

# ENGINEERING & TESTING SOLUTIONS, LLC

June 28, 2016

Civil & Environmental Consultants, Inc. (CEC)  
Attn: Mr. Guy Wantiez, P.E.  
308 Cates Street  
Maryville, Tennessee 37801

Subject: Interim Report of Subsurface Exploration  
**Five Points Phase II**  
Knoxville, Tennessee  
ETS Project Number 16-112

Dear Mr. Wantiez:

This reports represents our Interim Report of Subsurface Exploration for Five Points Phase II in Knoxville, Tennessee. Our services were performed in general accordance with the terms and conditions in our Proposal 16-28 dated June 16, 2016.

The purpose of this interim report was to determine general subsurface conditions as they are related to engineering properties and submit our preliminary data to aid in the bidding process for the proposed roadways and infrastructure. This report presents our prelim findings of subsurface conditions. Our formal report will be issued upon completion of drilling services and laboratory test results.

An assessment of site environmental conditions was beyond the scope of our services.

## Project Description

The proposed development is located at the intersection of Martin Luther King Jr. Avenue and McConnell Street. The site is bound on the north by South Olive Street, on the east by Bethel Avenue, and on the west by Martin Luther King Avenue. The property is generally grass covered with some existing area drains throughout. Elevations range from about 925 feet to 950 feet across the site.

This interim report is regarding new roadway infrastructure that will be constructed. The roadway will connect South Olive Street with McConnell Street just west of the existing housing development. There will also be new connector road from S. Olive Street and Ben Hur Avenue.



View of proposed development area

### Geological Conditions

The project site lies in the eastern portion of the Valley and Ridge Province. Review of the Knoxville Geologic Quadrangle indicates this site is geologically mapped to be underlain by Ottosee Shale. Ottosee Shale is a mixture of shale, limestone, some siltstone, and sandstone, and marble. Within a limited area, any one of these lithologic types may dominate a particular section. In the Knoxville Quadrangle, the Ottosee is mainly shale.

### Subsurface Conditions

A total of twelve soil test borings were requested for the proposed roadways and infrastructure. Due to utility conflicts, soil test boring B-7 was eliminated from our scope.

Subsurface conditions encountered at the boring locations are shown on the Soil Test Boring Records in Appendix B. The Soil Test Boring Records represent our interpretation of the subsurface conditions based on the field logs and visual examination of the field samples by our engineer. The lines designating the interfaces between various strata on the Soil Test Boring Records represent the approximate interface locations. The elevations listed should be considered approximate, as they were obtained by superimposing our boring locations on the provided preliminary site plan.

All borings were extended to their predetermined termination depths except boring B-12. Boring B-12 encountered auger refusal at a depth of about 16 feet.

Our soil test borings encountered fill soils and residual soils. Fill soils are soils that have been transported to their current location by man. Residual soils are soils that have developed from the in place weathering of the parent bedrock.

Fill soils generally consisting of a tan brown silty clay with rock fragments and occasional cinders/organics was encountered in a majority of our initial soil test borings. The depth of fill varied from about 2 feet to 3.5 feet. No documentation regarding placement of the fill soils has been provided. Standard Penetration Test (SPT) N values for the fill soils ranged from 5 to 16 blows per foot (bpf) indicating firm to very stiff consistencies. Moisture contents for the fill ranged from 14.6 percent to 28.1 percent.

Residual soils were penetrated in all soil test borings. The residual soils generally consisted of an orange brown silty/shaley clay. N values for the residuum ranged from 5 to 20 indicating firm to very stiff consistencies. Natural moisture contents for the residual soils varied from 18.9 percent to 34.3 percent.

Groundwater was not observed within our test borings at the time of drilling. We note, the borings were backfilled upon completion of drilling activities and therefore, long-term readings were not obtained.

Limitations and basis of recommendations

This interim report has been prepared in accordance with generally accepted geotechnical engineering practice for specific application to this project. Our findings are based upon standards of our practice in this area at the time this report is prepared.

Regardless of the thoroughness of a geotechnical exploration, there is always a possibility that conditions between test borings will differ from those at specific test boring locations and that conditions will not be as anticipated by the designers or contractors. In addition, the construction process may itself alter soil conditions. Therefore, experienced geotechnical personnel should observe and document the construction procedures used and the conditions encountered. Unanticipated conditions and inappropriate procedures will be reported to the design team, along with timely recommendations to solve the problems created. We recommend the owner retain ETS to provide this service, based upon our familiarity with the subsurface conditions, the project design and the intent of the recommendations.

We appreciate the opportunity to perform these services and are available to discuss any questions concerning this interim report. As previously mentioned, our final report will be issued upon completion of field work and laboratory testing.

Sincerely,  
ENGINEERING & TESTING SOLUTIONS, LLC



Trenton D. Smith, E.I.  
Staff Professional



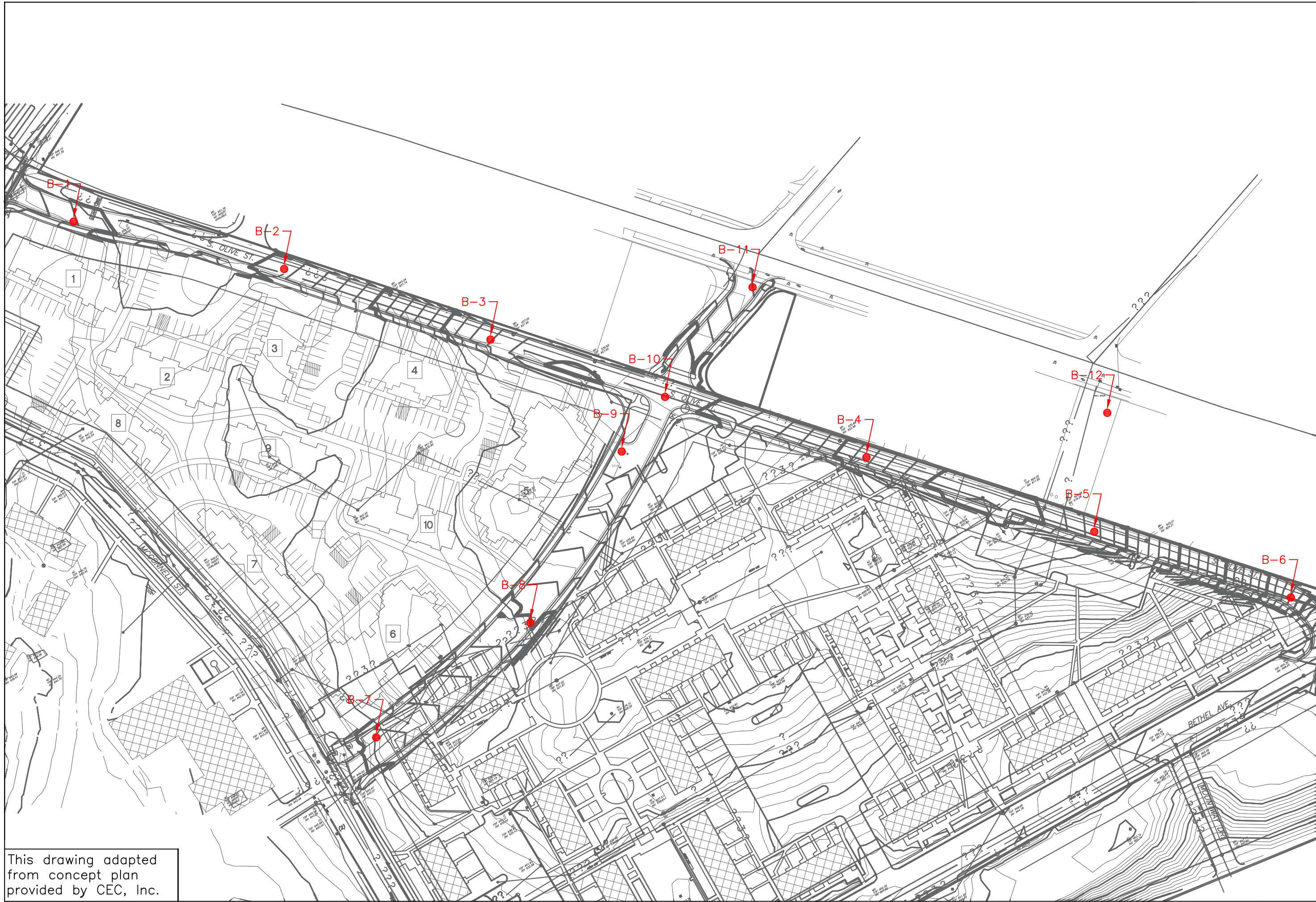
Chad B. Smock, P.E.  
Principal

Attachments

- Boring Location Plan Interim
- Soil Test Boring Records (B-1 through B-12)

**BORING LOCATION PLAN**





This drawing adapted from concept plan provided by CEC, Inc.

**BORING LOCATION PLAN INTERIM FIVE POINTS PHASE II KNOXVILLE, TENNESSEE ETS PROJECT 16-112**



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●
<b>BORING LOCATION</b>
<b>DRAWN</b> TDS
<b>CHECKED</b> CBS
<b>DATE</b> 6/24/16
<b>SCALE</b> 1"=100' ON 11X17
<b>JOB NO.</b> 16-112

**BLP**

**FIELD EXPLORATORY PROCEDURES  
BORING KEY SHEET  
BORING LOGS**

## FIELD EXPLORATORY PROCEDURES

### Soil Test Boring (Hollow Stem)

All boring and sampling operations were conducted in general accordance with ASTM D 1586. The borings were advanced by mechanically twisting continuous steel hollow-stem auger flights into the ground. At regular intervals, soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-tube sampler. The sampler was first seated 6 inches to penetrate any loose cuttings and then driven an additional foot with blows of a 140-pound hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot of penetration was recorded and is designated the "standard penetration resistance (SPT)." Proper evaluation of the penetration resistance provides an index to the soil's strength, density, and ability to support foundations.

Representative portions of the soil samples obtained from the split-tube sampler were sealed in bags and transported to our laboratory, where they were examined by our engineer to verify the driller's field classifications. Test Boring Records are attached, graphically showing the soil descriptions and penetration resistances.







# BORING LOG KEY SHEET

## LEGEND TO SOIL AND ROCK SYMBOLS

<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 20px; text-align: center;"></td> <td>TOPSOIL</td> </tr> <tr> <td style="text-align: center;"></td> <td>SILTSTONE</td> </tr> <tr> <td style="text-align: center;"></td> <td>BEDROCK</td> </tr> <tr> <td style="text-align: center;"></td> <td>DOLOMITE</td> </tr> <tr> <td style="text-align: center;"></td> <td>LIMESTONE</td> </tr> </table>		TOPSOIL		SILTSTONE		BEDROCK		DOLOMITE		LIMESTONE	<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 20px; text-align: center;"></td> <td>ASPHALT / GRAVEL</td> </tr> <tr> <td style="text-align: center;"></td> <td>ALLUVIAL</td> </tr> <tr> <td style="text-align: center;"></td> <td>RESIDUAL</td> </tr> <tr> <td style="text-align: center;"></td> <td>COLLUVIAL</td> </tr> <tr> <td style="text-align: center;"></td> <td>FILL</td> </tr> </table>		ASPHALT / GRAVEL		ALLUVIAL		RESIDUAL		COLLUVIAL		FILL
	TOPSOIL																				
	SILTSTONE																				
	BEDROCK																				
	DOLOMITE																				
	LIMESTONE																				
	ASPHALT / GRAVEL																				
	ALLUVIAL																				
	RESIDUAL																				
	COLLUVIAL																				
	FILL																				

## DRILLING SYMBOLS

	water table @ time of drilling		split spoon sample
	water table, 24 hours		undisturbed sample

Correlation of Penetration Resistance to Consistency and Relative density			
Silts & Clays		Sands & Gravels	
N Value	consistency	N Value	relative density
0-2	very soft	0-4	very loose
3-4	soft	5-10	loose
5-8	firm	11-30	firm
9-15	stiff	31-50	dense
16-30	very stiff	50+	very dense
31-50	hard		
50+	very hard		

Particle Size Identification	
Silts & Clays	Less than 0.075 mm
Fine Sand	0.075 mm to 0.425 mm
Medium Sand	0.425 mm to 2.00 mm
Coarse Sand	2.00 mm to 4.75 mm
Fine Gravel	4.75 mm to 19.0 mm
Coarse Gravel	19.0 mm to 75 mm
Cobbles	75 mm to 300 mm
Boulders	Greater than 300 mm

# ENGINEERING AND TESTING SOLUTIONS

D E P T H	<b>SOIL DESCRIPTION AND REMARKS</b> <small>SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS</small>	L E G E N D	E L E V	S A M P L E S				S P T (bpf)						L a b R e s u l t s				
				T Y P E	B L O W S				5	10	15	20	25	30	%	M	LL	PI
					1st 6"	2nd 6"	3rd 6"	N VALUE										
0	Asphalt / Stone		949															
5	Stiff tan brown silty/shaley clay - Residual		944	spt	6	5	9	14									30.9	
5	Boring Terminated @ 5.5 ft		944	spt	4	6	6	12									34.3	
10			939															
15			934															
20			929															
25			924															
30			919															
Remarks:				<b>SOIL TEST BORING RECORD</b>														
Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.				PROJECT: Five-Points Phase II														
This is an interpretation of subsurface conditions at the boring location. Subsurface conditions may differ at other times and other locations. The interfaces shown between strata are approximate. Transitions between strata may be gradual.				DRILLED: 6/23/2016 BORING: <b>B-1</b>														
				ETS PROJECT NO. 16-112														



# ENGINEERING AND TESTING SOLUTIONS

D E P T H	<b>SOIL DESCRIPTION AND REMARKS</b> <small>SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS</small>	L E G E N D	E L E V	S A M P L E S				S P T (bpf)						L a b R e s u l t s				
				T Y P E	B L O W S				5	10	15	20	25	30	%	M	LL	PI
					1st 6"	2nd 6"	3rd 6"	N VALUE										
0	Asphalt / Stone		937															
	Slightly stiff black clay with some cinders and trace of organics - Fill			spt	5	5	6	11					19.2					
5	Stiff orange brown silty clay - Residual		932	spt	4	5	7	12					29.8					
	Boring Terminated @ 5.5 ft																	
10			927															
15			922															
20			917															
25			912															
30			907															
Remarks:				<b>SOIL TEST BORING RECORD</b>														
Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.				PROJECT: Five-Points Phase II														
This is an interpretation of subsurface conditions at the boring location. Subsurface conditions may differ at other times and other locations. The interfaces shown between strata are approximate. Transitions between strata may be gradual.				DRILLED: 6/23/2016 BORING: <b>B-3</b>														
				ETS PROJECT NO. 16-112														









# ENGINEERING AND TESTING SOLUTIONS

D E P T H	SOIL DESCRIPTION AND REMARKS  SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS	L E G E N D	E L E V	SAMPLES				SPT (bpf)						Lab Results				
				T Y P E	BLOWS			5	10	15	20	25	30	M	LL	PI		
					1st 6"	2nd 6"	3rd 6"										N VALUE	
0	Note: Boring B-7 omitted from drilling scope due to utility conflict.		944															
5			939															
10			934															
15			929															
20			924															
25			919															
30			914															
Remarks:				<b>SOIL TEST BORING RECORD</b>														
Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.				PROJECT: <span style="float: right;">Five-Points Phase II</span> DRILLED: <span style="margin-left: 100px;">6/23/2016</span> <span style="margin-left: 100px;">BORING:</span> <span style="float: right;"><b>B-7</b></span> ETS PROJECT NO. <span style="float: right;">16-112</span>														
This is an interpretation of subsurface conditions at the boring location. Subsurface conditions may differ at other times and other locations. The interfaces shown between strata are approximate. Transitions between strata may be gradual.																		

# ENGINEERING AND TESTING SOLUTIONS

D E P T H	SOIL DESCRIPTION AND REMARKS <small>SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS</small>	L E G E N D	E L E V	SAMPLES				SPT (bpf)						Lab Results				
				T Y P E	BLOWS			N V A L U E	5	10	15	20	25	30	%	M	LL	PI
					1st 6"	2nd 6"	3rd 6"											
0	Asphalt / Stone		939															
	Stiff tan brown silty/sandy clay with traces of cinders - Fill			spt	9	8	8	16									14.6	
5	Stiff orange brown silty clay - Residual		934	spt	5	7	8	15									29.1	
	Boring Terminated @ 5.5 ft																	
10			929															
15			924															
20			919															
25			914															
30			909															
Remarks:				<b>SOIL TEST BORING RECORD</b>														
Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.				PROJECT: Five-Points Phase II														
This is an interpretation of subsurface conditions at the boring location. Subsurface conditions may differ at other times and other locations. The interfaces shown between strata are approximate. Transitions between strata may be gradual.				DRILLED: 6/23/2016 BORING: <b>B-8</b>														
				ETS PROJECT NO. 16-112														

# ENGINEERING AND TESTING SOLUTIONS

DEPTH	SOIL DESCRIPTION AND REMARKS <small>SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS</small>	LEGEND	ELEV	SAMPLES				SPT (bpf)						Lab Results				
				TYPE	BLOWS			N VALUE	5	10	15	20	25	30	%	M	LL	PI
					1st 6"	2nd 6"	3rd 6"											
0	grass / topsoil		937															
	Stiff orange brown silty clay - Fill			spt	6	7	9	16										
	Very stiff orange brown to tan brown shaley clay - Residual			spt	7	9	11	20										
5	Boring Terminated @ 5.5 ft		932															
10			927															
15			922															
20			917															
25			912															
30			907															

Remarks:

Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.

This is an interpretation of subsurface conditions at the boring location. Subsurface conditions may differ at other times and other locations. The interfaces shown between strata are approximate. Transitions between strata may be gradual.

## SOIL TEST BORING RECORD

PROJECT: Five-Points Phase II

DRILLED: 6/23/2016 BORING: **B-9**

ETS PROJECT NO. 16-112







# ENGINEERING AND TESTING SOLUTIONS

D E P T H	SOIL DESCRIPTION AND REMARKS <small>SEE KEY SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS</small>	L E G E N D	D E P T H	SAMPLES				SPT (bpf)						Lab Results			
				T Y P E	BLOWS				5	10	15	20	25	30	M	LL	PI
					1st 6"	2nd 6"	3rd 6"	N VALUE									
0	Grass / rootzone		0														
	Stiff grey brown silty clay with organics - Fill			spt	5	5	6	11								18.2	
5	Firm tan brown silty clay with shale - Possible residuum		-5	spt	3	2	3	5								22.2	
				spt	3	3	5	8								18.9	
10	Stiff tan brown and grey brown silty clay with completely weathered shale - Residual		-10	spt	4	4	7	11								24.9	
15	Auger Refusal @ 16 ft		-15	spt	6	21	50+	50+								28.3	
20	Note: No topographic information was available near B-12		-20														
25			-25														
30			-30														
Remarks:				<b>SOIL TEST BORING RECORD</b>													
Groundwater was not encountered at the time of drilling. Borings were backfilled after drilling. Therefore, long term groundwater depth was not measured.				PROJECT: Five-Points Phase II													
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				ETS PROJECT NO. 16-112													