

## **ADDENDUM NO. I**

DATE: June 29, 2017  
TO: All Proposers  
FROM: Janice McClelland, Assistant Purchasing Agent  
SUBJECT: Addendum No. 1 – Hockey Netting System  
BIDS TO BE OPENED: July 13, 2017, at 11:00:00 a.m.

This addendum becomes a part of the Contract Documents and modifies the original specifications as noted.

### **Item I. Information Shared at the Pre-Proposal Conference Held June 27, 2017**

#### **Submission Information**

Proposals must include the weight per pulley, as well as the total weight of the netting.

#### **Design Preferences**

If a winch motor is to be located on the north side of the arena, the City prefers that it be located in the crow's nest.

If a winch motor is to be located on the south side of the arena, the City prefers that it be located in the corner adjacent to Section FF.

#### **Design Requirements**

The City must be able to break down the netting system frame into sections of no more than 10 feet in length.

The design must work in conjunction with flags and theatrical banners, which hang roughly 50' from the ice floor.

#### **General Information**

The City shall be responsible for changing out the dasher mullion I-bolts where the netting clips on.

Design may incorporate the City's upstage batten.

### **Item II. Arena Drawings**

Four sets of drawings immediately follow this Addendum:

1. A structural evaluation of the roof framing over the Coliseum area was performed in 2011. The resulting 11-page report made recommendations as to rigging load tolerances. Note that the report is provided to potential proposers as a point of departure only and is not intended to obviate the need for a thorough evaluation of the present condition of the Coliseum roof framing by the awarded Contractor.
2. Drawings of east/west length of roof arches; roof framing; and beam, column, and joist schedules.
3. A dasher layout for the project site.
4. Laser measurements of the arena from floor to ceiling, measured from east to west.

**END OF ADDENDUM NO. I**

November 30, 2011

Mr. Dale Dunn  
City of Knoxville  
Knoxville Auditorium-Coliseum  
PO Box 2603  
Knoxville, TN 37901

**Re: Roof Framing Evaluation for Rigging Loads  
Knoxville Coliseum Roof Framing Evaluation and Scoreboard Relocation  
Knoxville, Tennessee  
CWE Project No. 2009145.00**

Dear Mr. Dunn,

The purpose of this letter is to convey and summarize the results of our structural evaluation of the roof framing over the coliseum area and to provide you with updated recommendations for the safe application of rigging loads for future events.

CWE conducted an initial walk-thru observation of the coliseum on Monday, June 6, 2011 with a follow-up framing observation on Friday, June 10, 2011 from the boom lift supplied and operated by Doug Simmons, Facility Operations Manager.

During our initial walk-thru, it was noted that the cantilever concrete frames which support the steel arch section of the roof had visible cracking as seen in photos 1 and 2. The exact extent of the cracking was partially masked by the painting conducted a few years ago. These cracks appear to have been present for a relatively long period of time. Coliseum personnel were not aware of their existence, and painting contractors had not brought it to their attention. These cracks appear to be fairly tight with no evidence of recent significant movement. However, given their critical location within primary framing members that possess no redundancy, we recommend that these cracks be closely monitored. We are available to assist in the development of a system and schedule for the monitoring and recording of any movements. We recommend cracks be monitored for a minimum period of a year. Readings should be recorded on a monthly cycle, after the application of rigging loads from each significant event/show, and during each significant snow occurrence. If significant movement occurs, these cracks should be further evaluated. Alternatively, CWE can provide the monitoring of cracks on an hourly rate or negotiated basis.

During our observation, we did not find any signs of steel corrosion or any permanent deformation/damage of individual structural steel framing members or connections. The recent "black-out" painting of the steel framed portion of the roof may have masked/covered-up any mild corrosion. The focus of our observation was on the primary W24x76 steel arches, the conventional steel trusses which span between the arches, and the underside of the bulb-tee purlins spanning between the trusses. During the observation, steel member sizes and orientations

were randomly verified with that shown on the original structural construction drawings. A significant deviation in the web member layout of the conventional steel trusses was observed and noted. Refer to the attached Intermediate Truss Profile sketch depicting the observed deviation. Our computer analysis model was adjusted accordingly, and based on our results, it is our opinion that the actual layout does not adversely affect the intended structural performance of these trusses.

As previously noted, a computer model of the steel framing system was generated to aid in the analysis of multiple scenarios for applied rigging loads and their effects on the structural system. A few screenshots of the analytical computer model have been included for your reference. During our review of the structural construction documents, we were unable to confirm the required material specification used for the design and construction of the steel portion of the roof framing system. Therefore, in our analysis model we have assumed the ultimate and yield strengths of the steel members to be 60,000 psi and 33,000 psi, respectively. This assumption was based on the wide use of material specification ASTM A7 for structural steel buildings from the late 1930's until the early 1960's. We also conducted a quick review of the critical section of the concrete frames, located in the cantilever roof beam at the face of the concrete column, supporting each side of the arches.

It is our opinion that the 4" diameter steel pin connections at each end of the steel arches are the limiting component for the entire system. Using the steel strength assumption, it appears these pins do not have significant reserve capacity beyond what is required to safely support the required load combinations of dead loads, roof live loads, wind loads, and snow loads. Therefore, we concur with the general rigging load restriction shown on the previous long used rigging guidelines (Refer to Attachment #1). This general restriction stated that suspended rigging loads from the roof structure shall not be concurrent with snow, heavy rain, or high wind events which may produce additional loads on the roof framing system.

As a starting point for our rigging load evaluation, we used information provided by a rigging contractor commonly used by the facility and Attachment #1. Based on multiple trial analyses using different rigging load configurations, we have provided updated guidelines for the safe application of rigging loads. Please refer to the Arch Loading Profiles noted for a depiction of these guidelines.

- A single suspended load of up to 2500lb applied directly to the arch at one small truss location.
  - "Arch Loading Profile – A".
- Four or five suspended loads of no more than 1500lb applied directly to the arch at every fourth small truss location.
  - "Arch Loading Profile – B" OR
  - "Arch Loading Profile – C".
- Multiple suspended loads of no more than 800lb applied directly to the arch at every other small truss location.
  - "Arch Loading Profile – D"

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Knoxville Coliseum Rigging Loads  
November 30, 2011

- Multiple suspended loads of no more than 400lb applied directly to the arch at every small truss location.
  - "Arch Loading Profile - E"

For the typical roof trusses, a maximum single load of 500 lb may be applied at any panel point. Do not hang loads greater than 6" away from panel points that would produce bending in the bottom chords. Refer to Roof Truss Loading Profile - F attached.

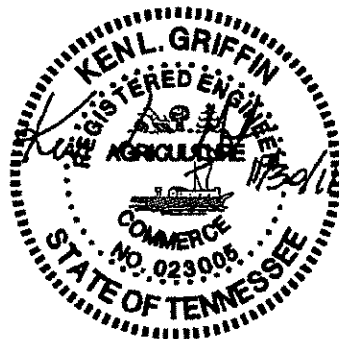
We are available for any discussion regarding these findings.

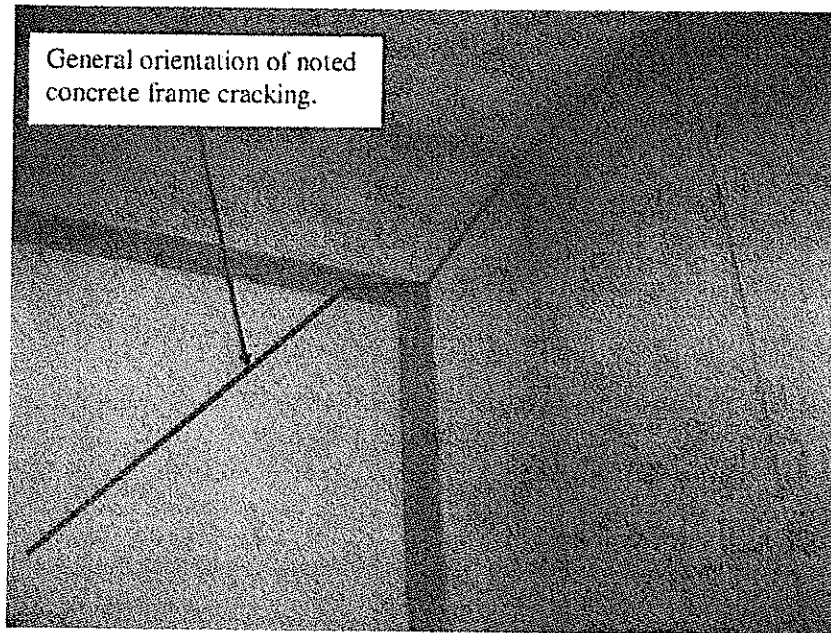
Sincerely,  
CARPENTER WRIGHT ENGINEERS, P.L.L.C.



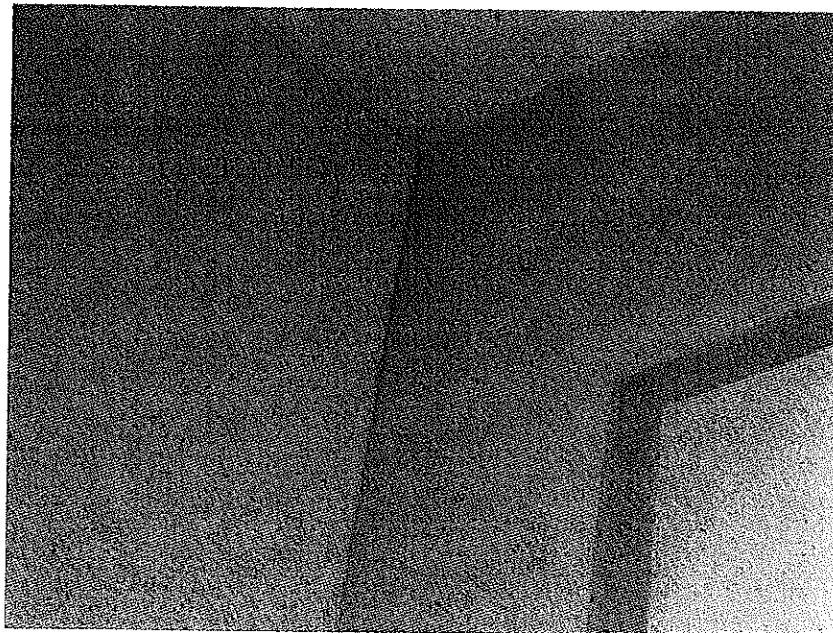
Michael R. Radcliffe, P.E.

Ken L. Griffin, P.E.  
Principal



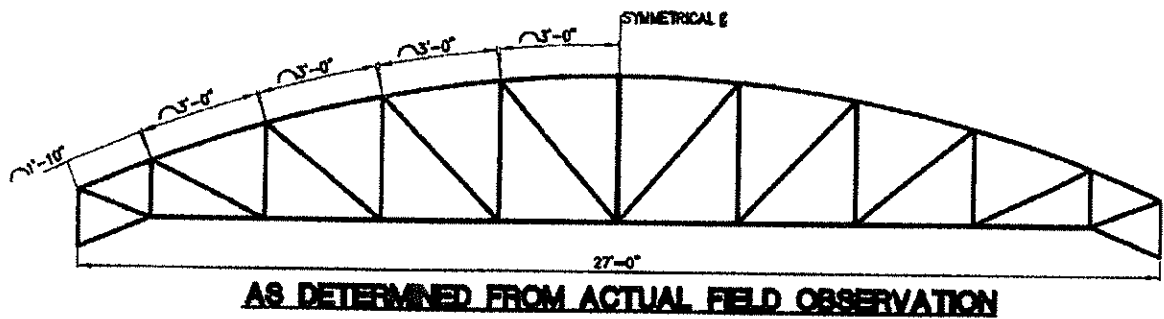
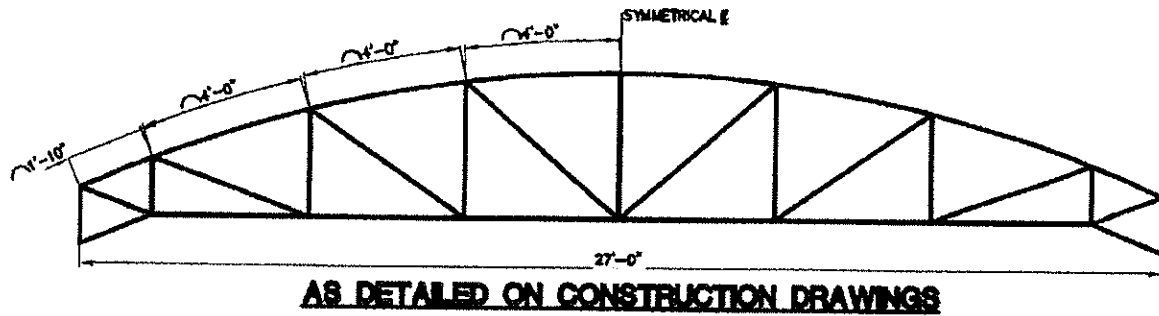


**Photo 1 – Sample Sketch of Concrete Frame Cracking at Fixed end of Cantilever  
(actual cracks have been painted over and are not visible in this photo)**

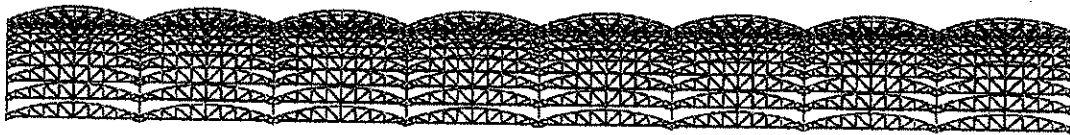


**Photo 2 – Sample of Concrete Frame Cracking at Fixed end of Cantilever**

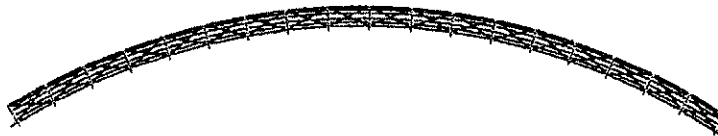
## INTERMEDIATE TRUSS PROFILE



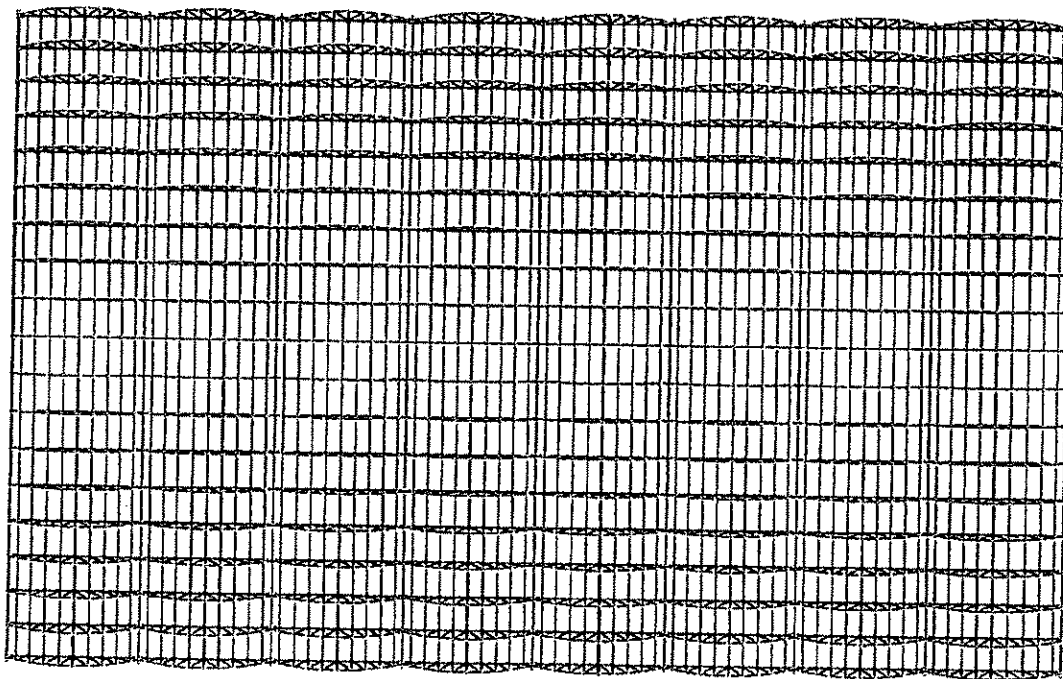
## SCREENSHOTS OF ANALYTICAL COMPUTER MODEL



**Side View of Computer Model**

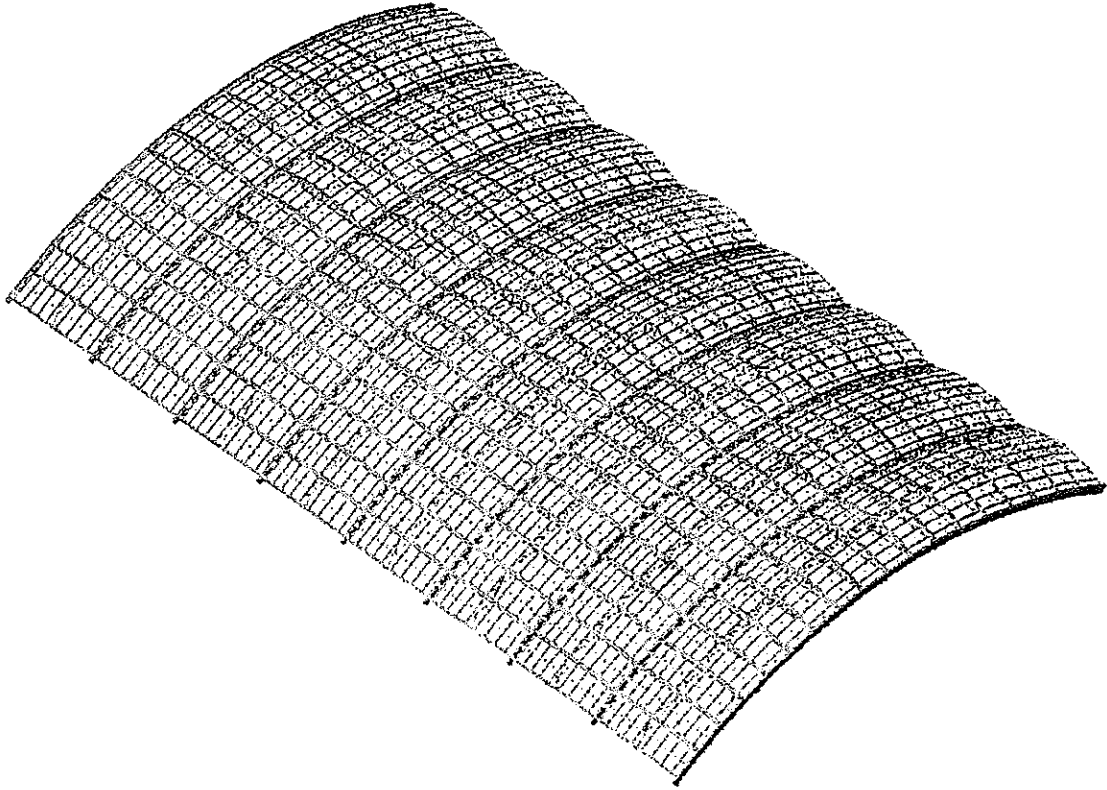


**End View of Computer Model**

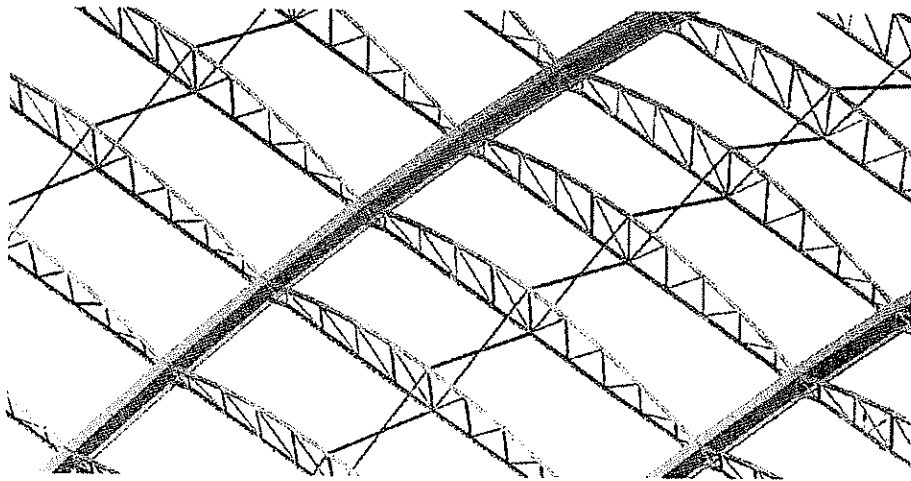


**Top View of Computer Model**



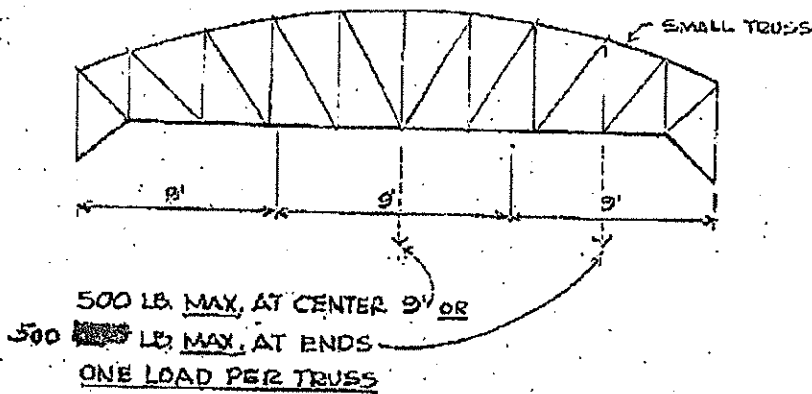
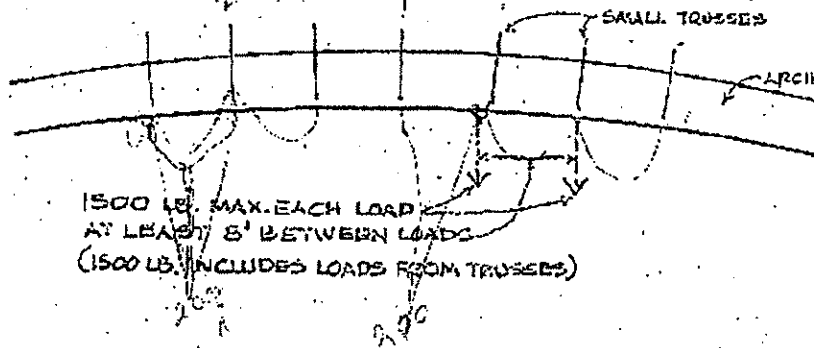


**Rendered Isometric View of Eight Bay Model**



**Rendered Isometric View Close-Up of Eight Bay Model**  
(Roof Deck & 'Bulb-T Purlins Not Shown for Clarity)

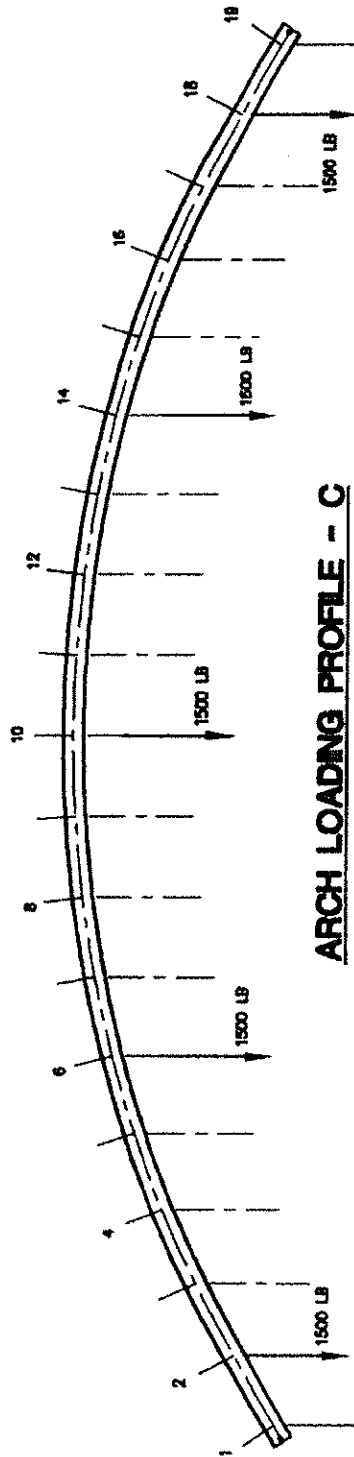
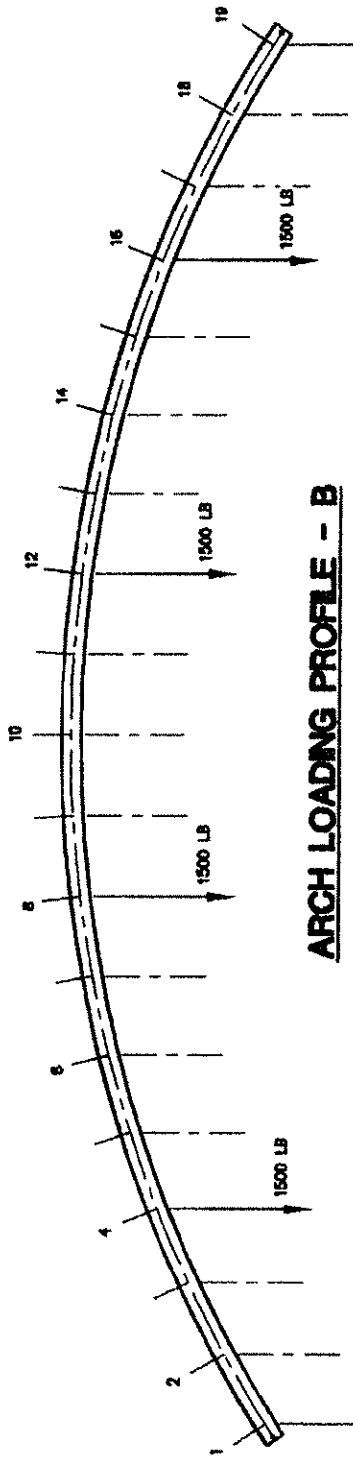
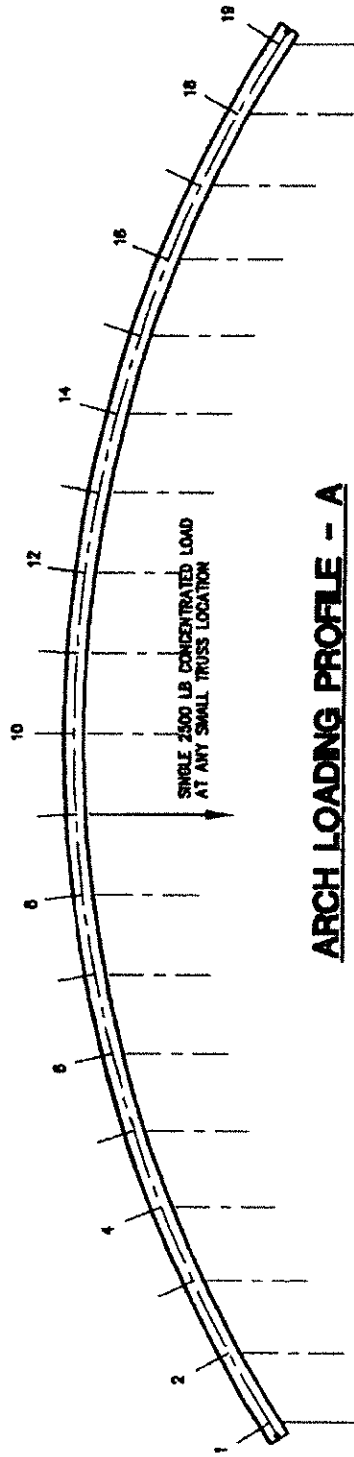
## ATTACHMENT #1 PREVIOUS RIGGING GUIDELINES



DO NOT SUSPEND LOADS FROM  
ROOF DURING SNOW, RAIN OR HIGH WIND

KNOXVILLE CIVIC COLISEUM

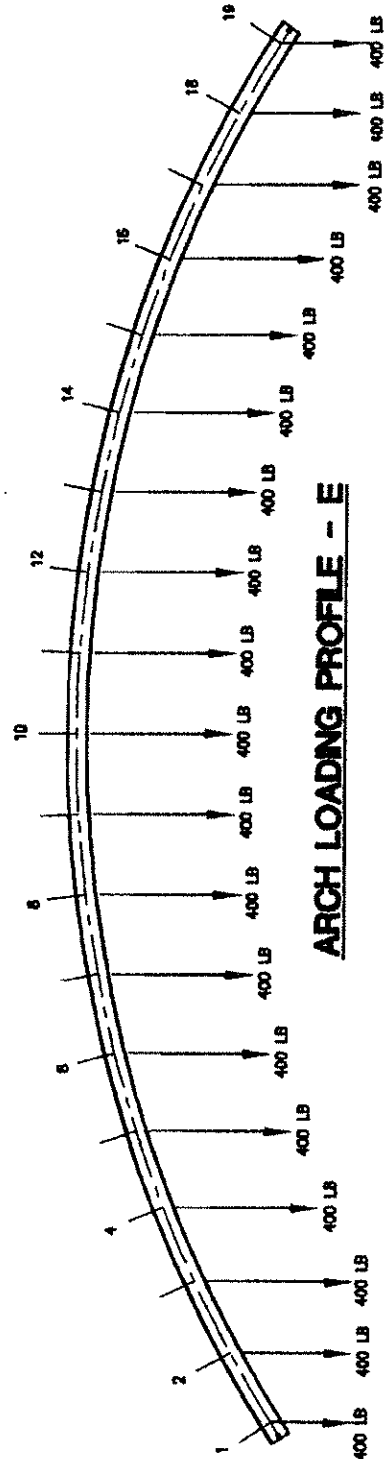
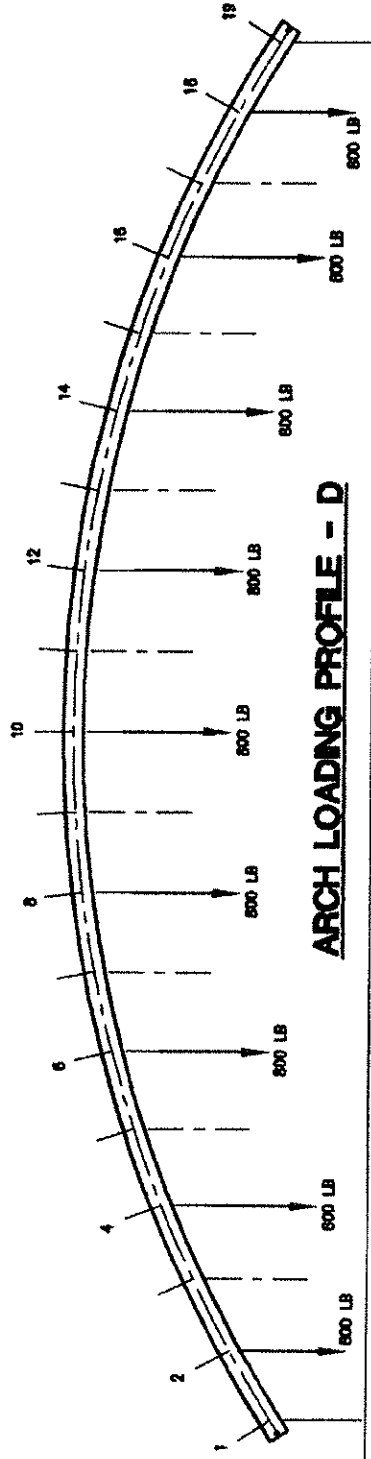
LOADS SHOWN TO INCLUDE ANY RIGGING LOADS IMPOSED ON TRUSSES.



PROJECT Knoxville Civic Coliseum NO. 2008145.00  
 COMPUTED BY MRR DATE 11/30/11 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_  
 SUBJECT Allowable Steel Arch Loading Profiles SHT 10 OF 11

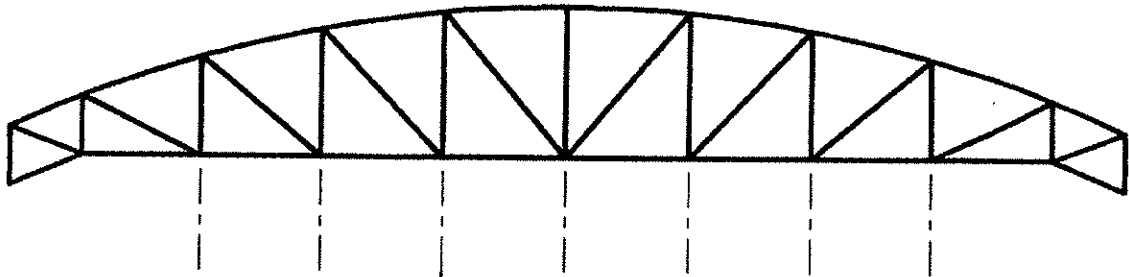
**Carpenter  
Wright  
Engineers** **CWE**  
 Structural Consultants

LOADS SHOWN TO INCLUDE ANY RIGGING LOADS IMPOSED ON TRUSSES.



PROJECT Knoxville Civic Coliseum NO. 2009145.00  
COMPUTED BY MFR DATE 11/30/11 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_  
SUBJECT Allowable Roof Truss Loading Profile SHT 11 OF 11

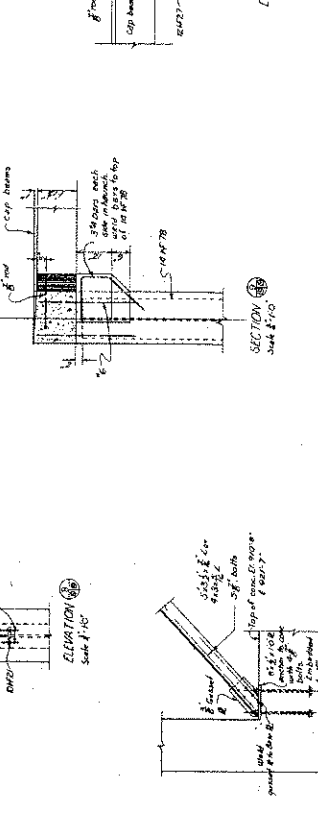
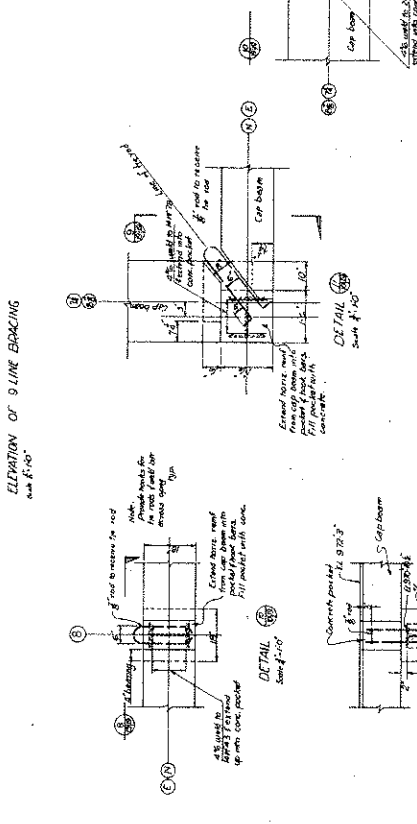
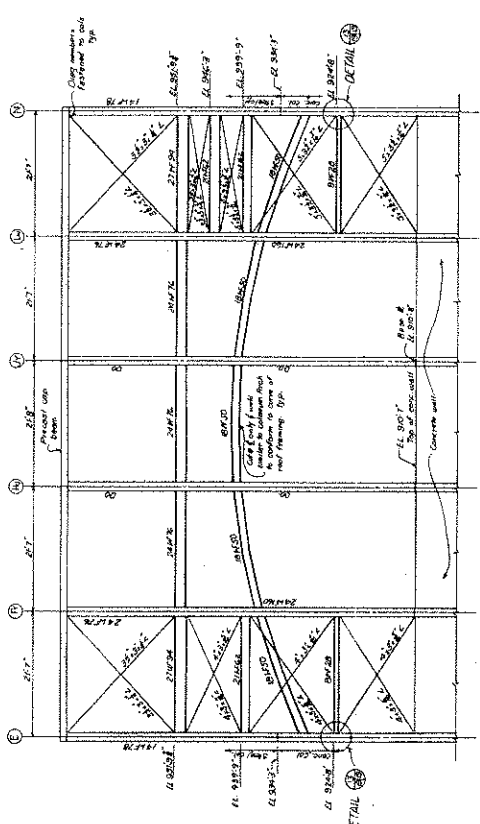
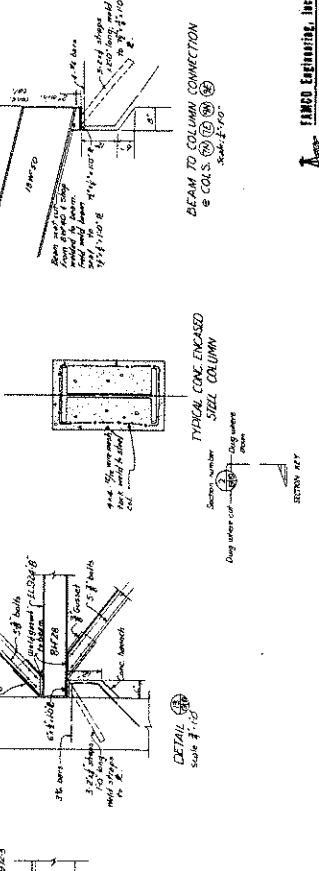
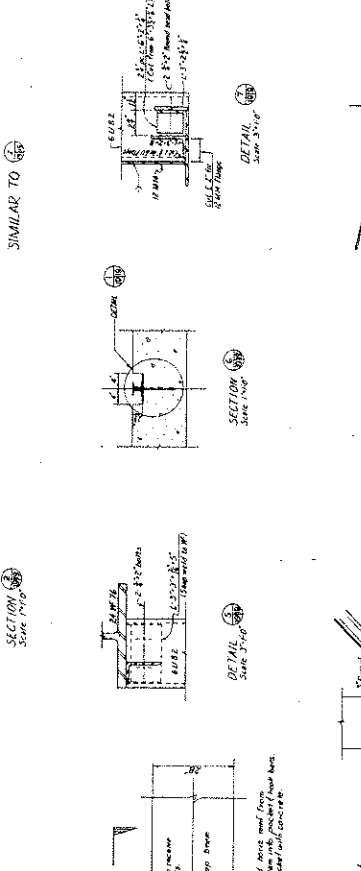
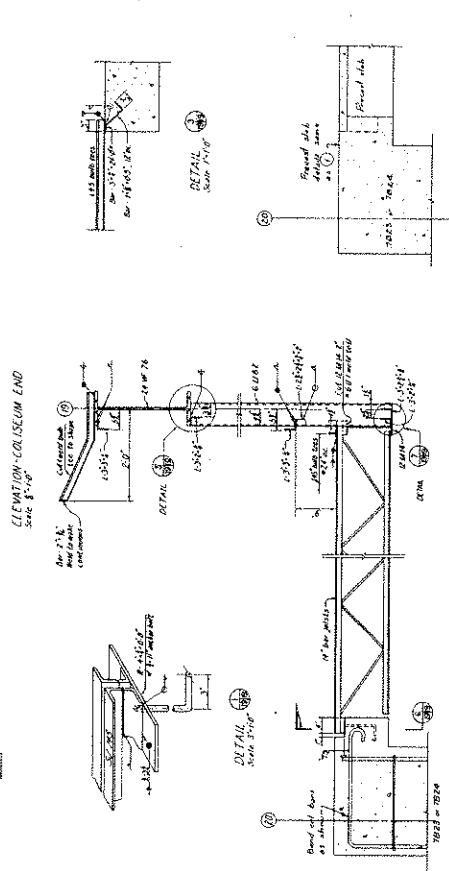
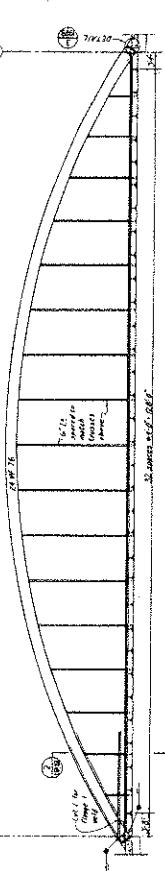
**Carpenter  
Wright  
Engineers** **C  
W  
E**  
Structural Consultants



SINGLE 500 LB CONCENTRATED  
LOAD AT ANY PANEL POINT

**ROOF TRUSS LOADING PROFILE - F**

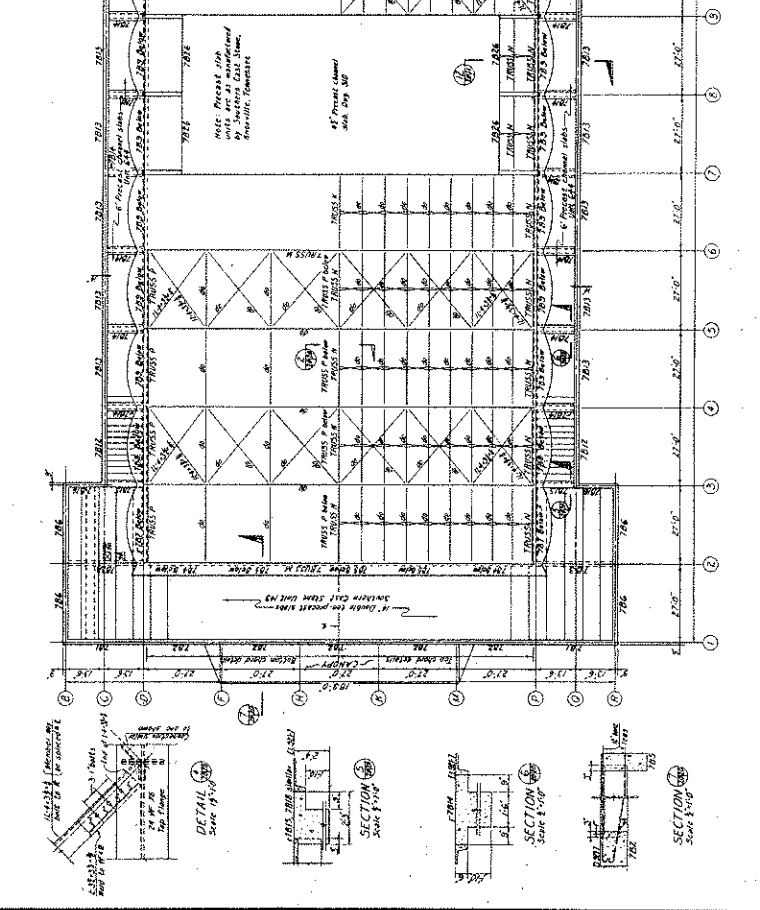
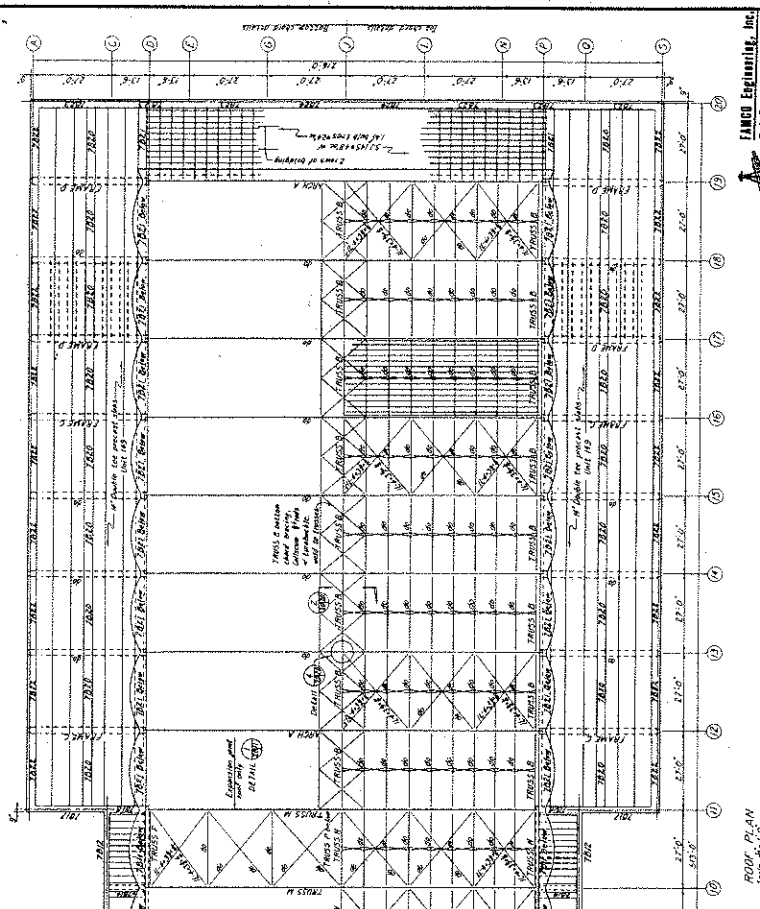
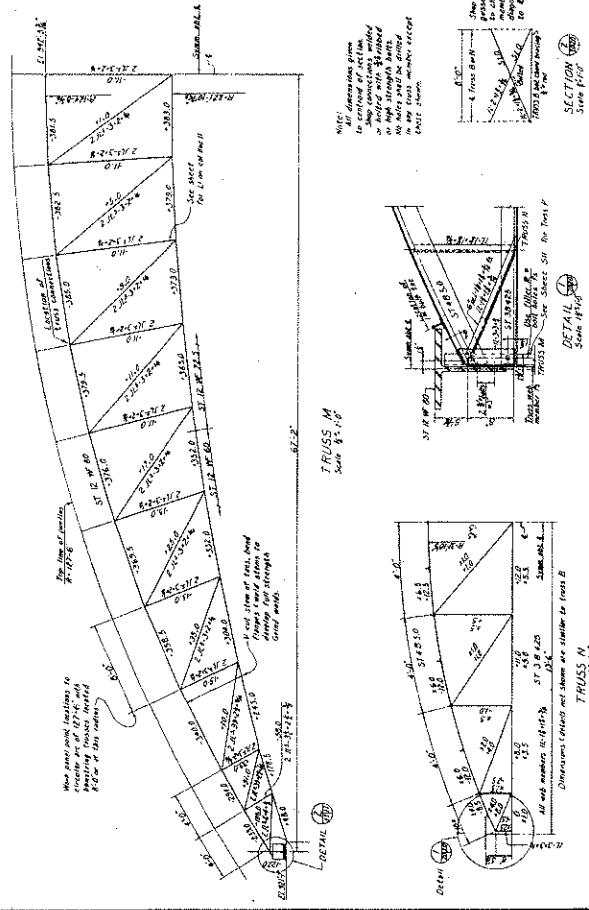
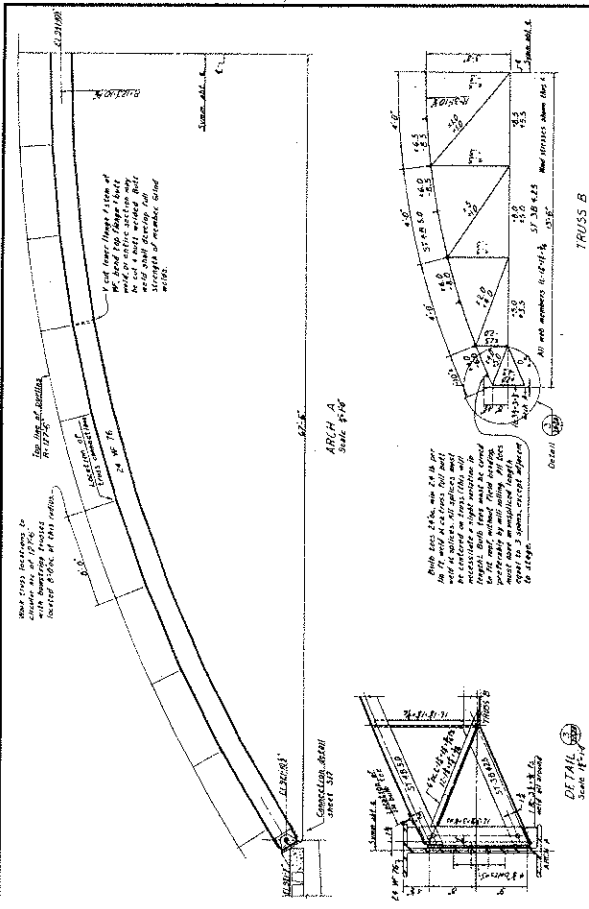
DO NOT APPLY CONCENTRATED  
LOADS BETWEEN PANEL POINTS  
THAT WILL PRODUCE BENDING  
OF TRUSS CHORDS.



**KNOXVILLE CIVIC AUDITORIUM - COLISEUM**

**PAINTER, WEEKS & McCARTY ARCHITECTS  
MORTON & SWEETSER ENGINEERS**

**FARCO ENGINEERING, INC.**  
Civil Engineer and Construction Administration  
KNOXVILLE - TENNESSEE  
605 N. UNIVERSITY AVENUE  
KNOXVILLE, TENNESSEE 37919  
PHONE 863-259-8889 FAX 863-259-8889



**KNOXVILLE CIVIC AUDITORIUM - COLISEUM**

PAINTER, WEEKS & McCARTY  
KNOXVILLE, TENNESSEE

ROOF FRAMING

LANCO Engineering, Inc.  
Chief Engineers and Consultants

ARCHITECT: LLOYD B. COOPER, INC. KNOXVILLE, TENNESSEE

NO. S20

MARK	CONVENTIONAL		STRIPE		LONG SPAN		LONG SPAN		LONG SPAN		LONG SPAN	LONG SPAN	LONG SPAN	LONG SPAN	LONG SPAN	LONG SPAN	LONG SPAN	LONG SPAN	
	MARK	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.	NO.									
281	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
282	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
283	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
284	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
285	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
286	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
287	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
288	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
289	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
290	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
291	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
292	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
293	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
294	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
295	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
296	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
297	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
298	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
299	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4
300	14	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4	1.4	2.4





# JOIST SCHEDULE

MARK	DIMENSIONS		SPECS	REMARKS
	B	D		
1.0	4 3/4"	8 1/2"	16 LBS PER LINEAL FOOT	STANDARD JOIST
1.1	5 3/4"	9 1/2"	18 LBS PER LINEAL FOOT	STANDARD JOIST
1.2	6 3/4"	10 1/2"	20 LBS PER LINEAL FOOT	STANDARD JOIST
1.3	7 3/4"	11 1/2"	22 LBS PER LINEAL FOOT	STANDARD JOIST
1.4	8 3/4"	12 1/2"	24 LBS PER LINEAL FOOT	STANDARD JOIST
1.5	9 3/4"	13 1/2"	26 LBS PER LINEAL FOOT	STANDARD JOIST
1.6	10 3/4"	14 1/2"	28 LBS PER LINEAL FOOT	STANDARD JOIST
1.7	11 3/4"	15 1/2"	30 LBS PER LINEAL FOOT	STANDARD JOIST
1.8	12 3/4"	16 1/2"	32 LBS PER LINEAL FOOT	STANDARD JOIST
1.9	13 3/4"	17 1/2"	34 LBS PER LINEAL FOOT	STANDARD JOIST
2.0	14 3/4"	18 1/2"	36 LBS PER LINEAL FOOT	STANDARD JOIST
2.1	15 3/4"	19 1/2"	38 LBS PER LINEAL FOOT	STANDARD JOIST
2.2	16 3/4"	20 1/2"	40 LBS PER LINEAL FOOT	STANDARD JOIST
2.3	17 3/4"	21 1/2"	42 LBS PER LINEAL FOOT	STANDARD JOIST
2.4	18 3/4"	22 1/2"	44 LBS PER LINEAL FOOT	STANDARD JOIST
2.5	19 3/4"	23 1/2"	46 LBS PER LINEAL FOOT	STANDARD JOIST
2.6	20 3/4"	24 1/2"	48 LBS PER LINEAL FOOT	STANDARD JOIST
2.7	21 3/4"	25 1/2"	50 LBS PER LINEAL FOOT	STANDARD JOIST
2.8	22 3/4"	26 1/2"	52 LBS PER LINEAL FOOT	STANDARD JOIST
2.9	23 3/4"	27 1/2"	54 LBS PER LINEAL FOOT	STANDARD JOIST
2.10	24 3/4"	28 1/2"	56 LBS PER LINEAL FOOT	STANDARD JOIST
2.11	25 3/4"	29 1/2"	58 LBS PER LINEAL FOOT	STANDARD JOIST
2.12	26 3/4"	30 1/2"	60 LBS PER LINEAL FOOT	STANDARD JOIST
2.13	27 3/4"	31 1/2"	62 LBS PER LINEAL FOOT	STANDARD JOIST
2.14	28 3/4"	32 1/2"	64 LBS PER LINEAL FOOT	STANDARD JOIST
2.15	29 3/4"	33 1/2"	66 LBS PER LINEAL FOOT	STANDARD JOIST
2.16	30 3/4"	34 1/2"	68 LBS PER LINEAL FOOT	STANDARD JOIST
2.17	31 3/4"	35 1/2"	70 LBS PER LINEAL FOOT	STANDARD JOIST
2.18	32 3/4"	36 1/2"	72 LBS PER LINEAL FOOT	STANDARD JOIST
2.19	33 3/4"	37 1/2"	74 LBS PER LINEAL FOOT	STANDARD JOIST
2.20	34 3/4"	38 1/2"	76 LBS PER LINEAL FOOT	STANDARD JOIST
2.21	35 3/4"	39 1/2"	78 LBS PER LINEAL FOOT	STANDARD JOIST
2.22	36 3/4"	40 1/2"	80 LBS PER LINEAL FOOT	STANDARD JOIST
2.23	37 3/4"	41 1/2"	82 LBS PER LINEAL FOOT	STANDARD JOIST
2.24	38 3/4"	42 1/2"	84 LBS PER LINEAL FOOT	STANDARD JOIST
2.25	39 3/4"	43 1/2"	86 LBS PER LINEAL FOOT	STANDARD JOIST
2.26	40 3/4"	44 1/2"	88 LBS PER LINEAL FOOT	STANDARD JOIST
2.27	41 3/4"	45 1/2"	90 LBS PER LINEAL FOOT	STANDARD JOIST
2.28	42 3/4"	46 1/2"	92 LBS PER LINEAL FOOT	STANDARD JOIST
2.29	43 3/4"	47 1/2"	94 LBS PER LINEAL FOOT	STANDARD JOIST
2.30	44 3/4"	48 1/2"	96 LBS PER LINEAL FOOT	STANDARD JOIST
2.31	45 3/4"	49 1/2"	98 LBS PER LINEAL FOOT	STANDARD JOIST
2.32	46 3/4"	50 1/2"	100 LBS PER LINEAL FOOT	STANDARD JOIST
2.33	47 3/4"	51 1/2"	102 LBS PER LINEAL FOOT	STANDARD JOIST
2.34	48 3/4"	52 1/2"	104 LBS PER LINEAL FOOT	STANDARD JOIST
2.35	49 3/4"	53 1/2"	106 LBS PER LINEAL FOOT	STANDARD JOIST
2.36	50 3/4"	54 1/2"	108 LBS PER LINEAL FOOT	STANDARD JOIST
2.37	51 3/4"	55 1/2"	110 LBS PER LINEAL FOOT	STANDARD JOIST
2.38	52 3/4"	56 1/2"	112 LBS PER LINEAL FOOT	STANDARD JOIST
2.39	53 3/4"	57 1/2"	114 LBS PER LINEAL FOOT	STANDARD JOIST
2.40	54 3/4"	58 1/2"	116 LBS PER LINEAL FOOT	STANDARD JOIST
2.41	55 3/4"	59 1/2"	118 LBS PER LINEAL FOOT	STANDARD JOIST
2.42	56 3/4"	60 1/2"	120 LBS PER LINEAL FOOT	STANDARD JOIST
2.43	57 3/4"	61 1/2"	122 LBS PER LINEAL FOOT	STANDARD JOIST
2.44	58 3/4"	62 1/2"	124 LBS PER LINEAL FOOT	STANDARD JOIST
2.45	59 3/4"	63 1/2"	126 LBS PER LINEAL FOOT	STANDARD JOIST
2.46	60 3/4"	64 1/2"	128 LBS PER LINEAL FOOT	STANDARD JOIST
2.47	61 3/4"	65 1/2"	130 LBS PER LINEAL FOOT	STANDARD JOIST
2.48	62 3/4"	66 1/2"	132 LBS PER LINEAL FOOT	STANDARD JOIST
2.49	63 3/4"	67 1/2"	134 LBS PER LINEAL FOOT	STANDARD JOIST
2.50	64 3/4"	68 1/2"	136 LBS PER LINEAL FOOT	STANDARD JOIST

**GENERAL NOTES:**  
 1. All columns shall be steel (unless otherwise noted).  
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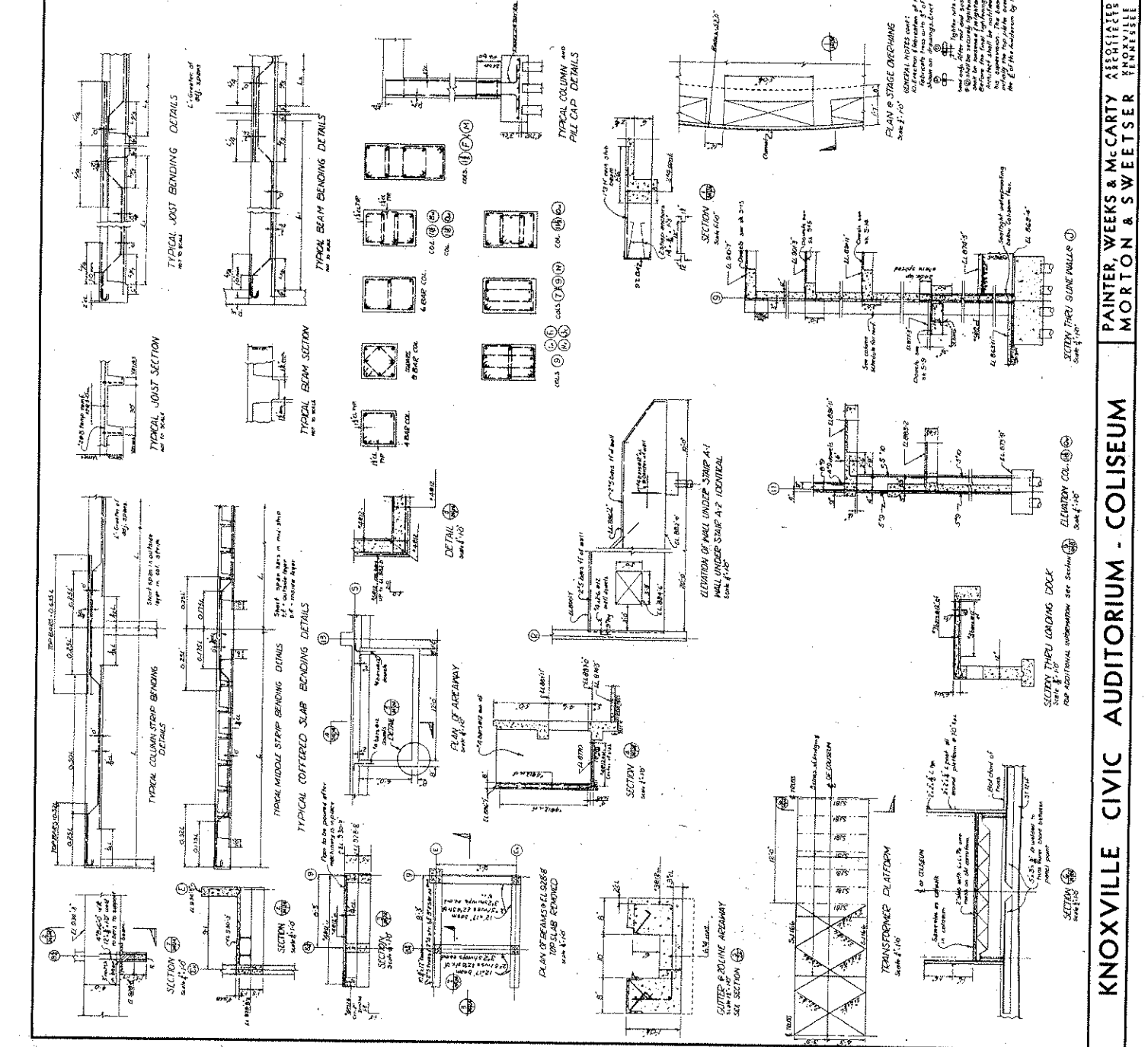
**JAMES ENGINEERING, INC.**  
 CIVIL ENGINEERS  
 ROSSVILLE - TENNESSEE

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**PAINTER, WEEKS & McCARTY**  
 ARCHITECTS  
 MORTON & SWEETSER

JOIST SCHEDULE / MISC DET

**S24**  
 DRAWING NO.

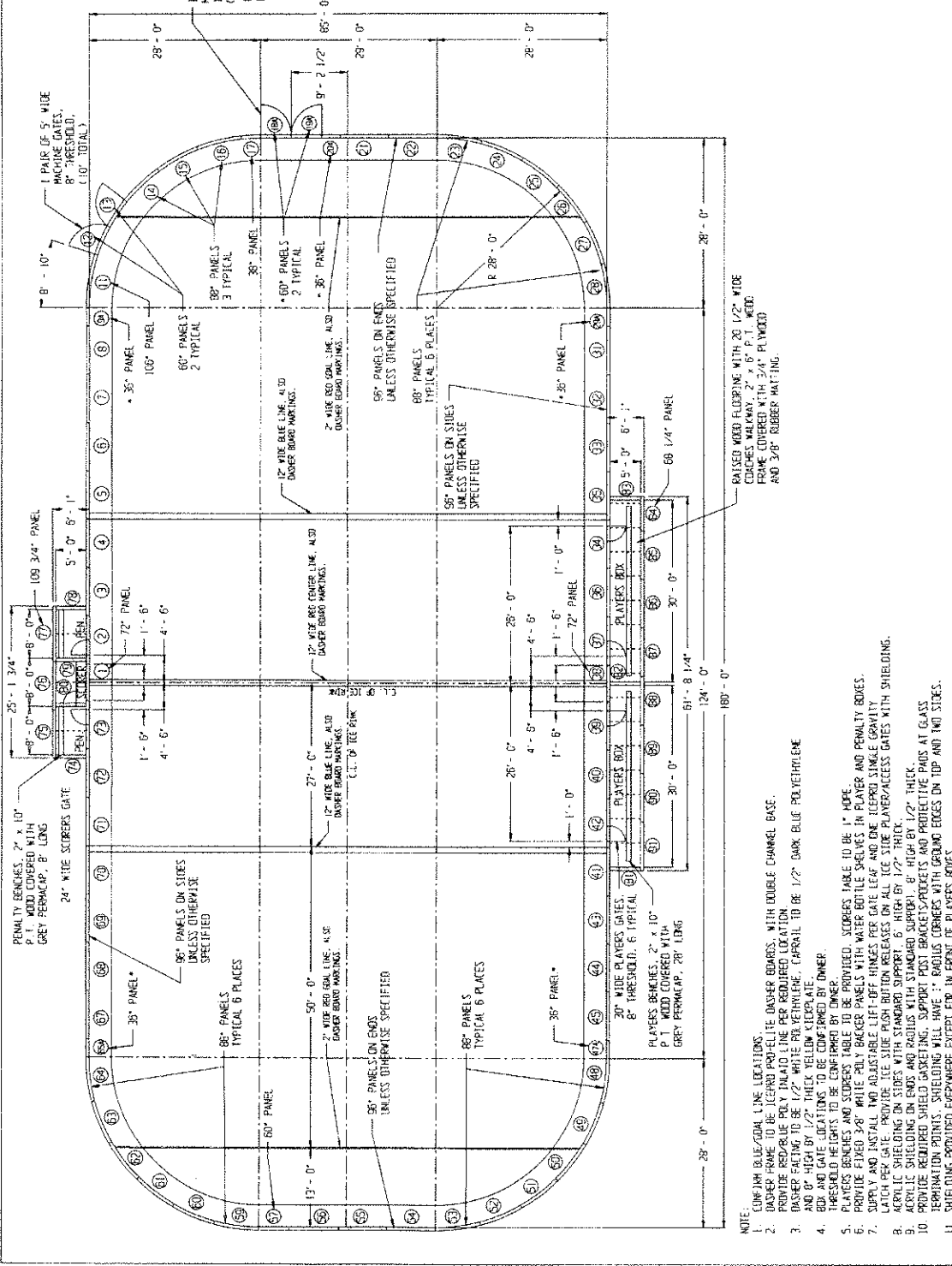
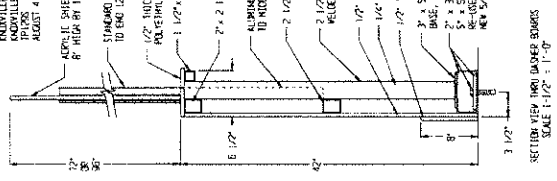


DESCRIPTION	QUANTITY
1. SWITCH PANELS (1.1 x 4.0) (AS PER BLUE LINE)	1
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**IcePro Canada Inc.**  
**PRO-ELITE**  
**DOUBLE CHANNEL BASE**  
**DASHER BOARD LAYOUT**

KNOXVILLE AUDITORIUM & COLISEUM  
 KNOXVILLE, TENNESSEE  
 AUGUST 4, 1999

NOTE: OWNER TO CONFIRM LOCATION OF MACHINE GATE.



**IcePro Canada Inc.**  
 2715 SLOUGH ST.  
 MISSISSAUGA, ONTARIO  
 L4T 1G2  
 DATE: AUGUST 4, 1999  
 DRAWN BY: LAURIE BULL  
 CHECKED BY:

**DASHER BOARD LAYOUT**  
**KNOXVILLE AUDITORIUM & COLISEUM**  
**KNOXVILLE, TENNESSEE**

DWG. NUMBER: JP 129-1  
 OF: 2  
 REVISIONS:

- NOTE:**
1. CENTER LINE LOCATIONS
  2. DASHER FRAME TO BE ICEPRO PRO-ELITE DASHER BOARDS WITH DOUBLE CHANNEL BASE
  3. PROVIDE PRO-ELITE BOX INLAND LINE PER PROVIDED LOCATION AND 8" HIGH BY 1/2" THICK YELLOW URETHANE CARROLL TO BE 1/2" DARK BLUE POLYETHYLENE
  4. THRESHOLD HEIGHTS TO BE CONFIRMED BY OWNER
  5. PROVIDE BENCHES AND SCORES TABLE TO BE PROVIDED. SCORES TABLE TO BE 1" HOPE
  6. PROVIDE BENCHES AND SCORES TABLE WITH WATER BOTTLE SHELVES IN PLAYER AND PENALTY BENCHES
  7. SUPPLY AND INSTALL TWO ADJUSTABLE LIFT-OFF HINGES PER GATE LEAF AND ONE ICEPRO SINGLE GRAVITY LATCH PER GATE. PROVIDE ICE SIDE PUSH BUTTON RELEASES ON ALL ICE SIDE PLAYER/ACCESS GATES WITH SHIELDING
  8. ACRYLIC SHIELDING ON SIDES WITH STANDARD SUPPORT. 6" HIGH BY 1/2" THICK
  9. PROVIDE BENCHES WITH SHIELDING WITH STANDARD SUPPORT. 6" HIGH BY 1/2" THICK
  10. PROVIDE PRO-ELITE SHIELDING ON ALL MACHINE GATES WITH SHIELDING PROTECTIVE FIBER GLASS TEMPERATURA PENETRATION RESISTANT. MAKE SURE PRO-ELITE SHIELDING WITH GROUND EDGES ON TOP AND SIDES.
  11. SHIELDING PROVIDED EVERYWHERE EXCEPT FOR IN FRONT OF PLAYERS BENCHES.

