

GENERAL STRUCTURAL NOTES

GENERAL NOTES:

- CONTRACTOR IS RESPONSIBLE FOR AND SHALL VERIFY AND COORDINATE ALL DIMENSIONS AND DETAILS BEFORE PROCEEDING WITH WORK. ANY DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ARCHITECT AND ENGINEERS.
 - DETAILS SHOWN IN ANY SECTION APPLY TO ALL SIMILAR SECTIONS AND CONDITIONS UNLESS NOTED OTHERWISE.
 - CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT ALL WORK IN PROGRESS UNTIL THE BUILDING IS COMPLETED.
 - ALL STRUCTURAL ITEMS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH APPROPRIATE PROVISIONS OF EACH OF THE FOLLOWING:
 - THE FLORIDA BUILDING CODE, (SEVENTH EDITION) 2020.
 - ACI STANDARD 318-14 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
 - BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402-16).
 - AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" 360-16
 - NDS FOR WOOD CONSTRUCTION WITH 2018 NDS SUPPLEMENT.
 - ASCE 7-16 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES".
 - THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS AND THE ARCHITECTURAL AND MECHANICAL DRAWINGS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO PERFORMING WORK. IN CASE OF CONFLICT THE MOST STRINGENT CONDITION SHALL APPLY.
 - ALL DIMENSIONS MUST BE COORDINATED WITH ARCHITECTURAL DRAWINGS AND WITH EQUIPMENT MANUFACTURER (I.E. WINDOW, DOOR, AIR HANDLER, ETC.). CONTRACTOR MUST OBTAIN AN ARCHITECTURAL DIRECTIVE IN CASE OF ANY CONFLICT. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN IN STRUCTURAL DRAWINGS.
 - ROOFTOP EQUIPMENT ANCHORAGE & OUTDOOR RACK MOUNTED EQUIPMENT ANCHORAGE: ALL ROOF TOP EQUIPMENT CURBS, ROOF TOP MECHANICAL EQUIPMENT, EQUIPMENT TIE DOWNS, AND CONNECTIONS OF ALL EQUIPMENT TO OUTDOOR RACKS OR BUILDING STRUCTURE FOR WIND LOADING ARE TO BE DESIGNED AND ENGINEERED BY A REGISTERED SPECIALTY ENGINEER RETAINED BY THE MECHANICAL EQUIPMENT SUPPLIER. SIGNED AND SEALED DRAWINGS AND CALCULATIONS ARE TO BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL. THE EQUIPMENT MANUFACTURER SHALL PROVIDE THE ATTACHMENT OF THE UNIT TO THE STRUCTURE AND SUBMIT TO THE ENGINEER LOADS, LOCATIONS, AND METHODS OF ATTACHMENT. THE STRUCTURAL ENGINEER WILL MAKE PROVISIONS IN THE DESIGN OF THE PRIMARY STRUCTURAL FRAME TO ACCOMMODATE THE LOADS AND ATTACHMENTS SUBMITTED BY THE MANUFACTURER.
 - ALLOWANCES FOR THIS PROJECT:
 - 50 CUBIC YARDS OF 4,000 PSI STRUCTURAL CONCRETE.
 - 5 TONS REINFORCED STEEL.
 - 5 TONS STRUCTURAL STEEL FRAMING.
 - \$50,000 ALLOWANCE FOR ADDITIONAL WOOD FRAMING AND CONNECTIONS.
- CONTRACTOR SHALL GIVE CREDIT TO OWNER FOR ANY UNUSED PORTION OF THIS ALLOWANCE AT THE END OF THE PROJECT.

CONCRETE AND REINFORCING:

- ALL CONCRETE WORK SHALL CONFORM TO THE LATEST ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI-318".
- ALL CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTHS AS INDICATED BELOW:

CONCRETE STRENGTH	MAX WATER CEMENT RATIO	TYPE AGGREGATE	LOCATION USED
4000 PSI	0.45	STONE	CONCRETE U.N.O
3000 PSI	0.52	STONE	SLAB ON GRADE AND FOUNDATIONS
- ALL REINFORCING STEEL SHALL BE INTERMEDIATE GRADE, NEW BILLET STEEL, DEFORMED BARS, CONFORMING TO ASTM A-615, GRADE 60. ALL BARS SHALL BE SECURELY SUPPORTED AND WIRED IN PLACE, PRIOR TO POURING CONCRETE. ALL REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A-706.
- ALL WELDED WIRE FABRIC (W.W.F.) IN FLAT SHEETS ONLY AND SHALL CONFORM TO ASTM A-185.
- UNLESS NOTED, ALL BARS MARKED CONTINUOUS SHALL BE SPLICED AT ALL LAP POINTS AND CORNERS AND DEVELOPED AT NON-CONTINUOUS ENDS AS PER TYPICAL DETAILS. SPLICE CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AND SPLICE CONTINUOUS BOTTOM BARS AT SUPPORTS.
- CONCRETE COVER FOR REINFORCING BARS SHOWN IN TYPICAL DETAILS.
- UNLESS NOTED, TEMPERATURE REINFORCING (ASTM A-615-60) TO BE 0.0018 X CONCRETE AREA.
- PROVIDE #4 @ 12" O.C., WITH STANDARD HOOK, TOP BARS IN ALL SLABS AT DISCONTINUOUS ENDS UNLESS OTHERWISE NOTED ON PLANS. LENGTH OF BARS 1/4 OF SPAN, MINIMUM 3'-0" UNLESS OTHERWISE NOTED PROVIDE #4 @ 12" O.C IN ALL CANTILEVERS, BAR LENGTH SHALL BE CANTILEVER SPAN PLUS 10'-0" PLUS STANDARD HOOK AT CANTILEVER ENDS.
- WHERE PIPE SLEEVES (UP TO 2" IN DIAMETER) PASS THROUGH CONCRETE BEAMS, PROVIDE ADDITIONAL STRIPP EACH SIDE OF SLEEVE, SLEEVES FOR PIPES 2" IN DIAMETER OR LARGER MUST BE STEEL OR CAST IRON, AND THE LOCATION MUST BE APPROVED BY THE STRUCTURAL ENGINEER.
- ALL CONSTRUCTION JOINTS SHALL BE THOROUGHLY CLEANED JUST BEFORE PLACING NEW CONCRETE IN ACCORDANCE WITH THE BUILDING CODE.
- FOR CHAMFER OF EXPOSED CORNERS OF BEAMS AND/OR COLUMNS, SEE ARCHITECTURAL DRAWINGS.
- CONTRACTOR SHALL COORDINATE PLACEMENT OF, OR BOX OUT FOR, ALL PIPE SLEEVES, OPENINGS, ETC. REQUIRED FOR VARIOUS TRADES.
- CONTRACTOR SHALL COORDINATE AND NOTIFY OTHER TRADES IN SUFFICIENT TIME TO ALLOW THEM TO SET ANCHORS, INSERTS, BOLTS, HANGERS, ETC., AS REQUIRED FOR THEIR USE.
- SEE ARCHITECTURAL DRAWINGS FOR DETAILS OF FLASHING REGLETS, FASCIA DETAILS, ETC.
- UNDER NO CIRCUMSTANCES SHALL CONCRETE BE PUMPED THROUGH ALUMINUM PIPES. CONCRETE SHALL NOT BE PLACED IN CONTACT WITH ALUMINUM, ALUMINUM MIXING DRUMS, TRUCK MIXERS, BUGGIES, CHUTES, CONVEYORS, TREMIE PIPES, AND OTHER EQUIPMENT MADE OF ALUMINUM SHALL NOT BE USED ON THIS PROJECT.
- SLUMPS OF OVER 4 INCHES WILL NOT BE PERMITTED UNLESS THE HRWR ADMIXTURE (SUPER PLASTICIZER) IS USED. MAXIMUM SLUMP IS THEN 8 INCHES UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- NO ADMIXTURE SHALL BE USED IN CONCRETE EXCEPT WITH THE PERMISSION OF THE ENGINEERS AND AFTER LABORATORY DESIGN MIX APPROVAL. ALL ADMIXTURES SHALL CONTAIN NO MORE CHLORIDE IONS THAN ARE PRESENT IN MUNICIPAL DRINKING WATER.
- WATER REDUCING ADMIXTURE SHALL CONFORM TO THE ASTM C-494, TYPE A, AND SHALL BE USED IN ALL CONCRETE.
- AIR ENTRAINING ADMIXTURE SHALL CONFORM TO ASTM C260. AIR CONTENT OF CONCRETE SHALL BE USED AS FOLLOWS:
 - FOR CONCRETE EXPOSED TO SOIL AND/OR WEATHER, 5%.
 - FOR INTERIOR WALLS, COLUMNS, AND SLABS, 3%.
- FLY ASH - ASTM C618, TYPE C OR TYPE F SHOULD BE USED BUT NOT TO EXCEED 20% CEMENTITIOUS CONTENT.

CONCRETE AND REINFORCING:

- ALL EXPOSED CONCRETE SHALL RECEIVE A CURING COMPOUND, THE CURING COMPOUND SHALL CONFORM TO ASTM C309 AND SHALL HAVE 30% SOLIDS. MINIMUM WATER/BLANKET CURING AS PER ACI RECOMMENDATION MAY BE USED OR ALTERNATE.
- UNLESS NOTED IN PROJECT SPECIFICATIONS, A TESTING LAB SHOULD PERFORM THE FOLLOWING TEST:
 - ATTENDANCE AT THE PROJECT SITE DURING ALL CONCRETE PLACING OPERATIONS
 - CONTROL THE ADDITION OF MIXING WATER TO MAINTAIN THE REQUIRED WATER/CEMENT RATIO AND INDICATED IN THE REPORT ANY ADDED WATER TO THE MIX AND THE LOCATION OF PLACEMENT.
 - ENSURE THAT THE CONCRETE IS OF THE PROPER TEMPERATURE WHEN PLACED.
 - AIR CONTENT TESTS - AT LEAST TWO TESTS SHALL BE MADE FOR EACH DAY'S PLACING OR FROM EACH BATCH OF CONCRETE FROM WHICH CYLINDERS ARE CAST.
 - SLUMP TESTS - AT FREQUENT INTERVALS TO PROPERLY CONTROL THE CONSISTENCY AND AT LEAST ONE AT THE TIME OF CASTING EACH GROUP OF CYLINDERS AND AT LEAST ONE TEST FOR EVERY 25 CUBIC YARDS.
 - CONCRETE COMPRESSION CYLINDERS SHALL BE TAKEN FROM THE CONCRETE OF EACH STRENGTH PLACED ON ANY ONE DAY AT LEAST ONE SET OF FIVE REPRESENTATIVE 6" X 12" TEST CYLINDERS. FOR LARGE PLACEMENTS ON ANY ONE DAY, THERE SHALL BE TAKEN NOT LESS THAN ONE SET OF FIVE REPRESENTATIVE TYPE CYLINDERS FOR EACH 100 CUBIC YARDS OF CONCRETE OF EACH STRENGTH PLACED. TWO CYLINDERS ARE TO BE TESTED AT 7 DAYS, TWO AT THE AGE OF 28 DAYS, AND THE FIFTH CYLINDER IN RESERVE FOR FURTHER TESTING. ASCERTAIN THAT THE TEST SPECIMENS ARE PROPERLY PROTECTED UNTIL SHIPPED TO THE TESTING LABORATORY. RECORD AND IDENTIFY EACH CYLINDER WITH THE LOCATION OF THE CONCRETE FROM WHICH THE SPECIMEN WAS TAKEN. KEEP MARKING IN SEQUENCE.

FOUNDATION NOTES:

- SITE SOIL FOR THIS PROJECT HAS BEEN INVESTIGATED BY THE FIRM OF ANDERSEN ANDRE CONSULTING ENGINEERS, INC. AND FOUND AS PRESENTED IN THEIR REPORT DATED APRIL 07, 2022, SUITABLE TO SUPPORT 2.5 KSF SPREAD FOOTINGS. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE ABOVE STATED CRITERIA.
- FILL AND SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER RECOMMENDATION AS CONTAINED IN THEIR REPORT STATED IN ITEM 1.
- ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES UNLESS OTHERWISE NOTED.
- BACKFILLING AGAINST FOUNDATION WALLS SHALL BE DONE CAREFULLY WITH SMALL COMPACTION EQUIPMENT. AFTER SLABS ON GROUND ARE IN PLACE AND CONCRETE HAS SET, NO TRUCKS, BULLDOZERS, ETC. SHALL BE ALLOWED CLOSER THAN 6'-0" TO ANY FOUNDATION WALL. ANY WALL 3'-0" OR HIGHER MUST BE BRACED DURING THE CONSTRUCTION PROCESS.
- NO FOUNDATIONS SHALL BE PLACED ABOVE 1' VERTICAL ON 2' HORIZONTAL SLOPES EXTENDED FROM THE CLOSEST EDGE OF ANY UNDISTURBED SOIL OR OTHER FOUNDATION STRUCTURE. BOTTOM OF FOOTINGS SHALL NOT BE LESS THAN 1'-0" BELOW EXISTING GRADE (U.N.O.).
- FOR FOUNDATIONS SIZE AND REINFORCING SEE SCHEDULE.
- CONTRACTOR SHALL TREAT SOIL BENEATH BUILDING FOR TERMITES.
- TERMITE PROTECTION INCLUDING PIPING SLEEVES MUST FOLLOW THE REQUIREMENTS OF SECTIONS 1816.1 AND 1816.2 OF THE FLORIDA BUILDING CODE, 7TH EDITION, 2020

MASONRY:

- DESIGN AND CONSTRUCTION SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ASCE 7-16) / TMS 402-16.
- MINIMUM NET COMPRESSIVE STRENGTH OF BLOCK ASSEMBLY SHALL BE 2000 P.S.I. (7m) MORTAR FOR MASONRY SHALL BE TYPE "S" OR "M".
- FOR ALL EXTERIOR AND INTERIOR BEARING, BED JOINTS ARE TO COVER 100% OF THE MASONRY SURFACES AND ALL HEAD JOINTS ARE TO COVER 100% OF THE PROJECTED AREA OF THE FACE SHELLS.
- FILL ALL CELLS AS REQUIRED WITH 3000 P.S.I. GROUT. SLUMP SHALL BE 8 TO 11 INCHES. SUBMIT DESIGN MIX FOR APPROVAL.
- MINIMUM HORIZONTAL JOINT REINFORCING SHALL BE 9 GAUGE HOT DIP GALVANIZED TRUSS OR LADDER TYPE JOINT REINFORCING AT 16" O.C. PROVIDE MANUFACTURE "T" AND "L" SHAPES FOR INTERSECTIONS AND CORNERS. (MINIMUM LAP 6").
- MINIMUM VERTICAL REINFORCING SHALL BE (1)-#4 OR (1)-#4 @ 32" O.C. (U.N.O.).
- PROVIDE ADDITIONAL VERTICAL REINFORCING BAR AT EVERY CORNER, INTERSECTION, CONTROL JOINT, AND OPENING EDGES (U.N.O.).
- MINIMUM SPLICE FOR VERTICAL REINFORCING IS SHOWN IN DETAIL 4-023. SPLICE FOR HORIZONTAL JOINT REINFORCING = 6".
- WALLS ARE DESIGNED TO BE BRACED BY FLOOR OR ROOF MEMBERS. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING DURING CONSTRUCTION.
- ALL CELLS BELOW FIRST FLOOR FINISHED ELEVATION MUST BE FULLY GROUT FILLED.
- ALL KNOCK OUT BLOCK HORIZONTAL BARS SHALL HAVE CORNER BARS AT ALL CORNERS AND WALL INTERSECTIONS. SIZE AND NUMBER OF CORNER BARS SHALL BE SAME AS HORIZONTAL BARS.
- ALL INTERSECTING WALLS AND CORNER WALLS SHALL BE LAID IN AN OVERLAPPING MASONRY BONDING PATTERN, WITH ALTERNATE UNITS HAVING A BEARING OF NOT LESS THAN 3 INCHES ON UNIT BELOW.

SHOP DRAWINGS:

- NO STRUCTURAL DRAWINGS SHALL BE REPRODUCED FOR USE AS SHOP DRAWINGS.
- ALL DIMENSIONAL COORDINATION SHALL BE DONE BY THE CONTRACTOR AND/OR HIS DETAILER.
- DETAILER SHALL CHECK ALL ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL ATTACHMENTS, CLIPS, OPENINGS, OR DUCT WORK AFFECTING STRUCTURAL MEMBERS. ALL ITEMS SHALL BE SHOWN ON SHOP DRAWINGS.
- ALL SHOP DRAWINGS SHALL BE SUBMITTED ELECTRONICALLY IN PDF FORMAT. DISTRIBUTION AS PER ARCHITECT INSTRUCTIONS.
- PROVIDE SUFFICIENT SPACE ON SHOP DRAWINGS NEAR TITLE BOX (ABOUT 40 SQUARE INCHES) FOR STAMPS AND ENGINEERS COMMENTS.
- THE SHOP DRAWINGS SHALL BEAR INITIALS OF DETAILER'S CHECKER AND CONTRACTOR PRIOR TO SUBMISSION.
- COMPLETED ERECTION PLANS SHALL BE SUBMITTED PRIOR TO OR IN CONJUNCTION WITH DETAIL DRAWINGS. BUT IN NO CASE SHALL DETAIL DRAWINGS BE SUBMITTED PRIOR TO ERECTION PLANS.
- DETAILER SHALL SUBMIT AN INDEX OF THE DETAIL DRAWINGS WITH EACH SHOP DRAWING SUBMITTAL.
- SHOP DRAWINGS NOT COMPLYING WITH ALL THE ABOVE ITEMS SHALL BE RETURNED FOR CORRECTIONS WITHOUT PROCESSING.
- RESUBMISSION OF SHOP DRAWINGS SHALL HAVE THE FOLLOWING CHANGES INCORPORATED: FIRST RESUBMISSION TO HAVE LETTER "A" ADDED TO DRAWING
 - NUMBER AND ANY CHANGES MARKED ON THE DRAWING MARKED 1 AT THE END OF EACH SHEET. ALL ITEMS TO BE NOTED IN REVISION BOX.
 - SUBSEQUENT RESUBMISSION SHALL BEAR CHANGES "B" AND 2 AND 3 ETC. AS IN 11A.
- CONTRACTOR SHALL HAVE SHOP DRAWINGS WHICH HAVE BEEN SATISFACTORILY REVIEWED BY THE ARCHITECT AND/OR ENGINEER AND APPROVED BY THE CONTRACTOR BEFORE PROCEEDING WITH ANY WORK.
- DETAILER SHALL USE THE SAME STRUCTURAL ELEMENTS NUMBERS IN HIS DETAILS AS THOSE SHOWN ON CONTRACT DRAWINGS.
- SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOULD BE SUBMITTED TO MCE WITH A MINIMUM TIME TO BE REVIEWED OF 10 WORKING DAYS. IN CASE OF A LARGE SUBMITTAL OR MORE THAN ONE SUBMITTAL FOR THE SAME PROJECT, AN ADDITIONAL WORKING DAY IS REQUIRED FOR EVERY 5 DRAWINGS/SHEETS OVER 30 DRAWINGS/SHEETS. THE TIME INDICATED ABOVE IS FOR MCE REVIEW ONLY. CONTRACTOR MUST INCLUDE ENOUGH TIME FOR DELIVERY, ARCHITECTURAL REVIEW, AND OWNERS REVIEW AND WORK THIS TIME IN THE PROJECT SCHEDULE, AS NEEDED.
- THERE SHALL BE NO DEVIATION FROM THESE CONSTRUCTION DOCUMENTS. IF ANY CHANGES ARE PROPOSED BY THE CONTRACTOR OR THE PROVIDER OF THE SHOP DRAWINGS, THEY SHOULD BE CLEARLY INDICATED, SIGNED AND SEALED DRAWINGS AND CALCULATIONS BY A FLORIDA PROFESSIONAL ENGINEER MUST BE PROVIDED. ANY CHANGES WITHOUT PROPER DOCUMENTATION INDICATED ABOVE WILL RESULT IN SOME REVISIONS BY THE ENGINEER OF RECORD AND/OR ARCHITECT. THE COST FOR THESE REVISIONS INCLUDING ENGINEER AND ARCHITECTURAL FEES SHALL BE PAID BY THE CONTRACTOR.

DELEGATED DESIGN

- SELECT SCOPE ITEMS IN THE PROJECT ARE CUSTOM DESIGNED AND ENGINEERED. THE ENGINEERING RESPONSIBILITY IS DELEGATED TO THE CONTRACTOR AND RELATED SUBCONTRACTORS.
- CONTRACTOR SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR SUCH ELEMENTS DESIGNATED TO BE DESIGNED BY A DELEGATED ENGINEER.
- DELEGATED ENGINEERING WILL ADDRESS ALL LOADING REQUIREMENTS INCLUDING WIND PRESSURES IN ACCORDANCE WITH THE LATEST FLORIDA BUILDING CODE. REFER TO THE COMPONENTS AND CLADDING PRESSURES PROVIDED FOR DESIGN PRESSURES ELEMENTS SHALL BE IN CONFORMANCE WITH.
- DELEGATED ENGINEERED DRAWINGS SHALL DEFINE MATERIAL THICKNESS, SIZING, CONNECTIONS, ETC. OF THE SUBMITTED SYSTEM.
- DELEGATED ENGINEERED DRAWINGS AND CALCULATIONS WILL BE REVIEWED AS PART OF THE SUBMITTAL PROCESS.
- BUILDING COMPONENTS THAT ARE NOT SPECIFIED AS DELEGATED TO OTHER ENGINEERS SHALL BE SUBMITTED WITH APPROPRIATE FLORIDA PRODUCT APPROVAL INFORMATION IN THE SUBMITTAL WHERE A FLORIDA PRODUCT APPROVAL DOES NOT EXIST FOR A COMPONENT REQUIRING APPROVAL, THE DESIGN SHALL BE DELEGATED TO AN ENGINEER ON THE CONTRACTOR'S TEAM.
- DELEGATED ENGINEERING AND DEFERRED SUBMITTALS:
 - DEFERRED SUBMITTALS SHALL HAVE THE SHOP DRAWINGS AND DELEGATED DESIGN SUBMITTALS (INCLUDING CALCULATIONS) SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.
 - DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE REGISTERED DESIGN PROFESSIONAL FOR REVIEW AND SHALL BE FORWARDED TO THE BUILDING OFFICIAL.
 - DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
 - THE FOLLOWING ITEMS ARE CONSIDERED DEFERRED SUBMITTALS BY THE REGISTERED DESIGN PROFESSIONAL:
 - EXTERIOR CURTAIN WALL SYSTEM
 - EXTERIOR AWNINGS, SUNSHADES, EYEBROWS AND CANOPIES
 - METAL FABRICATIONS, RAILINGS, LADDERS AND GRATINGS
 - PRE-FABRICATED / PRE-ENGINEERED WOOD TRUSSES
 - STRUCTURAL STEEL CONNECTIONS
 - ROOF TOP EQUIPMENT AND ANCHORAGES
 - STRUCTURAL PRECAST CONCRETE

WOOD FRAMING NOTES:

- ALL WOOD FRAMING SHALL BE IN COMPLIANCE WITH THE LATEST NDS EDITION FOR WOOD CONSTRUCTION. DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION.
- ALL LUMBER SHALL BE SOUND, SEASONED, AND FREE FROM WARP.
- ALL FRAMING LUMBER SHALL BE SOUTHERN YELLOW PINE #2 OR BETTER.
- INTERIOR NON-LOAD BEARING WALLS SHALL BE UTILITY GRADE OR BETTER.
- MINIMUM OF 3-PLY STUD COLUMNS TO BE INSTALLED AT BEAM OR GIRDER TRUSS BEARING LOCATIONS UNLESS NOTED OTHERWISE.
- INSTANT BLOCKING IN ALL WALL STUDS OVER 8'-0" AT MID-HEIGHT, AND SHEATHING JOINT, BRACE GABLE END AT 4'-0" O.C. AS SHOWN IN THE DRAWINGS.
- ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED OR OF NATURAL DURABLE WOOD.
- PRESSURE TREATED LUMBER SHALL BE IMPREGNATED WITH AN APPROVED TREATMENT IN ACCORDANCE WITH F.S. 11W-571 AND BARE THE AMERICAN WOOD PRESERVES INSTITUTE EQUALITY MARK LP-2.
- SHEATHING SHALL BE APA EXTERIOR GRADE RATED, AND INSTALLED WITH PLY-CLIPS AT 24" O.C. SEE NAILING SCHEDULE FOR SHEATHING CONNECTION.
- FLOOR SHEATHING SHALL BE A MINIMUM OF 5/8" TONGUE AND GROOVE TYPE SUPPORTED AT 24" O.C. MAX. UNLESS NOTED OTHERWISE IN PLAN.
- ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF EXTERIOR SHEATHING REQUIREMENTS. ALL NAILS EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED.
- ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG TIE OR APPROVED EQUAL. SUBMIT CUT SHEETS FOR ALL CONNECTION HARDWARE TO ENGINEER FOR APPROVAL. ALL NAIL HOLES SHALL BE FILLED OR AS REQUIRED BY THE MANUFACTURER TO ACHIEVE LOG CAPACITY.
- BRACING: TEMPORARY BRACING OF THE ROOF SYSTEM SHALL BE INSTALLED PER BC51-13 RECOMMENDATIONS AND SHALL BE UTILIZED AS THE PERMANENT BRACING FOR THE ROOF SYSTEM, UNLESS NOTED OTHERWISE.
- ALL WOOD FRAMING SHALL BE IN COMPLIANCE WITH THE LATEST NDS EDITION FOR WOOD CONSTRUCTION.

PRE-ENGINEERED WOOD PRODUCTS:

- ALL PRE-ENGINEERED WOOD PRODUCTS SHALL BE VERIFIED BY TRUSS MANUFACTURER. TRUSS MANUFACTURER SHALL HAVE THE AUTHORITY TO MAKE SUBSTITUTIONS FOR PRODUCTS SPECIFIED ON THE PLANS DUE TO AVAILABILITY OR ECONOMICS. CHANGES SPECIFIED BY THE TRUSS MANUFACTURER SHALL CONTROL. CHANGES MADE AFTER TRUSS ENGINEERING HAS BEEN PROVIDED TO ENGINEER OF RECORD, MUST BE APPROVED BY THE ENGINEER OF RECORD.
- FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY. TRUSS MANUFACTURER TO PROVIDE SEPARATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER.
- ALL PRE-ENGINEERED WOOD PRODUCTS ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS ENGINEER IS A DELEGATED ENGINEER FOR THIS PROJECT, AND AS SUCH, IS RESPONSIBLE FOR THE VALIDITY OF THE COMPONENTS PROVIDED. FRAMING LAYOUTS SHOWN MAY BE CHANGED BY THE TRUSS MANUFACTURER. THE DELEGATE ENGINEER IS RESPONSIBLE FOR PROVIDING A FINAL SEALED SET OF ALL CALCULATIONS AND LAYOUTS FOR THIS PROJECT TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO MANUFACTURE OF SAID COMPONENTS. ENGINEER OF RECORD HAS NOT REVIEWED THE PRE-ENGINEERED TRUSS MANUFACTURER'S COMPONENTS AT THIS TIME AND RESERVES THE RIGHT TO MAKE ANY CHANGES AFTER SUCH INFORMATION HAS BEEN PROVIDED FOR REVIEW. CONTRACTOR, AS PROJECT COORDINATOR, SHALL BE RESPONSIBLE FOR INSURING INFORMATION REQUESTED ABOVE HAS BEEN SUBMITTED TO ENGINEER OF RECORD IN A TIMELY MANNER WHEN AVAILABLE.
- ALL PRE-ENGINEERED TRUSSES TO BE DESIGNED USING THE MOST RECENT TPI CRITERIA. TRUSSES TO BE HANDLED AND INSTALLED USING MOST RECENT HIB RECOMMENDATIONS. TEMPORARY AND PERMANENT BRACING SHALL BE IN ACCORDANCE WITH BC51-13 RECOMMENDATIONS UNLESS NOTED OTHERWISE. OR MORE STRINGENT CODE REQUIREMENTS APPLY. TRUSS ENGINEER IS RESPONSIBLE FOR INDICATING ALL TRUSS TO TRUSS CONNECTORS, ALL COMPONENTS TO BE DESIGNED FOR BOTH GRAVITY AND UPLIFT LOAD CASES, INCLUDING BEAM COMPONENTS.
- UPON REVIEW, ENGINEER OF RECORD WILL PROVIDE A REVIEW LETTER INDICATING ANY CHANGE IN STRAPPING OR SUPPORT BASED ON THAT REVIEW. CONSTRUCTION COMMENCING PRIOR TO ENGINEER'S REVIEW IS SUBJECT TO MODIFICATION BASED ON REVIEW LETTER.

POST-INSTALLED ANCHORS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. SPECIAL INSPECTIONS ARE REQUIRED PER THE PROVISIONS SET FORTH BELOW. CONTRACTOR TO CONTACT MANUFACTURER'S REPRESENTATIVE FOR PROPER PRODUCT INSTALLATION TRAINING ON INITIAL ANCHORS.
- SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT CORE BITS, ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- EXPANSION ANCHORS SHALL BE STUD TYPE WITH A SINGLE PIECE OF THREE SECTION WEDGE AND ZINC PLATED IN ACCORDANCE WITH ASTM B633. THE ANCHORS SHALL MEET FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE A, CLASS I FOR CONCRETE EXPANSION ANCHORS. ANCHORS SHALL BE HILTI KWIK BOLT 3 AS SUPPLIED BY HILTI INC. TULSA OKLAHOMA. ANCHORS SHALL BE INSTALLED IN HOLES DRILLED WITH HILTI CARBIDE TIPPED DRILL BITS OR MATCHED TOLERANCE DIAMOND CORE BITS. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- INJECTED ADHESIVE ANCHORS SHALL BE USED FOR INSTALLATION OF THREADED RODS. ADHESIVE SHALL BE FURNISHED IN A SIDE BY SIDE REFILL PACK WHICH KEEP COMPONENT A AND B SEPARATE. INJECTION ADHESIVE SHALL BE HILTI HIT HY 200 AS SUPPLIED BY HILTI INC. TULSA OKLAHOMA. ANCHOR RODS MEET ASTM F1554 (36 KSI), NUTS AND WASHERS SHALL BE FURNISHED TO MEET THE REQUIREMENTS OF AN ASTM F1554 (36 KSI) STEEL ROD.

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LEAVE THIS AREA BLANK FOR ELECTRONIC SEAL

SEAL:

PROJECT: STATION #7

FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

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COMM. NO: 050219VB
DATE: 15 AUG. 2023
BY: AN
CH'KD: JBCV

SHEET NO.
S-1.1
OF TWENTY EIGHT



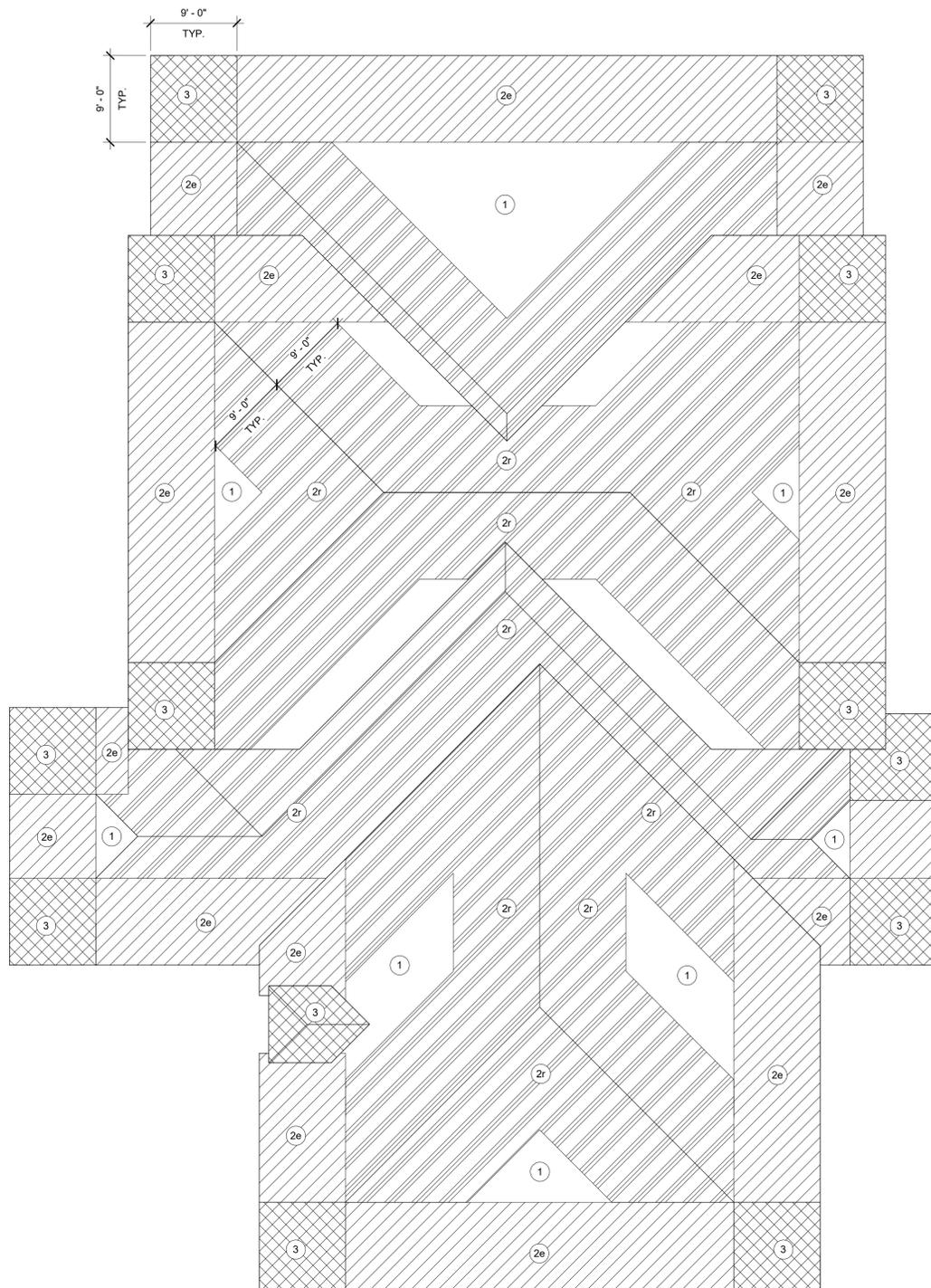
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22/08/2023 3:35:32 PM



Component	Zone	Pres (+ve) (psf)
10 sf	1	+59 PSF/106 PSF
10 sf	2e	+59 PSF/146 PSF
10 sf	2r	+59 PSF/146 PSF
10 sf	3	+59 PSF/146 PSF
10 sf	1oh	-138 PSF
10 sf	2eoh	-176 PSF
10 sf	2roh	-176 PSF
10 sf	3oh	-208 PSF
20 sf	1	+51 PSF/94 PSF
20 sf	2e	+51 PSF/130 PSF
20 sf	2r	+51 PSF/130 PSF
20 sf	3	+51 PSF/130 PSF
20 sf	1oh	-137 PSF
20 sf	2eoh	-169 PSF
20 sf	2roh	-169 PSF
20 sf	3oh	-186 PSF
50 sf	1	+40 PSF/78 PSF
50 sf	2e	+40 PSF/110 PSF
50 sf	2r	+40 PSF/110 PSF
50 sf	3	+40 PSF/110 PSF
50 sf	1oh	-135 PSF
50 sf	2eoh	-160 PSF
50 sf	2roh	-160 PSF
50 sf	3oh	-157 PSF
100 sf	1	+32 PSF/66 PSF
100 sf	2e	+32 PSF/95 PSF
100 sf	2r	+32 PSF/95 PSF
100 sf	3	+32 PSF/95 PSF
100 sf	1oh	-134 PSF
100 sf	2eoh	-153 PSF
100 sf	2roh	-153 PSF
100 sf	3oh	-135 PSF
200 sf	1	+32 PSF/66 PSF
200 sf	2e	+32 PSF/95 PSF
200 sf	2r	+32 PSF/95 PSF
200 sf	3	+32 PSF/95 PSF
200 sf	1oh	-133 PSF
200 sf	2eoh	-146 PSF
200 sf	2roh	-146 PSF
200 sf	3oh	-112 PSF
500 sf	1	+32 PSF/66 PSF
500 sf	2e	+32 PSF/95 PSF
500 sf	2r	+32 PSF/95 PSF
500 sf	3	+32 PSF/95 PSF
500 sf	1oh	-133 PSF
500 sf	2eoh	-146 PSF
500 sf	2roh	-146 PSF
500 sf	3oh	-112 PSF

LOAD SCHEDULE:

ROOF:

TOP CHORD DEAD LOADS

ROOFING & INSULATION = 12 PSF
 PLYWOOD SHEATHING = 3 PSF
 ROOF TRUSSES = 6 PSF
 21 PSF

BOTTOM CHORD DEAD LOADS

MECH / ELECT / PLUMB = 5 PSF
 CEILING & MISC. = 5 PSF
 10 PSF

DEAD LOAD (AVAILABLE TO RESIST UPLIFT) = 8 PSF

LIVE LOAD = 20 PSF

WIND DESIGN DATA:

CODE: FLORIDA BUILDING CODE, 2020 (7th Ed.) ASCE/SEI 7-16

BASIC WIND SPEED = 179 mph (Vult)
 139 mph (Vasd)

CATEGORY (RISK) = IV
 EXPOSURE = C
 BUILDING HEIGHT = 30 ft
 ENCLOSURE CLASSIFICATION = ENCLOSED
 INTERNAL PRESSURE COEFFICIENT = ± 0.18

* GLAZED OPENINGS IN RISK CATEGORY II, III, IV LOCATED IN HURRICANE PRONE REGIONS SHALL BE PROTECTED IN ACCORDANCE WITH FBC 2020 SEC. 1609.1.2

COMPONENTS AND CLADDING (ULTIMATE) UPLIFT PRESSURE SCHEDULE

PATTERN	ZONE
[Pattern]	1
[Pattern]	2r
[Pattern]	3
[Pattern]	2e
[Pattern]	2' ov
[Pattern]	3' ov

COMPONENTS AND CLADDING DESIGN WIND PRESSURE (ULTIMATE) FOR WALLS, DOORS & WINDOWS

PATTERN	ZONE
[Pattern]	4
[Pattern]	5

NOTE:

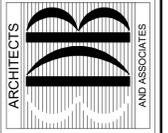
- ALL EXTERIOR DOORS & WINDOW ASSEMBLIES SHALL SATISFY THE REQUIREMENTS OF THE FLORIDA BUILDING CODE (SEVENTH EDITION 2020, SECTION 1709.5). ALL CONNECTIONS TO BUILDING STRUCTURE SHALL HAVE THE CAPACITY TO WITHSTAND THE PRESSURES INDICATED IN THIS SCHEDULE. PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.
- ALL WIND PRESSURE VALUES INDICATED ARE IN POUNDS PER SQUARE FOOT (PSF). MULTIPLY ULTIMATE WIND PRESSURE BY 0.60 FOR ASD PRESSURE.
- FOR NET UPLIFT CALCULATION THE WEIGHT OF THE STRUCTURAL MEMBER AND THE STRUCTURAL DECK SUPPORTED ARE THE ONLY TWO LOADS THAT CAN BE DEDUCTED FROM THE UPLIFT PRESSURES INDICATED ABOVE.

Component	Zone	Pres (+ve) (psf)
10 sf	4	+79 PSF/86 PSF
10 sf	5	+79 PSF/106 PSF
20 sf	4	+76 PSF/82 PSF
20 sf	5	+76 PSF/99 PSF
50 sf	4	+71 PSF/78 PSF
50 sf	5	+71 PSF/89 PSF
100 sf	4	+67 PSF/74 PSF
100 sf	5	+67 PSF/82 PSF
200 sf	4	+64 PSF/70 PSF
200 sf	5	+64 PSF/75 PSF
500 sf	4	+59 PSF/66 PSF
500 sf	5	+59 PSF/66 PSF

WIND DESIGN - ROOF PLAN
 SCALE: 1/8" = 1'-0"

WIND DESIGN DATA AND LOAD SCHEDULE

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 VERO BEACH, FLORIDA 32960
 PHONE: (772) 569-4320



SEAL:

PROJECT: **STATION #7**
 FOR: INDIAN RIVER COUNTY FIRE DISTRICT
 1840 25TH STREET
 VERO BEACH, FL 32960

NO.	DATE	REVISIONS
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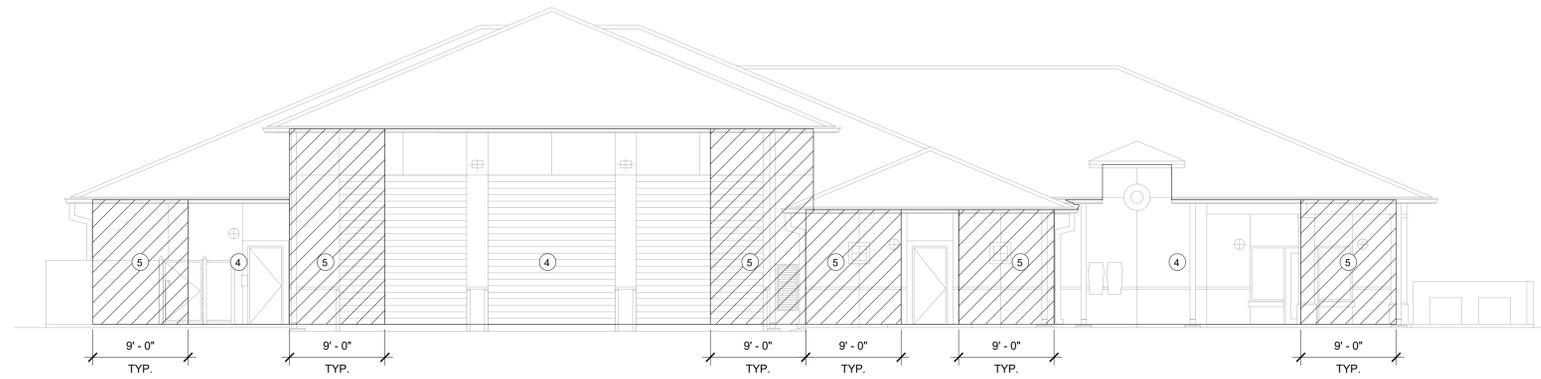
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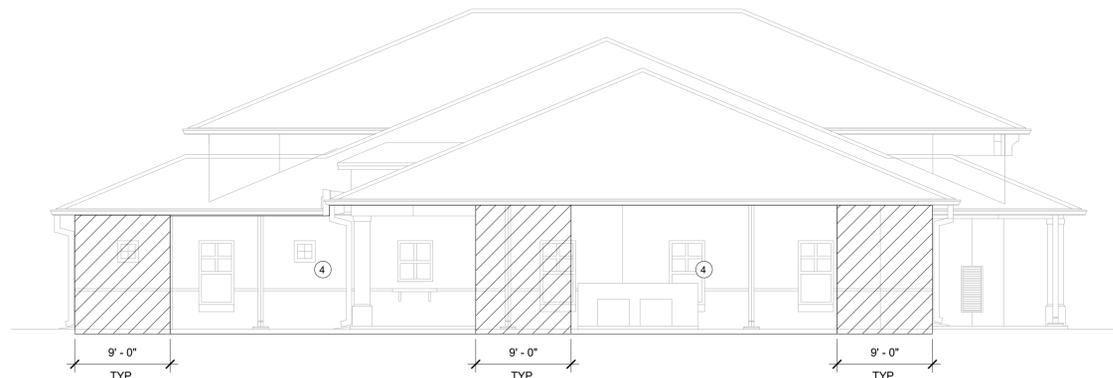
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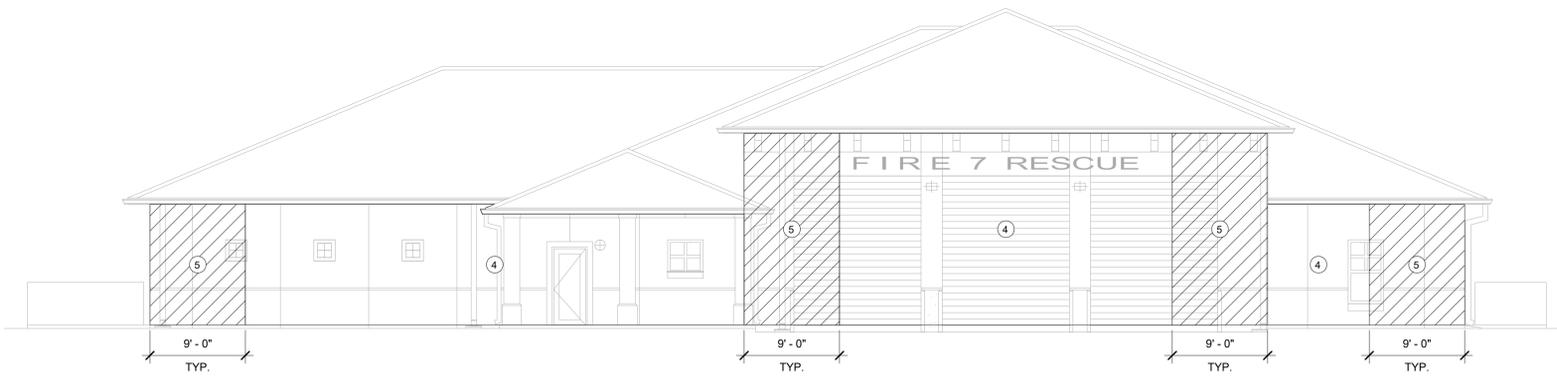
N WIND ELEVATION - NORTH
1/8" = 1'-0"



E WIND ELEVATION - EAST
1/8" = 1'-0"



W WIND ELEVATION - WEST
1/8" = 1'-0"



S WIND ELEVATION - SOUTH
1/8" = 1'-0"

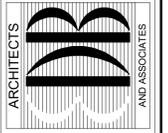
Component	Zone	Pres (+ve) (psf)
10 sf	4	+79 PSF/-86 PSF
10 sf	5	+79 PSF/-106 PSF
20 sf	4	+76 PSF/-82 PSF
20 sf	5	+76 PSF/-99 PSF
50 sf	4	+71 PSF/-78 PSF
50 sf	5	+71 PSF/-89 PSF
100 sf	4	+67 PSF/-74 PSF
100 sf	5	+67 PSF/-82 PSF
200 sf	4	+64 PSF/-70 PSF
200 sf	5	+64 PSF/-75 PSF
500 sf	4	+59 PSF/-66 PSF
500 sf	5	+59 PSF/-66 PSF

COMPONENTS AND CLADDING
DESIGN WIND PRESSURE
(ULTIMATE) FOR WALLS,
DOORS & WINDOWS

PATTERN	ZONE
	4
	5

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VERO BEACH, FLORIDA 32960
PHONE: (772) 569-4320



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PROJECT: **STATION #7**
FOR: INDIAN RIVER COUNTY FIRE DISTRICT
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WIND DESIGN DATA AND LOAD SCHEDULE

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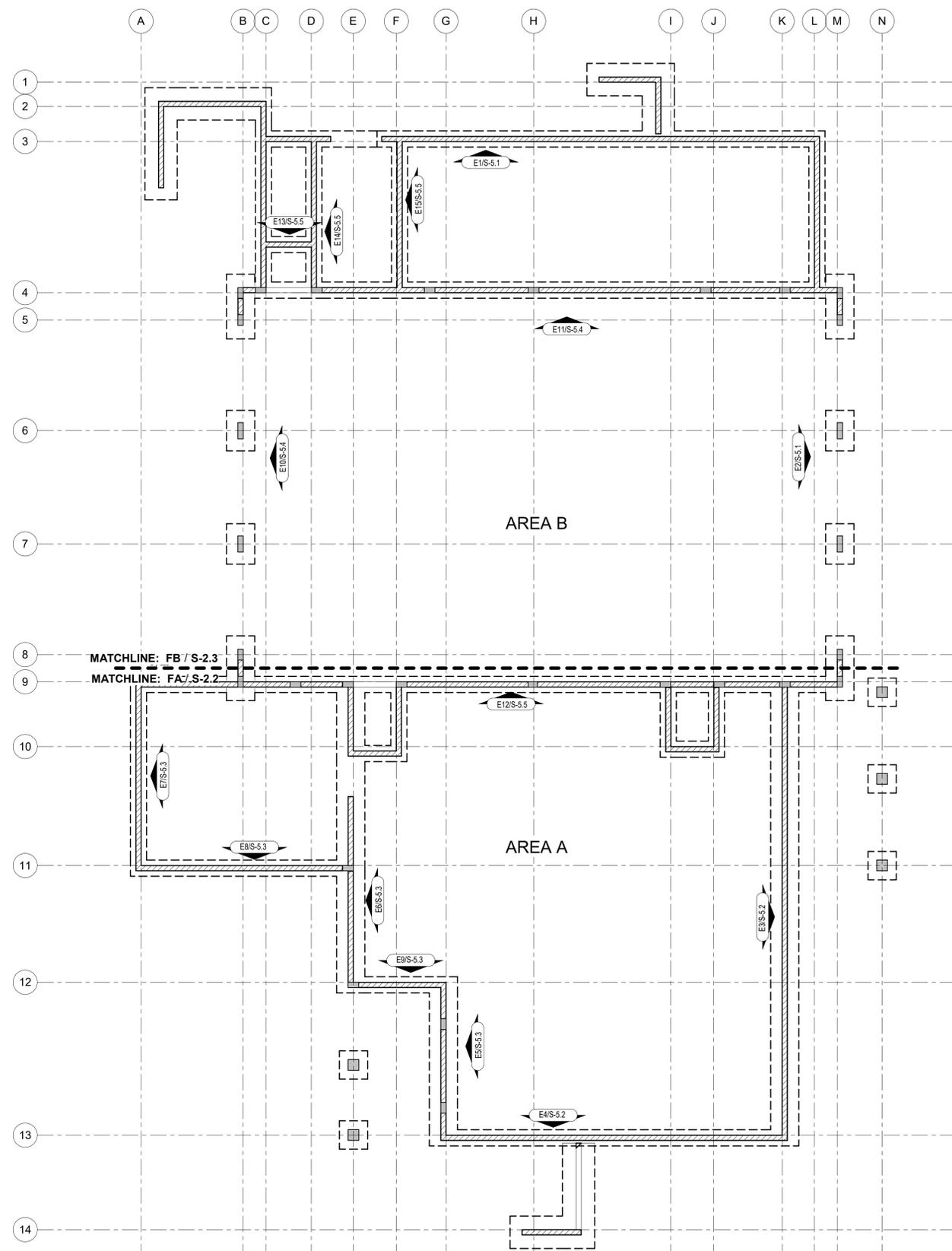
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SHEET NO. **S-2.1**
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00 FOUNDATION - OVERALL PLAN
1/8" = 1'-0"

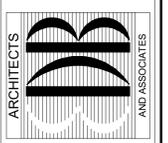


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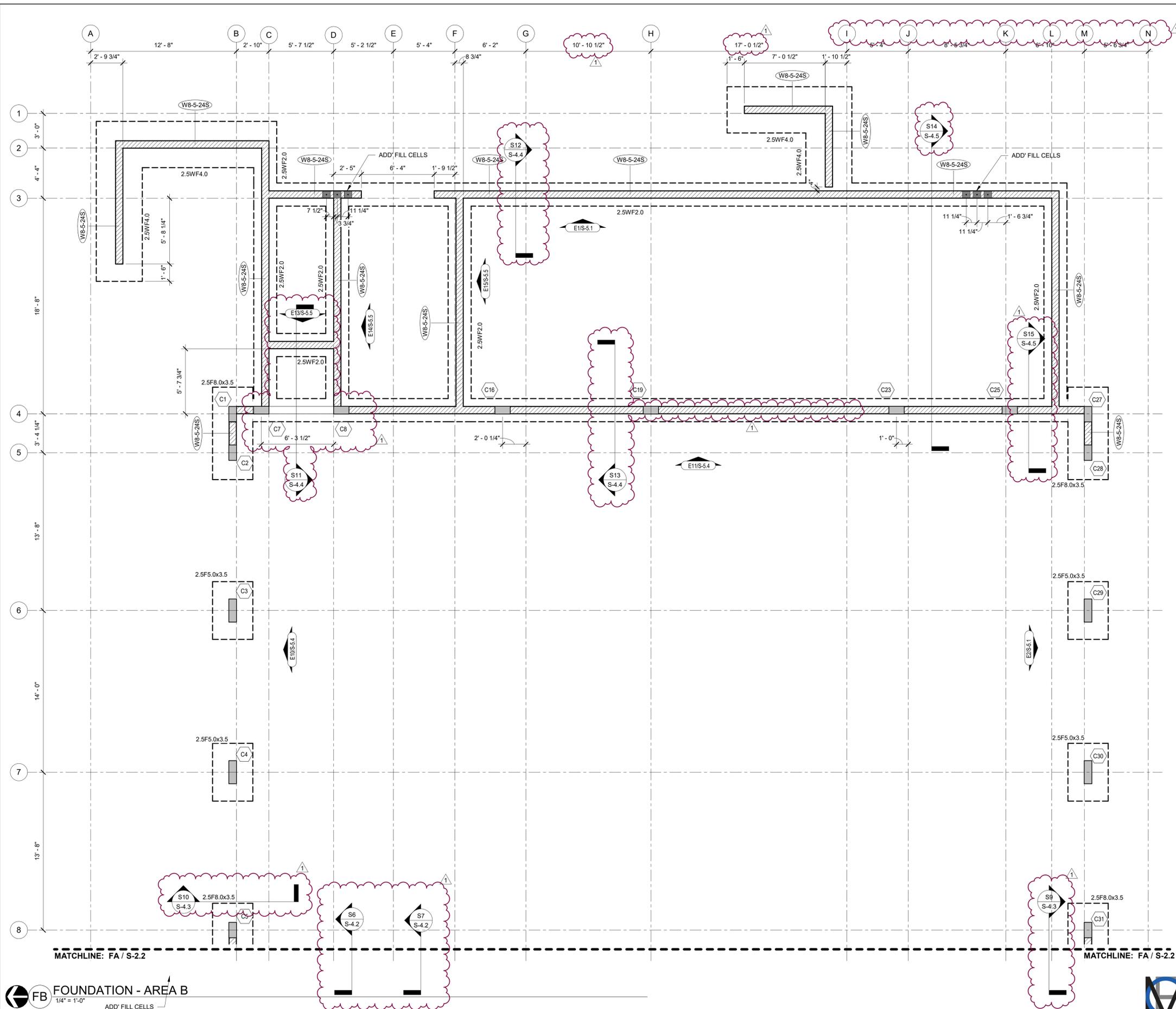
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WALL SCHEDULE

TYPE	WALL THICKNESS	TYPE OF WALL	WALL REINF.	REMARKS
W8-5-24S	7.5/8"	CMU	#5 AT 24" O.C.	SEE ELEVATION FOR ADD. REINFORCEMENT

COLUMN FOOTING SCHEDULE

MARK*	LENGTH	WIDTH	THICKNESS	TOP & BOTT. REINF.†	REMARKS
2.5F3.5	3'-6"	3'-6"	1'-6"	(4)#5	STD ACI HOOK
2.5F5.0x3.5	5'-0"	3'-6"	1'-6"	(4)#5 SHORT SIDE (6)#5 LONG SIDE	STD ACI HOOK
2.5F8.0x3.5	8'-0"	3'-6"	1'-6"	(4)#5 SHORT SIDE (12)#5 LONG SIDE	STD ACI HOOK

WALL FOOTING SCHEDULE

MARK*	WIDTH	THICKNESS	TOP & BOTT. REINF. CONT.	TOP & BOTT. REINF. TRANSV.	REMARKS
2.5WF2.0	2'-0"	1'-4"	(3)#5	#4 @ 24"	
2.5WF3.5	3'-6"	1'-4"	(5)#5	#4 @ 12"	
2.5WF4.0	4'-0"	1'-4"	(5)#5	#5 @ 8"	

* WHERE FOOTING MARK NUMBER HAS A SUFFIX B, I.E., WF5.0B, THE REINFORCING INDICATED SHALL BE APPLIED TO BOTTOM OF FOOTING ONLY.

- FOUNDATION PLAN NOTES:**
- FOR TYPICAL FOUNDATION DETAILS, SEE DRAWINGS S-3.1.
 - FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-1.1.
 - FOR FOOTING SIZE AND REINFORCING, SEE SCHEDULE ON THIS DRAWING.
 - () DENOTES TOP OF FOOTING ELEVATION.
 - TOP OF FOOTING: ELEVATION @ INTERIOR = (-1' - 4")
TYPICAL U.N.O.
ELEVATION @ EXTERIOR = (-1' - 4")
TYPICAL U.N.O.
 - FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
 - TYPICAL WALL REINFORCING SCHEDULE:
 - A. FOR CMU WALL REINFORCING, SEE SCHEDULE THIS SHEET. REINFORCING SHOWN ON SCHEDULE IS MINIMUM. SEE NOTES BELOW AND WALL ELEVATIONS FOR ADDITIONAL REINFORCING.
 - B. ■ INDICATES ADDITIONAL (1) VERTICAL IN GROUT FILLED CELL PROVIDE ADDITIONAL (1) VERTICAL IN FIRST (2) CELLS EACH SIDE OF ALL WALL OPENINGS, CORNERS AND INTERSECTIONS.
 - COLUMN DESIGNATION SHOWN THUS ○ ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-6.1.
 - T/COLUMN ELEVATION
 - COLUMN TYPE
 - CARRIED COLUMN
 - ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN WITH DOWELS TO MATCH U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
 - WHERE FOOTING MARK NUMBER HAS A SUFFIX B, I.E., WF5.0B, THE REINFORCING INDICATED SHALL BE APPLIED TO BOTTOM OF FOOTING ONLY.

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PROJECT: STATION #7
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REVISIONS

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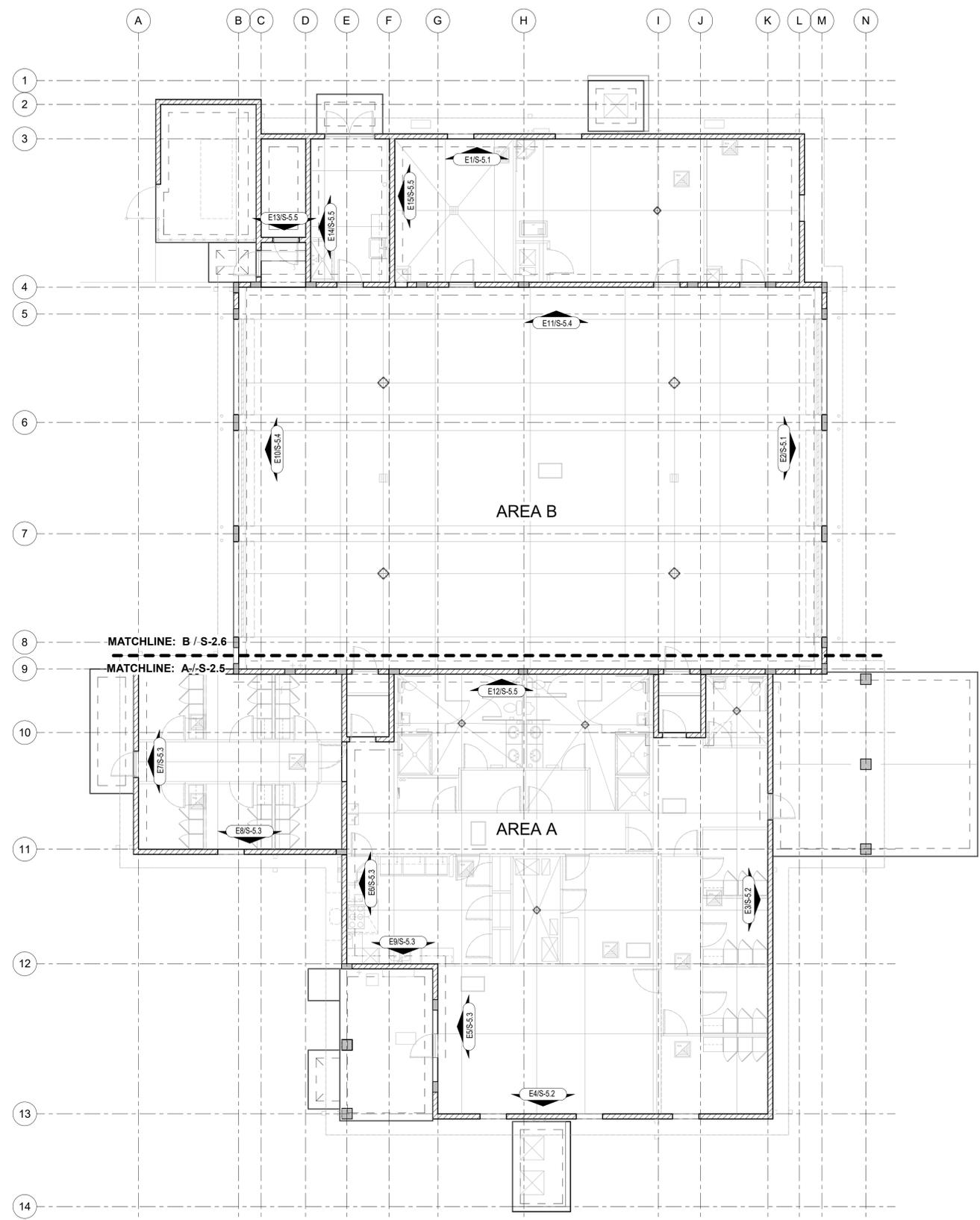
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FOUNDATION - AREA B
1/4" = 1'-0"
ADD FILL CELLS

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1 SLAB ON GRADE - OVERALL PLAN
1/8" = 1'-0"

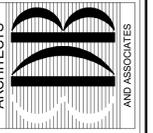


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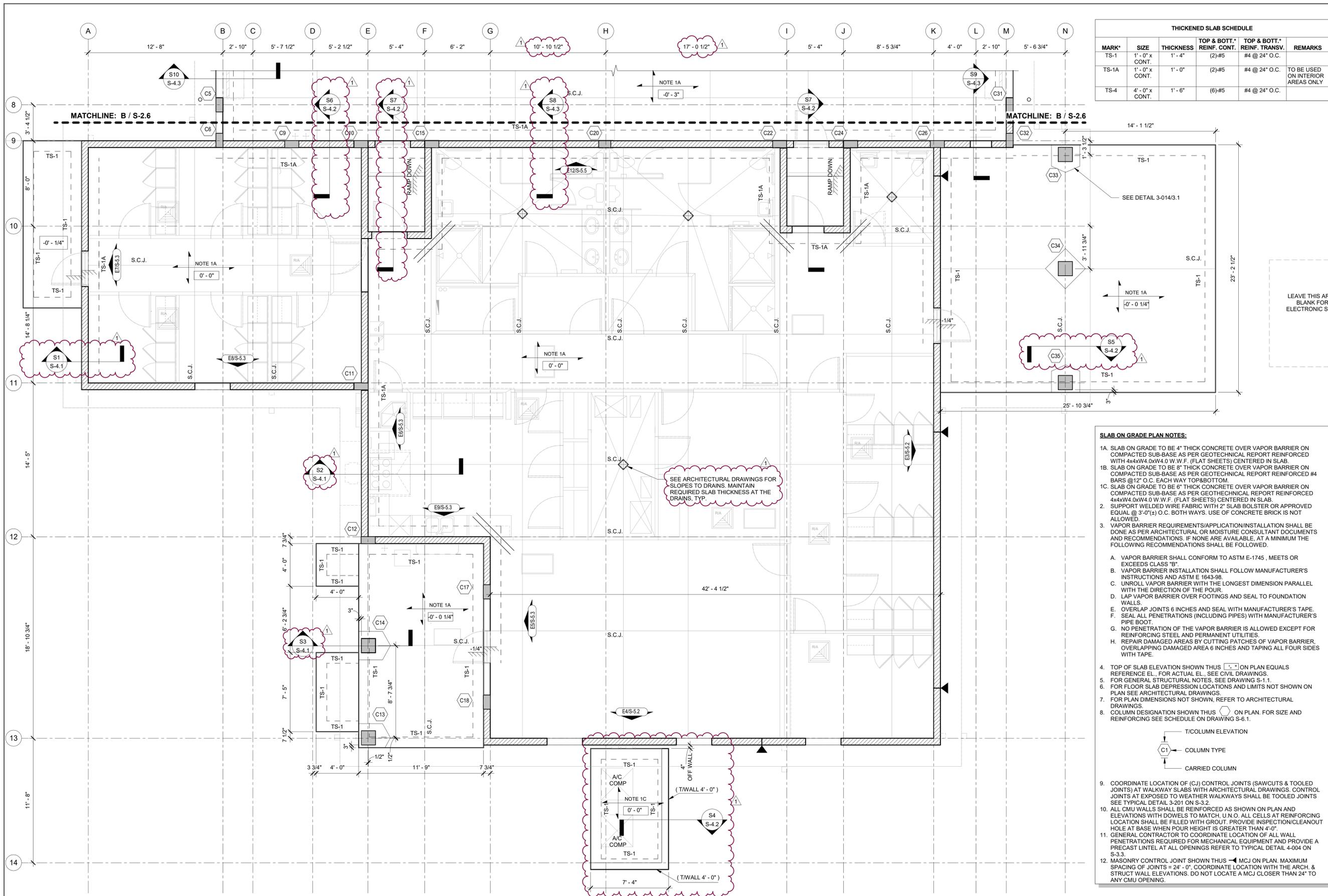
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THICKENED SLAB SCHEDULE					
MARK*	SIZE	THICKNESS	TOP & BOTT. REINF. CONT.	TOP & BOTT. REINF. TRANSV.	REMARKS
TS-1	1'-0" x CONT.	1'-4"	(2)#5	#4 @ 24" O.C.	
TS-1A	1'-0" x CONT.	1'-0"	(2)#5	#4 @ 24" O.C.	TO BE USED ON INTERIOR AREAS ONLY
TS-4	4'-0" x CONT.	1'-6"	(6)#5	#4 @ 24" O.C.	

- SLAB ON GRADE PLAN NOTES:**
- 1A. SLAB ON GRADE TO BE 4" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED WITH 4x4xW4.0xW4.0 W.W.F. (FLAT SHEETS) CENTERED IN SLAB.
 - 1B. SLAB ON GRADE TO BE 8" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED #4 BARS @ 12" O.C. EACH WAY TOP&BOTTOM.
 - 1C. SLAB ON GRADE TO BE 6" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED 4x4xW4.0xW4.0 W.W.F. (FLAT SHEETS) CENTERED IN SLAB.
 2. SUPPORT WELDED WIRE FABRIC WITH 2" SLAB BOLSTER OR APPROVED EQUAL @ 3'-0"(±) O.C. BOTH WAYS. USE OF CONCRETE BRICK IS NOT ALLOWED.
 3. VAPOR BARRIER REQUIREMENTS/APPLICATION/INSTALLATION SHALL BE DONE AS PER ARCHITECTURAL OR MOISTURE CONSULTANT DOCUMENTS AND RECOMMENDATIONS. IF NONE ARE AVAILABLE, AT A MINIMUM THE FOLLOWING RECOMMENDATIONS SHALL BE FOLLOWED.
 - A. VAPOR BARRIER SHALL CONFORM TO ASTM E-1745, MEETS OR EXCEEDS CLASS "B".
 - B. VAPOR BARRIER INSTALLATION SHALL FOLLOW MANUFACTURER'S INSTRUCTIONS AND ASTM E 1643-98.
 - C. UNROLL VAPOR BARRIER WITH THE LONGEST DIMENSION PARALLEL WITH THE DIRECTION OF THE POUR.
 - D. LAP VAPOR BARRIER OVER FOOTINGS AND SEAL TO FOUNDATION WALLS.
 - E. OVERLAP JOINTS 6 INCHES AND SEAL WITH MANUFACTURER'S TAPE.
 - F. SEAL ALL PENETRATIONS (INCLUDING PIPES) WITH MANUFACTURER'S PIPE BOOT.
 - G. NO PENETRATION OF THE VAPOR BARRIER IS ALLOWED EXCEPT FOR REINFORCING STEEL AND PERMANENT UTILITIES.
 - H. REPAIR DAMAGED AREAS BY CUTTING PATCHES OF VAPOR BARRIER, OVERLAPPING DAMAGED AREA 6 INCHES AND TAPING ALL FOUR SIDES WITH TAPE.
 4. TOP OF SLAB ELEVATION SHOWN THUS \square ON PLAN EQUALS REFERENCE EL., FOR ACTUAL EL., SEE CIVIL DRAWINGS.
 5. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-1.1.
 6. FOR FLOOR SLAB DEPRESSION LOCATIONS AND LIMITS NOT SHOWN ON PLAN SEE ARCHITECTURAL DRAWINGS.
 7. FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
 8. COLUMN DESIGNATION SHOWN THUS \square ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-6.1.
 - \square COLUMN ELEVATION
 - \square COLUMN TYPE
 - \square CARRIED COLUMN
 9. COORDINATE LOCATION OF (CJ) CONTROL JOINTS (SAWCUTS & TOOLED JOINTS) AT WALKWAY SLABS WITH ARCHITECTURAL DRAWINGS. CONTROL JOINTS AT EXPOSED TO WEATHER WALKWAYS SHALL BE TOOLED JOINTS SEE TYPICAL DETAIL 3-201 ON S-3.2.
 10. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN AND ELEVATIONS WITH DOWELS TO MATCH U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
 11. GENERAL CONTRACTOR TO COORDINATE LOCATION OF ALL WALL PENETRATIONS REQUIRED FOR MECHANICAL EQUIPMENT AND PROVIDE A PRECAST LINTEL AT ALL OPENINGS REFER TO TYPICAL DETAIL 4-004 ON S-3.3.
 12. MASONRY CONTROL JOINT SHOWN THUS \triangleleft MCJ ON PLAN. MAXIMUM SPACING OF JOINTS = 24' - 0". COORDINATE LOCATION WITH THE ARCH. & STRUCT WALL ELEVATIONS. DO NOT LOCATE A MCJ CLOSER THAN 24" TO ANY CMU OPENING.

SLAB ON GRADE - AREA A
1/4" = 1'-0"

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PHONE: (772) 569-4320

ARCHITECTS

PROJECT: **STATION #7**

FOR: **INDIAN RIVER COUNTY FIRE DISTRICT**
1840 25TH STREET
VERO BEACH, FL 32960

NO. DATE REVISIONS

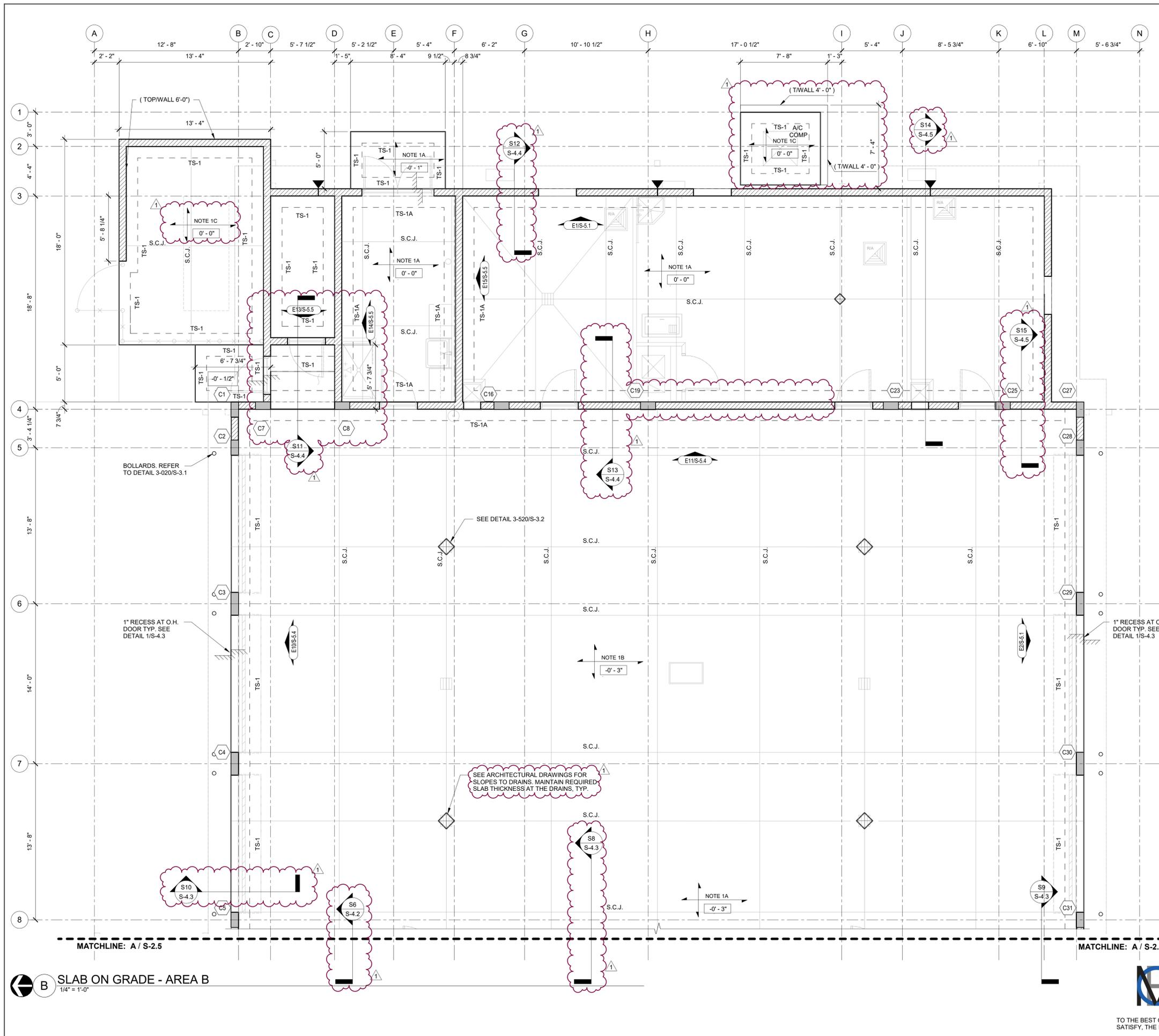
1 08/15/23 REVISION 1

SLAB ON GRADE - AREA A

COMM. NO: 050219VB
DATE: 15 AUG. 2023
BY: AN
CHK'D: JBCV

SHEET NO. **S-2.5**
OF TWENTY EIGHT

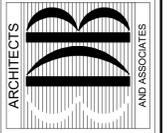
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THICKENED SLAB SCHEDULE					
MARK*	SIZE	THICKNESS	TOP & BOTT. REINF. CONT.	TOP & BOTT. REINF. TRANSV.	REMARKS
TS-1	1' - 0" x CONT.	1' - 4"	(2)-#5	#4 @ 24" O.C.	
TS-1A	1' - 0" x CONT.	1' - 0"	(2)-#5	#4 @ 24" O.C.	TO BE USED ON INTERIOR AREAS ONLY
TS-4	4' - 0" x CONT.	1' - 6"	(6)-#5	#4 @ 24" O.C.	

- SLAB ON GRADE PLAN NOTES:**
- 1A. SLAB ON GRADE TO BE 4" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED WITH 4x4xW4.0xW4.0 W.W.F. (FLAT SHEETS) CENTERED IN SLAB.
 - 1B. SLAB ON GRADE TO BE 8" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED #4 BARS @ 12" O.C. EACH WAY TOP&BOTTOM.
 - 1C. SLAB ON GRADE TO BE 6" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED 4x4xW4.0xW4.0 W.W.F. (FLAT SHEETS) CENTERED IN SLAB.
 2. SUPPORT WELDED WIRE FABRIC WITH 2" SLAB BOLSTER OR APPROVED EQUAL @ 3'-0"(±) O.C. BOTH WAYS. USE OF CONCRETE BRICK IS NOT ALLOWED.
 3. VAPOR BARRIER REQUIREMENTS/APPLICATION/INSTALLATION SHALL BE DONE AS PER ARCHITECTURAL OR MOISTURE CONSULTANT DOCUMENTS AND RECOMMENDATIONS. IF NONE ARE AVAILABLE, AT A MINIMUM THE FOLLOWING RECOMMENDATIONS SHALL BE FOLLOWED.
 - A. VAPOR BARRIER SHALL CONFORM TO ASTM E-1745, MEETS OR EXCEEDS CLASS "B".
 - B. VAPOR BARRIER INSTALLATION SHALL FOLLOW MANUFACTURER'S INSTRUCTIONS AND ASTM E 1643-98.
 - C. UNROLL VAPOR BARRIER WITH THE LONGEST DIMENSION PARALLEL WITH THE DIRECTION OF THE POUR.
 - D. LAP VAPOR BARRIER OVER FOOTINGS AND SEAL TO FOUNDATION WALLS.
 - E. OVERLAP JOINTS 6 INCHES AND SEAL WITH MANUFACTURER'S TAPE.
 - F. SEAL ALL PENETRATIONS (INCLUDING PIPES) WITH MANUFACTURER'S PIPE BOOT.
 - G. NO PENETRATION OF THE VAPOR BARRIER IS ALLOWED EXCEPT FOR REINFORCING STEEL AND PERMANENT UTILITIES.
 - H. REPAIR DAMAGED AREAS BY CUTTING PATCHES OF VAPOR BARRIER, OVERLAPPING DAMAGED AREA 6 INCHES AND TAPING ALL FOUR SIDES WITH TAPE.
 4. TOP OF SLAB ELEVATION SHOWN THUS \square ON PLAN EQUALS REFERENCE EL., FOR ACTUAL EL., SEE CIVIL DRAWINGS.
 5. FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-1.1.
 6. FOR FLOOR SLAB DEPRESSION LOCATIONS AND LIMITS NOT SHOWN ON PLAN SEE ARCHITECTURAL DRAWINGS.
 7. FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
 8. COLUMN DESIGNATION SHOWN THUS \square ON PLAN. FOR SIZE AND REINFORCING SEE SCHEDULE ON DRAWING S-6.1.
 - \square T/COLUMN ELEVATION
 - \square COLUMN TYPE
 - \square CARRIED COLUMN
 9. COORDINATE LOCATION OF (CJ) CONTROL JOINTS (SAWCUTS & TOOLED JOINTS) AT WALKWAY SLABS WITH ARCHITECTURAL DRAWINGS. CONTROL JOINTS AT EXPOSED TO WEATHER WALKWAYS SHALL BE TOOLED JOINTS SEE TYPICAL DETAIL 3-201 ON S-3.2.
 10. ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN AND ELEVATIONS WITH DOWELS TO MATCH U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
 11. GENERAL CONTRACTOR TO COORDINATE LOCATION OF ALL WALL PENETRATIONS REQUIRED FOR MECHANICAL EQUIPMENT AND PROVIDE A PRECAST LINTEL AT ALL OPENINGS REFER TO TYPICAL DETAIL 4-004 ON S-3.3.
 12. MASONRY CONTROL JOINT SHOWN THUS \blacktriangleleft MCJ ON PLAN. MAXIMUM SPACING OF JOINTS = 24' - 0". COORDINATE LOCATION WITH THE ARCH. & STRUCT WALL ELEVATIONS. DO NOT LOCATE A MCJ CLOSER THAN 24" TO ANY CMU OPENING.

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SEAL:

PROJECT: STATION #7

INDIAN RIVER COUNTY FIRE DISTRICT
 1840 25TH STREET
 VERO BEACH, FL 32960

SLAB ON GRADE - AREA B

NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

COMM. NO: 050219VB
 DATE: 15 AUG. 2023
 BY: AN
 CH'KD: JBCV

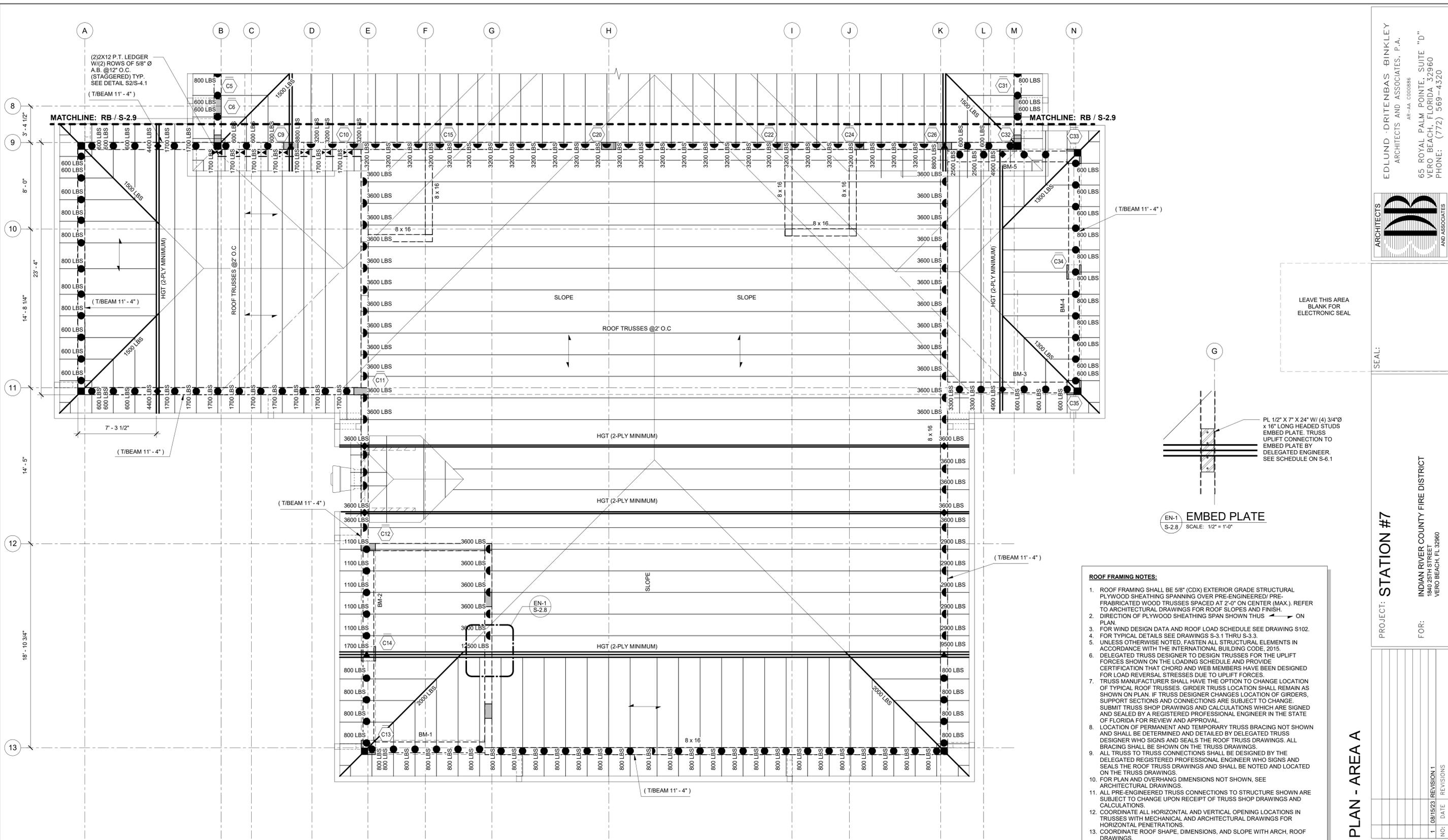
SHEET NO. **S-2.6**
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TO THE BEST OF OUR KNOWLEDGE AND BELIEF, THESE STRUCTURAL PLANS CONFORM TO AND SATISFY, THE FLORIDA BUILDING CODE, SEVENTH EDITION 2020, ACI 318-14 AND LOCAL CODES AS APPLICABLE

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- ROOF FRAMING NOTES:**
- ROOF FRAMING SHALL BE 5/8" (CDX) EXTERIOR GRADE STRUCTURAL PLYWOOD SHEATHING SPANNING OVER PRE-ENGINEERED/ PRE-FABRICATED WOOD TRUSSES SPACED AT 2'-0" ON CENTER (MAX.). REFER TO ARCHITECTURAL DRAWINGS FOR ROOF SLOPES AND FINISH. DIRECTION OF PLYWOOD SHEATHING SPAN SHOWN THUS ON PLAN.
 - FOR WIND DESIGN DATA AND ROOF LOAD SCHEDULE SEE DRAWING S102.
 - FOR TYPICAL DETAILS SEE DRAWINGS S-3.1 THRU S-3.3.
 - UNLESS OTHERWISE NOTED, FASTEN ALL STRUCTURAL ELEMENTS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2015.
 - DELEGATED TRUSS DESIGNER TO DESIGN TRUSSES FOR THE UPLIFT FORCES SHOWN ON THE LOADING SCHEDULE AND PROVIDE CERTIFICATION THAT CHORD AND WEB MEMBERS HAVE BEEN DESIGNED FOR LOAD REVERSAL STRESSES DUE TO UPLIFT FORCES.
 - TRUSS MANUFACTURER SHALL HAVE THE OPTION TO CHANGE LOCATION OF TYPICAL ROOF TRUSSES. GIRDER TRUSS LOCATION SHALL REMAIN AS SHOWN ON PLAN. IF TRUSS DESIGNER CHANGES LOCATION OF GIRDERS, SUPPORT SECTIONS AND CONNECTIONS ARE SUBJECT TO CHANGE. SUBMIT TRUSS SHOP DRAWINGS AND CALCULATIONS WHICH ARE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA FOR REVIEW AND APPROVAL.
 - LOCATION OF PERMANENT AND TEMPORARY TRUSS BRACING NOT SHOWN AND SHALL BE DETERMINED AND DETAILED BY DELEGATED TRUSS DESIGNER WHO SIGNS AND SEALS THE ROOF TRUSS DRAWINGS. ALL BRACING SHALL BE SHOWN ON THE TRUSS DRAWINGS.
 - ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE DELEGATED REGISTERED PROFESSIONAL ENGINEER WHO SIGNS AND SEALS THE ROOF TRUSS DRAWINGS AND SHALL BE NOTED AND LOCATED ON THE TRUSS DRAWINGS.
 - FOR PLAN AND OVERHANG DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
 - ALL PRE-ENGINEERED TRUSS CONNECTIONS TO STRUCTURE SHOWN ARE SUBJECT TO CHANGE UPON RECEIPT OF TRUSS SHOP DRAWINGS AND CALCULATIONS.
 - COORDINATE ALL HORIZONTAL AND VERTICAL OPENING LOCATIONS IN TRUSSES WITH MECHANICAL AND ARCHITECTURAL DRAWINGS FOR HORIZONTAL PENETRATIONS.
 - COORDINATE ROOF SHAPE, DIMENSIONS, AND SLOPE WITH ARCH. ROOF DRAWINGS.
 - MECHANICAL UNITS SHOWN ARE GENERAL LAYOUT. G.C. TO COORDINATE WITH TRUSS MANUFACTURE AND MECHANICAL DWGS FOR EXACT PLACEMENT AND ROOF PENETRATION REQUIREMENT FOR ROOF FRAMING.

CONCRETE BEAM SCHEDULE					
MARK	BEAM WIDTH	BEAM DEPTH	CONT. REINFORCING	TIE REINFORCING	REMARKS
BM-1	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-2	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-3	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-4	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-5	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM

WOOD TRUSS CONNECTOR SCHEDULE								
MARK	LOCATION	QUANTITY	NO. OF PLIES	MANUF.	CONNECTOR MODEL NO.	MAX. UPLIFT LOAD	FASTENERS TO TRUSS	FASTENERS TO THE BEAM
●	RT	1	1	SIMPSON	HETA16	1810 LBS	(9) 0.148" X 1-1/2"	EMBEDMENT 8" MIN. ATR
◀▶	RT	1	1	SIMPSON	FGTR	4725 LBS	(18) 1/4" X 3" SDS	(2) 1/2" X 5" TITEN HD
▶▶	RT	2	1	SIMPSON	MTS 20	1980 LBS	2 (14) 0.148" X 3" NAILS	-
◆	HGT	1	2	SIMPSON	VTG	4940 LBS	(16) 1/4" X 3" SDS	(1) 5/8" ATR. EMBEDMENT 8" MIN.
▲	HGT	1	2	SIMPSON	HGT - 2	10690 LBS	(16) 0.148" X 3"	(2) 5/8" ATR. EMBEDMENT 8" MIN.
■	HJT	2	1	SIMPSON	HTS16	1445 LBS	2 (8) 0.148" X 1-1/2"	2 (8) 0.148" X 1-1/2"

RA ROOF PLAN - AREA A
1/4" = 1'-0"

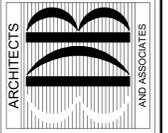
MASTER CONSULTING ENGINEERS, INC.
STRUCTURAL CONSULTANTS

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PROJECT: STATION #7
INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

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SEAL:

PROJECT: STATION #7

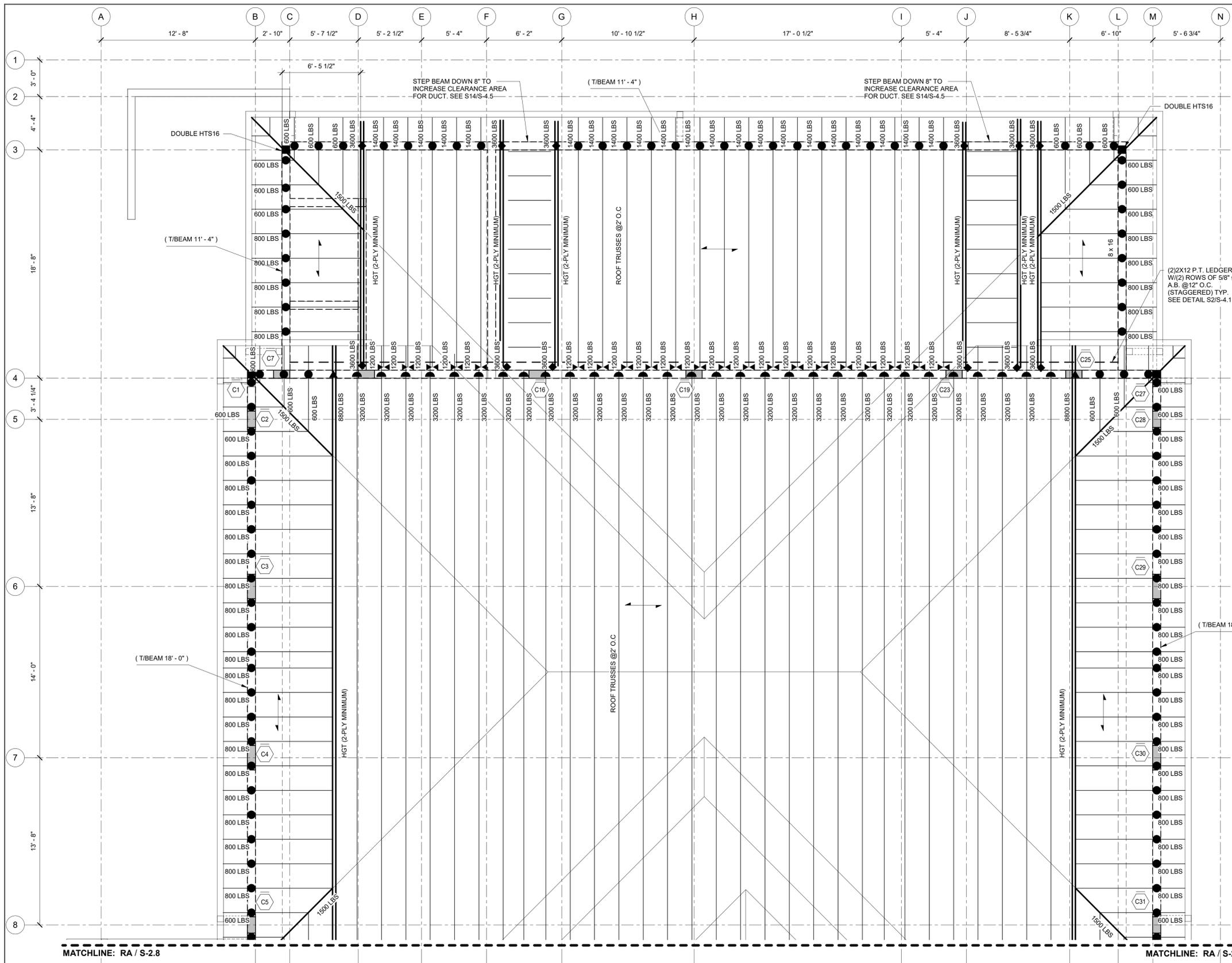
NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

COMM. NO: 050219VB
DATE: 15 AUG. 2023
BY: AN
CHK'D: JBCV

SHEET NO. **S-2.8**
OF TWENTY EIGHT

TO THE BEST OF OUR KNOWLEDGE AND BELIEF, THESE STRUCTURAL PLANS CONFORM TO AND SATISFY, THE FLORIDA BUILDING CODE, SEVENTH EDITION 2020, ACI 318-14 AND LOCAL CODES AS APPLICABLE

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- ROOF FRAMING NOTES:**
- ROOF FRAMING SHALL BE 5/8" (CDX) EXTERIOR GRADE STRUCTURAL PLYWOOD SHEATHING SPANNING OVER PRE-ENGINEERED/ PRE-FABRICATED WOOD TRUSSES SPACED AT 2'-0" ON CENTER (MAX.). REFER TO ARCHITECTURAL DRAWINGS FOR ROOF SLOPES AND FINISH.
 - DIRECTION OF PLYWOOD SHEATHING SPAN SHOWN THUS → ON PLAN.
 - FOR WIND DESIGN DATA AND ROOF LOAD SCHEDULE SEE DRAWING S102.
 - FOR TYPICAL DETAILS SEE DRAWINGS S-3.1 THRU S-3.3.
 - UNLESS OTHERWISE NOTED, FASTEN ALL STRUCTURAL ELEMENTS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2015.
 - DELEGATED TRUSS DESIGNER TO DESIGN TRUSSES FOR THE UPLIFT FORCES SHOWN ON THE LOADING SCHEDULE AND PROVIDE CERTIFICATION THAT CHORD AND WEB MEMBERS HAVE BEEN DESIGNED FOR LOAD REVERSAL STRESSES DUE TO UPLIFT FORCES.
 - TRUSS MANUFACTURER SHALL HAVE THE OPTION TO CHANGE LOCATION OF TYPICAL ROOF TRUSSES. GIRDER TRUSS LOCATION SHALL REMAIN AS SHOWN ON PLAN. IF TRUSS DESIGNER CHANGES LOCATION OF GIRDERS, SUPPORT SECTIONS AND CONNECTIONS ARE SUBJECT TO CHANGE. SUBMIT TRUSS SHOP DRAWINGS AND CALCULATIONS WHICH ARE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA FOR REVIEW AND APPROVAL.
 - LOCATION OF PERMANENT AND TEMPORARY TRUSS BRACING NOT SHOWN AND SHALL BE DETERMINED AND DETAILED BY DELEGATED TRUSS DESIGNER WHO SIGNS AND SEALS THE ROOF TRUSS DRAWINGS. ALL BRACING SHALL BE SHOWN ON THE TRUSS DRAWINGS.
 - ALL TRUSSES TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE DELEGATED REGISTERED PROFESSIONAL ENGINEER WHO SIGNS AND SEALS THE ROOF TRUSS DRAWINGS AND SHALL BE NOTED AND LOCATED ON THE TRUSS DRAWINGS.
 - FOR PLAN AND OVERHANG DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
 - ALL PRE-ENGINEERED TRUSS CONNECTIONS TO STRUCTURE SHOWN ARE SUBJECT TO CHANGE UPON RECEIPT OF TRUSS SHOP DRAWINGS AND CALCULATIONS.
 - COORDINATE ALL HORIZONTAL AND VERTICAL OPENING LOCATIONS IN TRUSSES WITH MECHANICAL AND ARCHITECTURAL DRAWINGS FOR HORIZONTAL PENETRATIONS.
 - COORDINATE ROOF SHAPE, DIMENSIONS, AND SLOPE WITH ARCH. ROOF DRAWINGS.
 - MECHANICAL UNITS SHOWN ARE GENERAL LAYOUT. G.C. TO COORDINATE WITH TRUSS MANUFACTURE AND MECHANICAL DWGS FOR EXACT PLACEMENT AND ROOF PENETRATION REQUIREMENT FOR ROOF FRAMING.

CONCRETE BEAM SCHEDULE					
MARK	BEAM WIDTH	BEAM DEPTH	CONT. REINFORCING	TIE REINFORCING	REMARKS
BM-1	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-2	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-3	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-4	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM
BM-5	12"	12"	(4) #6 T&B CONT.	#3 @4" O.C.	(2) #6 AT THE TOP AND (2) #6 AT THE BOTTOM

WOOD TRUSS CONNECTOR SCHEDULE								
MARK	LOCATION	QUANTITY	NO. OF PLIES	MANUF.	CONNECTOR MODEL NO.	MAX. UPLIFT LOAD	FASTENERS TO TRUSS	FASTENERS TO TIE BEAM
●	RT	1	1	SIMPSON	HETA16	1810 LBS	(9) 0.148" X 1-1/2"	EMBEDMENT 8" MIN. ATR
◐	RT	1	1	SIMPSON	FGTR	4725 LBS	(18) 1/4" X 3" SDS	(2) 1/2" X 5" TITEN HD
◑	RT	2	1	SIMPSON	MTS 20	1980 LBS	2 (14) 0.148" X 3" NAILS	-
◆	HGT	1	2	SIMPSON	VGT	4940 LBS	(16) 1/4" X 3" SDS	(1) 5/8" ATR. EMBEDMENT 8" MIN.
▲	HGT	1	2	SIMPSON	HGT - 2	10690 LBS	(16) 0.148" X 3"	(2) 5/8" ATR. EMBEDMENT 8" MIN.
■	HJT	2	1	SIMPSON	HTS16	1445 LBS	2 (8) 0.148" X 1-1/2"	2 (8) 0.148" X 1-1/2"

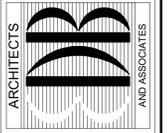
ROOF PLAN - AREA B
1/4" = 1'-0"



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ROOF PLAN - AREA B

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SEAL:

PROJECT: STATION #7

FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

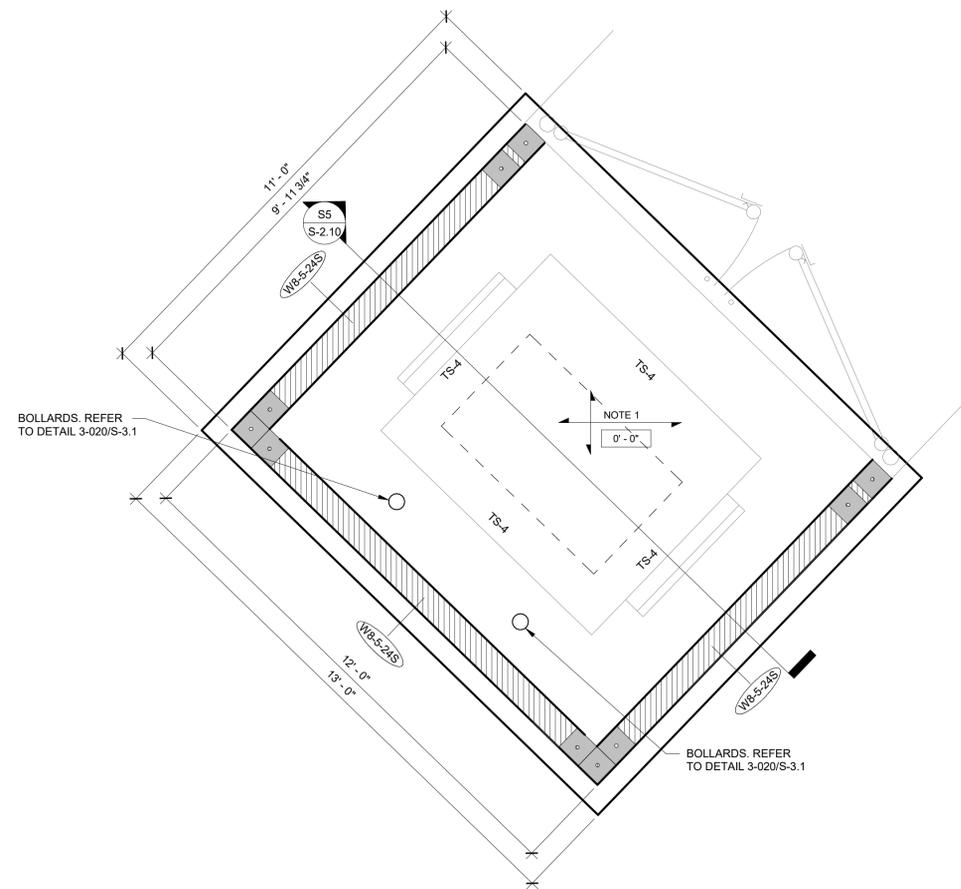
NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

COMM. NO: 050219VB
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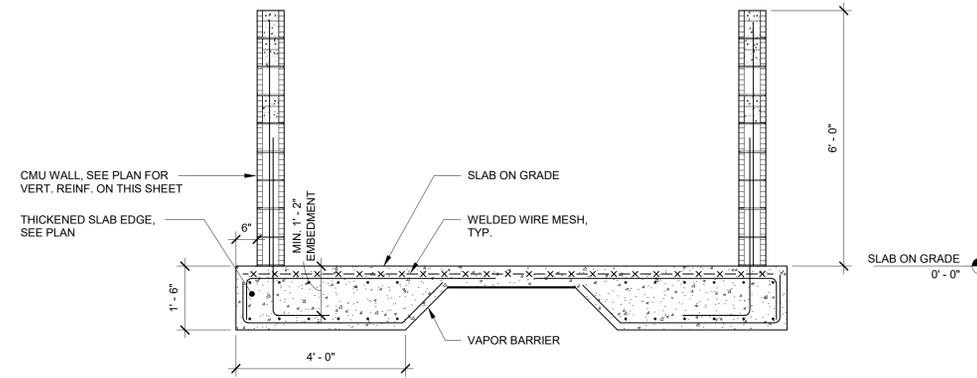
SHEET NO. S-2.9 OF TWENTY EIGHT

TO THE BEST OF OUR KNOWLEDGE AND BELIEF, THESE STRUCTURAL PLANS CONFORM TO AND SATISFY, THE FLORIDA BUILDING CODE, SEVENTH EDITION 2020, ACI 318-14 AND LOCAL CODES AS APPLICABLE

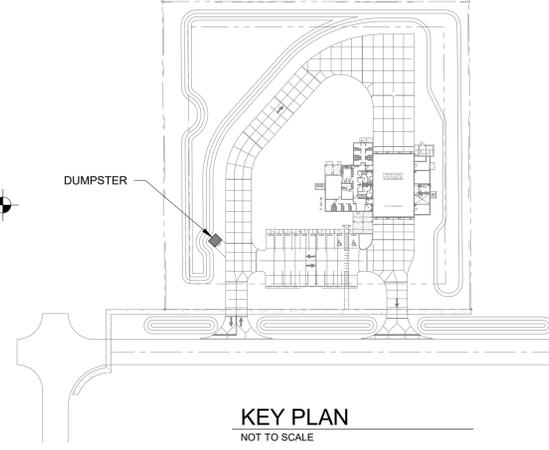
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1 DUMPSTER PLAN
1/2" = 1'-0"



S5 DUMPSTER SECTION
S-2.10 SCALE: 1/2" = 1'-0"



KEY PLAN
NOT TO SCALE

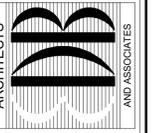
FOUNDATION/SLAB ON GRADE PLAN NOTES:

- SLAB ON GRADE TO BE 6" THICK CONCRETE OVER VAPOR BARRIER ON COMPACTED SUB-BASE AS PER GEOTECHNICAL REPORT REINFORCED 4x4xW4.0xW4.0 W.W.F. (FLAT SHEETS) CENTERED IN SLAB. SUPPORT WELDED WIRE FABRIC WITH 2" SLAB BOLSTER OR APPROVED EQUAL @ 3'-0" O.C. BOTH WAYS. USE OF CONCRETE BRICK IS NOT ALLOWED.
- VAPOR BARRIER REQUIREMENTS/APPLICATION/INSTALLATION SHALL BE DONE AS PER ARCHITECTURAL OR MOISTURE CONSULTANT DOCUMENTS AND RECOMMENDATIONS. IF NONE ARE AVAILABLE, AT A MINIMUM THE FOLLOWING RECOMMENDATIONS SHALL BE FOLLOWED.
 - VAPOR BARRIER SHALL CONFORM TO ASTM E-1745, MEETS OR EXCEEDS CLASS "B".
 - VAPOR BARRIER INSTALLATION SHALL FOLLOW MANUFACTURER'S INSTRUCTIONS AND ASTM E 1643-98.
 - UNROLL VAPOR BARRIER WITH THE LONGEST DIMENSION PARALLEL WITH THE DIRECTION OF THE POUR.
 - LAP VAPOR BARRIER OVER FOOTINGS AND SEAL TO FOUNDATION WALLS.
 - OVERLAP JOINTS 6 INCHES AND SEAL WITH MANUFACTURER'S TAPE.
 - SEAL ALL PENETRATIONS (INCLUDING PIPES) WITH MANUFACTURER'S PIPE BOOT.
 - NO PENETRATION OF THE VAPOR BARRIER IS ALLOWED EXCEPT FOR REINFORCING STEEL AND PERMANENT UTILITIES.
 - REPAIR DAMAGED AREAS BY CUTTING PATCHES OF VAPOR BARRIER, OVERLAPPING DAMAGED AREA 6 INCHES AND TAPING ALL FOUR SIDES WITH TAPE.
- TOP OF SLAB ELEVATION SHOWN THUS [] ON PLAN EQUALS REFERENCE EL., FOR ACTUAL EL., SEE CIVIL DRAWINGS.
- FOR GENERAL STRUCTURAL NOTES, SEE DRAWING S-1.1.
- FOR FLOOR SLAB DEPRESSION LOCATIONS AND LIMITS NOT SHOWN ON PLAN SEE ARCHITECTURAL DRAWINGS.
- FOR PLAN DIMENSIONS NOT SHOWN, REFER TO ARCHITECTURAL DRAWINGS.
- TYPICAL WALL REINFORCING SCHEDULE:
 - FOR CMU EXTERIOR WALL REINFORCING SEE SCHEDULE THIS SHEET.
 - INDICATES ADDITIONAL (1) VERTICAL IN GROUT FILLED CELL.
 - PROVIDE ADDITIONAL (1) VERTICAL IN FIRST (2) CELLS EACH SIDE OF ALL WALL OPENINGS, CORNERS AND INTERSECTIONS.
- COORDINATE LOCATION OF (CJ) CONTROL JOINTS (SAWCUTS & TOOLED JOINTS) AT WALKWAY SLABS WITH ARCHITECTURAL DRAWINGS. CONTROL JOINTS AT EXPOSED TO WEATHER WALKWAYS SHALL BE TOOLED JOINTS SEE TYPICAL DETAIL 3-201 ON S-3.2.
- ALL CMU WALLS SHALL BE REINFORCED AS SHOWN ON PLAN WITH DOWELS TO MATCH U.N.O. ALL CELLS AT REINFORCING LOCATION SHALL BE FILLED WITH GROUT. PROVIDE INSPECTION/CLEANOUT HOLE AT BASE WHEN POUR HEIGHT IS GREATER THAN 4'-0".
- LOCATION OF MASONRY CONTROL JOINT NOT SHOWN. COORDINATE WITH ARCHITECTURAL ELEVATIONS. SEE TYPICAL DETAIL 4-002 ON S-3.3.
- GENERAL CONTRACTOR TO COORDINATE LOCATION OF ALL WALL PENETRATIONS REQUIRED FOR MECHANICAL EQUIPMENT AND PROVIDE A PRECAST LINTEL AT ALL OPENINGS REFER TO TYPICAL DETAIL 4-004 ON S-3.3.
- MASONRY CONTROL JOINT SHOWN THUS [] MCJ ON PLAN. MAXIMUM SPACING OF JOINTS = 24'-0". COORDINATE LOCATION WITH THE ARCH. BUILDING ELEVATIONS.

THICKENED SLAB SCHEDULE

MARK*	SIZE	THICKNESS	TOP & BOTTT.* REINF. CONT.	TOP & BOTTT.* REINF. TRANSV.	REMARKS
TS-1	1'-0" x CONT.	1'-4"	(2)#5	#4 @ 24" O.C.	
TS-1A	1'-0" x CONT.	1'-0"	(2)#5	#4 @ 24" O.C.	TO BE USED ON INTERIOR AREAS ONLY
TS-4	4'-0" x CONT.	1'-6"	(6)#5	#4 @ 24" O.C.	

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SEAL:

PROJECT: STATION #7

FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

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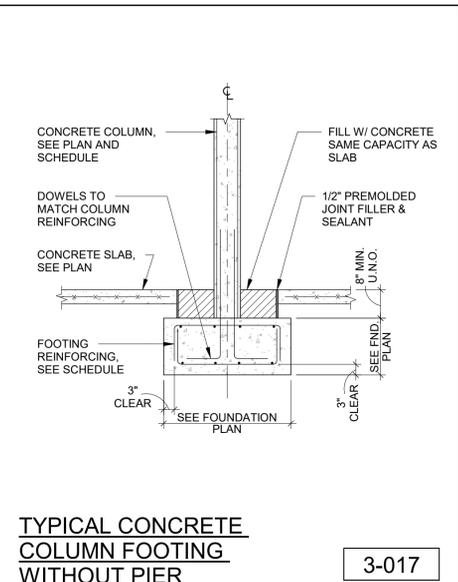
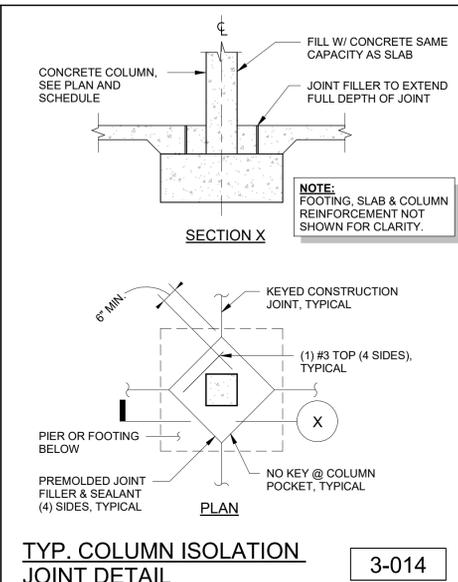
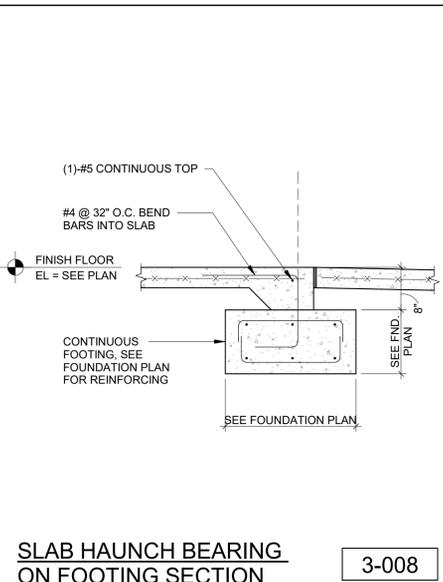
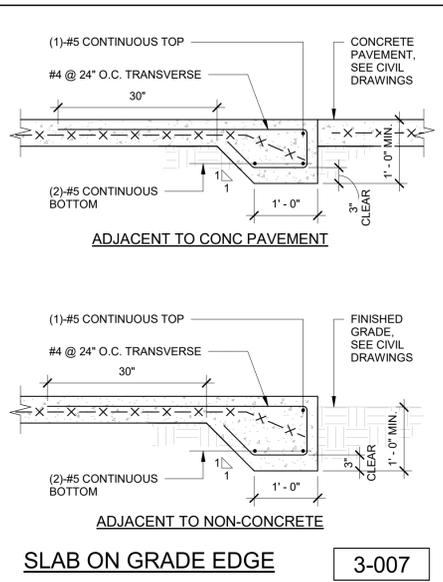
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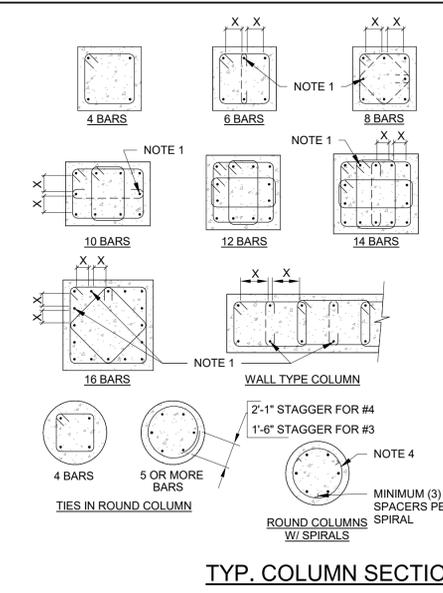
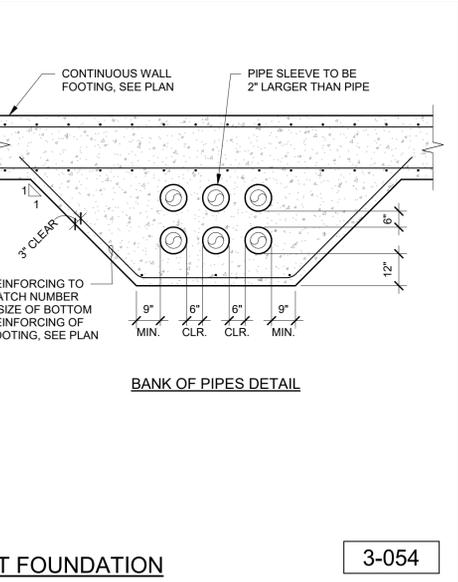
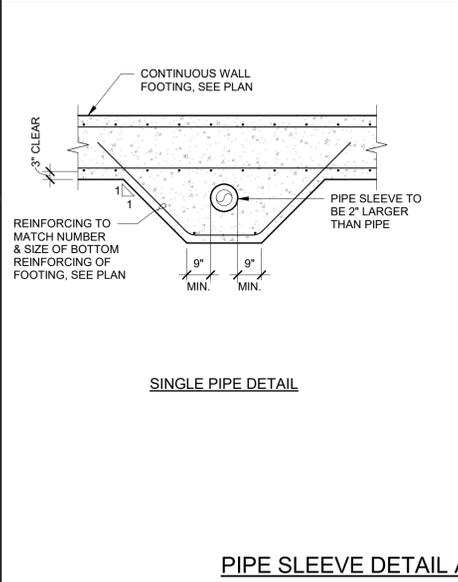
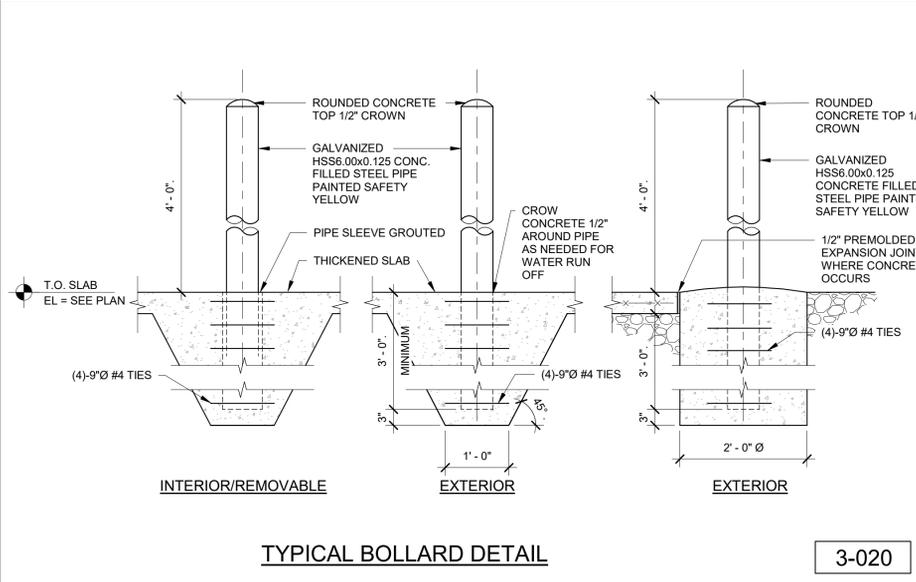
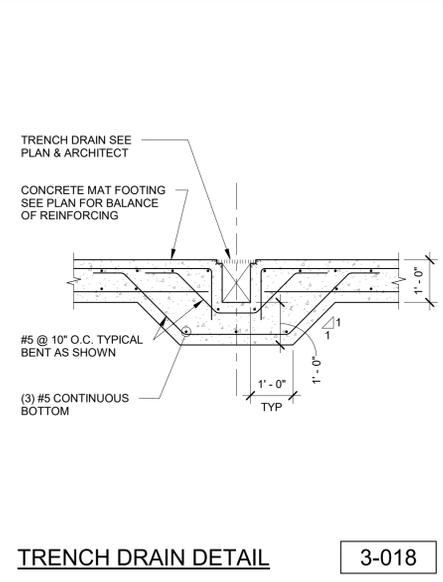
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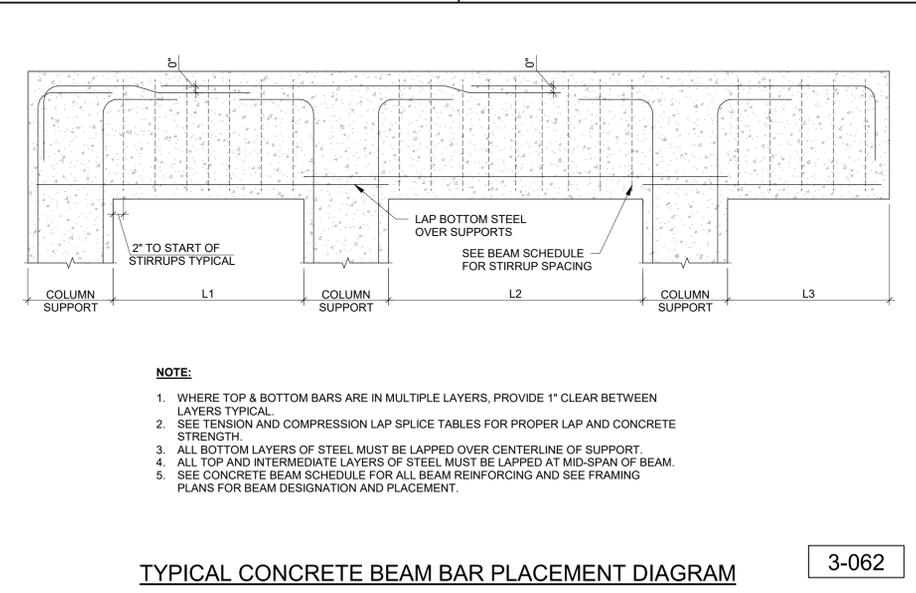
		SIZE AND SPACING OF COL. TIES									
A	B	8"	10"	12"	14"	16"	18"	20"	22"	24" OR MORE	
#5	#3@8	#3@10	#3@10	#3@10	#3@10	#3@10	#3@10	#3@10	#3@10	#3@10	
#6	#3@8	#3@10	#3@12	#3@12	#3@12	#3@12	#3@12	#3@12	#3@12	#3@12	
#7	#3@8	#3@10	#3@12	#3@14	#3@14	#3@14	#3@14	#3@14	#3@14	#3@14	
#8	#3@8	#3@10	#3@12	#3@14	#3@16	#3@16	#3@16	#3@16	#3@16	#3@16	
#9,#10	#3@8	#3@10	#3@12	#3@14	#3@16	#3@18	#3@18	#3@18	#3@18	#3@18	
#11#14 & #18	#4@8	#4@10	#4@12	#4@14	#4@16	#4@18	#4@20	#4@22	#4@22	#4@22	

A = SIZE OF VERTICAL REINFORCING
B = LEAST DIMENSION OF COLUMN

NOTES:

- THESE BARS MUST BE TIED AS SHOWN BY DASHED LINES WHEN "X" DISTANCE IS OVER 6 INCHES AND NEED NOT BE TIED WHEN "X" IS 6" OR LESS.
- MINIMUM CLEAR COVER 1 1/2" TO TIES (2" FOR EXPOSED CONDITION).
- FOR VERTICAL COLUMN REINFORCING, SEE PLAN AND COLUMN SCHEDULE.
- FOR SPIRAL SEE COLUMN SCHEDULE.

TYPICAL CONCRETE BEAM BAR PLACEMENT DIAGRAM 3-062



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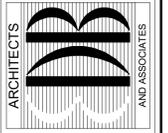
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BY: AN
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SHEET NO. **S-3.1**
OF TWENTY EIGHT

TYPICAL DETAILS

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THE MINIMUM CLEAR COVER FOR REINFORCEMENT BARS SHALL BE ONE BAR DIAMETER OR THE VALUES TABULATED BELOW, WHICHEVER IS THE GREATER.	
SLABS (LT. WT. CONC. OR STONE CONC.)	1"
GIRDERS AND BEAMS (TO STIRRUPS)	1 1/2"
JOISTS (STONE OR LT. WT.) BOTTOM BARS	1 1/4"
TIED COLUMNS AND PIERS	
SURFACE EXPOSED TO EARTH AND WEATHER (TO TIES)	2"
OTHER SURFACES (TO TIES)	1 1/2"
FOUNDATION ELEMENTS	
FORMED SURFACES	2"
SURFACES PLACED AGAINST EARTH	3"
WALLS	
SURFACES EXPOSED TO EARTH	2"
SURFACES EXPOSED TO WEATHER	1 1/2"
OTHER SURFACES	1"

$f_c = 3000$ PSI, NORMAL WEIGHT

BAR SIZE	LAP CLASS	TENSION LAP SPLICES				COMPRESSION LAP SPLICES
		LAP LENGTH PER SPACING AND COVER CASE				
		CASE 1		CASE 2		
TOP BARS	OTHER BARS	TOP BARS	OTHER BARS			
#3	A	22	17	32	25	12
	B	28	22	42	32	
#4	A	29	22	43	33	15
	B	37	29	56	43	
#5	A	36	28	54	41	19
	B	47	36	70	54	
#6	A	43	33	64	50	23
	B	56	43	84	64	
#7	A	63	48	94	72	26
	B	81	63	122	94	
#8	A	72	55	107	82	30
	B	93	72	139	107	
#9	A	81	62	121	93	34
	B	105	81	157	121	
#10	A	91	70	136	105	38
	B	118	91	177	136	
#11	A	101	78	151	116	42
	B	131	101	196	151	

NOTES:

CASE 1
BEAMS AND COLUMNS:
 CONCRETE COVER \geq TO BAR DIAMETER, C-C BAR SPACING \geq 2X BAR DIAMETER AND WITH STIRRUPS OR TIES THROUGHOUT TENSION LAP SPlice LENGTH NOT LESS THAN THE CODE MINIMUM.

OTHER MEMBERS:
 CONCRETE COVER \geq TO THE BAR DIAMETER AND C-C BAR SPACING \geq TO 3X BAR DIAMETER.

CASE 2
BEAMS AND COLUMNS:
 CONCRETE COVER < BAR DIAMETER AND C-C BAR SPACING < 2X BAR DIAMETER.

OTHER MEMBERS:
 CONCRETE COVER < BAR DIAMETER OR C-C BAR SPACING < 3X BAR DIAMETER.

* LAP CLASS "B" IS TO BE USED UNLESS OTHERWISE SPECIFIED IN THESE CONTRACT DOCUMENTS.

$f_c = 4000$ PSI, NORMAL WEIGHT

BAR SIZE	LAP CLASS	TENSION LAP SPLICES				COMPRESSION LAP SPLICES
		LAP LENGTH PER SPACING AND COVER CASE				
		CASE 1		CASE 2		
TOP BARS	OTHER BARS	TOP BARS	OTHER BARS			
#3	A	19	15	28	22	12
	B	24	19	36	28	
#4	A	25	19	37	29	15
	B	32	25	48	37	
#5	A	31	24	47	36	19
	B	40	31	60	47	
#6	A	37	29	56	43	23
	B	48	37	72	56	
#7	A	54	42	81	63	26
	B	70	54	106	81	
#8	A	62	48	93	71	30
	B	80	62	121	93	
#9	A	70	54	105	81	34
	B	91	70	136	105	
#10	A	79	61	118	91	38
	B	102	79	153	118	
#11	A	87	67	131	101	42
	B	113	87	170	131	

NOTES:

CASE 1
BEAMS AND COLUMNS:
 CONCRETE COVER \geq TO BAR DIAMETER, C-C BAR SPACING \geq 2X BAR DIAMETER AND WITH STIRRUPS OR TIES THROUGHOUT TENSION LAP SPlice LENGTH NOT LESS THAN THE CODE MINIMUM.

OTHER MEMBERS:
 CONCRETE COVER \geq TO THE BAR DIAMETER AND C-C BAR SPACING \geq TO 3X BAR DIAMETER.

CASE 2
BEAMS AND COLUMNS:
 CONCRETE COVER < BAR DIAMETER AND C-C BAR SPACING < 2X BAR DIAMETER.

OTHER MEMBERS:
 CONCRETE COVER < BAR DIAMETER OR C-C BAR SPACING < 3X BAR DIAMETER.

* LAP CLASS "B" IS TO BE USED UNLESS OTHERWISE SPECIFIED IN THESE CONTRACT DOCUMENTS.

TYPICAL CONCRETE COVER FOR REINFORCING BARS

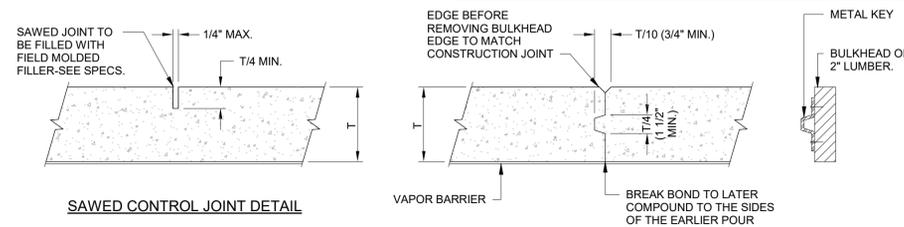
3-064

TENSION AND COMPRESSION LAP SPLICES WITH $f_c = 3000$ PSI

3-065

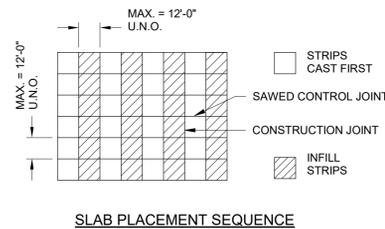
TENSION AND COMPRESSION LAP SPLICES WITH $f_c = 4000$ PSI

3-066



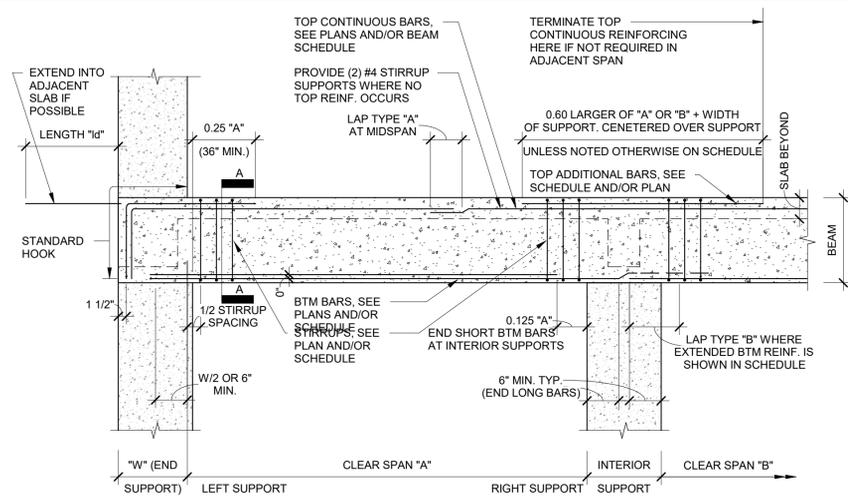
BULKHEAD DETAILS FOR CONSTRUCTION JOINT

- NOTES:**
- CONCRETE FOR SLAB ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AS INDICATED ON THE "CONCRETE AND REINFORCING" NOTES.
 - SUPERPLASTICIZER SHALL BE USED IN SLAB ON GRADE CONCRETE - SEE SPECIFICATIONS.
 - SLAB ON GRADE CONCRETE MIX SHALL HAVE A WATER-CEMENT RATIO AS INDICATED IN SPECS AND OR NOTES.
 - CONSTRUCTION JOINTS SHALL BE LOCATED A MINIMUM OF 5'-0" AWAY FROM ANY OTHER JOINTS TO WHICH THEY ARE PARALLEL.
 - SAW CUTTING SHALL BE DONE WITH A POWER SAW WITH A MASONRY CUTTING BLADE. CUTTING SHALL BE DONE AS SOON AS CONCRETE HARDENS ENOUGH SO THAT THE BLADE DOES NOT DISLodge THE AGGREGATES.
 - WHERE SAWCUT IS DISCONTINUED AT A TRANSVERSE JOINT, STOP CUT 2" SHORT.
 - SLAB REINFORCING NOT SHOWN FOR CLARITY.



TYPICAL SLAB ON GRADE JOINT DETAILS

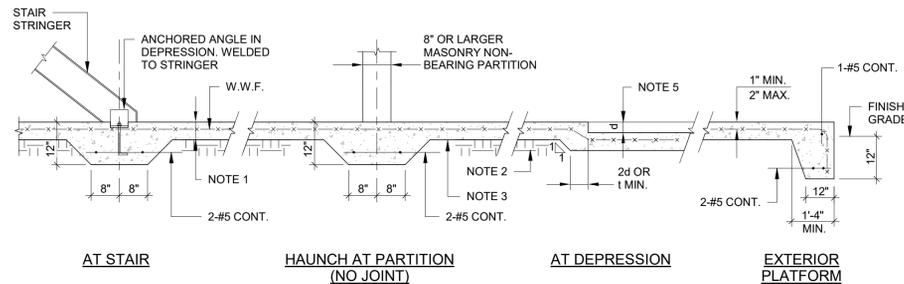
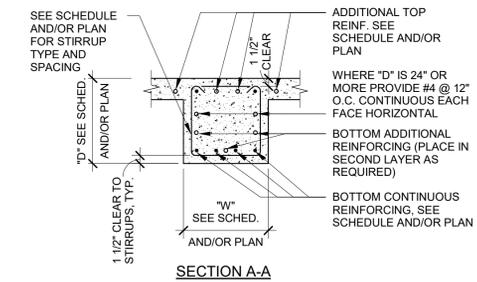
3-201



TYPICAL CONCRETE BEAM REINFORCING DETAIL

3-250

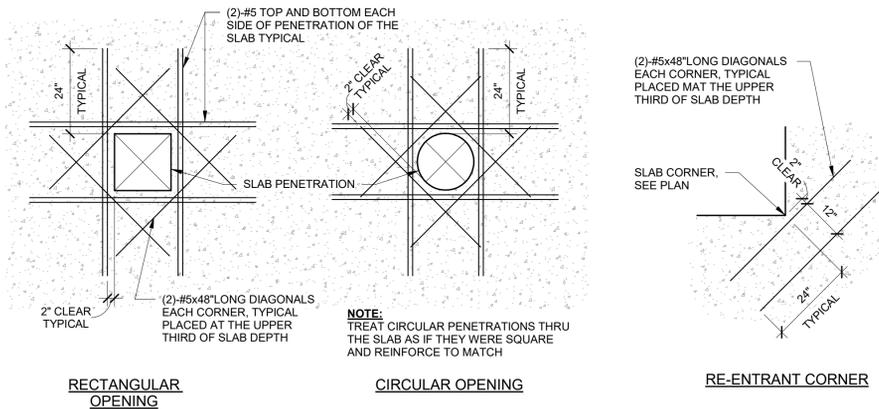
- NOTES:**
- LEFT AND RIGHT ENDS OF BEAMS FOR LOCATING TOP BARS ARE FIXED BY THE POSITION OF THE BEAM MARK ON THE FRAMING PLAN.
 - TOP BARS AT THE RIGHT ENDS OF BEAMS ARE SCHEDULED FOR ALL SPANS. TOP BARS AT LEFT ENDS ARE SCHEDULED FOR SINGLE SPANS AND FOR THE END SPAN AT THE LEFT OF A SERIES OF CONTINUOUS BEAMS.
 - WHERE CONTINUOUS BARS ARE SCHEDULED, THEY ARE IN ADDITION TO REINFORCING CALLED FOR UNDER TOP BARS LEFT OR RIGHT.
 - PROVIDE #3 @ 12" O.C. STIRRUPS WHERE NO STIRRUPS ARE SCHEDULED. TOP CONTINUOUS REINFORCING TO EXTEND ACROSS TOP OF BEAM. PROVIDE LAP TYPE "A" AT CENTER OF SPAN UNLESS OTHERWISE NOTED WHERE DIFFERENT "TOP CONTINUOUS" REINFORCING IS SCHEDULED FOR ADJACENT SPANS. USE LARGER AMOUNT OVER COLUMNS OR GIRDERS.
 - MINIMUM (2) BOTTOM BARS TO BE FULL LENGTH UNLESS OTHERWISE NOTED.
 - FOR "d" DISTANCE SEE DETAIL 1.



- UNLESS NOTED ON PLANS:**
- SEE FOUNDATION NOTES FOR SLAB THICKNESS AND REINFORCING.
 - SLABS SHALL BEAR ON COMPACTED FILL.
 - COMPACTED FILL SHALL BE COVERED WITH VAPOR BARRIER AT HAUNCHES. SEE SPECS.
 - UNDER MACHINE EQUIPMENT DEEPEN SLAB TO 8" AND ADD TWO LAYERS OF WELDED WIRE FABRIC BOTTOM, U.N.O.
 - FOR DEPRESSIONS - SEE ARCHITECTURAL DRAWINGS.
 - AT CONSTRUCTION JOINTS USE WOOD FORMS WITH SHEAR KEYS.
 - STOP WIRE MESH AT CONSTRUCTION JOINTS. CUT EVERY OTHER WIRE AT SAWCUT JOINT.
 - WHERE SLAB IS DOWEL INTO WALL, FIRST SLAB CONSTRUCTION JOINT TO BE NOT MORE THAN 15 FEET FROM WALL.
 - FOR EXPANSION JOINT LOCATION SEE PLAN. FOR DETAILS SEE ARCHITECTURAL DRAWINGS.

TYPICAL SLAB ON GRADE DETAILS

3-202



TYPICAL SLAB REINFORCING AT PENETRATIONS & CORNERS

3-520

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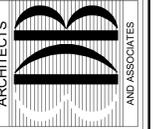


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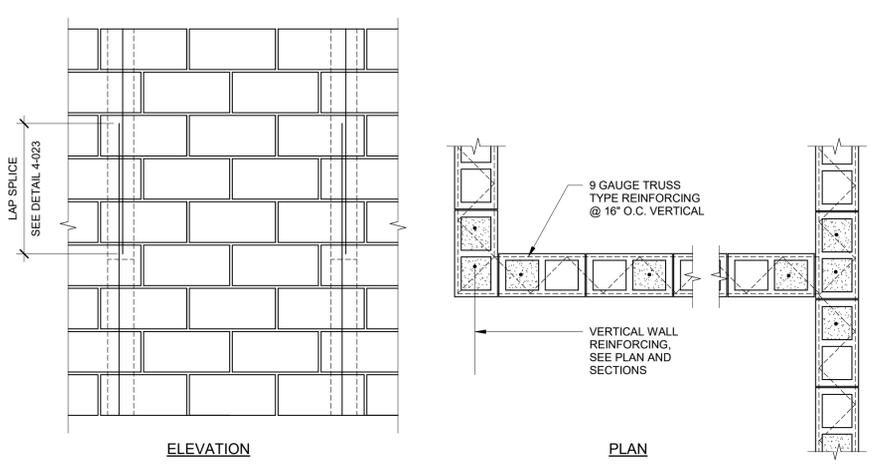
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TYPICAL DETAILS

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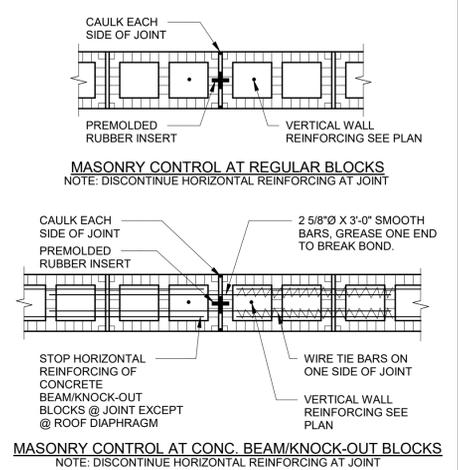
ELEVATION

PLAN

NOTE:
PROVIDE INSPECTION PORT AT BOTTOM OF LIFT GREATER THAN 4' - 0" IN HEIGHT.

TYP MASONRY WALL DETAIL

4-001



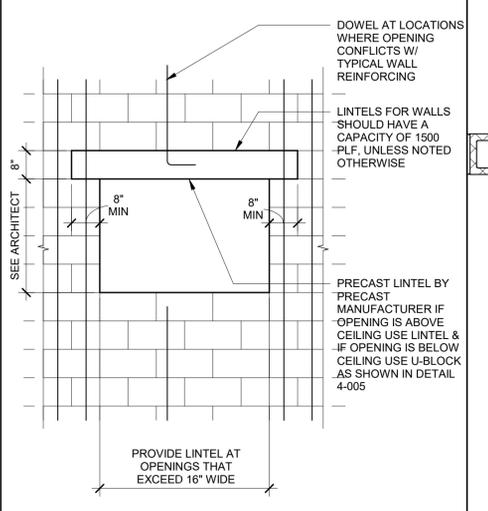
MASONRY CONTROL AT REGULAR BLOCKS

MASONRY CONTROL AT CONC. BEAM/KNOCK-OUT BLOCKS

NOTE:
1. MAXIMUM SPACING OF CONTROL JOINT EQUALS 24'-0" OR THREE TIMES WALL HEIGHT (WHICH EVER IS SMALLER)

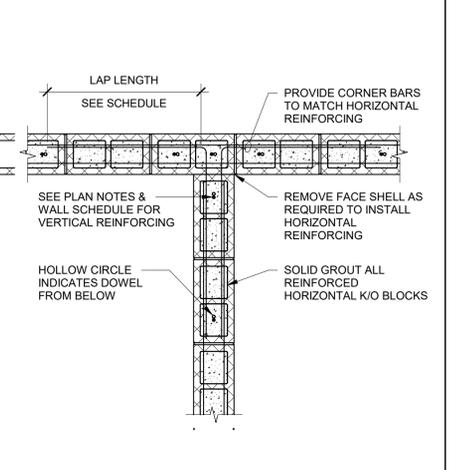
MASONRY CONTROL JOINT

4-002



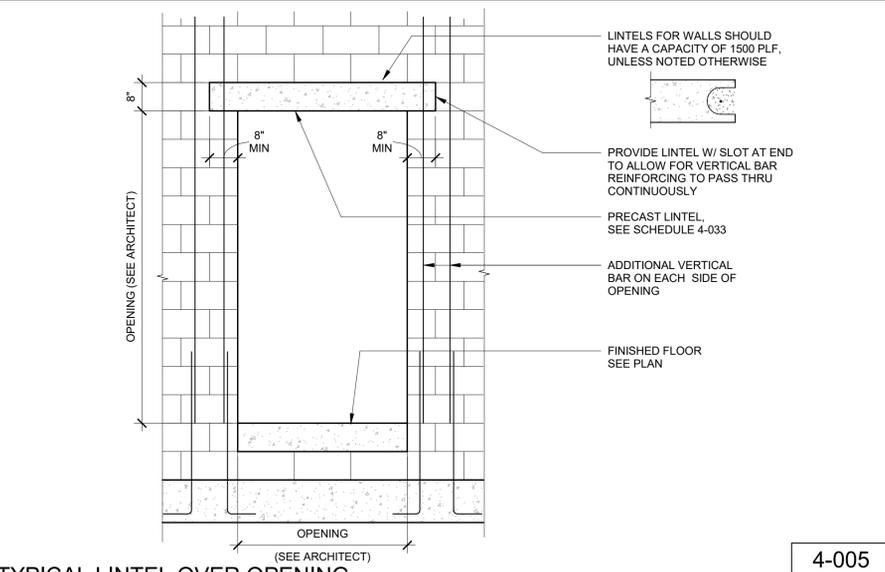
TYPICAL LINTEL OVER DUCT OPENING

4-004



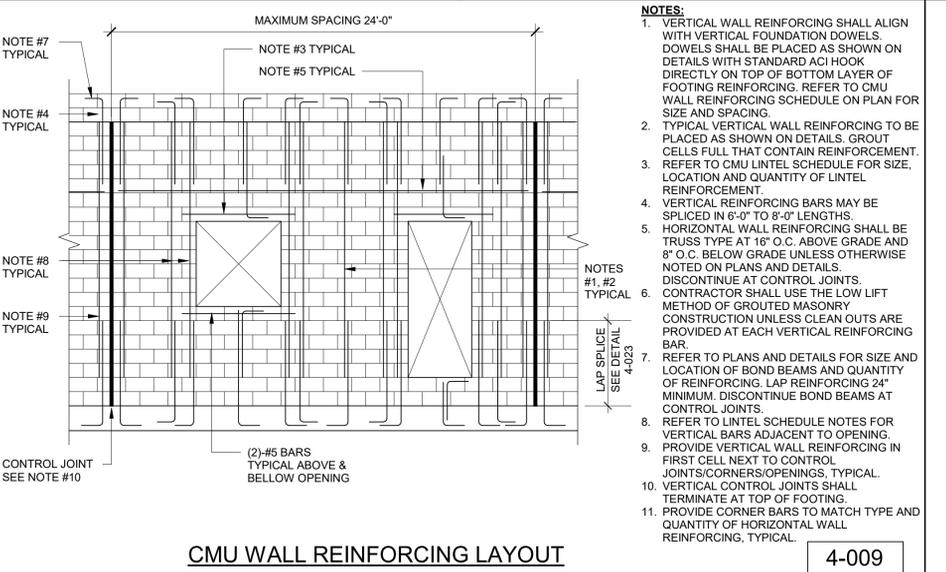
TYPICAL CMU K/O BLOCK REINFORCING LAYOUT

4-008



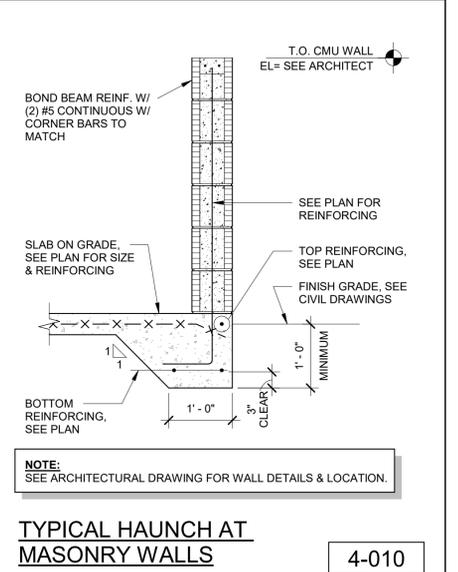
TYPICAL LINTEL OVER OPENING

4-005



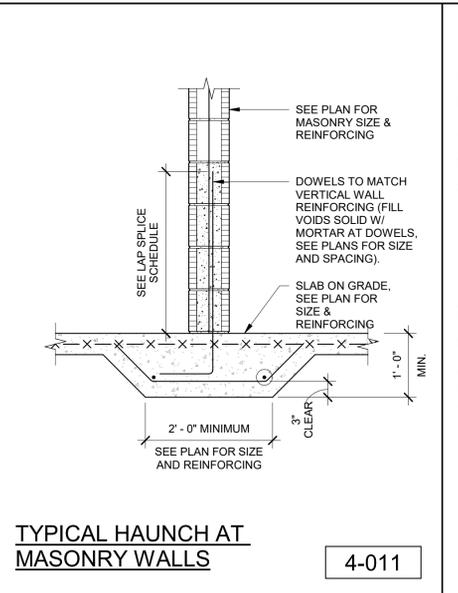
CMU WALL REINFORCING LAYOUT

4-009



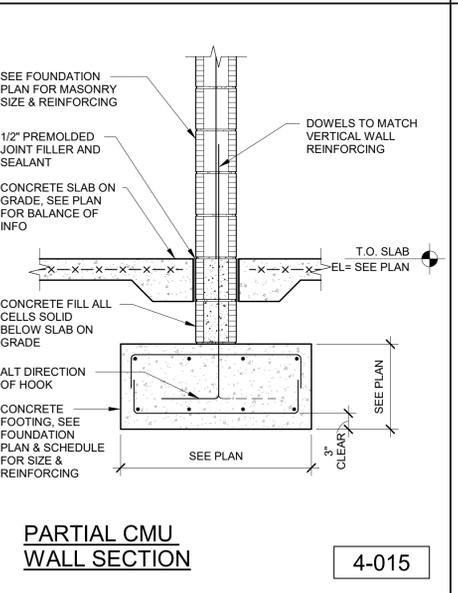
TYPICAL HAUNCH AT MASONRY WALLS

4-010



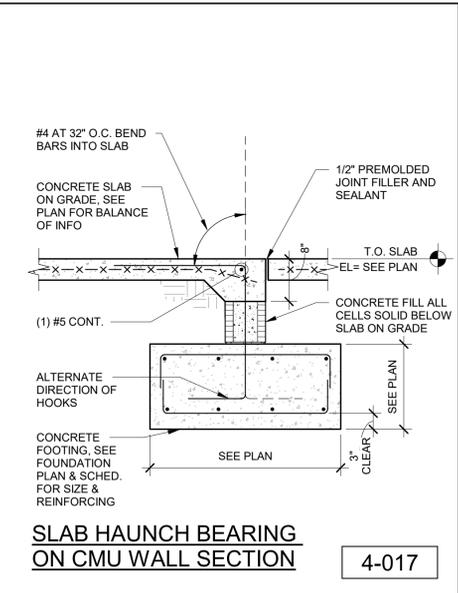
TYPICAL HAUNCH AT MASONRY WALLS

4-011



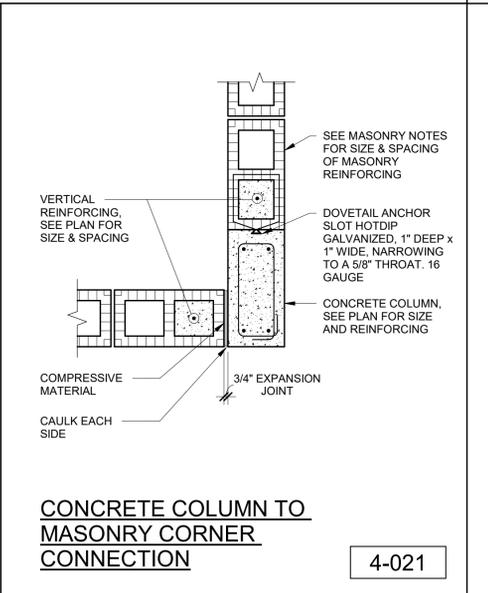
PARTIAL CMU WALL SECTION

4-015



SLAB HAUNCH BEARING ON CMU WALL SECTION

4-017



CONCRETE COLUMN TO MASONRY CORNER CONNECTION

4-021

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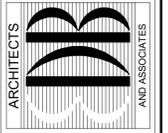


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22/08/2023 3:35:43 PM



SEAL:

PROJECT: STATION #7

FOR: INDIAN RIVER COUNTY FIRE DISTRICT
 1840 25TH STREET
 VERO BEACH, FL 32960

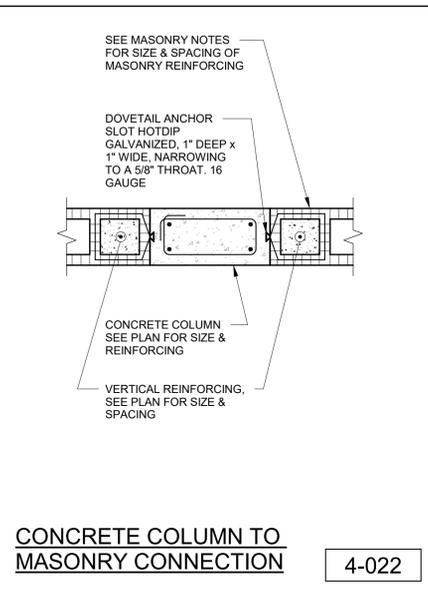
NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

COMM. NO: 050219VB
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 BY: AN
 CH/KD: JBCV

SHEET NO. S-3.4

OF TWENTY EIGHT

TYPICAL DETAILS

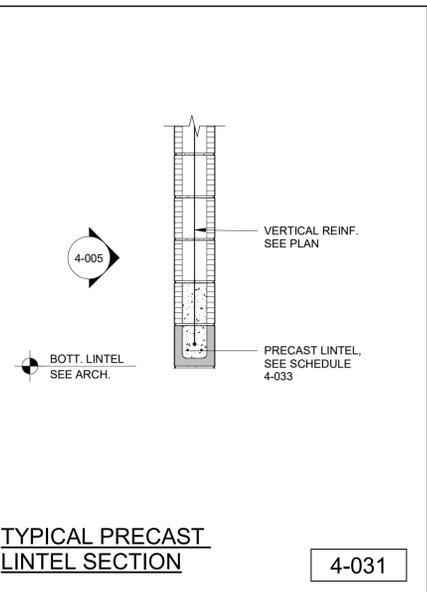


LAP SPLICE SCHEDULE FOR SINGLE REINFORCED 8" CMU WALL

BAR SIZE	LAP SPLICE	REMARKS
#3	18"	
#4	24"	
#5	30"	
#6	36"	

LAP SPLICE SCHEDULE FOR DOUBLE REINFORCED 8" CMU WALL

BAR SIZE	LAP SPLICE	REMARKS
#3	18"	
#4	24"	
#5	30"	
#6	36"	

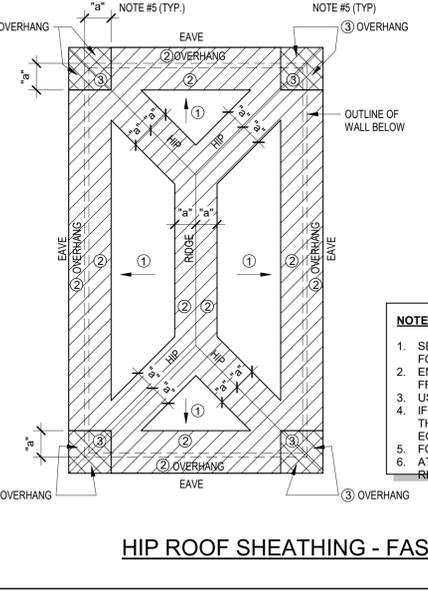
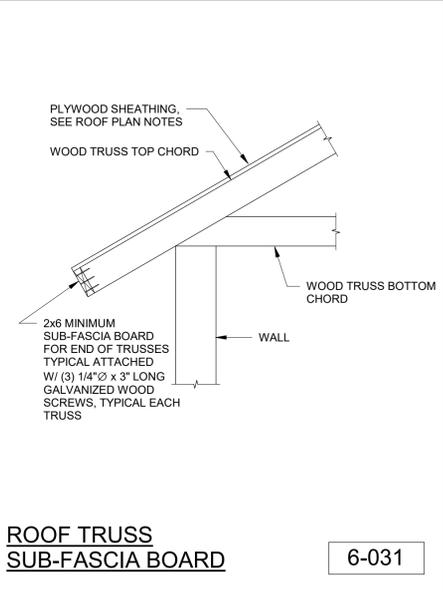
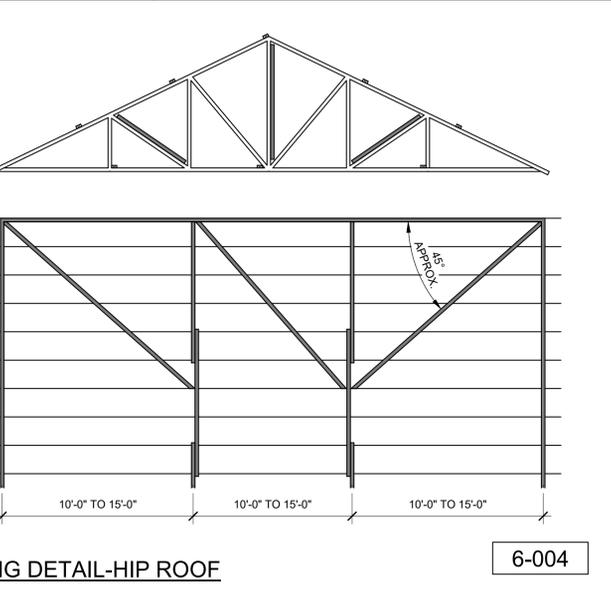
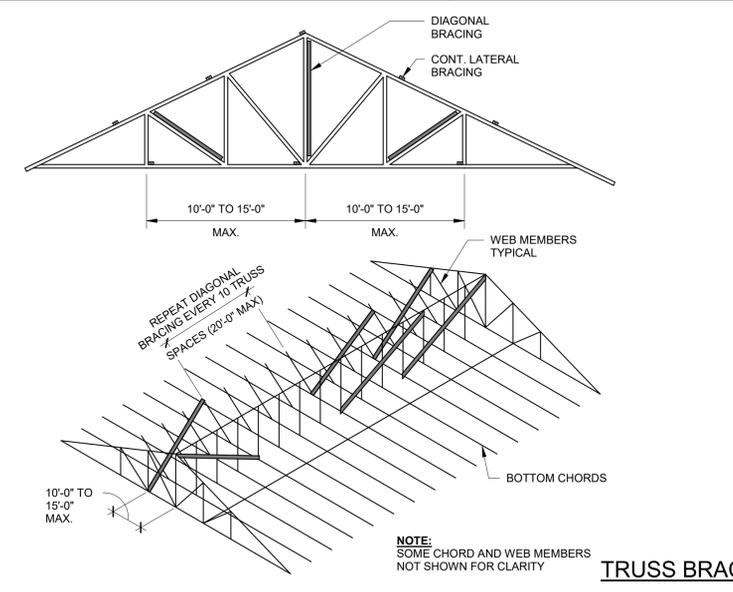
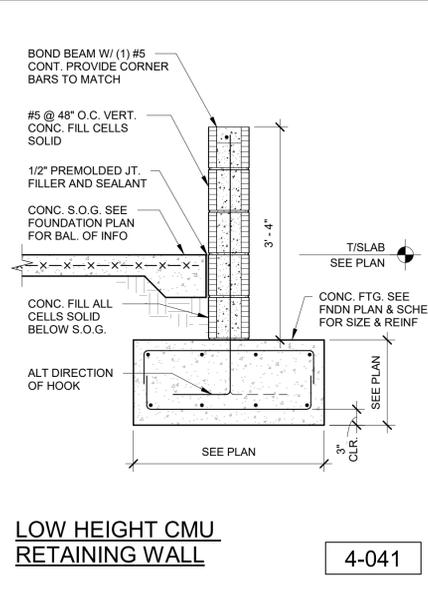


CMU LINTEL SCHEDULE

WALL THICKNESS	LINTEL DEPTH	BOTTOM REINFORCING	TOP REINFORCING	STIRRUPS & SPACING	NOTES
PCL-1	8"	(2)#5	-	-	-
PCL-2	16"	-	-	-	-
PCL-3	24"	-	-	-	-
CIPL-2	24"	-	-	-	-
CIPL-3	32"	-	-	-	-

NOTES:

- PRECAST CONCRETE LINTELS CAN BE INSTALLED AT ALL OPENINGS PER SCHEDULE. CAST IN PLACE CONCRETE LINTELS SHALL BE PLACED WHERE SHOWN ON PLANS. SUBSTITUTED UPON APPROVAL BY ENGINEER.
- GROUTED LINTEL (THROUGH) BLOCKS SHALL EXTEND A MINIMUM OF 8" BEYOND FACE OF OPENING EACH SIDE UNLESS NOTED OTHERWISE. THE FIRST TWO VERTICAL CELLS EACH SIDE OF THE OPENING SHALL BE GROUTED AND CONTAIN (2) #5 VERTICAL EACH FACE.
- NOTE: THE FIRST CELL EACH SIDE OF THE OPENING SHALL BE GROUTED TO LINTEL LEVEL. SECOND CELL SHALL BE GROUTED TO ROOF LEVEL.
- SEE SPECIFIC DETAIL FOR BRICK LEDGE SUPPORT ANGLE OF LOOSE LINTEL SCHEDULE.
- CONSTRUCTION/CONTROL JOINT SHALL NOT BE LOCATED WITHIN WIDTH OF OPENING, NOR SHALL IT BE LOCATED WITHIN 24" OF THE OPENING ON EITHER SIDE.
- CONCRETE LINTEL BEARING EACH SIDE OF LINTEL TO BE EQUAL TO DEPTH OF LINTEL, TYPICAL UNLESS NOTED OTHERWISE.

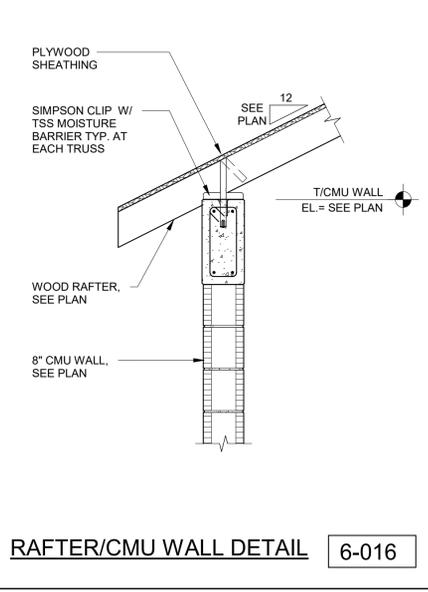
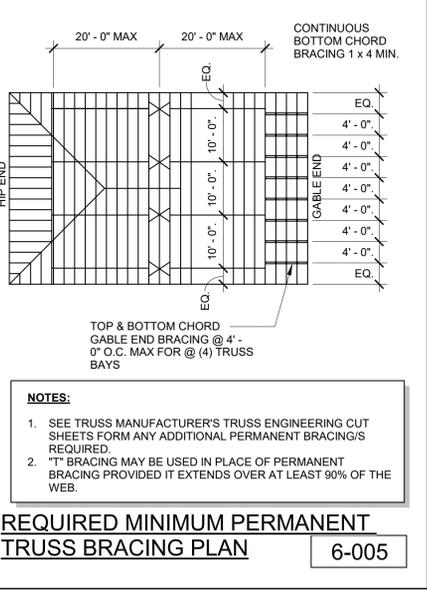


ROOF SHEATHING FASTENING SCHEDULE

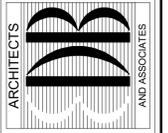
WIND VELOCITY (3-SECOND GUST)	PANEL LOCATION	ROOF FASTENING ZONE			
		1	2	3	4
120 mph f.g	SUPPORTED PANEL END AND EDGES	6	6	6	6
	PANEL FIELD	12	6	4	6

NOTES:

- SEE STRUCTURAL NOTES FOR SHEATHING SPECIFICATIONS AND DRAWING S000 FOR WIND DESIGN DATA.
- END JOINTS OF THE ROOF SHEATHING SHALL BE STAGGERED AND OCCUR OVER FRAMING MEMBER.
- USE MINIMUM OF 10d COMMON OR 8d RING SHANK NAILS.
- IF A PNEUMATIC NAILER WITH FASTENER SIZE LESS THAN .131"Ø x 2 1/2" IS USED, THE CONTRACTOR MUST CONTACT THE ENGINEER OF RECORD FOR AN EQUIVALENT NAIL SPACING.
- FOR DIMENSION "a" MINIMUM EDGE DISTANCE REFER TO WIND DESIGN DATA.
- AT UNSUPPORTED PANEL EDGES PROVIDE ADDITIONAL 2X BLOCKING AS REQUIRED.



NOTE:
 IN CASE OF CONFLICT BETWEEN INFORMATION SHOWN ON THIS DRAWING, SECTION DRAWINGS OR ARCHITECTURAL DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO INFORM THE ARCHITECT AND ENGINEER OF SUCH CONFLICT IN ORDER TO RECEIVE A CLARIFICATION BEFORE PROCEEDING TO WORK. DIMENSIONS, SIZES AND REINFORCEMENT OF STRUCTURAL ELEMENTS SHOWN IN THIS DRAWING ARE MINIMUM TO BE USED UNLESS NOTED OTHERWISE (U.N.O.) IN PLANS.



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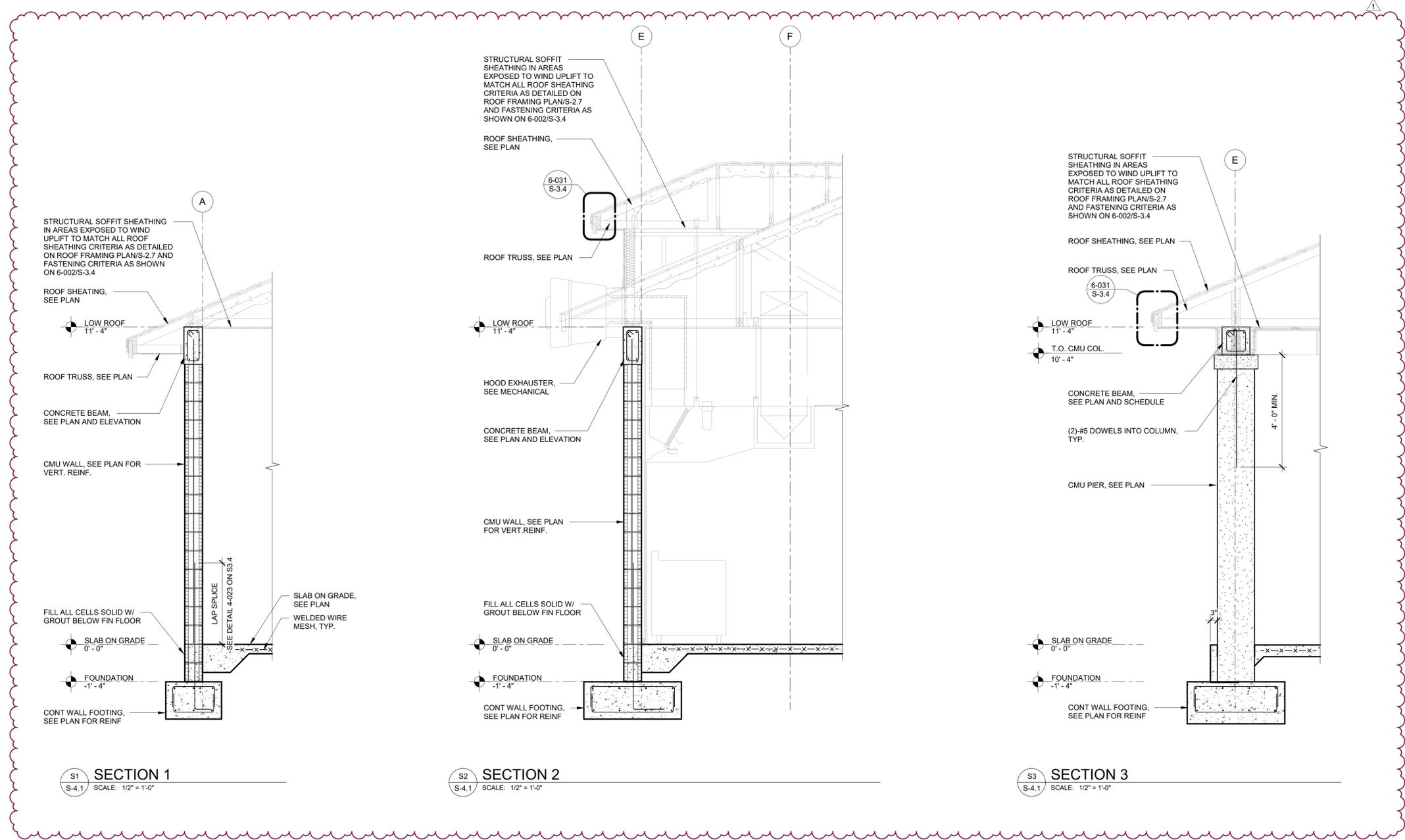
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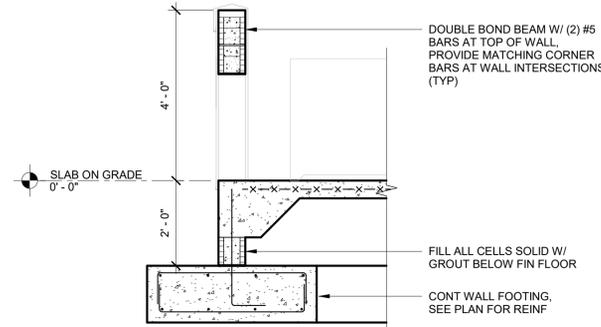
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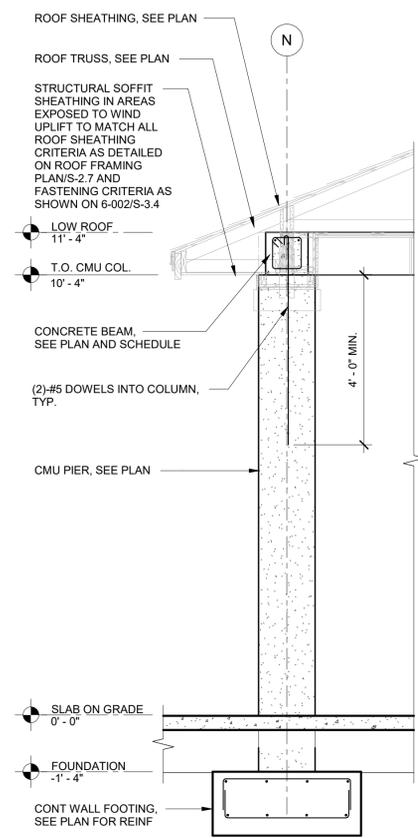
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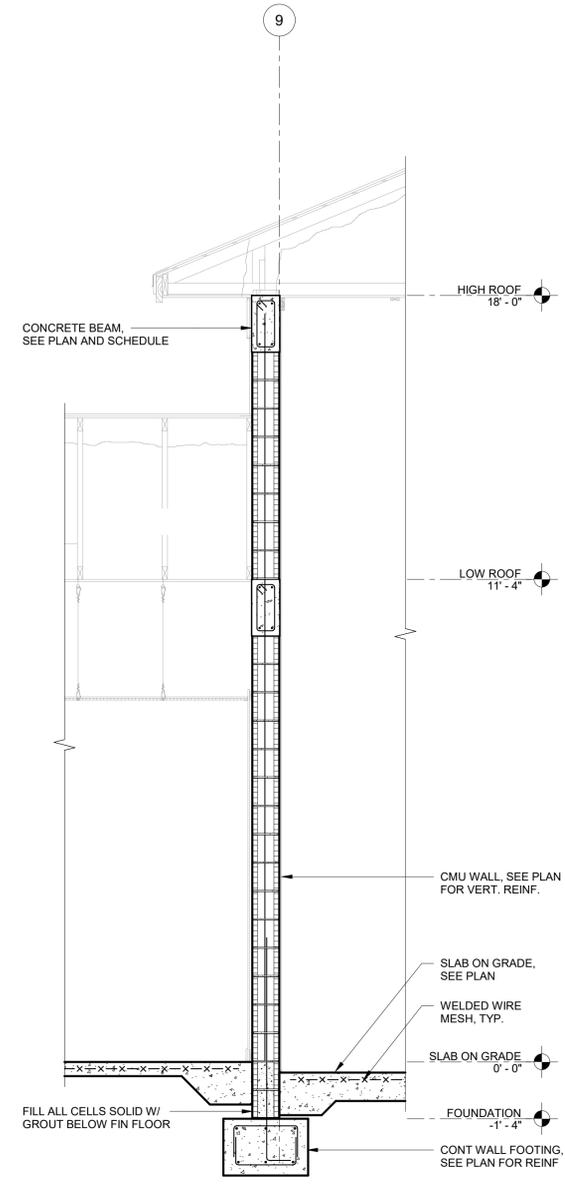
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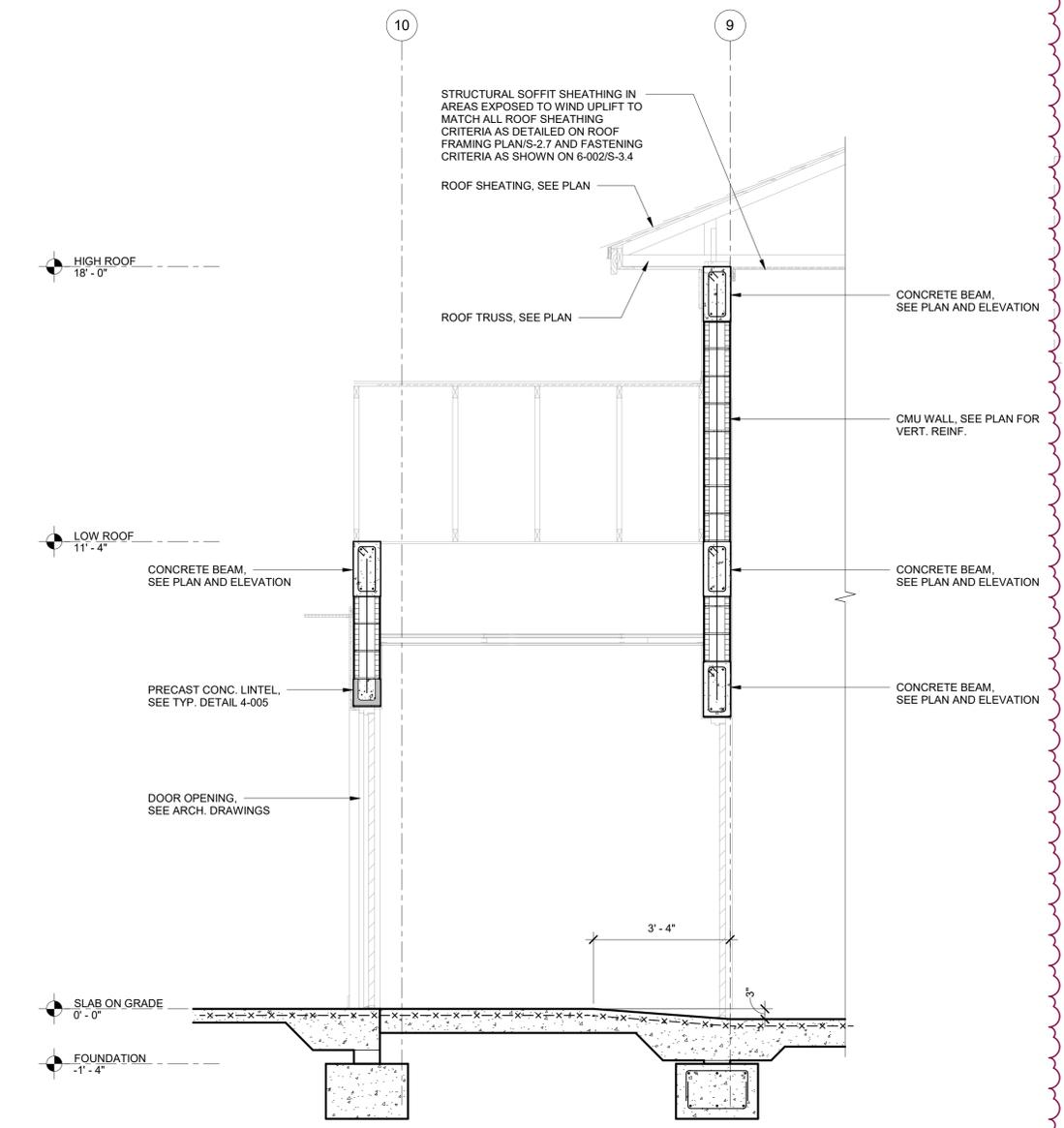
S4 SECTION 4
S-4.2 SCALE: 1/2" = 1'-0"



S5 SECTION 5
S-4.2 SCALE: 1/2" = 1'-0"



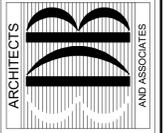
S6 SECTION 6
S-4.2 SCALE: 1/2" = 1'-0"



S7 SECTION 7
S-4.2 SCALE: 1/2" = 1'-0"

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AR-AA 000886
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VERO BEACH, FLORIDA 32960
PHONE: (772) 569-4320



SEAL:

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1840 25TH STREET
VERO BEACH, FL 32960

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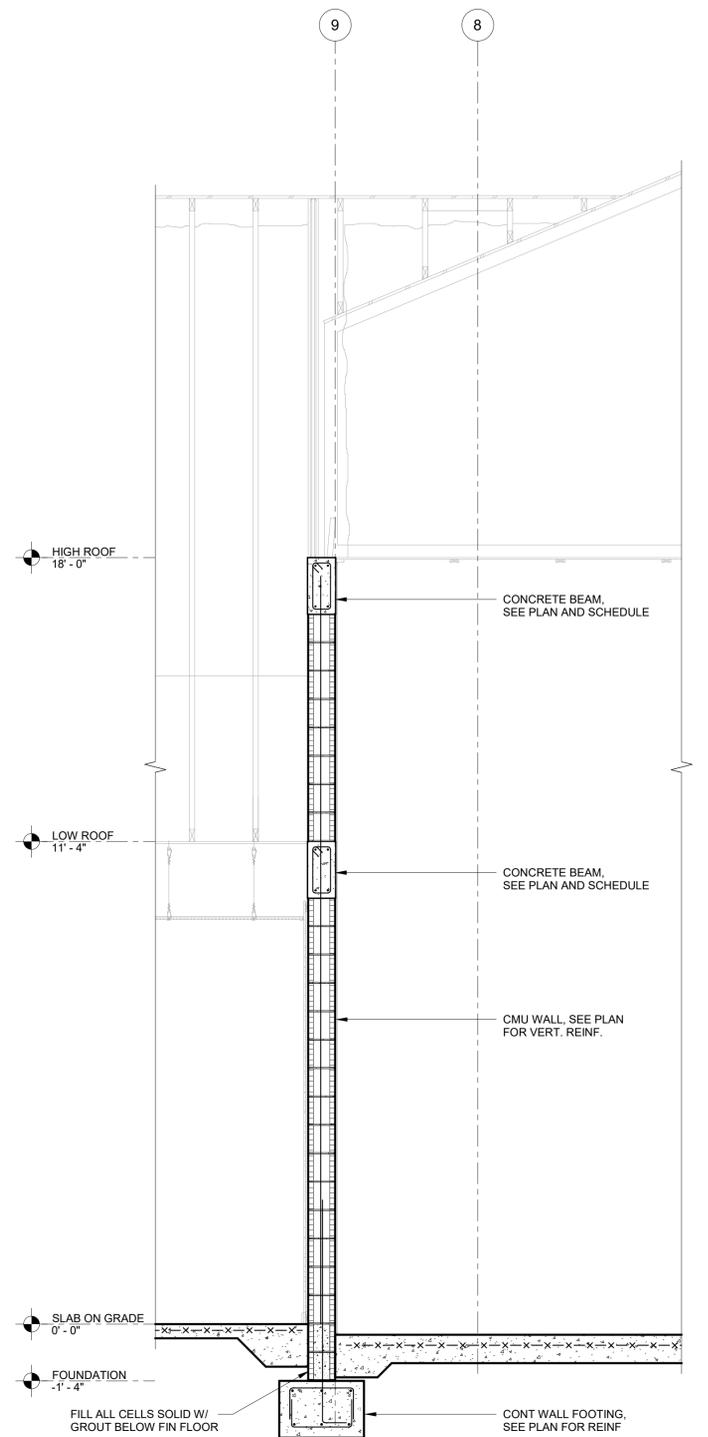


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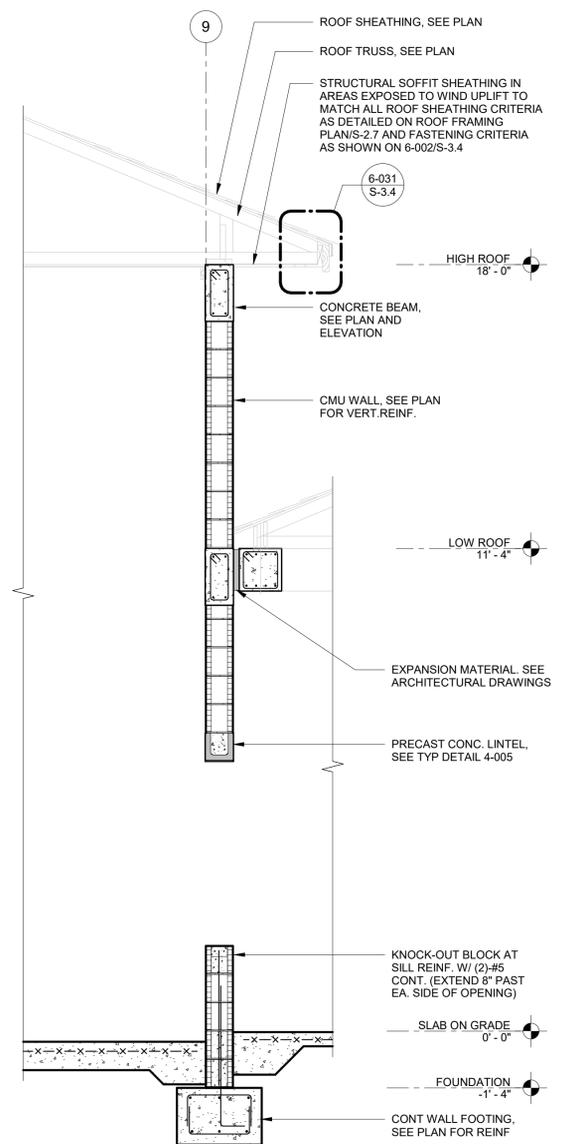
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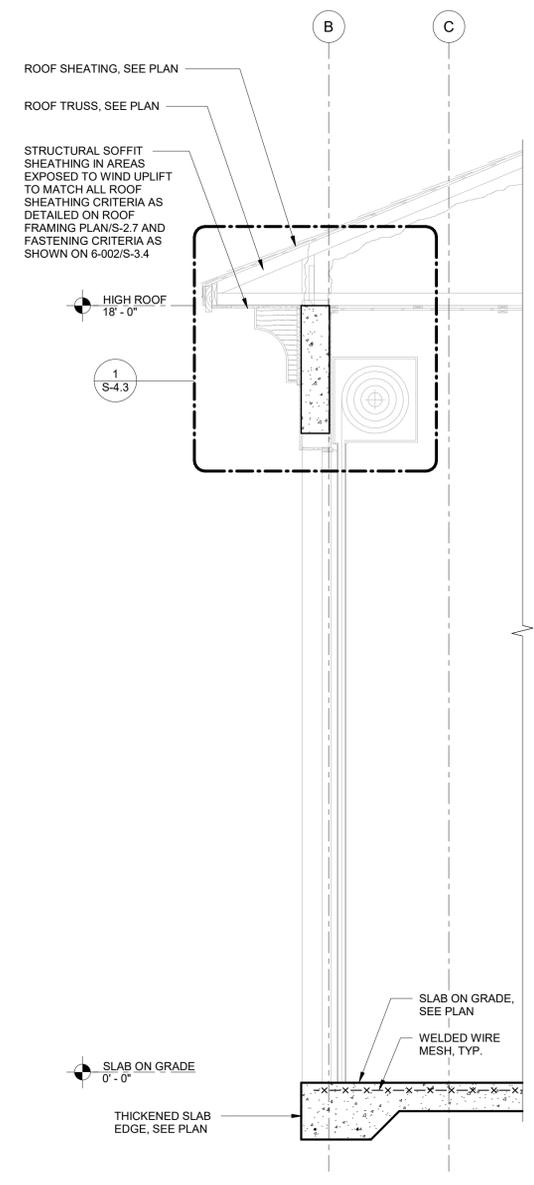
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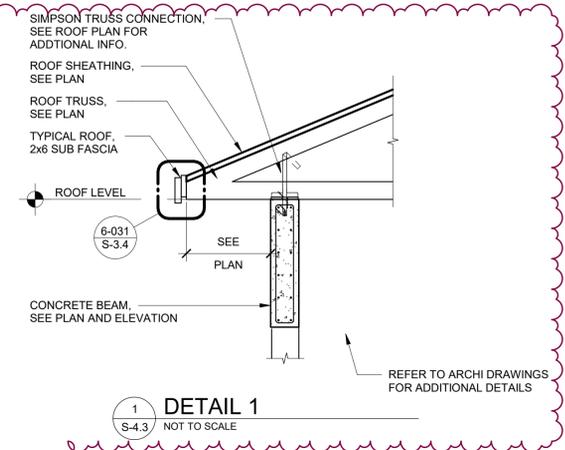
S8 SECTION 8
S-4.3 SCALE: 1/2" = 1'-0"



S9 SECTION 9
S-4.3 SCALE: 1/2" = 1'-0"



S10 SECTION 10
S-4.3 SCALE: 1/2" = 1'-0"



1 DETAIL 1
S-4.3 NOT TO SCALE

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ARCHITECTS AND ASSOCIATES

PROJECT: STATION #7
FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
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SECTION NO. S-4.3
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SECTIONS AND DETAILS

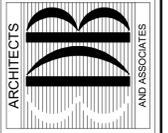
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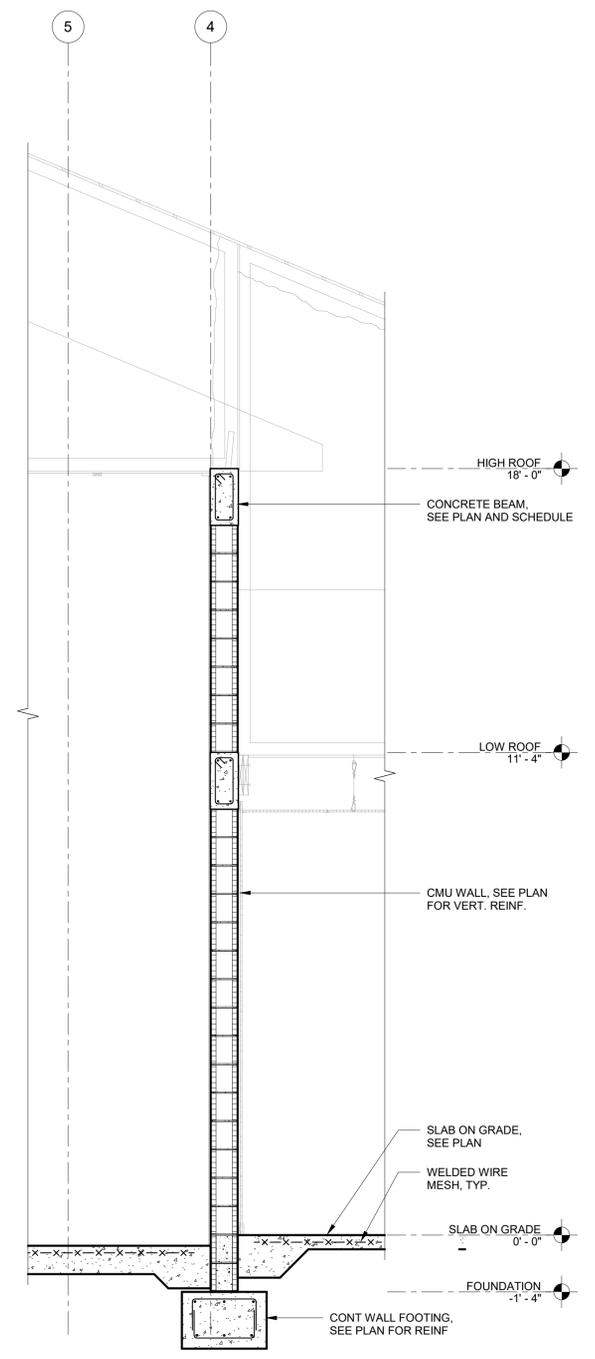
SECTIONS AND DETAILS

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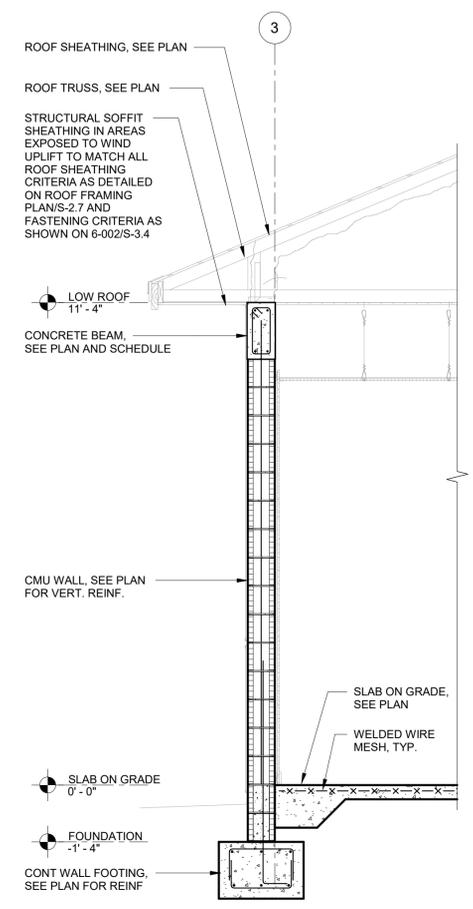
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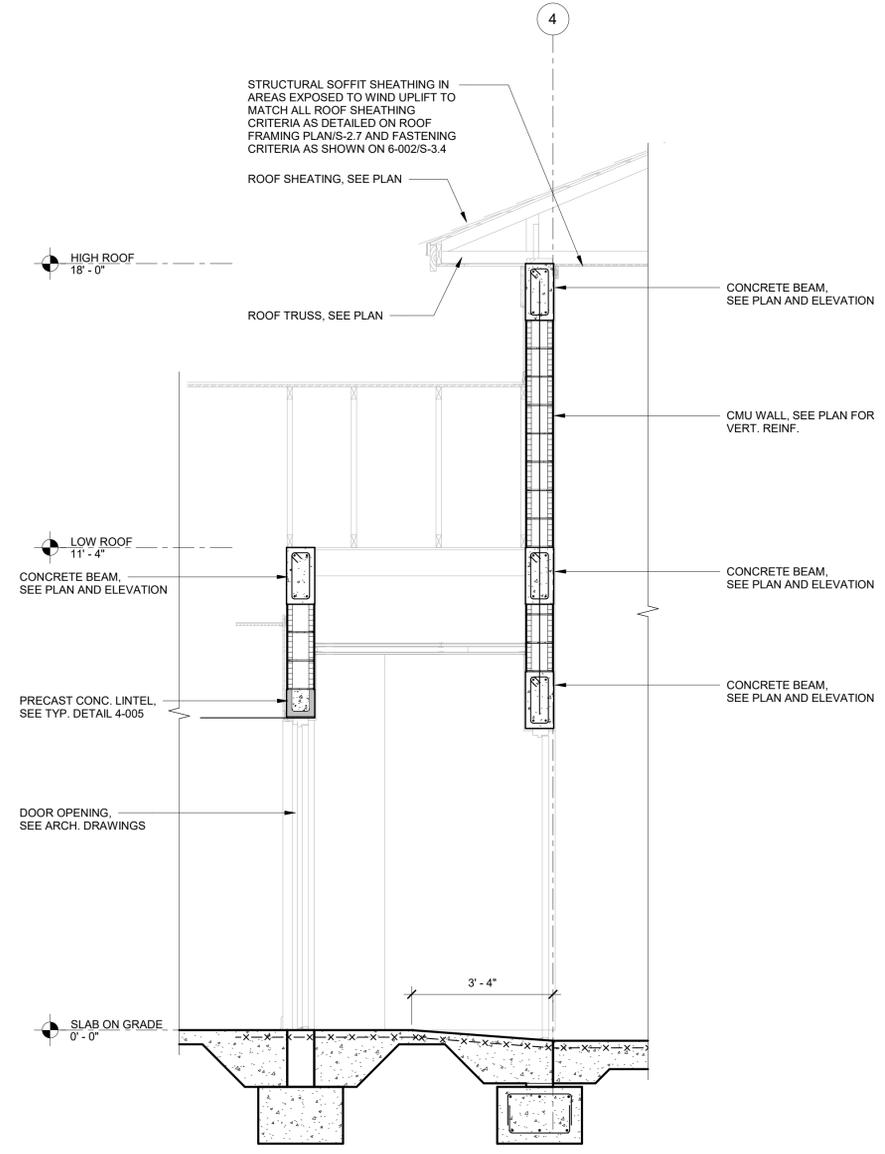
SHEET NO.
S-4.4
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S13 SECTION 13
 S-4.4 SCALE: 1/2" = 1'-0"



S12 SECTION 12
 S-4.4 SCALE: 1/2" = 1'-0"



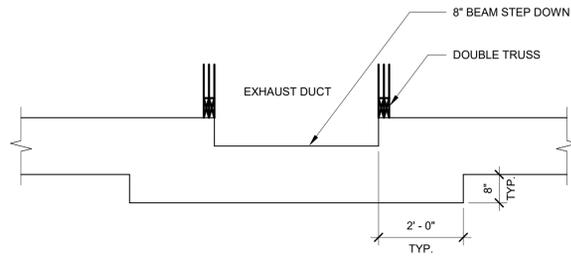
S11 SECTION 11
 S-4.4 SCALE: 1/2" = 1'-0"



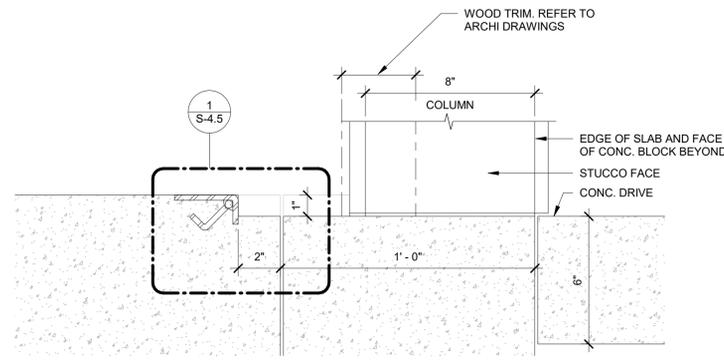
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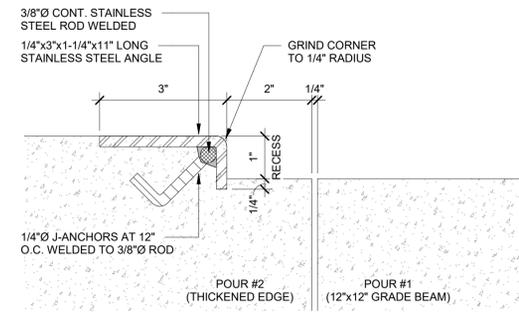
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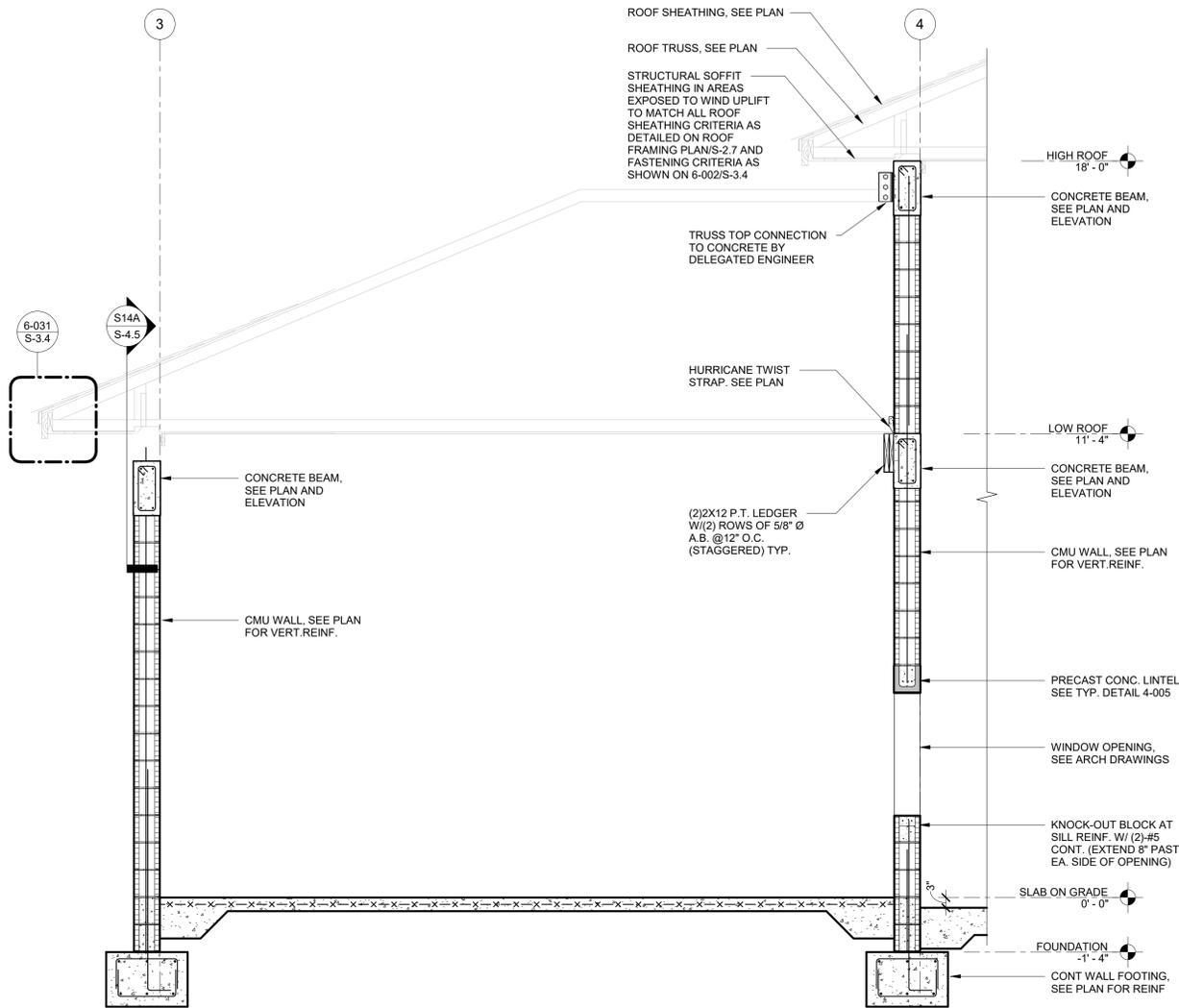
S14A
S-4.5 SECTION 14A
SCALE: 1/2" = 1'-0"



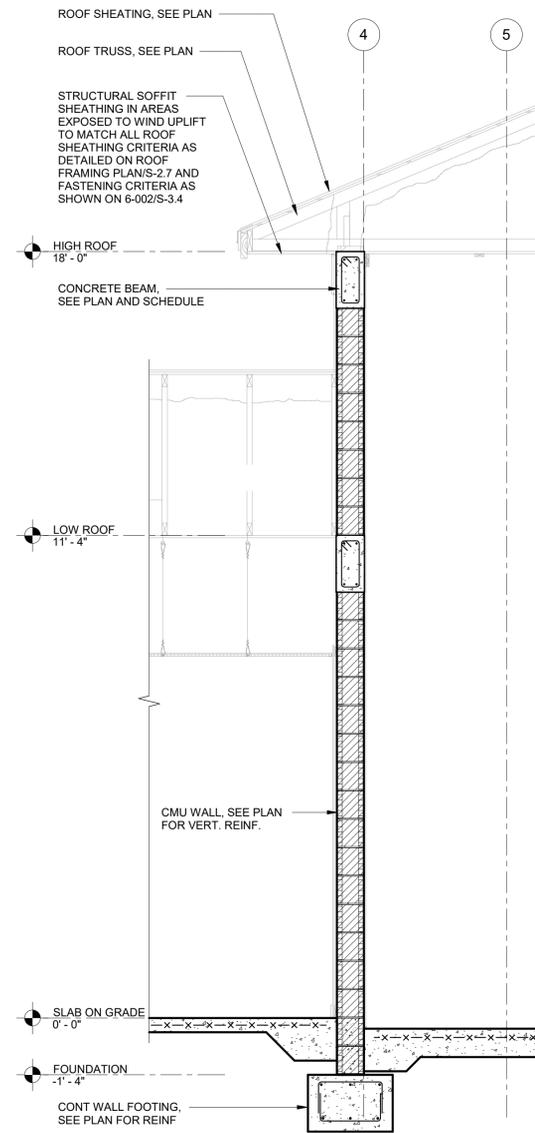
1
S-4.5 TYPICAL DETAIL AT APPARATUS BAY OVERHEAD DOORS
SCALE: 3" = 1'-0"



DETAIL 1
SCALE: 6" = 1'-0"



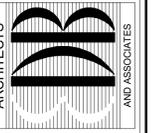
S14
S-4.5 SECTION 14
SCALE: 1/2" = 1'-0"



S15
S-4.5 SECTION 15
SCALE: 1/2" = 1'-0"

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VERO BEACH, FL 32960

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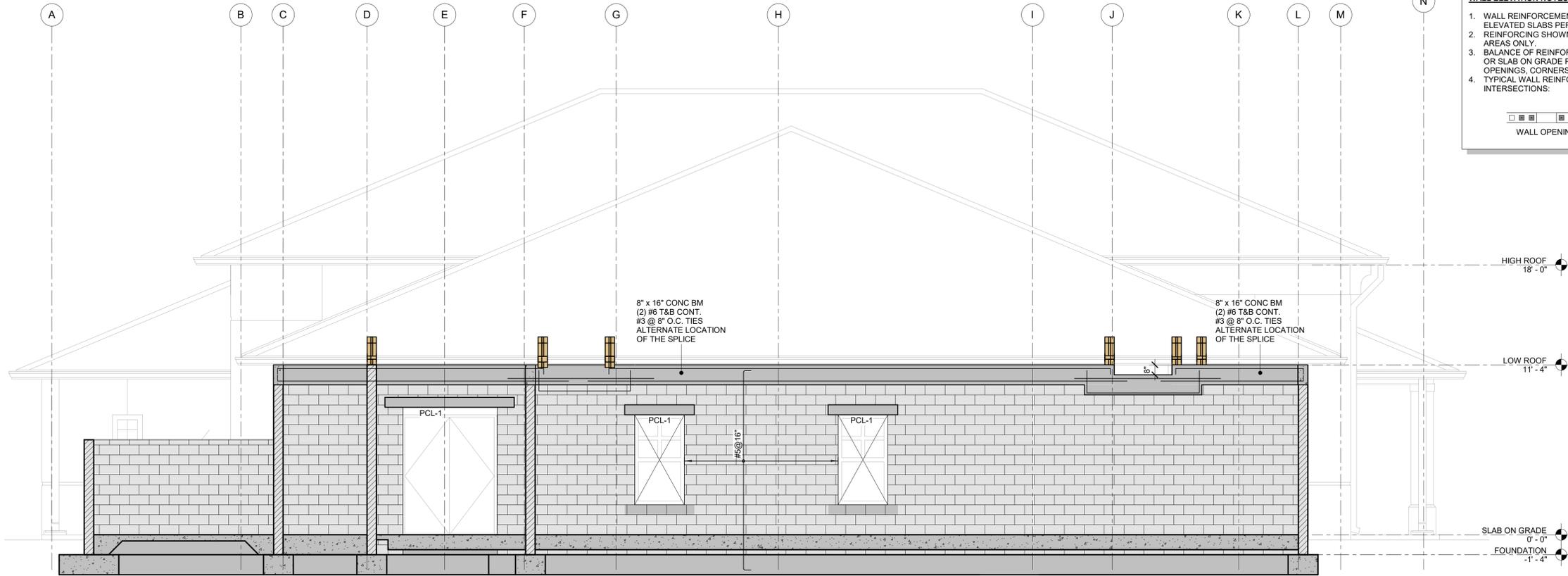


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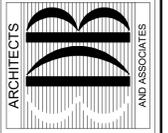


WALL ELEVATION NOTES:

1. WALL REINFORCEMENT TO BE SPLICED ABOVE SLAB ON GRADE AND ELEVATED SLABS PER TYPICAL DETAILS.
2. REINFORCING SHOWN ON WALL ELEVATIONS APPLY TO THE SPECIFIED AREAS ONLY.
3. BALANCE OF REINFORCING PER THE SCHEDULE SHOWN ON FOUNDATION OR SLAB ON GRADE PLANS INCLUDING TYPICAL REINFORCING AT WALL OPENINGS, CORNERS, AND INTERSECTION.
4. TYPICAL WALL REINFORCING AT OPENINGS, CORNERS, AND INTERSECTIONS:



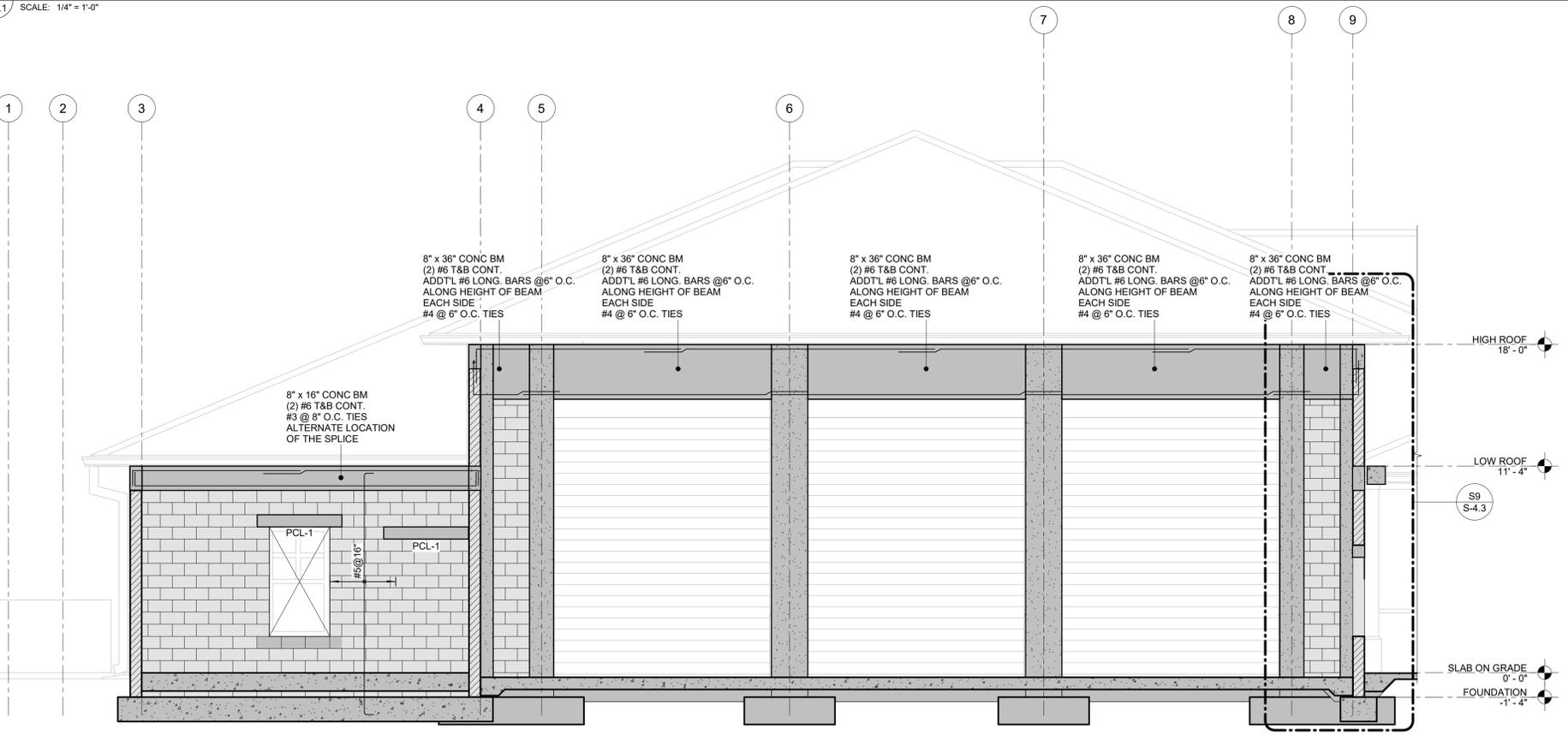
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SEAL:

E1 ELEVATION 1
S-5.1 SCALE: 1/4" = 1'-0"



E2 ELEVATION 2
S-5.1 SCALE: 1/4" = 1'-0"

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ELEVATIONS

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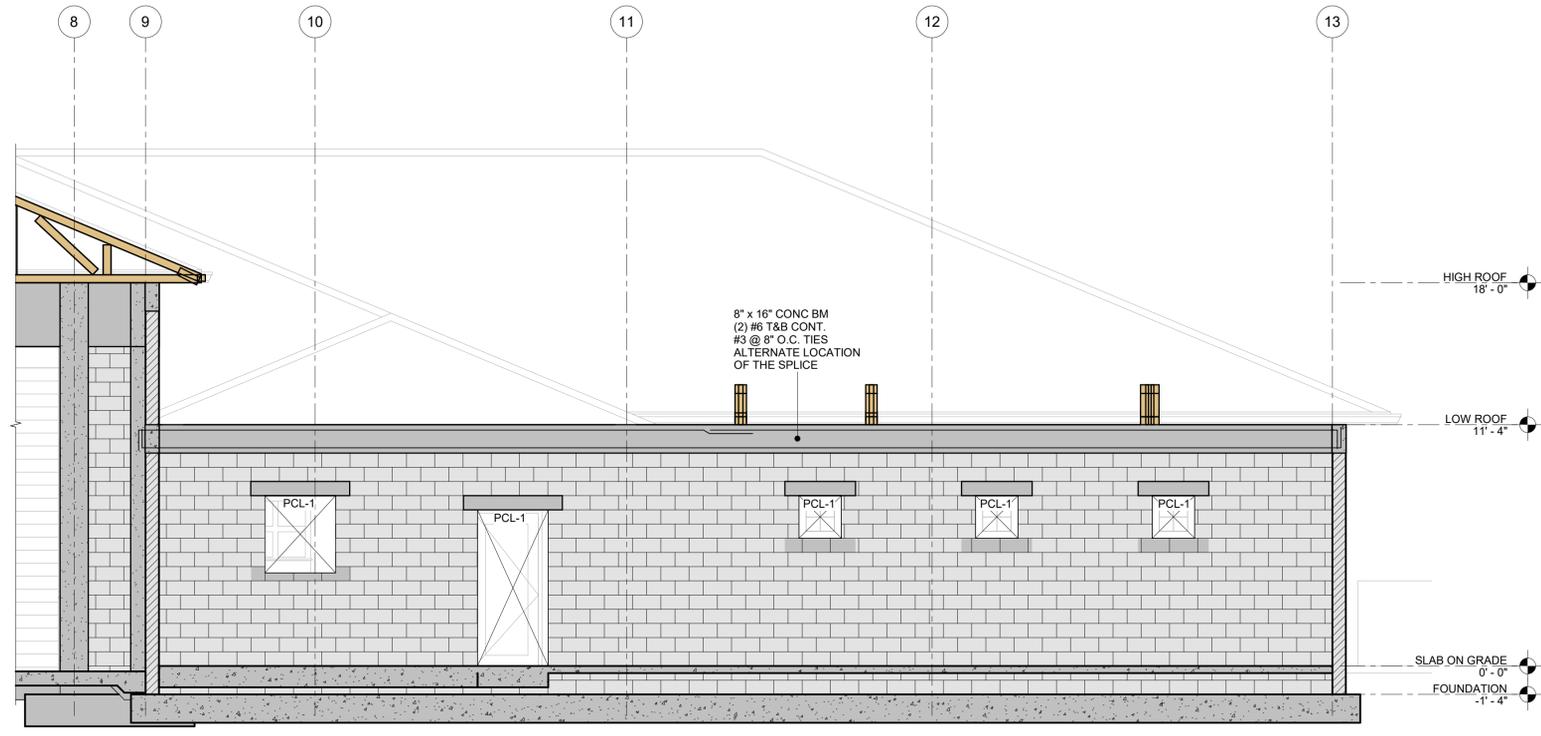
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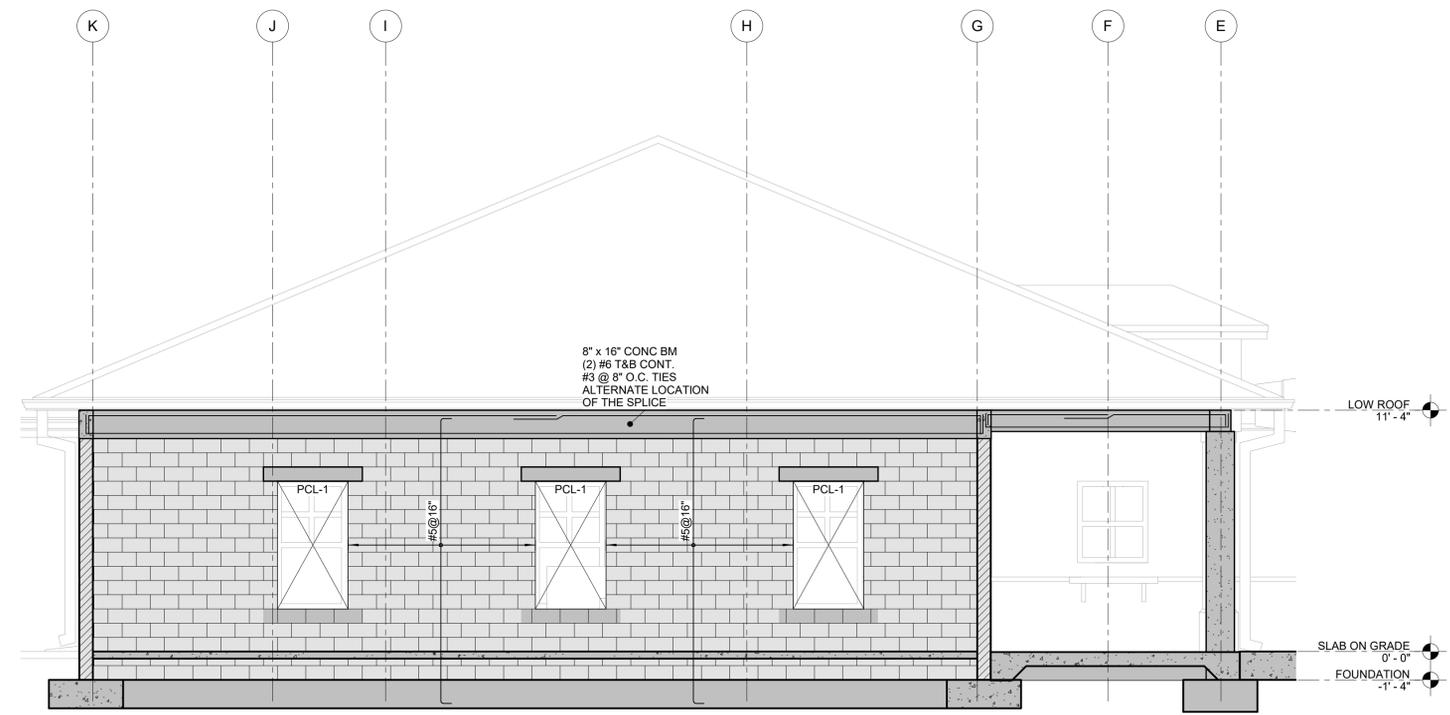
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E3
S-5.2 ELEVATION 3
SCALE: 1/4" = 1'-0"



E4
S-5.2 ELEVATION 4
SCALE: 1/4" = 1'-0"

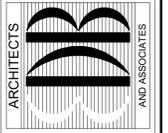
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4. TYPICAL WALL REINFORCING AT OPENINGS, CORNERS, AND INTERSECTIONS:



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NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

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DATE: 15 AUG. 2023
BY: AN
CHK'D: JBCV

SHEET NO.
S-5.2
OF TWENTY EIGHT



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22/08/2023 3:35:48 pm

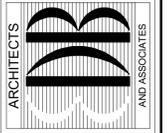
ELEVATIONS

WALL ELEVATION NOTES:

1. WALL REINFORCEMENT TO BE SPICED ABOVE SLAB ON GRADE AND ELEVATED SLABS PER TYPICAL DETAILS.
2. REINFORCING SHOWN ON WALL ELEVATIONS APPLY TO THE SPECIFIED AREAS ONLY.
3. BALANCE OF REINFORCING PER THE SCHEDULE SHOWN ON FOUNDATION OR SLAB ON GRADE PLANS INCLUDING TYPICAL REINFORCING AT WALL OPENINGS, CORNERS, AND INTERSECTION.
4. TYPICAL WALL REINFORCING AT OPENINGS, CORNERS, AND INTERSECTIONS:

WALL OPENING CORNER INTERSECTION

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ARCHITECTS AND ASSOCIATES, P.A.
AR-AA 000886
65 ROYAL PALM POINTE, SUITE "D"
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SEAL:

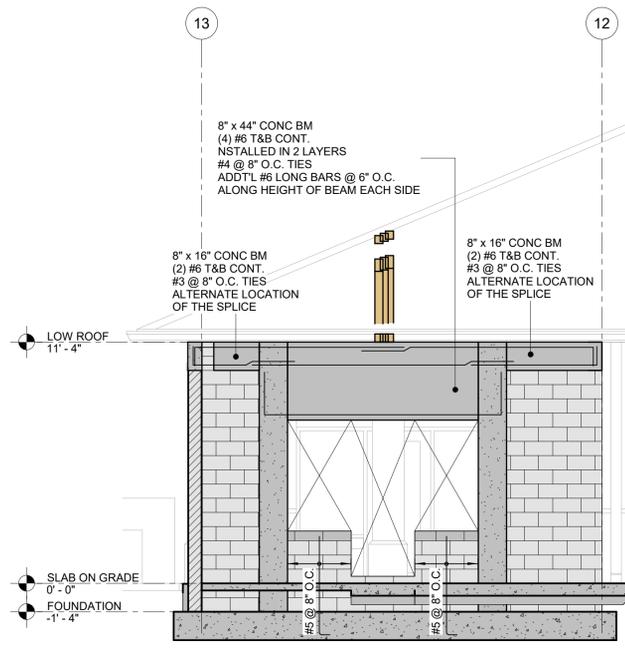
PROJECT: **STATION #7**
FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

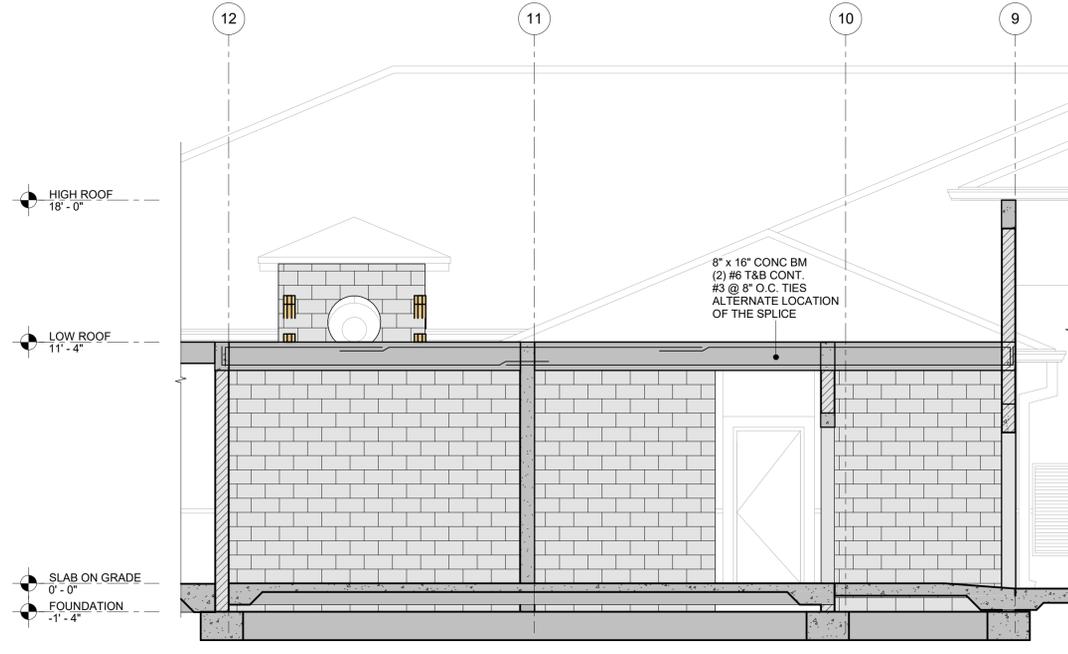
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CH'KD: JB/CV

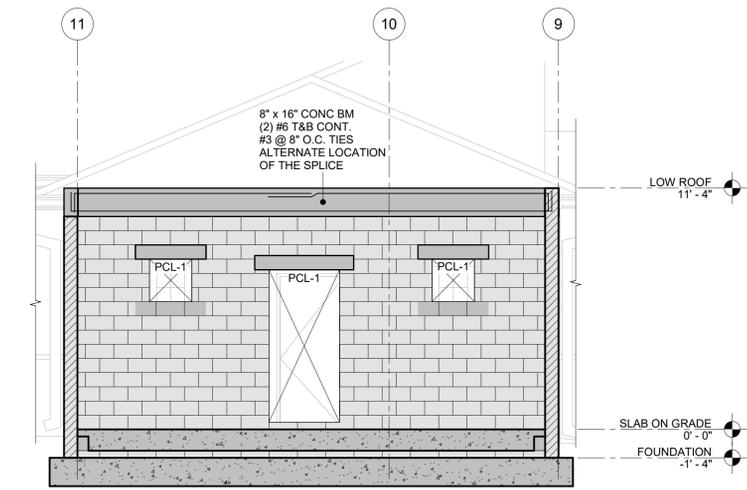
SHEET NO.
S-5.3
OF TWENTY EIGHT



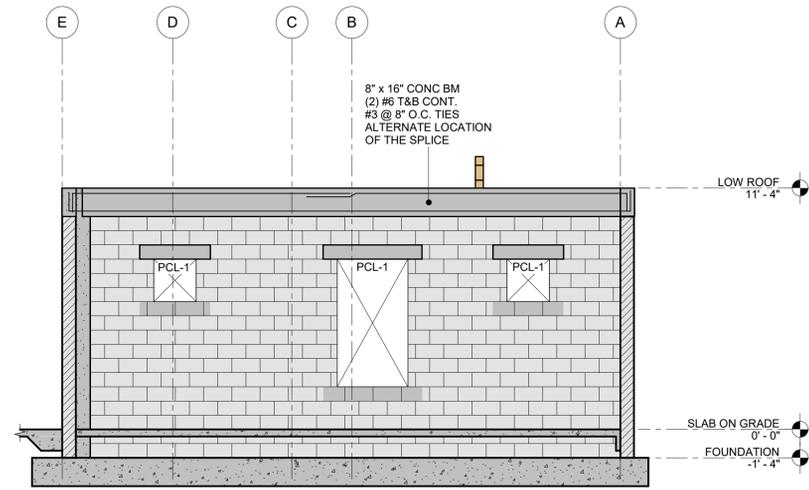
E5 ELEVATION 5
S-5.3 SCALE: 1/4" = 1'-0"



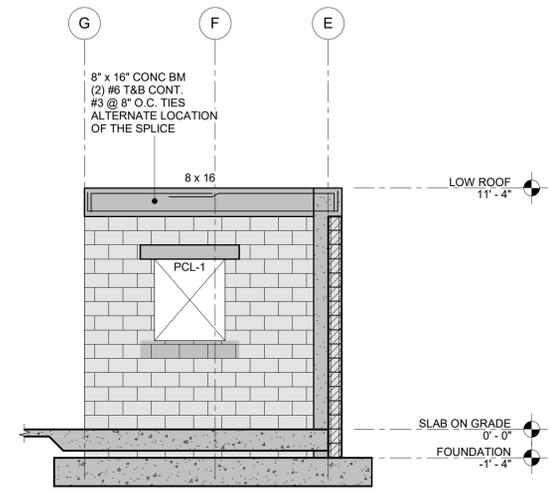
E6 ELEVATION 6
S-5.3 SCALE: 1/4" = 1'-0"



E7 ELEVATION 7
S-5.3 SCALE: 1/4" = 1'-0"



E8 ELEVATION 8
S-5.3 SCALE: 1/4" = 1'-0"



E9 ELEVATION 9
S-5.3 SCALE: 1/4" = 1'-0"



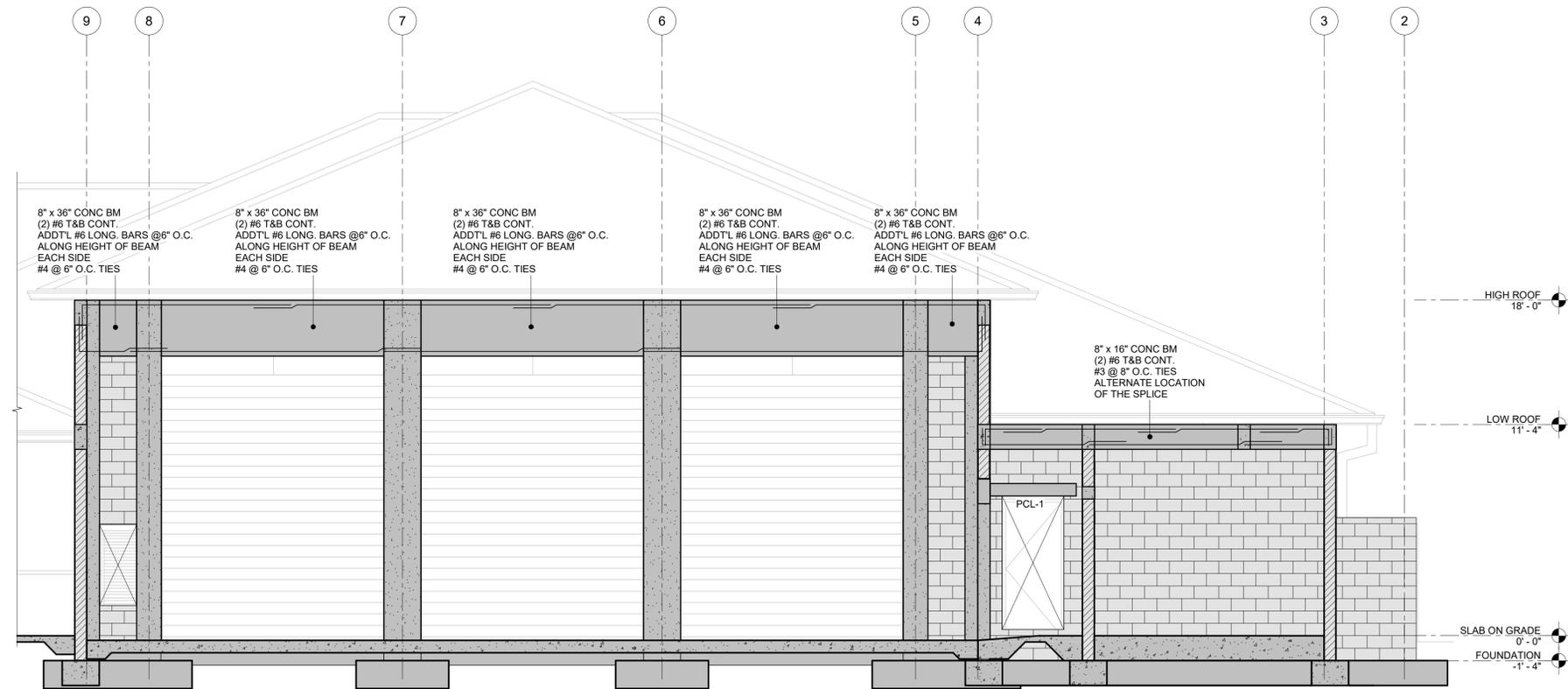
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ELEVATIONS

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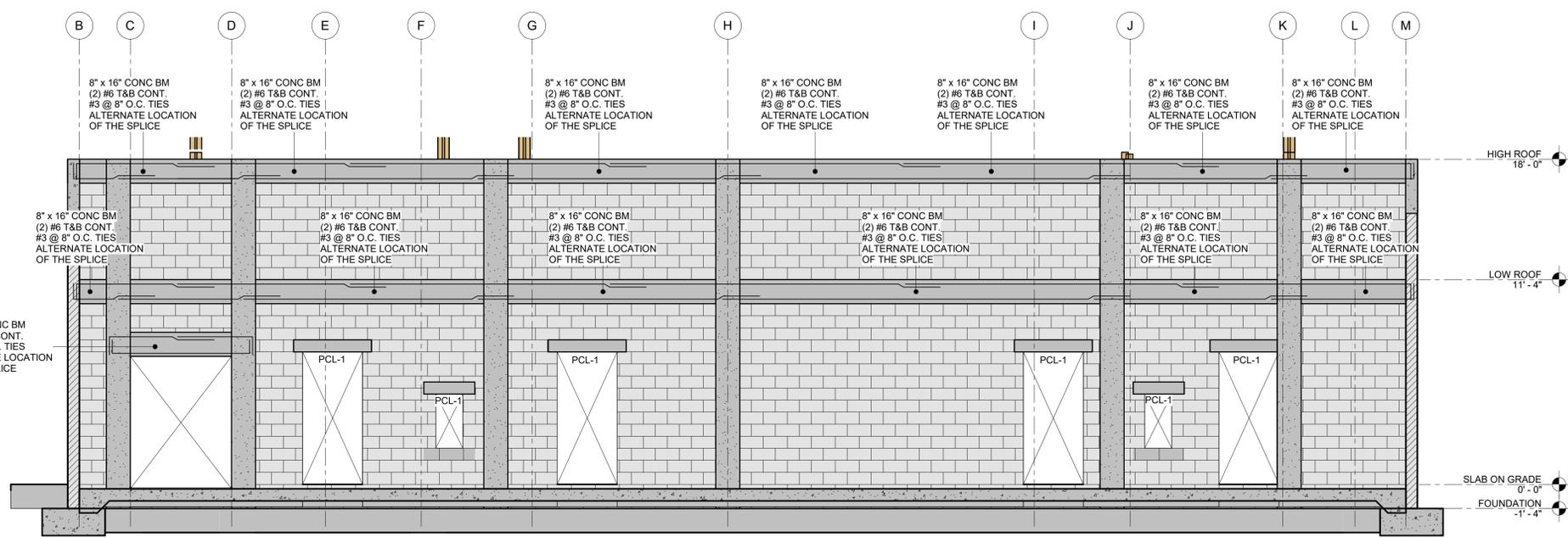


WALL ELEVATION NOTES:

1. WALL REINFORCEMENT TO BE SPLICED ABOVE SLAB ON GRADE AND ELEVATED SLABS PER TYPICAL DETAILS.
2. REINFORCING SHOWN ON WALL ELEVATIONS APPLY TO THE SPECIFIED AREAS ONLY.
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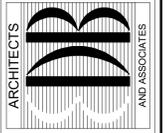
 WALL OPENING
  CORNER
  INTERSECTION

E10 ELEVATION 10
S-5.4 SCALE: 1/4" = 1'-0"



E11 ELEVATION 11
S-5.4 SCALE: 1/4" = 1'-0"

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SEAL:

LEAVE THIS AREA
BLANK FOR
ELECTRONIC SEAL

PROJECT: STATION #7
FOR: INDIAN RIVER COUNTY FIRE DISTRICT
1840 25TH STREET
VERO BEACH, FL 32960

NO.	DATE	REVISIONS
1	08/15/23	REVISION 1

ELEVATIONS

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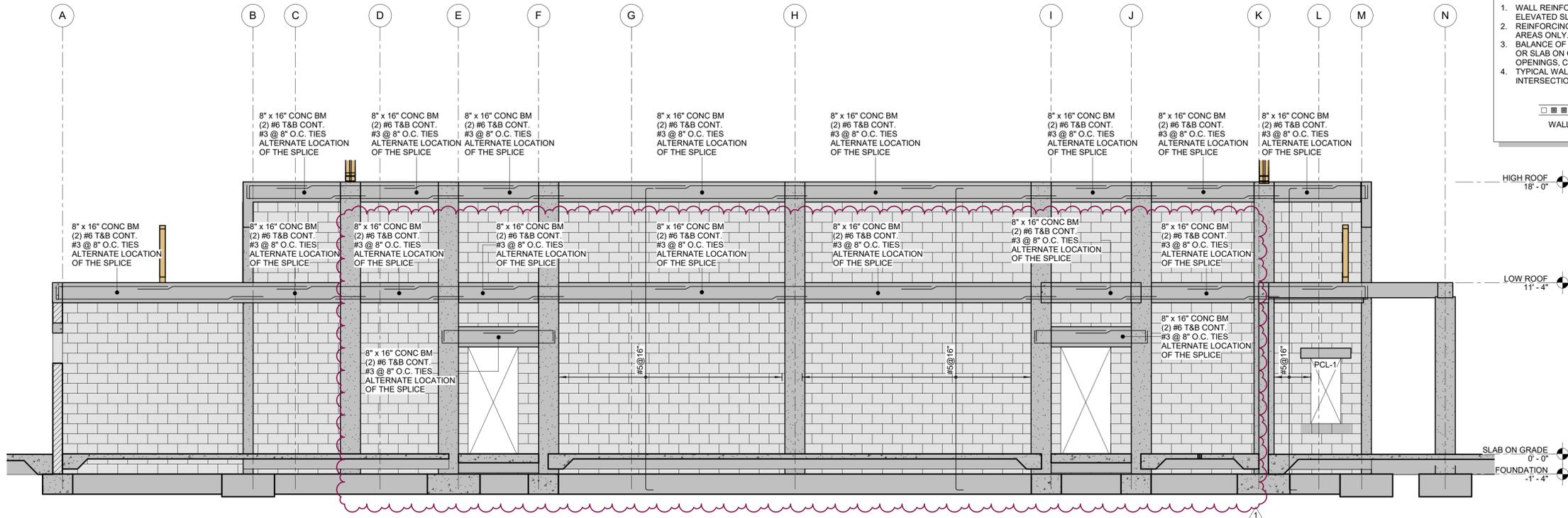
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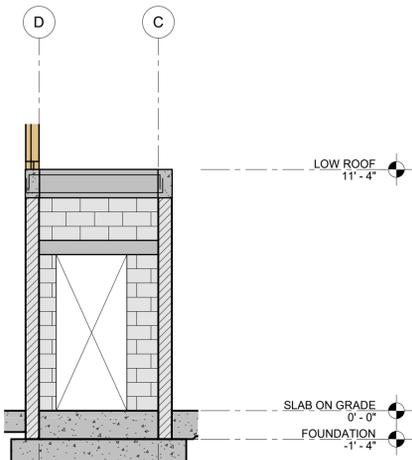


WALL ELEVATION NOTES:

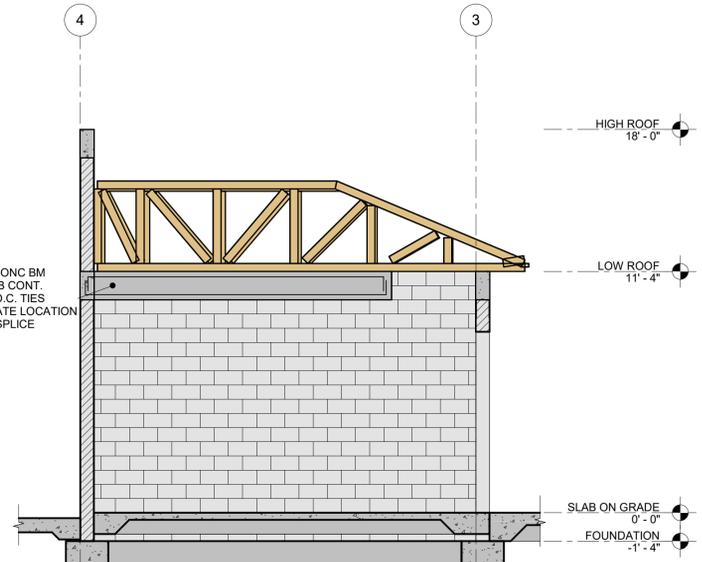
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WALL OPENING
 CORNER
 INTERSECTION

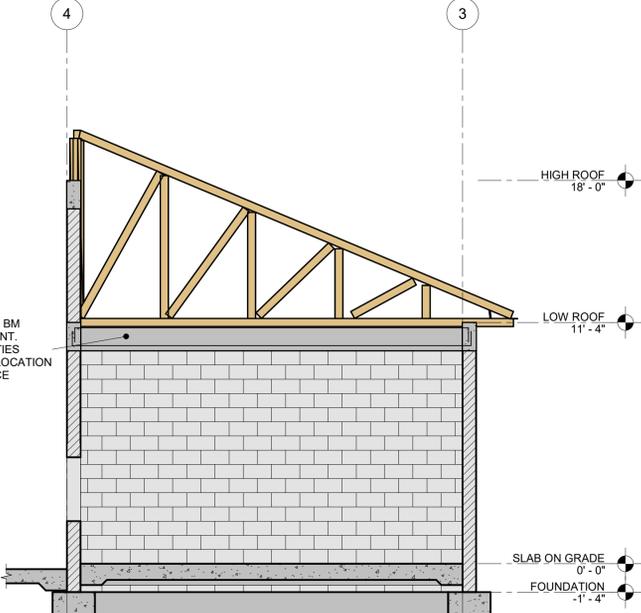
E12 ELEVATION 12
S-5.5 SCALE: 1/4" = 1'-0"



E13 ELEVATION 13
S-5.5 SCALE: 1/4" = 1'-0"



E14 ELEVATION 14
S-5.5 SCALE: 1/4" = 1'-0"



E15 ELEVATION 15
S-5.5 SCALE: 1/4" = 1'-0"

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SEAL:

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ELEVATIONS

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