GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT

Ballston Pond Geo Boring Arlington, Virginia



PREPARED FOR:

RK&K 10306 Eaton Place Willow Wood II, Suite 240 Fairfax, VA 22030

PREPARED BY:



Engineers Planners Surveyors Contractors

May 16, 2012

Attn: Mr. Bill Springer RK&K 10306 Eaton Place Willow Wood II, Suite 240 Fairfax, VA 22030

REF: Report of Subsurface Investigation and Studies

Ballston Pond Geo Boring

Arlington, Virginia AB Job No. 12-033

Dear Mr. Springer:

AB Consultants, Inc. (ABC) is pleased to submit this soil report containing the results of the geotechnical investigation for the above referenced site. The project site is an existing stormwater management (SWM) pond that located in Arlington, Virginia. During our field operation, we noticed that large portion of the pond is covered with sediments and vegetation. A hiker path is found along the east and south boundary of the pond. The purpose of this study was to explore the subsurface conditions for the site improvement. It is understood that boardwalks that branch out from existing hiker path are proposed at various locations. Improvement may also include dredging of sediment in the SWM pond.

Field Investigation

Due to the limited accessibility of proposed boring locations, soil borings were only able to be performed by hand auger at site on April 27, 2012 by ABC. Borings were located in field by us and boring plan is included in the Appendix. To obtain information of the subsurface conditions, six (6) 5- to 7-ft deep borings were drilled on site. Boring depths were restricted by the shallow refusal and groundwater. A representative portion of sample was placed in a plastic bag and was transported to laboratory. Dynamic Cone Penetration (DCP) test were performed in accordance with ASTM Special Technical Publication #399 for each test pits at depth intervals of 1, 3 and 5 ft.

In the procedure of DCP test, the dynamic portable penetrometer is driven into the ground with a 15-pound hammer, free falling a distance of 20 inches. The blows required to advance the dynamic cone to a specified distance are recorded. The values are corrected to penetration resistance values which were obtained using the Standard Penetration Tests (SPT) in accordance with ASTM D1586. The values are shown on boring logs at the depths of their occurrence. The N-value is the sum of last two penetration resistance values.

Groundwater level was monitored in boring holes. Samples obtained from borings were inspected by a geotechnical engineer and the field logs were edited accordingly. The

Geotechnical Report

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May 16, 2012

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final logs that indicate the subsurface conditions encountered are included in the Appendix. All samples obtained from soil borings will be retained in our laboratory for a period of thirty (30) days from the date of this report. They will be available for inspection during this period. After that time, the samples will be discarded.

Per the request of your office, three (3) water samples were also collected and forwarded to assigned testing laboratory for sediment analysis. Locations of water samples are included in the boring plan.

<u>Laboratory Testing Program</u>

Laboratory tests were performed on selected representative samples. Natural moisture contents were performed on all soil samples, and results are included in boring logs. Atterberg limits and sieve analysis were conducted on selected samples. Atterberg limits results are shown in test pits logs in correspondence with the sample depths and results of sieve analyses are presented in the Appendix. Results of some laboratory tests are summarized in the following table.

	SUMMARY OF LABORATORY TEST RESULTS														
Boring	Sample	Atterberg	g Limits		Sieve	e Analysi	s (% Pas	sing)							
No.	Depth (ft)	LL	PI	3/4"	3/8"	#4	#10	#40	#200	Classif.					
B-1	1 to 3	ı	ı	88.1	77.5	69.5	64.8	57.5	27.8	SM					
D-1	3 to 5	38	18	_	ı	-	-	ı	67.2	CL					
B-2	3 to 5	ı	ı	100	99.0	97.5	90.0	78.3	46.5	SM					
B-3	1 to 3	ı	ı	100	94.0	83.0	72.9	51	26.1	SM					
D-3	3 to 5	-	-	100	93.6	72.5	58.4	40.8	7.9	SP-SM					
B-4	3 to 5	-	-	87.2	86.7	84.1	83.3	71.9	19.7	SM					
B-5	1 to 3	-	ı	92.8	86.4	78.8	62.3	17.5	1.4	SP					
B-6	3 to 5	-	-	100	87.7	83.9	78.5	55.5	18.5	SM					

Subsurface Soil Conditions and Groundwater Observations

Various soil types were grouped into the major zones noted on the boring logs. A brief explanation of the terms and notes used in the logs is included with this report. The stratification lines designating the interfaces between earth materials on the boring logs are approximate; in situ, the transitions may be gradual. Detailed soil description and depth of various soil strata are given in boring logs, together with DCP blow counts with depths. In general, the encountered soils are grouped into major types and summarized as follows:

Topsoil: Topsoil was encountered in all boring. Topsoil is defined as the more high-organic, weathered surficial soils horizon capable of supporting vegetation

Type A: Below Topsoil, brown and gray silty fine sand to sand with gravel was encountered in most of all borings and extended 2.5-ft below existing ground or to completion depth of borings. Corrected N-values of these soil types were ranging from 7 to 24 blows per foot.

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Type B: Localized brown fine sandy clay was encountered underneath Type A and extended to the completion depths in boring B-1. Corrected N-value of this soil type was 24 blows per foot.

Groundwater observations were made in boring during drilling, and after completion of drilling operations. As noted on boring logs, groundwater was observed in boring B-3, B-5 and B-6. Fluctuations in the level and quantity of ground water will occur due to variations in rainfall, temperature, soil permeability and other factors not evident at the time of the water level measurements recorded on test pit logs.

Findings and Summary

Based on information revealed from soil borings, the engineering properties of encountered soils are summarized in the following table. Soil parameters were based on laboratory results, empirical correlation from the corrected N-values and published information.

		SUMMARY OF EN	COUNTERED	SOIL PROPE	RTIES	
Boring Number	DCP Test at Elevation	Sample Description	Uncorrected N-Value	Corrected N-Value	Range of Effective Unit Weight, γ (pcf)	Range of Friction Angle, ¢ (deg.)
B-1	254	Silty fine sand (SM)	11	11	115 to 120	28 to 32
D-1	252	Fine sandy clay (CL)	37	24	115 to 120	<15
	262	Silty fine sand (SM)	30	23	120 to 125	30 to 34
B-2	260	Silty fine sand (SM)	35	24	120 to 125	30 to 34
	258	Silty fine sand (SM)	34	24	120 to 125	30 to 34
B-3	Silty fine sand (SM)		16	16	115 to 120	28 to 32
D-3	257	Silty sand (SP-SM)	14	14	115 to 120	30 to 34
B-4	259	Silty fine sand (SM)	16	16	115 to 120	28 to 32
D-4	257	Silty fine sand (SM)	32	24	120 to 125	30 to 34
B-5	254	Sand (SP)	9	9	110 to 115	28 to 32
D-3	252	Sand (SP)	9	9	110 to 115	28 to 32
B-6	257	Silty fine sand (SM)	7	7	110 to 115	26 to 30
D-0	255	Silty fine sand (SM)	7	7	110 to 115	26 to 30

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Due to the site access restriction, field borings were manual dug at locations that were closed to the proposed improvements. Subsoil conditions may be varied. recommended that during construction of the boardwalk and improvement of SWM facility, the soil encountered at and below the planned structures elevations, to be verified in field by geotechnical engineer.

General Limitations

This report is based upon the data obtained from the test holes performed at indicated location and from any other information discussed in this report. This report does not reflect any variations that may occur across the site. If variations appear evident, the summary of this report should then be reviewed by ABC geotechnical engineer in light of the new information.

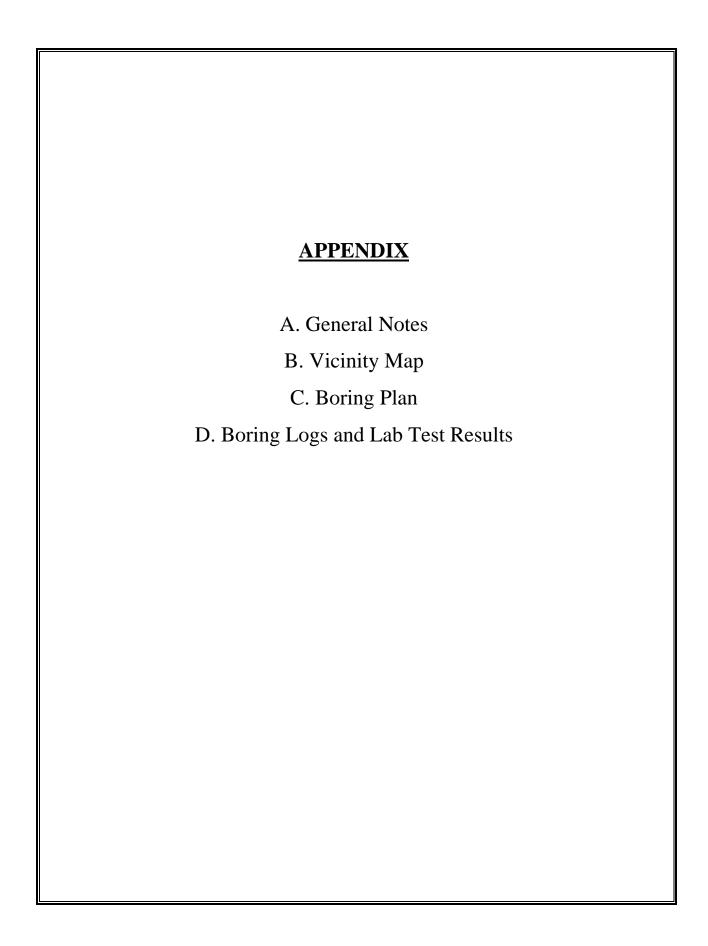
It has been a pleasure serving you on this project. If you have any questions regarding this report, or if we can be of further service in any way, please contact us.

> KIM HOU CHA NO. 039138

Very truly yours,

AB Consultants, Inc.

Kim-Hou Chan, r.L.
Geotechnical & Field Services



GENERAL NOTES

Drilling and Sampling Symbols



N = Standard penetration, blows per foot of a 140 lbs hammer for 30" drop

RQD = Rock Quality Designation

LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index

Cohesionless Soils

If the sand or silt content of a soil is great enough, the soil becomes non-cohesive or semi-cohesive. The soil classification becomes SAND or SILT with the other soil constituents being modifying.

Based on N-Value

10 to 29 Blows......Medium Dense

Cohesive Soils

If clay content is sufficient so that clay dominates soil properties, then CLAY becomes the major soil constituent as modifier. Other minor soil constituents may be added according to classification breakdown for cohesion less soils: i.e. silty clay, trace of some sand, trace of gravel.

Based on N-Value

Based on Penetrometer Value

 Below 0.25
 Very Soft
 1.00 to 1.99
 Stiff

 0.25 to 0.49
 Soft
 2.00 to 3.99
 Very Stiff

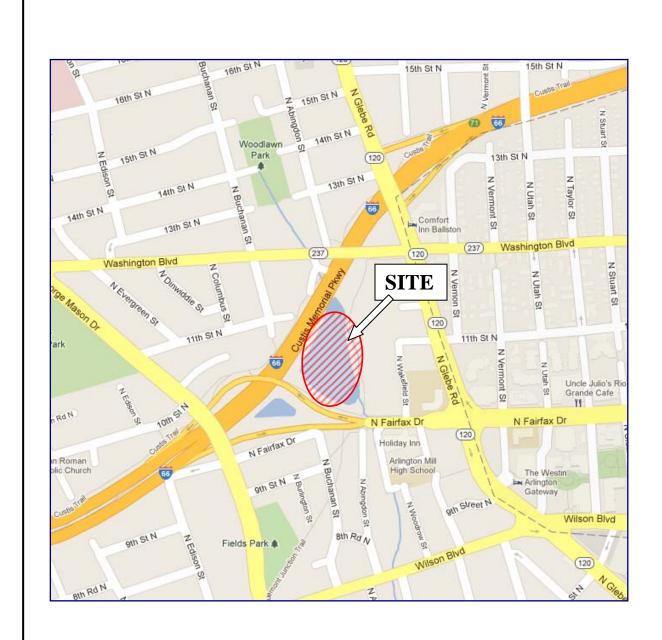
 0.50 to 0.99
 Firm
 Over 4.00
 Hard

Quantity Modifiers

<u>Term</u>	% of Dry Weight
trace	0 to 10
little	11 to 20
some	21 to 35
and/with	36 to 50

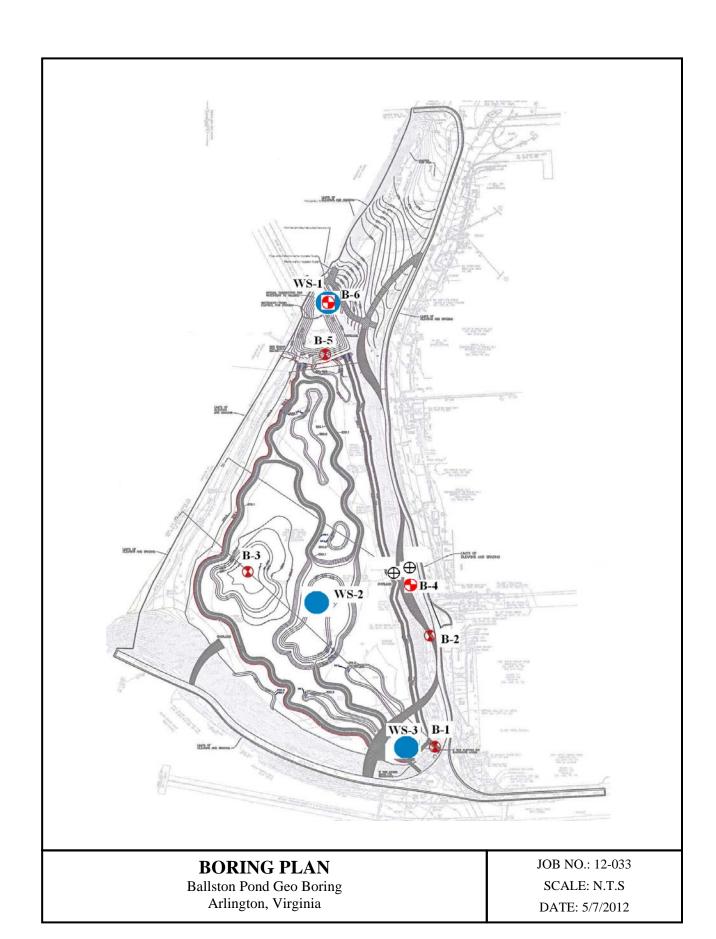
Particle Size Identifications

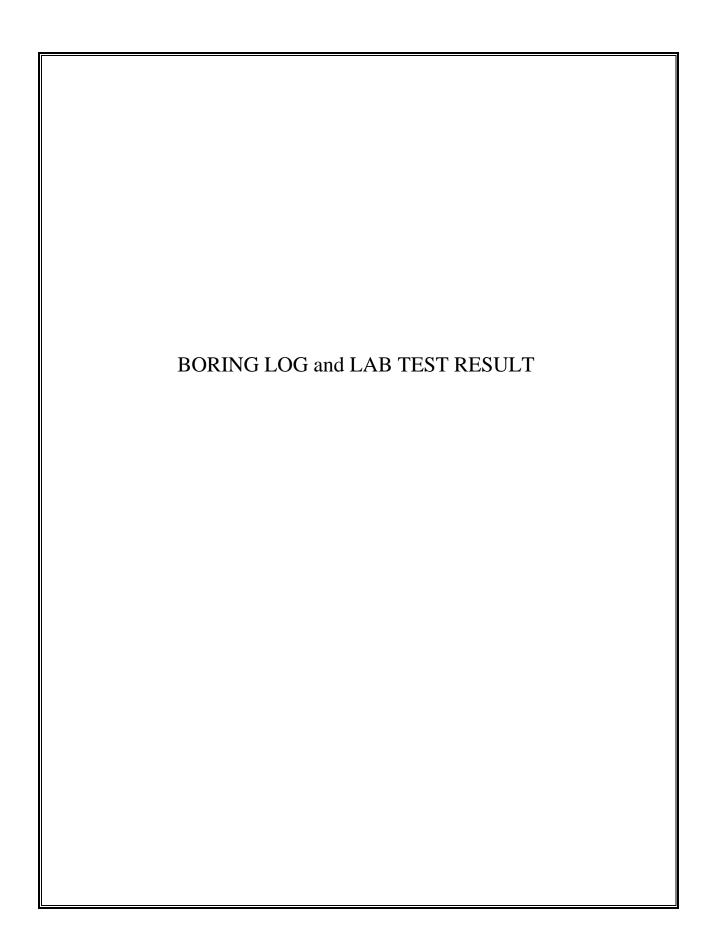
		Over 8 inch diameter3 inch to 8 inch
Gravel	Coarse	1 inch to 3 inch
	Medium	1/2 inch to 1 inch
	Fine	4.75 mm to 1/2 inch
Sand	Coarse	2 mm to 4.75 mm
	Medium	0.425 mm to 2 mm
	Fine	0.075 mm to 0.425 mm
Silt/Clay		Below 0.075 mm



VICINITY MAP

Ballston Pond Geo Boring Arlington, Virginia JOB NO.: 12-033 SCALE: N.T.S DATE: 5/7/2012





Proje	ct No. 12-033		LOG (OF BOI	REH	OLE B-	1						;	Sheet 1 of 1	
CLIENT	Γ:	R.K.& I	·		PROJ	ECT:		Ba	Ilstor	ı Po	ond				
ARCHI	TECT/ENGINEER:	11.11.01	`		SITE:			gton	n						
						Virginia SAMPLES TESTS									
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	CE ELEV.: 255.0 ft. 6" Topsoil			74 1 ^N · 7/4		шии		-	==			0.0	*	ЩФП	
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2.5 C	Dense brown FINE	SANDY CL	AY (CL)	252.5 (1)											
4.5				250.5		6-17-20 N=37	2	AU	5.25	17			67	LL = 38 PL = 18 PI = 20	
	Auger Refusal @ 4	.5 ft													
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ML WL	Dry	+ax: 3	Fax: 301-306-3092				LOGGED BY:				APPROVED:				

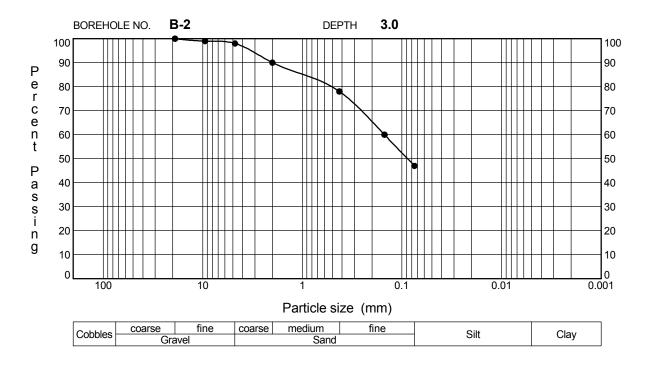
Projec	ct No. 12-033		LOG O	F BOI	REH	OLE B-	2						;	Sheet 1 of 1	
CLIENT	<u>.</u>	R.K.& I	·		PROJ	ECT:		ı Po	ond						
ARCHIT	TECT/ENGINEER:	IX.IX.OCI	`		SITE:			Arlington Virginia							
						SA	MPLE	S	Virg	ınıa		TES	STS		
				GRAPHIC LOG	ОЕРТН (FT)	BLOWS/6" N - VALUE RQD	NUMBER	TYPE	IN. RECOVERED IN. DRIVEN	MOISTURE (%)	DRY DENSITY (PCF)	Qu (TSF)	% PASSING #200 SIEVE	REMARKS/ ADDITIONAL DATA	
	CE ELEV.: 263.0 f "Topsoil	t.		<u>7, 7, 7,</u> <u>Q</u>	. – .	<u>az</u>	ž	F		Σ	<u> </u>	٥Ŀ	%#	<u> </u>	
0.5	Dense brown SILipravel and rock fr	TY FINE SAN agments	D (SM) with	2.5		17-17-13 N=30 13-15-20 N=35	2	AU	5.25	12			47		
6.0			25	57.0	5	14-16-18 N=34	3	AU	5.25	16					
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WL Dry @ 0 Hrs WL Dry After 24 Hrs					e: 301-	306-3091 6-3092			LER:		ADP	ASS'T DRILLER: MB/JL			
<u> </u>	NL Dry After 24 Hrs					000-003 <u>Z</u>			GED BY:		APPROVED:				

Proj	ect N	o. 12-03	33		LOG	OF BC	REH	OLE B	-3						;	Sheet 1 of 1		
CLIE	NT:		R	R.K.& F	<u> </u>		PRO	JECT:		Ba	allstor	ı Po	nd					
ARCI	HITECT	/ENGINEE					SITE	:			Arling Virgi	gton						
								SA	AMPLE	S			i	TES	STS			
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0.7	Medii SANI	um dense D (SM) wit	brown and	d gray d grave	SILTY FINE	<u>7</u> 259.3 √ 5.		6-6-10 N=16	1	AU	5.25	17			26			
3.0		um dense some grav		LTY SA	AND (SP-SM)	257.0		6-6-8 N=14	2	AU	5.25	25			8			
5.0	Porol	nolo kont (caving in	cannot	t advance.	255.0	5-											
		-	-	Carino	auvance.													
BOKING LOG ABBU 12-033 BALLSI ONE POND.GPJ AR CONS.GDI S/16/12	End	of Boring (@ 5 π															
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WL	Ā	▼ 2.5 @ 0 Hrs Phon											ADP ASS'T DRILLER: MB/JL					
K WL	$ar{m{\Lambda}}$	0.5	After 2	- 24 Hrs Fax: 301-306-3092 LOGGED BY:									APPROVED:					

	 Proj	ject No. 12-033	LOG OF BORE						REHOLE B-4						
Ì	CLIE	NT:	R.K.& I	Κ		PRO	JECT:		Ba	Ilstor	ı Po	nd			Sheet 1 of 1
Ì	ARCI	HITECT/ENGINEER:				SITE	SITE: Arlington Virginia								
ŀ							SAMPLES TESTS								
	SURF	ACE ELEV.: 260.0 1	ft.		GRAPHIC LOG	DEPTH (FT)	BLOWS/6" N - VALUE RQD	NUMBER	ТҮРЕ	IN. RECOVERED IN. DRIVEN	MOISTURE (%)	DRY DENSITY (PCF)	Qu (TSF)	% PASSING #200 SIEVE	REMARKS/ ADDITIONAL DATA
Ì		6" Topsoil			74 1 ^N · 7/4	_		_			_			0 4	
	0.5	Medium dense to SAND (SM) with	dense gray S roots	259. ILTY FINE	5 1/2 2/2		6-7-9 N=16	1	AU	5.25	11				
	4.0			256.	0		27-16-16 N=32	2	AU	5.25	11			20	
		Auger Refusal @	4 ft												
J AB_CONS.GDT 5/16/12		Hand auger was in 5 ft radius	done at two (2	2) other locations											
BORING LOG AB09 12-033 BALLSTONE POND.GPJ AB_CONS.GDT_5/16/12	WA WL	ATER LEVEL OBSER Dry Dry	RVATIONS @ Drilling @ 0 Hrs		9450 <i>i</i> Lanha	Annap ım, MI	Iltants, In polis Road D 20706 -306-3091	C.	DRIL	RTED: L CO.:		7/12 ABC		L RIG:H	4/27/12 and Auger
30R	WL	Dry	After 24 Hrs Fax: 301-306-3092 LOGGED BY:									APPROVED:			

Pr	oject N	No. 12-03	33		LOG OF BOREHOLE B-5								;	Sheet 1 of 1			
CL	IENT:		F	R.K.& F	<		PRO	JECT:		Ba	allstor	ı Po	ond				
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								SA	MPLE	S	viig	IIIIa	1	TES	STS		
SUF		:LEV.: 255.0) ft.			GRAPHIC LOG	DEPTH (FT)	BLOWS/6" N - VALUE RQD	NUMBER	TYPE	IN. RECOVERED IN. DRIVEN	MOISTURE (%)	DRY DENSITY (PCF)	Qu (TSF)	% PASSING #200 SIEVE	REMARKS/ ADDITIONAL DATA	
0.		opsoil			25/	1.5											
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4.	5				250).5		4-5-4 N=9	2	AU	5.25	15			1		
		hole kept o	caving in,	cannot													
BORING LOG AB09 12-033 BALLSTONE POND.GPJ AB_CONS.GDT 5/16/12	End	of Boring (@ 4.5 ft														
33 BALLSTON				-													
309 12-(Itants, Ir	IC.	STAI	RTED:		7/12		SHED:	4/27/12	
WL WL		1.75	@ Dri 			Lanha	ım, Mİ	olis Road D 20706			L CO.:		ABC DRILL RIG: Hand Auger				
MP MP		0.75		Phone: 301-306-3091 DRILLER: r 24 Hrs Fax: 301-306-3092 Logged BY:								ADP ASS'T DRILLER: MB/JL APPROVED:					

Project No. 12-033	LOG OF BOREHOLE B-6							Sheet 1 of 1			
CLIENT:	R.K.& K	PRO	JECT:		Ва	Ilstor	ı Po	nd			
ARCHITECT/ENGINEER:	11110	SITE	:			Arling Virgi	gton				
			SA	MPLE:	S	viigi	IIIIa		TES	STS	
SURFACE ELEV.: 258.0 ft.	BRAPHICI OG	DEPTH (FT)	BLOWS/6" N - VALUE RQD	NUMBER	TYPE	IN. RECOVERED IN. DRIVEN	MOISTURE (%)	DRY DENSITY (PCF)	Qu (TSF)	% PASSING #200 SIEVE	REMARKS/ ADDITIONAL DATA
6" Topsoil	257.5 ^V	· .\\\.\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\									
Loose dark brown SILT	Y FINE SAND (SM)		3-3-4 N=7	1	AU	5.25	55				
		- - -	3-4-3 N=7	2	AU	5.25	45			19	
5.0	253.0	5—									
Borehole kept caving in	, cannot advance.										
End of Boring @ 5 ft Figure 15033 BALISTONE FORD AB CONS. GD 15.033 BALIS											
WATER LEVEL OBSERVATIO	AD		ıltants, In	C.	STAF	RTED:	4/27	7/12	FINIS	SHED:	4/27/12
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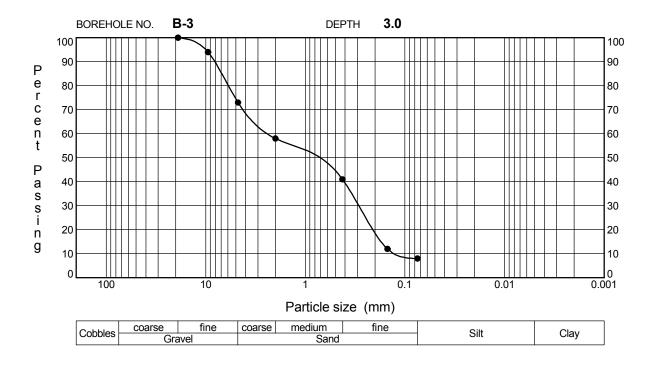


AB Consultants, Inc. 9450 Annapolis Road Lanham, MD 20706 Phone: 301-306-3091

Fax: 301-306-3092

GRAIN SIZE DISTRIBUTION

CLIENT: R.K.& K PROJECT NO.: 12-033 PROJECT: Ballston Pond

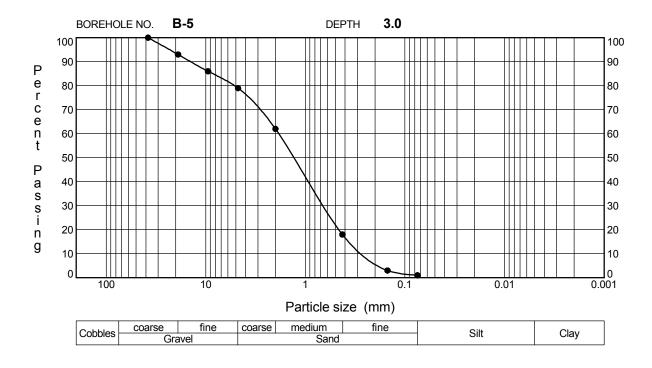




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