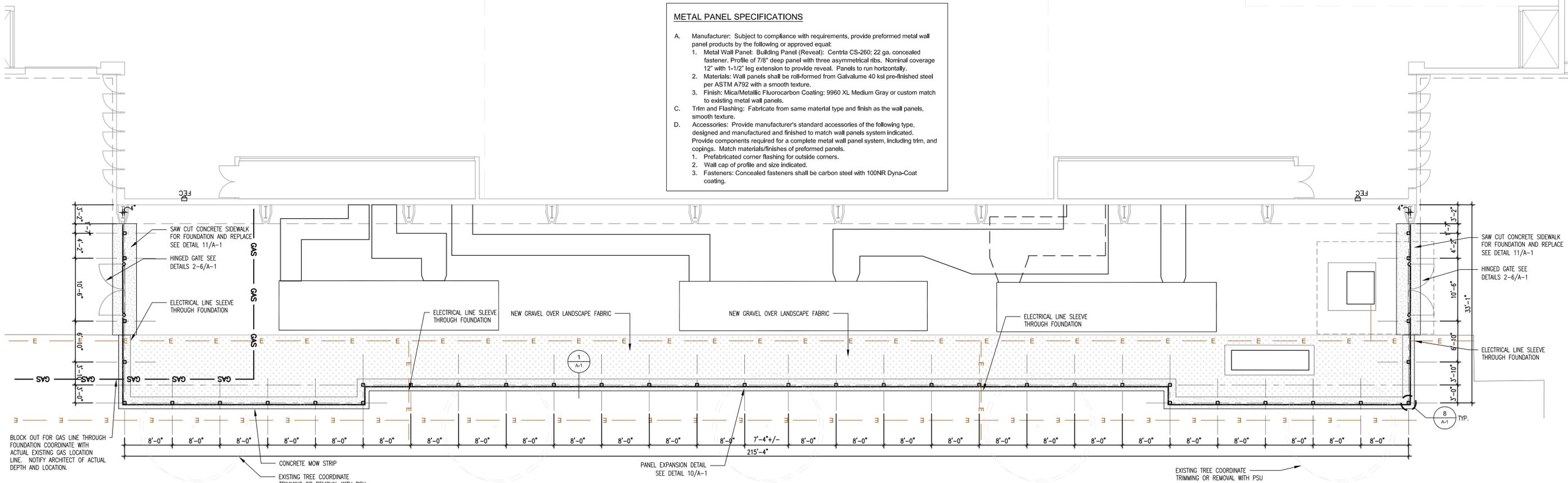




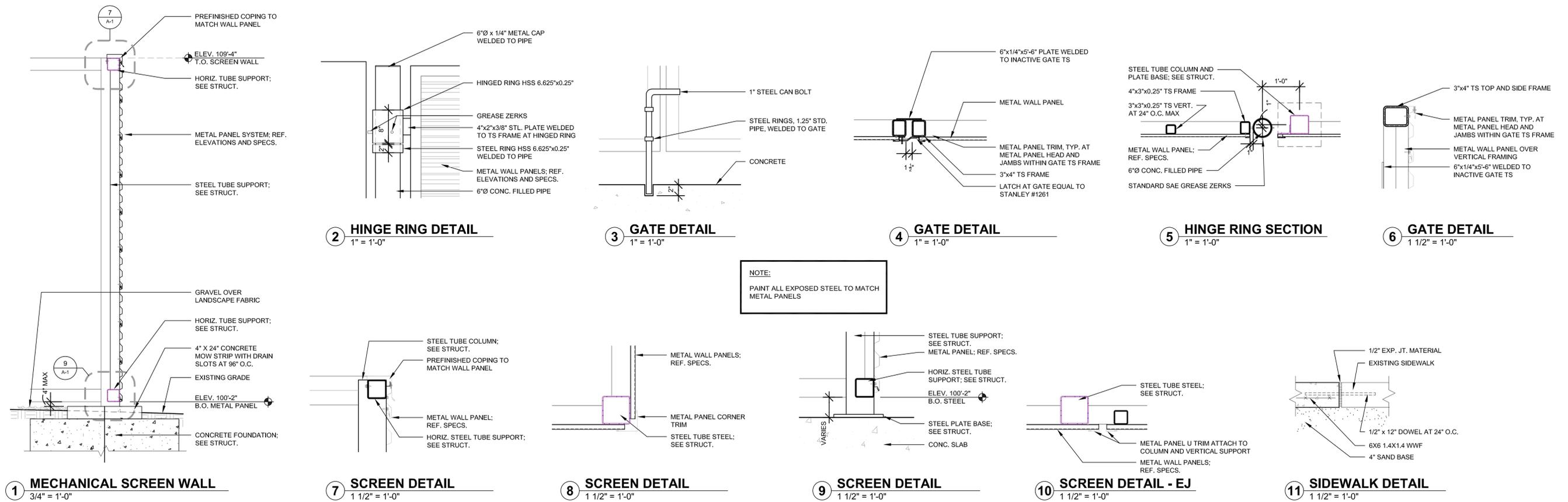
**METAL PANEL SPECIFICATIONS**

- A. Manufacturer: Subject to compliance with requirements, provide preformed metal wall panel products by the following or approved equal:
  1. Metal Wall Panel: Building Panel (Reveal): Centria CS-260; 22 ga. concealed fastener. Profile of 7/8" deep panel with three asymmetrical ribs. Nominal coverage 12" with 1-1/2" leg extension to provide reveal. Panels to run horizontally.
  2. Materials: Wall panels shall be roll-formed from Galvalume 40 ksi pre-finished steel per ASTM A792 with a smooth texture.
  3. Finish: Mica/Metallic Fluorocarbon Coating; 9960 XL Medium Gray or custom match to existing metal wall panels.
- C. Trim and Flashing: Fabricate from same material type and finish as the wall panels, smooth texture.
- D. Accessories: Provide manufacturer's standard accessories of the following type, designed and manufactured and finished to match wall panels system indicated. Provide components required for a complete metal wall panel system, including trim, and copings. Match materials/finishes of preformed panels.
  1. Prefabricated corner flashing for outside corners.
  2. Wall cap of profile and size indicated.
  3. Fasteners: Concealed fasteners shall be carbon steel with 100NR Dyna-Coat coating.



**MECHANICAL SCREEN PLAN**

1/8" = 1'-0"



**NOTE:**  
PAINT ALL EXPOSED STEEL TO MATCH METAL PANELS



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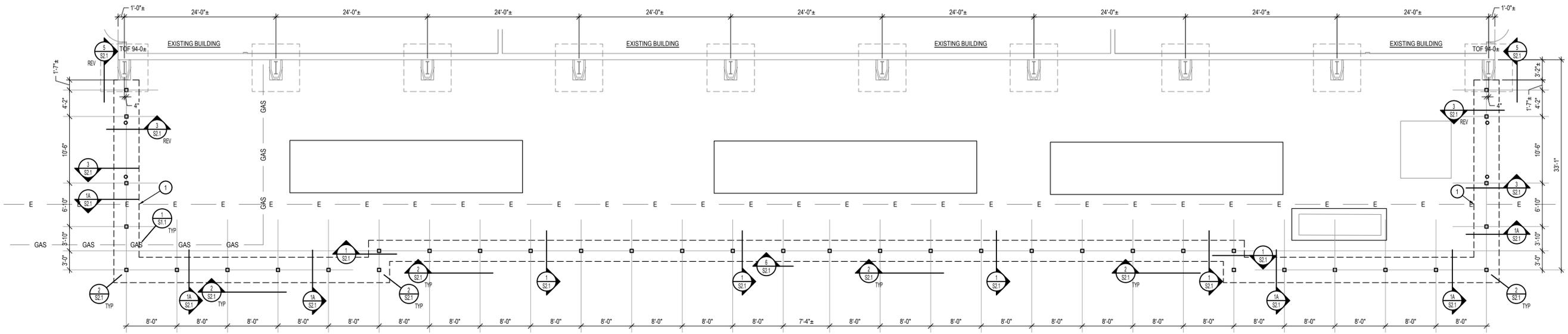
PITTSBURG STATE UNIVERSITY  
WEEDE MECHANICAL ENCLOSURE  
1701 S. BROADWAY, PITTSBURG, KANSAS 66762  
BUILDING NUMBERS 38500-00031  
DATE: 03/30/2022  
DRAWN BY: [REDACTED]  
CHECKED BY: [REDACTED]

FOUNDATION PLAN  
AND GENERAL NOTES

-

S1.1

ORIGINAL CONTRACT  
DOCUMENTS



### FOUNDATION PLAN



KEYED PLAN NOTE:  
1 PROVIDE SLEEVE AT EXISTING ELECTRICAL LINE PER GENERAL STRUCTURAL NOTES.

**GENERAL STRUCTURAL NOTES**  
GENERAL CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. FIELD VERIFY (FV) ALL EXISTING DIMENSIONS, ELEVATIONS, AND CONDITIONS. NOTIFY THE ARCHITECT FOR DIRECTION IF THE ACTUAL EXISTING CONDITIONS DIFFER FROM THE CONDITIONS SHOWN OR IMPLIED ON THE DRAWINGS.  
VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS.  
SEE THE ARCHITECTURAL DRAWINGS FOR THE EXACT DIMENSIONS FOR OPENINGS IN THE WALL SYSTEMS. THE CONTRACTOR SHALL DESIGN, PROVIDE, AND MAINTAIN TEMPORARY BRACING, SHORING, GUYING, ETC. AND OTHER METHODS AS REQUIRED TO PREVENT ANY EXCESSIVE LOADING AND TO STABILIZE THE STRUCTURAL ELEMENTS DURING CONSTRUCTION. THESE METHODS SHALL REMAIN IN PLACE UNTIL ALL MEMBERS AND FINAL CONNECTIONS HAVE BEEN COMPLETED.  
THE WALL FRAMING AND FOUNDATION SYSTEMS ARE DESIGNED PER THE INTERNATIONAL BUILDING CODE - 2012 EDITION.  
STRUCTURAL STEEL ERECTION SHALL COMPLY WITH OSHA STANDARD 29 CFR PART 1926, SUBPART R AND ALL OTHER GOVERNING REGULATIONS. STRUCTURAL STEEL SUPPLIERS AND FABRICATORS SHALL INCORPORATE THE REQUIREMENTS OF THIS STANDARD INTO THE MATERIALS FABRICATED AND SUPPLIED ON THIS PROJECT.  
THE CONTRACTOR SHALL RETAIN A LICENSED GEOTECHNICAL ENGINEER TO VERIFY THAT THE EXISTING SOIL CONDITIONS WILL PROVIDE A MINIMUM NET ALLOWABLE TOTAL LOAD BEARING PRESSURE OF 2000 PSF. LONG-TERM SETTLEMENT AT THIS BEARING PRESSURE SHALL NOT EXCEED 3/4 INCHES. DIFFERENTIAL SETTLEMENT ACROSS THE STRUCTURE SHALL NOT EXCEED ONE-HALF OF THE TOTAL SETTLEMENT. NOTIFY THE ARCHITECT/ENGINEER FOR FURTHER DIRECTION IF THE EXISTING SOIL CONDITIONS ARE NOT CAPABLE OF PROVIDING THE DEFINED FOUNDATION DESIGN CRITERIA.

**DESIGN LOADS**  
BUILDING STRUCTURE IS DESIGNED FOR THE FOLLOWING LOADS AND CRITERIA:  
RISK CATEGORY: I  
WIND: BASIC WIND SPEED (3-SECOND GUST): V = 105 MPH ULTIMATE  
WIND EXPOSURE CATEGORY: VASD = 81 MPH NOMINAL  
INTERNAL PRESSURE COEFFICIENT: C = .18

**CAST-IN-PLACE CONCRETE**  
CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES AND MINIMUM COMPRESSIVE STRENGTHS AT 28-DAYS.  
FOOTINGS: 3000 PSI WITH A MAX. W/C RATIO OF 0.50  
EXTERIOR SLABS: 4500 PSI WITH A MAX. W/C RATIO OF 0.45  
CONCRETE SHALL BE PROPORTIONED FOR A 2" TO 5" SLUMP RANGE AT THE POINT OF PLACEMENT.  
CEMENT SHALL BE TYPE I OR II CONFORMING TO ASTM C150. FLY ASH CONFORMING TO ASTM C618, TYPE C OR F MAY BE USED TO REPLACE A MAXIMUM OF 20% OF THE CEMENT OR 100 POUNDS PER CUBIC YARD OF CONCRETE, WHICHEVER IS LESS.  
AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33. AGGREGATES SHALL BE PROPORTIONED SUCH THAT MIX DESIGN SHALL CONTAIN A MINIMUM OF 50% COARSE AGGREGATES BY GRADATION REQUIREMENTS SET FORTH IN ASTM C33. COARSE AGGREGATE SHALL MEET NO. 67 GRADING REQUIREMENTS.  
EXTERIOR EXPOSED CONCRETE SHALL HAVE FROM 4 TO 7% ENTRAINED AIR.  
CONCRETE SHALL CONTAIN A WATER-REDUCING ADMIXTURE MEETING ASTM C494, TYPE A OR F, AT A DOSAGE TO PROVIDE THE NECESSARY FLOWABILITY AND WORKABILITY WITHIN THE SPECIFIED SLUMP RANGE.  
CONCRETE SHALL BE IN STRICT CONFORMANCE WITH THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE". NO ALUMINUM SHALL BE PLACED IN THE CONCRETE.  
CONCRETE PLACEMENT, CURING, AND HOT AND COLD WEATHER CONCRETING SHALL COMPLY WITH ACI 301, ACI 308R, ACI 305.1, ACI 306R, AND ACI 306.1.  
NO ELECTRICAL CONDUIT SHALL BE CAST IN A STRUCTURAL CONCRETE SYSTEM OR CONCRETE SLABS ON METAL DECK WITHOUT APPROVAL FROM THE ARCHITECT/ENGINEER.  
CAST-IN-PLACE CONCRETE SHALL BE OBTAINED FOR TESTING PER ASTM C172 AND TESTED AS FOLLOWS:  
a. OBTAIN ONE SET OF FOUR TEST CYLINDERS FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE LESS THAN 25 CUBIC YARDS, PLUS ONE SET OF FOUR TEST CYLINDERS FOR EACH ADDITIONAL 50 CUBIC YARDS OR FRACTION THEREOF.  
b. SLUMP: ONE TEST AT POINT OF DISCHARGE PER ASTM C143 FOR EACH SET OF TEST CYLINDERS TAKEN. PERFORM ADDITIONAL SLUMP TEST ON TRUCK LOADS WHEN CONSISTENCY SEEMS TO HAVE CHANGED.  
c. CONCRETE TEMPERATURE: ONE TEST PER ASTM C1064 FOR EACH SET OF TEST CYLINDERS TAKEN OR HOURLY WHEN AIR TEMPERATURE IS BELOW 40°F OR ABOVE 90°F.  
d. AIR CONTENT: VOLUMETRIC METHOD PER ASTM C173 OR PRESSURE METHOD PER ASTM C231 FOR EACH SET OF TEST CYLINDERS.  
e. COMPRESSION TEST SPECIMENS: ONE SET OF FOUR STANDARD CYLINDERS PER ASTM C31 AT THE SPECIFIED FREQUENCY.  
f. COMPRESSIVE STRENGTH TESTS: ONE SET OF FOUR CYLINDERS PER ASTM C39. TEST ONE CYLINDER AT 7-DAYS, TWO CYLINDERS AT 28-DAYS, AND HOLD ONE IN RESERVE TO BE TESTED AS DIRECTED.  
PERSONNEL TRAINED AND CERTIFIED IN CONCRETE SAMPLING SHALL PERFORM ALL CONCRETE TESTING AND SAMPLING. TEST RESULTS SHALL BE SUBMITTED TO THE ARCHITECT, ENGINEER, AND CONTRACTOR WITHIN 24 HOURS OF COMPLETING TESTS. CONCRETE TESTING SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY.  
SUBMIT THE CONCRETE MIX DESIGN TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.

**REINFORCING STEEL**  
REINFORCING SHALL MEET ASTM A615 - 60,000, UNLESS NOTED OTHERWISE.  
REINFORCING STEEL SHALL HAVE ADEQUATE COVERAGE AS INDICATED IN ACI 318 FOR THE GIVEN EXPOSURE. REINFORCING SHALL BE CONTINUOUS AND LAPPED A MINIMUM OF 24 INCHES OR 36 BAR DIAMETERS WHICHEVER IS GREATER, UNLESS OTHERWISE NOTED.  
REINFORCING SHALL BE DETAILED ACCORDING TO THE ACI DETAILING MANUAL AND SHALL BE PREPARED UNDER THE SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF KANSAS. PROVIDE CORNER LAP BARS TO MATCH IN SIZE AND SPACING OF ALL TRENCH FOOTING HORIZONTAL BARS. PROVIDE 30 POUNDS OF EXTRA BARS OF VARIOUS SIZES TO BE USED AS DIRECTED. INCLUDE LABOR FOR PLACING SAME.  
PROVIDE 3-INCH SLAB BOLSTER WITH CONTINUOUS BOTTOM PLATE AT 4'-0" MAXIMUM CENTERS FOR POSITIONING ALL FOOTING BOTTOM BARS.  
MARK EACH BUNDLE OF THE REINFORCING WITH WEATHERPROOF TAGS.  
SUBMIT THE REINFORCING STEEL SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.

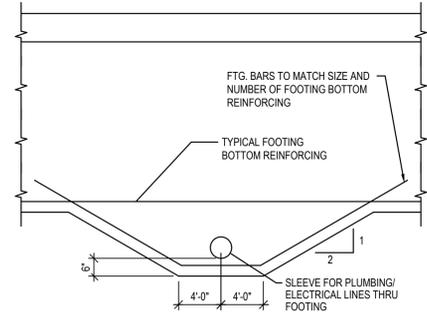
**STRUCTURAL STEEL**  
STRUCTURAL STEEL SHALL MEET THE LATEST AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS." THE TYPICAL STEEL FRAMING CONNECTION DETAILS SHOWN ON THE DRAWINGS REPRESENT THE GENERAL TYPE OF CONNECTION DETAIL EXPECTED TO BE IMPLEMENTED IN THE CONNECTION DESIGN, UNLESS SPECIFICALLY APPROVED OTHERWISE.  
STRUCTURAL STEEL SHOP DRAWINGS SHALL BE PREPARED UNDER THE SUPERVISION OF A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF KANSAS.  
STEEL PLATES AND SHAPES SHALL MEET ASTM A36. STRUCTURAL STEEL TUBING SHALL MEET ASTM A500, GRADE C, FY = 50 KSI.  
PROVIDE TENSION CONTROL BOLTS MEETING ASTM F3125, GRADE F1852 (TYPE 1) AT ALL PRETENSIONED (TYPE PT) BOLTED CONNECTIONS.  
UNHEADED ANCHOR RODS SHALL BE ASTM F1554, GRADE 36 OR ASTM F1554, GRADE 55 (SUPPLEMENT S1). THREADED STEEL RODS SHALL MEET ASTM A307, GRADE B; ASTM F1554, GRADE 36; OR AN APPROVED EQUAL OR GREATER STRENGTH THREADED ROD. THREADED RODS CAST IN CONCRETE OR POST-INSTALLED IN CONCRETE OR MASONRY SHALL BE THOROUGHLY CLEANED OF ALL SURFACE OILS.  
PROVIDE 3/8" PLATE WASHERS ABOVE ALL OVERSIZED HOLES (HOLE DIAMETERS GREATER THAN 1/16" LARGER THAN ANCHOR DIAMETER) IN THE COLUMN BASE PLATES. PROVIDE STANDARD HOLE SIZE IN PLATE WASHERS. ANCHOR RODS SET IN CONCRETE SHALL BE FURNISHED WITH DOUBLE NUTS AND SHALL BE SET WITH A TEMPLATE.  
PROVIDE STANDARD SIZE HOLES FOR ALL BOLTS AND ANCHORS IN STEEL FRAMING MEMBERS UNLESS NOTED OTHERWISE (1/16" LARGER HOLE THAN DIAMETER OF BOLT OR ANCHOR).  
WELDING SHALL CONFORM TO AWS D1.1, "STRUCTURAL WELDING CODE - STEEL". ALL WELDS SHALL BE AWS PREQUALIFIED WELDED JOINTS. NO UNAUTHORIZED WELDS WILL BE ACCEPTED.  
E70XX ELECTRODES SHALL BE USED FOR ALL WELDING, UNLESS NOTED OTHERWISE.  
GALVANIZED STRUCTURAL STEEL SHALL CONFORM TO ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION HARDWARE.  
HOT-DIP GALVANIZED STEEL FRAMING MEMBERS AS SPECIFIED WHERE SPECIFICALLY NOTED ON THE DRAWINGS. PROVIDE VENTING RELIEF HOLES AS REQUIRED BUT LOCATE ON THE BOTTOM SIDE OR AT SIMILAR NON-VISIBLE LOCATIONS WHERE THE MEMBERS ARE EXPOSED ON THE EXTERIOR OF THE BUILDING. SHOW OR NOTE THE LOCATIONS OF VENTING HOLES ON THE SHOP DRAWING SUBMITTAL.  
NON-METALLIC SHRINKAGE-RESISTANT GROUT SHALL MEET ASTM C 1107.  
TOUCH-UP PRIMER FOR GALVANIZED SURFACES SHALL BE SSPC-PAINT 20, TYPE II, ORGANIC WITH A METALLIC ZINC CONTENT OF 95 PERCENT BY WEIGHT IN DRY FILM AND A SOLIDS CONTENT OF 62 PERCENT BY VOLUME. APPLY TWO COATS WITH A MINIMUM 1.5 MILS DRY FILM THICKNESS PER COAT. USE ZRC GALVILITE GALVANIZING COMPOUND BY ZRC WORLDSIDE OR APPROVED EQUIVALENT.  
SUBMIT THE STRUCTURAL STEEL SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO BEGINNING CONSTRUCTION.

**POST-INSTALLED ANCHORS**  
POST-INSTALLED ANCHORS AND POST-INSTALLED REINFORCING BARS SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. HOLES SHALL BE DRILLED WITH THE REQUIRED BIT TYPE AND SIZE TO PROVIDE THE MINIMUM EMBEDMENT LENGTH SPECIFIED IN THE STRUCTURAL DRAWINGS. HOLES SHALL BE CLEANED PRIOR TO INSTALLING THE ANCHOR OR REINFORCING BAR WITH THE BRUSH AND COMPRESSED AIR METHOD OR WITH THE MANUFACTURER'S PROPRIETARY DRILL BIT AND DUST EXTRACTION SYSTEM.  
THE INSTALLATION OF ALL POST-INSTALLED ANCHORS AND POST-INSTALLED REINFORCING BARS SHALL BE PERFORMED BY PERSONNEL TRAINED AND CERTIFIED BY THE AMERICAN CONCRETE INSTITUTE/CONCRETE REINFORCING STEEL INSTITUTE OR TRAINED BY THE POST-INSTALLED ANCHOR AND/OR ADHESIVE MANUFACTURER FOR THE TYPE BEING USED.  
POST-INSTALLED ANCHOR TYPES SHALL BE AS FOLLOWS:  
ADHESIVE ANCHORS OR REINFORCING BARS INSTALLED INTO CONCRETE SHALL USE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM OR AN APPROVED EQUAL. HILTI HIT-RE 500 V3, SIMPSON STRONG-TIE SET-3G, DEWALT AC208+, AND DEWALT PURE 110+ ARE APPROVED EQUAL ADHESIVE ANCHORING SYSTEMS FOR ADHESIVE ANCHORS OR REINFORCING BARS INSTALLED INTO CONCRETE.

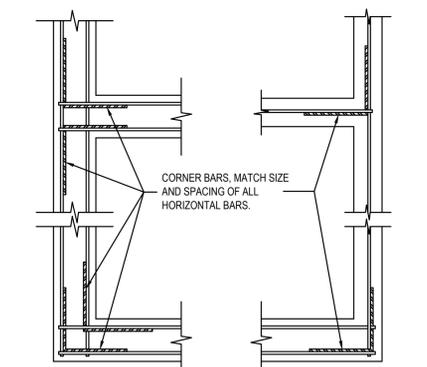
### SPECIAL REQUIREMENTS AT FOUNDATION SYSTEMS AND STRUCTURAL FRAMING SYSTEMS

- A. SLEEVES OR BLOCKOUTS IN TRENCH FOOTINGS**
- SLEEVES OR BLOCKOUTS 10 INCHES OR GREATER IN ANY DIRECTION MUST BE APPROVED PRIOR TO INSTALLING IN ANY TRENCH FOOTINGS.
  - LOCATE SLEEVES OR BLOCKOUTS WHERE THE EDGE OF THE OPENING CLOSEST TO THE TOP OR BOTTOM OF THE TRENCH FOOTING IS 12 INCHES OR GREATER UNLESS SPECIFICALLY APPROVED OTHERWISE.
  - PROVIDE 2#5 HORIZONTAL BARS, 4'-0" LONGER THAN THE OPENING DIMENSION AND CENTERED ON THE OPENING, ABOVE AND BELOW THE TRENCH FOOTING OPENINGS, ONE BAR EACH FACE. PROVIDE ONE ADDITIONAL TRENCH FOOTING STIRRUP PLACED WITH 2 INCHES TO 3 INCHES EACH SIDE OF THE TRENCH FOOTING SLEEVE OR BLOCKOUT FOR OPENINGS GREATER THAN 6 INCHES.
  - NO SLEEVES OR BLOCKOUTS SHALL OCCUR WITHIN THE COLUMNS OR PLASTERS CAST INTEGRAL WITH THE TRENCH FOOTINGS.
  - PROVIDE A MINIMUM OF 12 INCHES OF CONCRETE BETWEEN THE ADJACENT SLEEVES OR BLOCKOUTS UNLESS SPECIFICALLY APPROVED OTHERWISE.
  - CORED OPENINGS SHALL MEET THE SAME REQUIREMENTS AS SLEEVED OPENINGS.
- B. DIMENSIONAL AND ELEVATION CHECK AND TOLERANCE**
- AS SOON AS POSSIBLE AFTER THE COMPLETION OF THE NOTED ITEMS, THE CONTRACTOR SHALL PERFORM A DIMENSIONAL AND ELEVATION CHECK OF THE CONSTRUCTED ITEMS TO CONFIRM IF THE ITEMS HAVE BEEN BUILT WITHIN AN ACCEPTABLE TOLERANCE. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER FOR DIRECTION IF THE NOTED ITEM HAVE NOT BEEN BUILT WITHIN THE SPECIFIED TOLERANCE.
  - THE PLACEMENT OF ALL ANCHOR RODS OR EMBEDS IN THE CONCRETE FOUNDATION SYSTEMS FOR THE ATTACHMENT OF STRUCTURAL STEEL FRAMING AND PRECAST CONCRETE SYSTEMS SHALL BE CHECKED FOR ACCURACY IN THE DIMENSIONAL LOCATION AND ELEVATION. THE ACCEPTABLE TOLERANCE IN THE PLACEMENT OF ANCHOR RODS AND EMBEDS CAST INTO CONCRETE FOR STRUCTURAL STEEL AND PRECAST CONCRETE CONNECTIONS SHALL BE AS DEFINED IN ACI 117, SPECIFICATION FOR CONCRETE CONSTRUCTION AND MATERIALS AND COMMENTARY SECTION 2.3, AND THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES SECTION 7.  
a. IT IS RECOMMENDED THAT THE CHECK BE PERFORMED PRIOR TO DELIVERY OF THE FABRICATED STRUCTURAL STEEL MEMBERS TO THE SITE SO THAT ANY REQUIRED MODIFICATIONS OR ADJUSTMENTS TO THE STRUCTURAL MEMBERS CAN BE MADE AS DIRECTED BY THE ARCHITECT/ENGINEER IN THE SHOP RATHER THAN IN THE FIELD. FIELD CUTTING TO ENLARGE HOLES OR MODIFY FRAMING MEMBERS OR CONNECTING PLATES OR ANGLES SHALL NOT BE PERFORMED PRIOR TO RECEIVING DIRECTION FROM THE ARCHITECT/ENGINEER.

THICKEN FOOTING AS SHOWN WHERE SUBGRADE PLUMBING/ELECTRICAL LINES OCCUR LESS THAN 2'-0" BELOW BOTTOM OF FOOTING, TYPICAL



1 TYP. DETAIL  
NO SCALE



2 CORNER BAR DETAIL  
NO SCALE

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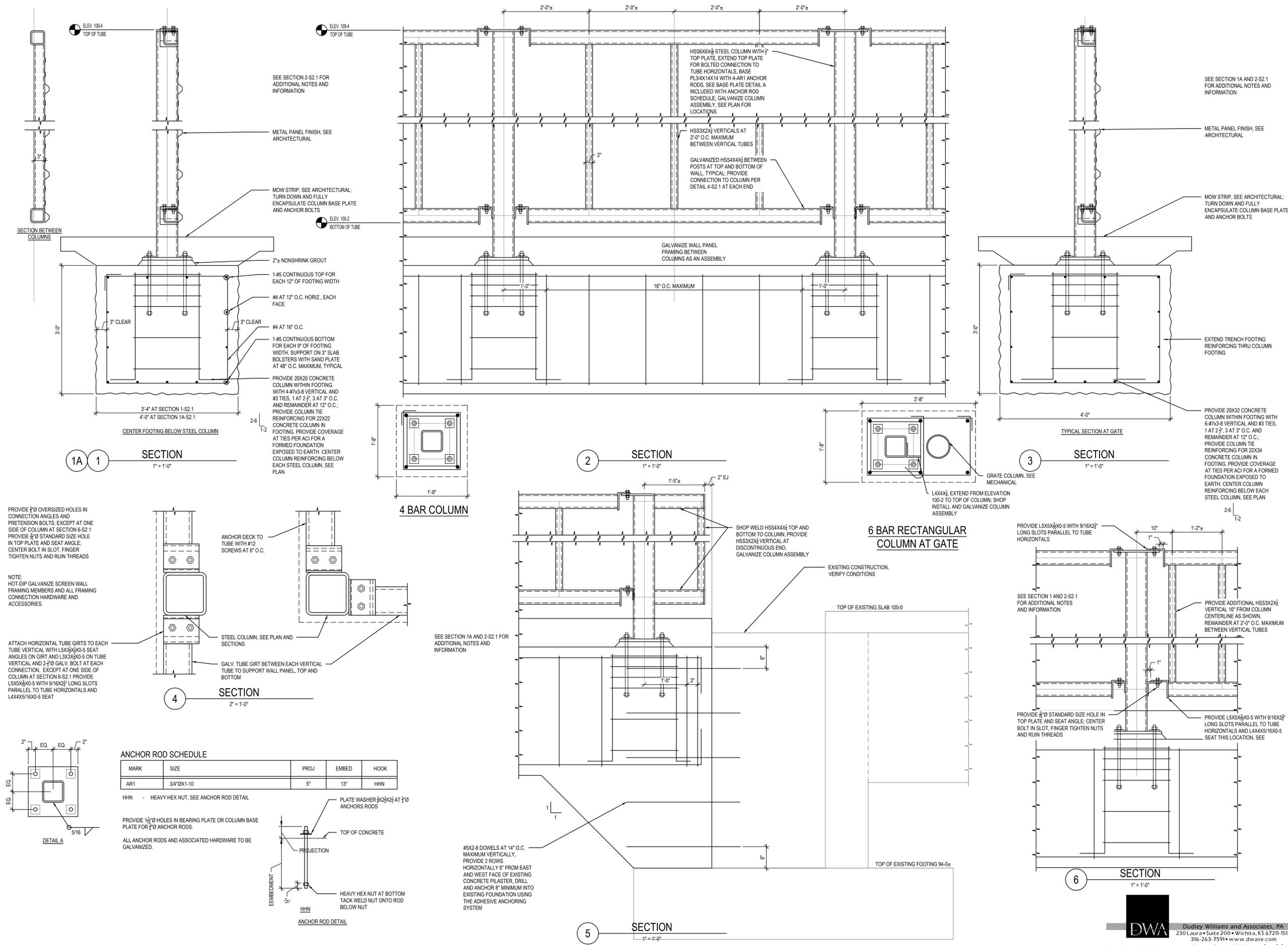
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SECTIONS AND DETAILS

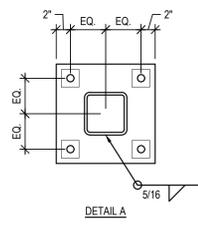
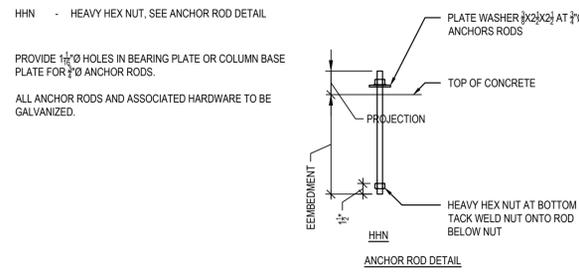
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S2.1

ORIGINAL CONTRACT DOCUMENTS



**ANCHOR ROD SCHEDULE**

MARK	SIZE	PROJ	EMBED	HOOK
AR1	3/4"ØX1-10	5"	13"	HHN



PROVIDE 3/8" OVERSIZED HOLES IN CONNECTION ANGLES AND PRETENSION BOLTS EXCEPT AT ONE SIDE OF COLUMN AT SECTION 6-S2.1 PROVIDE 3/8" STANDARD SIZE HOLE IN TOP PLATE AND SEAT ANGLE. CENTER BOLT IN SLOT, FINGER TIGHTEN NUTS AND RUIN THREADS

NOTE: HOT-DIP GALVANIZE SCREEN WALL FRAMING MEMBERS AND ALL FRAMING CONNECTION HARDWARE AND ACCESSORIES.

ATTACH HORIZONTAL TUBE GIRTS TO EACH TUBE VERTICAL WITH L5X5X10-5 SEAT ANGLES ON GIRT AND L3X3X10-5 ON TUBE VERTICAL AND 2x1/2 GALV. BOLT AT EACH CONNECTION, EXCEPT AT ONE SIDE OF COLUMN AT SECTION 6-S2.1 PROVIDE L5X5X10-5 WITH 9/16X2 1/2 LONG SLOTS PARALLEL TO TUBE HORIZONTALS AND L4X4X5/16X10-5 SEAT

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