aqua-Swirl[®] model size, and serial number.

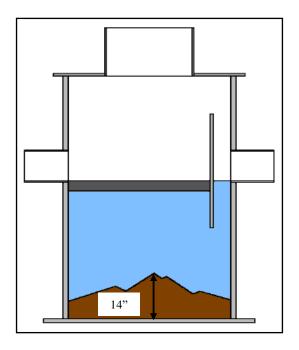
The only tools needed to inspect the Aqua-Swirl[®] system are a flashlight and a measuring device such as a stadia rod or pole. Given the easy and direct accessibility provided, floating oil and debris can be observed directly from the surface. Sediment depths can easily be determined by lowering a measuring device to the top of the sediment pile and to the surface of the water.

It should be noted that in order to avoid underestimating the volume of sediment in the chamber, the measuring device must be carefully lowered to the *top* of the sediment pile. Keep in mind that the finer sediment at the top of the pile may offer less resistance to the measuring device than the larger particles which typically occur deeper within the sediment pile.

The Aqua-Swirl[®] design allows for the sediment to accumulate in a semi-conical fashion as illustrated below. That is, the depth to sediment as measured below the water surface may be less in the center of the swirl chamber; and likewise, may be greater at the edges of the swirl chamber.



Sediment inspection using a stadia rod



Maximum recommended sediment depth prior to cleanout is 14 inches for all Aqua-Swirl[®] models

Aqua-Swirl[®] Cleanout Procedure

Cleaning the Aqua-Swirl[®] is simple and quick. Free-floating oil and floatable debris can be observed and removed directly through the 30-inch service access riser provided. A vacuum truck is typically used to remove the accumulated sediment and debris. An advantage of the Aqua-Swirl[®] design is that the entire sediment storage area can be reached with a vacuum hose

from the surface reaching all the sides. Since there are no multiple or limited (blind) access chambers in the Aqua-Swirl[®], there are no restrictions to impede on-site maintenance tasks.

Disposal of Recovered Materials

AquaShieldTM recommends that all maintenance activities be performed in accordance with appropriate health and safety practices for the tasks and equipment being used. AquaShieldTM also recommends that all materials removed from the Aqua-Swirl[®] and any external structures (e.g, bypass features) be handled and disposed in full accordance with any applicable local and state requirements.



Vacuum (vactor) truck quickly cleans the single open access swirl chamber

Aqua-Swirl[®] Inspection and Maintenance Work Sheets on following pages

Aqua-Swirl[®] Inspection and Maintenance Manual Work Sheets

SITE and OWNER INFORMATION

Site Name:	
Site Location:	
Date:	Time:
Inspector Name:	
Inspector Company:	Phone #:
Owner Name:	
Owner Address:	
Owner Phone #:	Emergency Phone #:

INSPECTIONS

I. Floatable Debris and Oil

- 1. Remove manhole lid to expose liquid surface of the Aqua-Swirl[®].
- 2. Remove floatable debris with basket or net if any present.
- 3. If oil is present, measure its depth. Clean liquids from system if one half (½) inch or more oil is present.

Note: Water in Aqua-Swirl[®] can appear black and similar to oil due to the dark body of the surrounding structure. Oil may appear darker than water in the system and is usually accompanied by oil stained debris (e.g. Styrofoam, etc.). The depth of oil can be measured with an oil/water interface probe, a stadia rod with water finding paste, a coliwasa, or collect a representative sample with a jar attached to a rod.

II. Sediment Accumulation

- 1. Lower measuring device (e.g. stadia rod) into swirl chamber through service access provided until top of sediment pile is reached.
- 2. Record distance to top of sediment pile from top of standing water: ______ inches.
- 3. Maximum recommended sediment depth prior to cleanout is 14 inches for all models. Consult system shop drawing for treatment chamber depth as measured from the inlet pipe invert to base of the unit.

III. Diversion Structures (External Bypass Features)

If a diversion (external bypass) configuration is present, it should be inspected as follows:

- 1. Inspect weir or other bypass feature for structural decay or damage. Weirs are more susceptible to damage than off-set piping and should be checked to confirm that they are not crumbling (concrete or brick) or decaying (steel).
- 2. Inspect diversion structure and bypass piping for signs of structural damage or blockage from debris or sediment accumulation.
- 3. When feasible, measure elevations on diversion weir or piping to ensure it is consistent with site plan designs.
- 4. Inspect downstream (convergence) structure(s) for sign of blockage or structural failure as noted above.

CLEANING

Schedule cleaning with local vactor company or AquaShieldTM to remove sediment, oil and other floatable pollutants. The captured material generally does not require special treatment or handling for disposal. Site-specific conditions or the presence of known contaminants may necessitate that appropriate actions be taken to clean and dispose of materials captured and retained by the Aqua-Swirl[®]. All cleaning activities should be performed in accordance with property health and safety procedures.

AquaShieldTM always recommends that all materials removed from the Aqua-Swirl[®] during the maintenance process be handled and disposed in accordance with local and state environmental or other regulatory requirements.

MAINTENANCE SCHEDULE

I. During Construction

Inspect the Aqua-Swirl[®] every three (3) months and clean the system as needed. The Aqua-Swirl[®] should be inspected and cleaned at the end of construction regardless of whether it has reached its maintenance trigger.

II. First Year Post-Construction

Inspect the Aqua-Swirl[®] every three (3) months and clean the system as needed.

Inspect and clean the system once annually regardless of whether it has reached its sediment or floatable pollutant storage capacity.

III. Second and Subsequent Years Post-Construction

If the Aqua-Swirl[®] did not reach full sediment or floatable pollutant capacity in the First Year Post-Construction period, the system can be inspected and cleaned once annually.

If the Aqua-Swirl[®] reached full sediment or floatable pollutant capacity in less than 12 months in the First Year Post-Construction period, the system should be inspected once every six (6) months and cleaned as needed. The Aqua-Swirl[®] should be cleaned annually regardless of whether it reaches its sediment or floatable pollutant capacity.

IV. Bypass Structures

Bypass structures should be inspected whenever the Aqua-Swirl[®] is inspected. Maintenance should be performed on bypass structures as needed.

MAINTENANCE COMPANY INFORMATION

Company Name:			
Street Address:			
City:		State/Prov.:	Zip/Postal Code:
Contact:			Title:
Office Phone:		Cell Phone	e:
	AC'	FIVITY LOG	
Date of Cleaning:			pection should be 3 months from for first year).
Time of Cleaning:	Start:	En	d:
Date of Next Inspectio	on:		
Floatable debris preser	nt: Yes	No	
Notes:			_
Oil present: Yes Measurement r	No Oil de method and notes:		
ST	RUCTURAL CONI	DITIONS and OBS	SERVATIONS

Structural dar	nage:	Yes	No	Where:
Structural wea	ar:	Yes	No	Where:
Odors present	-• ~•	Yes	No	Describe:
Clogging:	Yes	No	Descri	be:
Other Observ	ations:			

NOTES

Additional Comments and/or Actions To Be Taken	Time Frame

ATTACHMENTS

- Attach site plan showing Aqua-Swirl[®] location.
- Attach detail drawing showing Aqua-Swirl[®] dimensions and model number.
- If a diversion configuration is used, attach details showing basic design and elevations (where feasible).

Aqua-Swirl[®]

TABULAR MAINTENANCE SCHEDULE

Date Construction Started:

Date Construction Ended:

During Construction

		Month										
Activity	1	2	3	4	5	6	7	8	9	10	11	12
Inspect and Clean as needed			Х			Х			Х			X
Inspect Bypass and maintain as needed			Х			Х			Х			X
Clean System*												X*

* The Aqua-Swirl[®] should be cleaned <u>once a year</u> regardless of whether it has reached full pollutant storage capacity. In addition, the system should be cleaned at the <u>end of construction</u> regardless of whether it has reach full pollutant storage capacity.

First Year Post-Construction

		Month										
Activity	1	2	3	4	5	6	7	8	9	10	11	12
Inspect and Clean as needed			Х			Х			Х			Х
Inspect Bypass and maintain as needed			Х			Х			Х			Х
Clean System*												X*

* The Aqua-Swirl[®] should be cleaned <u>once a year</u> regardless of whether it has reached full pollutant storage capacity.

Second and Subsequent Years Post-Construction

		Month										
Activity	1	2	3	4	5	6	7	8	9	10	11	12
Inspect and Clean as needed												X*
Inspect Bypass, maintain as needed												X*
Clean System*												X*

* If the Aqua-Swirl[®] did <u>not</u> reach full sediment or floatable pollutant capacity in the First Year Post-Construction period, the system can be inspected and cleaned once annually.

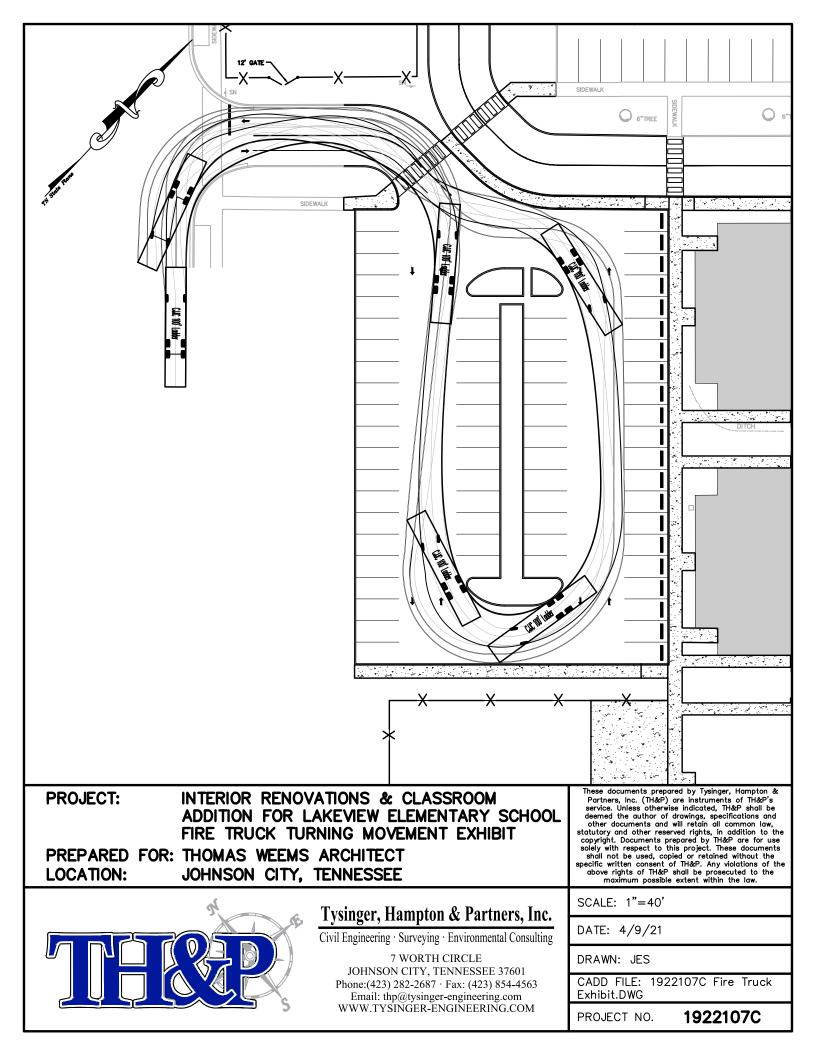
If the Aqua-Swirl[®] <u>reached</u> full sediment or floatable pollutant capacity in less than 12 months in the First Year Post-Construction period, the system should be inspected once every six (6) months or more frequently if past history warrants, and cleaned as needed. The Aqua-Swirl[®] should be cleaned annually regardless of whether it reaches its full sediment or floatable pollutant capacity.

APPENDIX G

Fire Truck Exhibit



APPENDIX



ELECTRICAL ADDENDUM INTERIOR RENOVATIONS AND CLASSROOM ADDITION FOR LAKE RIDGE ELEMENTARY SCHOOL Johnson City, Tennessee May 13, 2021

1. Refer to Electrical Drawings E101B, E101C, E101D, E103A, E201B, and E203A:

The "Existing Ceiling Work Note" on each of these drawings shall be changed to read as follows:

Due to HVAC system replacement throughout existing school, ceiling grids, ceiling tiles, etc. will have to be removed and reinstalled in order for new systems to be installed. As part of the ceiling removal/reinstallation work, electrical contract work will include removal and reinstallation of associated electrical/communications items including, but not limited to, lighting fixtures, exit signs, emergency lighting units, lighting control devices, occupancy sensors, security system motion detectors, CCTV cameras, network wireless access points (WAP), speakers, etc. Carefully coordinate all work with other trades and GC as required. Refer to architectural drawings for limits of ceiling grid and ceiling tile removal in the existing building as well as "ceiling new scope of work" notes on architectural drawings which outline contractor responsibilities to replace items if items are damaged during this process. Any additional ceiling work shown outside areas indicated on architectural drawings will be covered by allowance as noted on architectural drawings.

2. Refer to electrical drawings and project manual:

As a clarification, attention is called to the fact that electrical drawings and project manual set forth the fire alarm scope of work to include a complete new fire alarm system for the new addition. In the existing building, existing fire alarm system devices, equipment, wiring, etc. shall remain in place and be maintained in operation unless noted otherwise on drawings. Exception to this is that new duct type smoke detectors will be provided in replacement HVAC units in the existing building where noted on drawings. Further, electrical drawings and project manual require that the new FACU being provided in the new addition is to be interfaced with the existing FACU which is to remain in the existing school such that an alarm condition at either new or existing FACU will activate an evacuation alarm on all new and existing audio/visual notification appliances throughout building.

VREELAND ENGINEERS, INC.

PTA Request for Payment

Date 9 6 19 Name Laura Linn **Board Position** Zip Code Address Make Check Payable To Play ground Equipment com Item(s) bought and purpose/line item: Amount \$9005.00 Sunshade Amount Amount Amount Amount Amount Total Amount Due \$ 9005.00 ***SALES TAX CANNOT BE REIMBURSED*** For Treasurer's Use Only Date Paid_9/6/19 Check Number 3164 Account/Line Item Long Term Purchases Amount \$9005.00 Approved By: Date 9/6/19 Date 9/6/19 lingols President_ Treasurer



PlaygroundEquipment.com 1 Playground Drive Greenfield, IN 46140 Phone: (800) 667-0097 Fax: (317) 855-9247

Ship Via: Freight Request By: Morgan Quote Out: 8/27/2019

Quote #716526

Visit: PlaygroundEquipment.com for more great deals

Bill To

Ship To:

Lake Ridge School PTA Laura Linn 1001 Lake Ridge Square Johnson City, TN 37601 USA lauramlinn@gmail.com Ph: (423) 737-7788 Fax:

JC Schools Maintenance Department Renee Wood 2735 E Oakland Ave Johnson City, TN 37601 USA Ph: (423) 737-7788

Product ID	Description	Weight	Qty	Price	Amount
RD243412IG	24' x 34' Rectangular Fabric Shade, 12' Entrance Height, In- Ground Mount, WITH Glide Elbow	1,931 lbs	1	\$7,583.00	\$7,583.00
JOTE ONLY				Subtotal:	\$7,583.00
VALID FOR 30	DAYS FROM DATE OF ISSUE			Shipping:	\$1,422.00
				Tax Rate:	0%
				Sales Tax:	\$0.00
			т	otal Weight:	1,931 lbs
				Installation:	
				Total:	\$9,005.00

Notes

Ships via Freight from GA by appointment for delivery. Customer responsible for unloading shipment.

COLORS

Posts: Iced Coffee Fabric: Forest Green

Please note, this quote does not include sealed engineered drawings. You will need to contact your local building commission to see if they require sealed engineered drawings in order to pull a permit. Your building commission may deny a permit to erect the shade thout the renderings. Please make sure you know if these are required before ordering. Please note, sealed engineered drawings may change the shade to a custom unit and may affect the price of the shade as the shade will then need to be designed to the specifications of the city/state you are located in. Shades are nonreturnable and nonrefundable.

DOCUMENT 01 2519

SUBSTITUTION REQUEST FORM

-							
Т	O: PATH CONSTRUCTION		Project Name	e: LAKE	RIDGE	ELEM	SCHOOL
	ATTN:DAVID NELSON PROJECT MANAGER						
S	pecified Item: DIV 8	7	Proposed Su	hetituto:	METAT.	PPODI	CILC THO
H	OLLOW METAL DOORS AND FRAMES	-	CORBIN,			PRODU	
1.	The following are attached (Mark all that apply):						
	Complete Description		Catalog				
_	Laboratory Tests		Specifica				
2.	This substitution will have the following effects or	n dimen	sions, gauges,	weights,	etc.:		
	NONE						
3.	This substitution will have the following effects or	n wiring,	piping, ductwo	ork, etc.:			
	N/A						
4.	This substitution will have the following effects or	n other t	rades:				
	NONE						
5.	This substitution will have the following effect on	construc	tion Schedule	s:			
	NONE						
6.	The proposed substitute(s) differs from the specif	fied proc	luct(s) in qualit	y and per	formance	as follow	/s:
	QUALITY IS A+ STEEL DOOR INST	the second se					
7.	Manufacturers guarantees for the substitute(s) an	id the sp	ecified produc	t(s) are (c	check one)		
	X The Same. No Change.	Ľ	Different. I			•	
8.	Information on the availability of maintenance sen is provided on an attached sheet if applicable. Thi	vices an is attach	d replacement ment is:	materials	for propo	sed subs	stitute(s)
	X Attached.	Γ	Not Applic	able.			
9.	Names, addresses, and phone numbers of fabrica on an attached sheet if applicable. This attachmer	ntors and			substitute	(s) are p	rovided
[X Attached.		Not Applica	able.			
TWA	x #019-030 01	2519-1			Substitut	tion Requ	uest Form

10. If the cost substitution is accepted, it will result in	1:		
X No Cost Impact.	Cost Increase	\$	
	Cost Decrease	\$	
11. License fees or royalties are pending on the prop	posed substitution:		
X No.	Yes. Describe Be	elow.	

12. The undersigned or the firm represented shall pay for additional studies, investigations, submittals, redesign, and analysis by the Designer necessitated by this substitution request.

Substitutions must be requested in accordance with applicable Contract requirements. After bidding, substitutions are to be submitted only by Contractor. Substitute products should not be ordered or installed without written acceptance.

Date:	MAY 5, 2021							
Submitted By:	JERRY D. MCCLANAHAN							
Signature:	lour D. Melantra							
Firm Name:	APPALACHIAN COMMERCIAL PRODUCTS, LLC							
Address:	250 BIRCH STREET SUITE "B"							
	BLOUNTVILLE, TN 37617							
Telephone:	423-323-2952							
Designer's Revie	w Comments:							
Accepted.	Rejected.							
APPROVED	Rejected – Received Too Late.							
	:04 pm, May 17, 2021 Rejected – Submittal Incomplete.							
Additional Comm	ents:							
For The Designer								
Date:								
- Name:								
- Signature:								
	END OF SECTION							
	END OF SECTION							



Standards As Tough As Steel.™

30200 Detroit Road, Cleveland, OH 44145 Phone: 440 899-0010 Fax: 440 892-1404 www.steeldoor.org

January 19, 2021

1

Mr. David McConnell MPI 319 North Hills Road Corbin, KY 40701

Dear David.

This letter acknowledges receipt of MPI's certification criteria as outlined by the SDI Technical Committee for 2021:

ASTM A1008-2016 Standard Specification for Steel, Sheet, Cold-Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low-Alloy with Improved Formability, Solution Hardened, and Bake Hardenable ASTM A653-2015e1 Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process ANSI/SDI A250.4-11 Test Procedure and Acceptance Criteria for — Physical Endurance for Steel Doors, Frames and ANSI/SDI A250.10-11 Test Procedure and Acceptance Criteria for Prime Painted Steel Surfaces for Steel

Upon review of the material provided, the Steel Door Institute has determined that MPI is in compliance with the certification requirements and is therefore approved to continue to utilize "SDI Certified" branding.

Thank you for your participation in SDI's 2021 Certification program.

Very truly yours,

J. J. Wherry **Managing Director**

Ceco Milan, TN

Curries Mason City, IA

Mesker Huntsville, TN

> Republic McKenzie, TN

Deansteel San Antonio, TX

MPI Corbin, KY

SMP Culver City, CA

DCI Fontana, CA

Pioneer Carlstadt, NI

Steelcraft Cincinnati, OH

HMX Phoenix, AZ

Premier Monroe, LA



National Association of Architectural Metal Manufacturers

Architectural Metal Products Division Expanded Metal Manufacturers Association Division

Hollow Metal Manufacturers Association Division Metal Bar Grating Division

January 11, 2019

To Whom It May Concern:

This will confirm that *The MPI Group LLC* is a member in good standing of the Hollow Metal Manufacturers Association (HMMA) Division of NAAMM, the National Association of Architectural Metal Manufacturers and is entitled to all rights and privileges of membership.

As such, *The MPI Group LLC* endorses the technical product standards and specifications published by the HMMA Division.

Sincerely,

Jeff Church Executive Vice President

800 Roosevelt Rd. • Bldg. C-312 • Glen Ellyn, IL 60137 630/942-6591 • FAX: 630/790-3095 E-Mail: info@naamm.org • http://www.naamm.org



February 25, 2019

RE: ANSI 250.4 Physical Endurance Levels of Doors & Frames and ANSI 250.8 SDI Classifications

The MPI Group has successfully tested both of our door product lines to the ANSI 250.4 standards at Intertek Testing Labs and the results are as follows.

Polystyrene Doors (#3195456MID-002): tested a 18 gauge door to 1 million cycles, with 0.818" max deflection with 300lb load applied, and 0.044" permanent deflection after load released to successfully pass Performance Level A using a Level 2 (18 gauge door) with no restriction of Model seam.

Steel Stiffen Doors (#WHI-495-SP-0631): tested a 16 gauge door to 4 million cycles (3 million beyond the test standard). At the highest testing standard of 1 million cycle testing, max deflection of 0.210" with 300lb load applied and 0.010" permanent deflection after the load is removed to successfully pass Performance Level A using a Level 3 (16 gauge door) with no restriction of Model seam. As indicated above the testing continued after recording the 1 million cycle until they reached 4 million cycles and they documented max deflection of 0.354" with 300lb load applied and 0.011" permanent deflection after removing the load, due to the testing standard only coving to Performance Level A this part of the test didn't yield any additional performance levels and was for informational purposes only.

ANSI 250.8 Section 2.04 (not a testing standard)

Level 1 - 0.032" (20 gauge) Door Level 2 - 0.042" (18 gauge) Door Level 3 - 0.056" (16 gauge) Door Level 4 - 0.067" (14 gauge) Door

ANSI 250.8 Section 2.04 (not a testing standard) Model 1 – Open Seam Door Edge Model 2 – Seamless Door Edge

ANSI 250.4 Section 5 (cycle testing), 6 (twist testing), & 7 (acceptance criteria) Performance Level A – Max Deflection of 1 ¼" at 300lb load and 1 million cycles

Performance Level B – Max Deflection of 1 ¼" at 300lb load and 500 thousand cycles Performance Level C – Max Deflection of 2 ½" at 300lb load and 250 thousand cycles *Max 1/8 permanent deflection for any performance level once the load is removed (ANSI 250.4 Section 7.1.8)

Please contact us if additional information is needed.

Very truly yours,

- Mc Conell

David McConnell Products and Services Manager

DOCUMENT 01 2519

SUBSTITUTION REQUEST FORM

To: Tom Weems	Project Name: Lake Ridge Elementary
Thomas Weems Architect 3203 Hanover Road Johnson City, TN 37604	1001 Lake Ridge Square Johnson City, TN 37601
Specified Item: Kawneer	Proposed Substitute: Quaker Windows
AA 6400 Fixed and Project-Out	M600 Fixed and Project-Out Windows
1. The following are attached (Mark all that apply):	
Complete Description	X Catalog
X Laboratory Tests	Specification Data
2. This substitution will have the following effects on	dimensions, gauges, weights, etc.:
Quaker M600 is 3 1/4" deep, compared to 4" fo	or Kawneer's AA 6400.
3. This substitution will have the following effects on	wiring, piping, ductwork, etc.:
N/A	
4. This substitution will have the following effects on	other trades:
N/A	
5. This substitution will have the following effect on c	construction Schedules:
N/A	
6. The proposed substitute(s) differs from the specifi	ed product(s) in quality and performance as follows:
Window depth dimension only.	
7. Manufacturers guarantees for the substitute(s) and	d the specified product(s) are (check one):
X The Same. No Change.	Different. Describe Below.
	vices and replacement materials for proposed substitute(s)
is provided on an attached sheet if applicable. Thi	s attachment is: KGI has access to replacement parts and can Not Applicable.form maintenance on proposed window.
X Attached.	
9. Names, addresses, and phone numbers of fabrica on an attached sheet if applicable. This attachmer	ators and suppliers for proposed substitute(s) are provided nt is:
X Attached.	Not Applicable.

10. If the cost substitution is accepted, it will result in:	X To be determined
No Cost Impact.	Cost Increase \$
	Cost Decrease \$
11. License fees or royalties are pending on the proposed	d substitution:
X No.	Yes. Describe Below.

12. The undersigned or the firm represented shall pay for additional studies, investigations, submittals, redesign, and analysis by the Designer necessitated by this substitution request.

Substitutions must be requested in accordance with applicable Contract requirements. After bidding, substitutions are to be submitted only by Contractor. Substitute products should not be ordered or installed without written acceptance.

Date:	5/10/21						
Submitted By:	By: Neil Johnson						
Signature:							
Firm Name:	Keller Glasco						
Address:	2711 East Oakland Avenue						
	Johnson City, TN 37601						
Telephone: 423-282-1210, extension 111							
Designer's Review Comments:							
Accepted.	Accepted. Kejected.						
DEVIEWED	Rejected – Received Too Late.						
REVIEWED Image: Constrained and the second and the							
Additional Comm	ents:						
Not an equal	product.						
For The Designer	r.						
Date:							
Name:							
Signature:							
END OF SECTION							

DOCUMENT 01 2519

SUBSTITUTION REQUEST FORM

To:	TOM WEENS	Project Nar	ne: LAKE RIDGE ES	
Sn	ecified Item:	Proposed	Substitute:	
	SEE ATTACITED RES		ATTACITED RFS	
1. ⁻	The following are attached (Mark all that apply):	Catalo	g	
	Laboratory Tests		ication Data	
2.	This substitution will have the following effects of	on dimensions, gaug	es, weights, etc.:	
	NONE			
З.	This substitution will have the following effects of	on wiring, piping, du	ctwork, etc.:	
	NONE			
4.	This substitution will have the following effects of	on other trades:		
	NONE			
5.	This substitution will have the following effect on construction Schedules:			
	NONE			
6.	The proposed substitute(s) differs from the spe	cified product(s) in c	uality and performance as follows:	
	SUBSTITUED PRODUCTS	SANE EQU	IAL ON BETTER	
7.	Manufacturers guarantees for the substitute(s)			
	The Same. No Change.		ent. Describe Below.	
	SEE ATTACHED RF	S + PATA	PAGES	
8.	Information on the availability of maintenance s is provided on an attached sheet if applicable.	services and replace	ement materials for proposed substitute(s)	
	Attached.		Applicable.	
9.	Names, addresses, and phone numbers of fab on an attached sheet if applicable. This attach	pricators and supplie ment is:	rs for proposed substitute(s) are provided	
	Attached.		Applicable.	
-	MA #010 020	01 2519-1	Substitution Request Forn	

10. If the cost substitution is accepted, it will result in:

1

No Cost Impact.	Cost Increase	\$					
	Cost Decrease	\$ <u>73</u> D					
 License fees or royalties are pending on the proposed substitution: 							
No.	Yes. Describe B	elow.					

12. The undersigned or the firm represented shall pay for additional studies, investigations, submittals, redesign, and analysis by the Designer necessitated by this substitution request.

Substitutions must be requested in accordance with applicable Contract requirements. After bidding, substitutions are to be submitted only by Contractor. Substitute products should not be ordered or installed without written acceptance.

Date:	MAY 10, 20	21					
Submitted By:	PAUL HOTOVE	С.					
Signature:	Jane 7 That						
Firm Name:	CERTAINTEED ARCHITECTURAL						
Address:	20 MOONES ROAD						
	MALVERN, PF	7 19355					
Telephone:	615-947-65	02					
Designer's Revi	ew Comments:						
Accepted.		Rejected.					
APPROVED		Rejected – Rec	ceived Too Late.				
	5:02 pm, May 17, 2021	Rejected – Sut	omittal Incomplete.				
Additional Com	ments:						
For The Design	er:						
Date:							
Name:							
Signature:							
		END OF SECTION					
TIN/A #040 020		01 0510 0	Substitution Deguest Form				