

GENERAL NOTES:

- 1. TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE FLORIDA BUILDING CODE, 2020 7TH EDITION.
2. THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DOCUMENTS. USE THESE NOTES IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
3. COMPLY WITH REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
4. ALL REFERENCED STANDARDS REFER TO THE EDITION IN FORCE AT THE TIME THESE PLANS AND SPECIFICATIONS ARE ISSUED FOR BIDDING.
5. REVIEW ALL CONTRACT DOCUMENTS, DIMENSIONS AND SITE CONDITIONS AND COORDINATE WITH FIELD DIMENSIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT/ENGINEER. DO NOT CHANGE SIZE OR DIMENSIONS OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
6. ANY DISCREPANCIES, OMISSIONS, OR VARIATIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PERIOD SHALL BE IMMEDIATELY COMMUNICATED IN WRITING TO THE ARCHITECT/ENGINEER.
7. PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE. EACH CONTRACTOR SHALL PROTECT HIS WORK, ADJACENT PROPERTY AND THE PUBLIC. EACH CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLIGENCE.
8. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY AND CONSTRUCTION PROCEDURES.
9. DO NOT SCALE DRAWINGS; USE DIMENSIONS.
10. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS IN STRUCTURE NOT SHOWN ON STRUCTURAL DRAWINGS.
11. DETAILS LABELED "TYPICAL DETAILS" ON THE DRAWINGS APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT THEY ARE KEYPED IN AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT/ENGINEER.
12. REVISIONS ARE IDENTIFIED BY A REVISION NUMBER WITHIN A TRIANGLE. ALL REVISIONS ISSUED ON A SINGLE DATE WILL BE IDENTIFIED BY THE SAME REVISION NUMBER ISSUED CONSEQUENTLY.
13. CURRENT REVISIONS ARE ENCIRCLED BY AN IRREGULAR "CLOUD", AS WELL AS FLAGGED WITH THE CURRENT REVISION NUMBER. CLOUDS ARE REMOVED FROM PREVIOUSLY ISSUED REVISIONS.
14. DESIGN LOADS AND CRITERIA:
FLOOR LIVE LOAD 100 PSF
PARTITION LOAD 15 PSF
ROOF LIVE LOAD 20 PSF
FLOOR DEAD LOAD SELF WEIGHT
WIND CRITERIA ASCE 7-10
ULTIMATE WIND SPEED 160 MPH EXPOSURE "C"
RISK CATEGORY II
IMPORTANCE FACTOR 1.00
STRUCTURE TYPE OPEN

SHOP DRAWING SUBMITTALS:

- 1. THE FOLLOWING REQUIREMENTS IN NO WAY REDUCE OR LIMIT ANY ADDITIONAL REQUIREMENTS OF SPECIFICATIONS.
2. REVIEW OF SUBMITTALS BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AS PRESENTED BY THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF QUANTITIES OR DIMENSIONS WILL BE MADE. ONLY THOSE SHOP DRAWINGS REQUIRED BY THE CONTRACT DOCUMENTS TO BE SUBMITTED WILL BE REVIEWED. ALL OTHERS WILL BE RETURNED WITHOUT COMMENT.
3. IN ACCORDANCE WITH THE SPECIFICATIONS, SUBMIT A COPY OF THE SHOP DRAWING SUBMITTAL REGISTER TO THE STRUCTURAL ENGINEER, SHOWING DATES OF SUBMITTAL FOR EACH SPECIFIC STRUCTURAL SECTION OF THE WORK, CONSISTENT WITH THE FOLLOWING CRITERIA:
A. ALLOW ADEQUATE TIME FOR TRANSIT AND PROCESSING BEFORE FABRICATION. THE STRUCTURAL ENGINEER WILL REVIEW AN AVERAGE SUBMITTAL WITHIN 10 WORKING DAYS OF RECEIPT BY THEM.
B. SCHEDULE AND SUBMIT SHOP DRAWINGS FOR SPECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN THEIR ENTIRETY. SHOP DRAWINGS FOR SIMILAR FLOORS SHALL BE SUBMITTED IN THE SAME PACKAGE.
C. SUBMIT SHOP DRAWINGS IN A TIMELY MANNER, CONSISTENT WITH THE ABOVE REQUIREMENTS.
4. ALL CHANGES AND ADDITIONS MADE ON RESUBMITTALS MUST BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RESUBMITTALS MUST BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT / ENGINEER REVIEW WILL BE LIMITED TO THE ITEMS CAUSING THE RESUBMITTAL. DO NOT REPRODUCE THE CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS.
5. SHOP DRAWINGS NOT MEETING THE ABOVE CRITERIA OR SUBMITTED AFTER FABRICATION WILL NOT BE REVIEWED AND WILL BE RETURNED WITHOUT COMMENT.
6. RESPONSIBILITIES OF DETAILERS AND FABRICATORS:
A. GENERAL - SUBMIT SHOP DRAWINGS AND ANY OTHER SPECIAL INFORMATION NECESSARY FOR PROPER FABRICATION, ERECTION, AND PLACEMENT OF STRUCTURAL FABRICATIONS. INCLUDE PLANS, ELEVATIONS, AND SECTIONS. CLEARLY SHOW ANCHORAGES, CONNECTIONS, AND ACCESSORY ITEMS. THE DETAILER MUST INTERPRET THE CONTRACT DOCUMENTS AND CLEARLY CONVEY THIS INTERPRETATION TO THE FIELD IN THE FORM OF PLACING OR ERECTION DRAWINGS.
B. CONCRETE REINFORCING DETAILER - PROVIDE PLACING DRAWINGS FOR FABRICATION AND PLACING OF REINFORCING STEEL. THESE DRAWINGS SHALL INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING: BAR LISTS, SCHEDULES, BENDING DETAILS, PLACING DETAILS, PLACING PLANS, AND PLACING ELEVATIONS.
- CLEARLY SHOW ELEVATIONS OF ALL FOUNDATION WALLS. INDICATE CONTROL JOINTS, EXPANSION JOINTS, LINTELS, CONCRETE BOND BEAMS, AND OPENINGS.
- DETAILS OF ALL REINFORCING WITH LOCATIONS OF SPLICES, AND HOOKS, FILASTERS.
- CLEARLY SHOW GRADE BEAM ELEVATIONS AND SECTIONS. INDICATE BAR LENGTHS, HOOKS, STIRRUP SPACING, LAP SPLICES, OFFSETS, AND LOCATION OF BARS WITH RESPECT TO ALL SUPPORTS.
- CLEARLY SHOW COLUMN ELEVATIONS AND SECTIONS. INDICATE DOWELS, OFFSETS, LAP SPLICES, AND TIES. PLAN SECTIONS OF ALL COLUMNS MUST CLEARLY BE SHOWN.
- CLEARLY SHOW FOUNDATION REINFORCING. INDICATE BAR LENGTHS, LOCATION AND SPLICES OF CONTINUOUS BARS, AND BAR SUPPORTS.
- CLEARLY SHOW LOCATIONS OF ALL DOWELS ON PLAN. INDICATE FOOTING STEP LOCATIONS AND PROVIDE DETAILS.
7. FOR ADDITIONAL CRITERIA APPLICABLE TO SHOP DRAWINGS REQUIRING ENGINEERING INPUT BY A SPECIALTY ENGINEER, SEE BELOW

FORMWORK, SHORING & RESHORING:

- 1. PROVIDE, AS A PACKAGE, SHORING AND RESHORING DRAWINGS PREPARED BY OR UNDER THE DIRECT SUPERVISION OF A SPECIALTY ENGINEER.
2. THE SHORING AND RESHORING DRAWINGS SHALL CONTAIN, AS A MINIMUM, THE FOLLOWING:
A. LOCATION, SIZE, AND TYPE OF ALL SHORING.
B. LOCATION, SIZE, AND TYPE OF ALL RESHORING.
C. LOCATION, SIZE, AND TYPE OF ALL MUD SILLS, BLOCKING, TEMPORARY LATERAL BRACING AND OTHER ACCESSORIES NECESSARY TO SAFELY SUPPORT AND BRACE THE STRUCTURE DURING CONSTRUCTION.
D. THE SEQUENCE OF INSTALLATION LOAD RELIEF AND REMOVAL OF ALL SHORING AND RESHORING.
3. SUBMIT ONE RECORD COPY OF THE SHORING AND RESHORING DRAWINGS, SIGNED, SEALED AND DATED BY THE SPECIALTY ENGINEER, TO THE ARCHITECT, ENGINEER, AND INSPECTOR.
4. DESIGN AND ERECT FORMS AND SHORES IN ACCORDANCE WITH ACI 347 AND AS SPECIFIED HEREIN. DESIGN FORMS AND SHORES FOR HORIZONTAL CONCRETE MEMBERS FOR NOT LESS THAN DEAD LOAD PLUS 50 PSF CONSTRUCTION LOAD, EQUIPMENT LOAD AND FOR THE CUMULATIVE LOADS OF SUPPORTED FLOOR, DESIGN WOOD SHORES WITH A SAFETY FACTOR OF 3 AND METAL SHORES WITH A SAFETY FACTOR OF 2.
5. THE MAXIMUM SUPERIMPOSED CONSTRUCTION LOAD APPLIED TO FLOORS SUPPORTING SHORES OR RESHORES SHALL NOT EXCEED 75% OF THE SPECIFIED LOAD.
6. REMOVAL OF FORMWORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. REMOVE FORMS IN SUCH A MANNER TO INSURE JOB SAFETY AND TO PREVENT DAMAGE TO AND DEFLECTION OF THE STRUCTURE EXCEPT FOR FORMWORK NOT SUPPORTING CONCRETE. SUCH AS BEAMS SIDES, WALLS AND COLUMNS. FORMS MAY BE REMOVED NO SOONER THAN 72 HOURS AFTER COMPLETING CONCRETE POUR PROVIDED CONCRETE STRENGTH IS 70% OF THE SPECIFIED 28 DAY STRENGTH AND NOT LESS THAN 3000 PSI. RESHORE EACH DAY AS IT IS STRIPPED, AS SOON AS FORMS ARE REMOVED.
7. DO NOT REMOVE SHORING UNTIL CONCRETE HAS REACHED 7 DAY STRENGTH, BUT NOT LESS THAN 3000 PSI. AS A MINIMUM, SHORING AND RESHORING SHALL REMAIN IN PLACE AS FOLLOWS: (TIME MEASURED FROM COMPLETION OF CONCRETE POUR).

Table with 3 columns: SLABS, SHORES, RESHORES. Rows include BEAMS, SPANS LESS THAN 30'-0" (7 DAYS, 14 DAYS), BEAMS, SPANS GREATER THAN 30'-0" (7 DAYS, 14 DAYS).

- 8. THE SHORING AND RESHORING IS TO BE INSPECTED BY THE SPECIALTY ENGINEER OR HIS AUTHORIZED REPRESENTATIVE PRIOR TO EACH CONCRETE POUR. HE SHALL SUBMIT A WRITTEN INSPECTION REPORT TO THE PROJECT ARCHITECT, BUILDING ENGINEER, INSPECTING AGENCY AND GENERAL CONTRACTOR STATING THAT THE WORK IS IN GENERAL COMPLIANCE WITH THE SHORING AND RESHORING DRAWINGS.
THE SHORING AND RESHORING REPORT SHALL CONTAIN, AS A MINIMUM, THE FOLLOWING:
A. NAME AND LOCATION OF PROJECT, NAME OF SPECIALTY ENGINEER AND FIELD REPRESENTATIVE.
B. PERMIT NUMBER, DATE, TIME OF DAY, WORKING CONDITIONS INCLUDING WEATHER AND TEMPERATURE.
C. ITEMS REQUIRING CORRECTIONS.
D. ACCEPTED DEVIATIONS FROM SHORING AND RESHORING DRAWINGS.
E. AREAS ACCEPTED AND RELEASED FOR CONCRETE POUR.

SHALLOW FOUNDATIONS:

- 1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF FOR FOOTINGS AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
2. SOIL COMPACTION SHALL BE FIELD CONTROLLED BY A SOILS ENGINEER OR TESTING LABORATORY. A REPORT BEARING THE SIGNATURE AND EMBOSSED SEAL OF THE REGISTERED PROFESSIONAL ENGINEER WHO CONTROLLED AND TESTED THE SOIL COMPACTION SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD AS AN INDICATION THAT THE REQUIREMENTS OF THE CONTRACT DOCUMENTS HAVE BEEN FOLLOWED.
3. SOIL COMPACTION, FILL, AND ITS REPLACEMENT SHALL BE FIELD CONTROLLED BY THE TESTING AGENCY OR GEOTECHNICAL ENGINEER OF RECORD. THE TESTING AGENCY SHALL RANDOMLY SELECT ALL TEST LOCATIONS.
4. THE CONTRACTOR IS RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER FROM EXCAVATIONS AND DEWATERING OPERATIONS IN SUCH A WAY AS NOT TO CAUSE INCONVENIENCE TO THE WORK AND DAMAGE TO THE STRUCTURAL ELEMENTS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

REINFORCED CONCRETE:

- 1. USE STRUCTURAL CONCRETE AND CONCRETING PRACTICES CONFORMING TO ACI-316 AND 301 AND PROPORTION CONCRETE IN ACCORDANCE WITH ACI-318 CH. 4 AND MEETING A MIN. ULTIMATE COMPRESSIVE STRENGTH IN 28 DAYS AS FOLLOWS:
FOOTING 4000 PSI
GRADE BEAMS 4000 PSI
ELEVATED SLABS 5000 PSI
WALLS 5,000 PSI

Table with 2 columns: FOOTING/PILECAP, WALLS, ELEVATED SLABS. Values: 3", 2", 3", 1", 1", 1".

- 2. PROVIDE CURRENT (MAX. 1 YEAR OLD) STATISTICAL DATA FOR EACH CONCRETE MIX DESIGN SUBMITTED. WHERE CONCENTRATION OF REINFORCING STEEL HINDERS PROPER CONSOLIDATION OF CONCRETE USE CONCRETE CONTAINING A SUPERPLASTICIZER (N.R.W.R.) ADMIXTURE, ASTM C494 TYPE F. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 7" IF CONCRETE IS PUMPED, SLUMP MAY BE INCREASED TO 6" AT THE TRUCK, PROVIDED THE SLUMP SPECIFIED IN NOTE 2 IS MAINTAINED AT THE DISCHARGE END. USE A MINIMUM 4-INCH PUMP, UNLESS PRE-APPROVED BY ARCHITECT. TAKE CONCRETE SAMPLES FOR SLUMP AT TRUCK AND AT DISCHARGE END. TAKE CONCRETE SAMPLES FOR CYLINDER TESTING AT DISCHARGE END.
3. USE ASTM A-615 GR. 60 FOR ALL REINFORCING STEEL, CONFORM TO ACI-301, ACI-315, ACI-318, AND CRSI "MANUAL OF STANDARD PRACTICE". ALL REINFORCING SHALL BE ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLEDS WITH BAR SPACERS IN ACCORDANCE WITH THE ABOVE REQUIREMENTS. PROVIDE CLASS 'B' LAP SPLICE FOR CONTINUOUS BARS, U.O.N. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN UNLESS OTHERWISE SPECIFIED. HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS U.O.N. USE 1" COVER OVER REINFORCING EXCEPT AS FOLLOWS:

Table with 3 columns: BOTTOM, TOP, SIDES. Values: 3", 2", 3", 1", 1", 1".

- 5. USE PLAIN COLD-DRAWN ELECTRICALLY-WELDED STEEL WIRE FABRIC CONFORMING TO ASTM A 185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICES SHALL BE MEASURED BETWEEN THE OUTERMOST CROSS WIRES OF EACH FABRIC SHEET AND SHALL BE NOT LESS THAN TWICE THE SPACING OF THE CROSS WIRES PLUS 2".
6. SLEEVE ALL PIPES THROUGH SLABS INDIVIDUALLY, UNLESS APPROVED BY THE ENGINEER. WHERE PIPES OR DUCTS PENETRATE THE SLAB, A MAXIMUM OF TWO SLAB BARS MAY BE CUT PROVIDED THEY ARE #5 BARS OR SMALLER, PROVIDED SPLICED BARS ARE PLACED ALONGSIDE THE OPENING IN EACH DIRECTION WITH A 36 BAR DIAMETER SPLICE AT THE END OF EACH CUT BAR. SPLICE BARS SHALL HAVE THE EQUIVALENT CROSS-SECTIONAL AREA AS THE CUT BARS. FOR OPENINGS LARGER THAN 6" NOT SHOWN ON THE STRUCTURAL DRAWINGS SUBMIT SHOP DRAWINGS SHOWING SIZE AND LOCATION FOR THE ENGINEER'S REVIEW. PROVIDE (1) # 5x6'-0" EACH WAY DIAGONALLY AT CORNERS OF ALL OPENINGS LARGER THAN 12", UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL:

- 1. ALL STEEL WORK (INCLUDING FABRICATION AND ERECTION) SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION 9TH EDITION AND PROJECT SPECIFICATIONS. USE THE FOLLOWING:
A. ROLLED SHAPES, PLATES, AND BARS: ASTM A992 50 KSL
B. COLD-FORMED TUBING: ASTM A500, GRADE B.
C. HOT-FORMED STEEL TUBING: ASTM A501
D. STEEL PIPE: ASTM A3, TYPE E OR S, GRADE B.
2. USE STRUCTURAL STEEL THAT IS FULLY WELDABLE WITHIN GRADES AND FROM ANY GRADE TO ANY OTHER GRADE. WELD ALL SHOP CONNECTIONS, U.O.N.
3. ALL SHOP AND FIELD WELDING SHALL CONFORM TO THE STRUCTURAL WELDING CODE AWS D1.1, LATEST EDITION, PUBLISHED BY THE AMERICAN WELDING SOCIETY (AWS). USE ELECTRODES CONFORMING TO AWS D1.1, E70 SERIES, U.O.N. SHOW ALL SHOP WELDS ON THE FABRICATION DRAWINGS AND ALL FIELD WELDS ON THE ERECTION DRAWINGS. ALL SHOP AND FIELD WELDERS, WELDING OPERATORS, AND TACKERS SHALL BE CERTIFIED ACCORDING TO A.W.S. PROCEDURES FOR THE WELDING PROCESS AND WELDING POSITION USED. SUBMIT CERTIFICATES TO ARCHITECT/ENGINEER FOR THEIR RECORDS BEFORE BEGINNING WELDING.
4. ALL JOINT WELDING PROCEDURES TO BE USED SHALL BE PREPARED BY THE FABRICATOR OR CONTRACTOR AS WRITTEN PROCEDURE SPECIFICATIONS AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR THEIR RECORD. ALL JOINT WELDING PROCEDURES SHALL BE QUALIFIED PRIOR TO USE ACCORDING TO AWS PROCEDURES. SUBMIT PROPERLY DOCUMENTED EVIDENCE OF QUALIFICATION TESTS TO ARCHITECT/ENGINEER FOR THEIR RECORDS.
5. A325 BOLTS SHALL BE USED UNLESS NOTED OTHERWISE:
A. A325 BOLTS SHALL CONFORM TO ASTM A 325 TYPE 1, HIGH STRENGTH BOLTS FOR STRUCTURAL STEEL JOINTS. DO NOT USE TYPE 2 BOLTS.
B. PROVIDE HARDENED WASHERS CONFORMING TO ASTM F 436. PLACE HARDENED WASHERS UNDER PART BEING TURNED.
C. ALL BOLTS SHALL BE NEW AND DOMESTICALLY MANUFACTURED. DO NOT REUSE BOLTS. BOLTS AND NUTS SHALL BE WAX DIPPED BY THE BOLT SUPPLIER OR LUBRICATED WITH JOHNSON'S STICK WAX 140.
D. BEARING-TYPE BOLTS (A-325N) SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITIONS. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PILES IN A JOINT ARE IN FIRM CONTACT. THIS MAY BE ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH.
E. SLIP CRITICAL TYPE BOLTS (A325SC) SHALL BE TIGHTENED TO THE MINIMUM FASTENER TENSION INDICATED IN SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR ASTM A490 BOLTS, SECTION 5, TABLE 3. TENSION SHALL BE DETERMINED SOLELY BY USE OF DIRECT TENSION INDICATORS, COMPLYING WITH ASTM F959-85, OR "LOHR" TENSION CONTROL BOLTS. HARDENED WASHERS SHALL BE USED FOR FRICTION-TYPE BOLTS. PROVIDE ONLY DOMESTICALLY MANUFACTURED BOLTS AND D.T.I.'S FRICTION-TYPE BOLTED CONNECTIONS MAY BE INSTALLED.
6. CONNECTIONS NOT COMPLETELY DETAILED ON STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY FABRICATOR'S SPECIALTY ENGINEER ACCORDING TO AISC SPECIFICATIONS AND THE SPECIFIED LOADS AND IN COMPLIANCE WITH APPLICABLE PARAGRAPHS OF THIS SECTION. USE A-307 BOLTS FOR ERECTION BOLTS AND F1554-50 FOR ANCHOR BOLTS OR WHEN SPECIFICALLY CALLED FOR ON THE DRAWINGS.
7. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO METAL SURFACES. DO NOT FLAME CUT HOLES OR ENLARGE HOLES BY BURNING.
8. SPLICING OF STRUCTURAL STEEL MEMBERS IN THE FIELD OR IN THE SHOP IS PROHIBITED EXCEPT WHERE INCLUDED ON THE DRAWINGS.
9. ALL EXTERIOR STRUCTURAL PLATES ANGLES AND EXTERIOR BEAMS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A-153.
10. REFER TO ARCHITECTURAL PLANS FOR FIREPROOFING OF STRUCTURAL STEEL MEMBERS. REFER TO SPECIFICATIONS FOR PAINTING OF EXPOSED STRUCTURAL STEEL.
11. SUBMIT STRUCTURAL STEEL SHOP DRAWINGS TO ARCHITECT/ENGINEER FOR REVIEW BEFORE FABRICATION OF STRUCTURAL STEEL.
12. PROVIDE TEMPORARY BRACING AS NECESSARY TO INSURE A STABLE STRUCTURE DURING CONSTRUCTION.
13. DO NOT REPRODUCE THE STRUCTURAL DRAWINGS FOR USE AS SHOP DRAWINGS.
14. NO CUTTING OF SECTIONS, FLANGES, WEBS, OR ANGLES SHALL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.

Wind Loads - Open Buildings: 0.25 ≤ h/L ≤ 1.0

Type of roof = Monoslope free Roofs
Wind Flow = Clear
G = 0.85
Roof Angle = 1.2 deg

Main Wind Force Resisting System

Kz = Kh (case 2) = 1.01
Base pressure (qh) = 45.5 psf

Table for Roof pressures - Wind Normal to Ridge. Columns: Wind Flow, Load Case, Cn, p. Values for Clear Wind Flow cases A and B.

NOTE: 1) Cnw and Cnl denote combined pressures from top and bottom roof surfaces.
2) Cnw is pressure on windward half of roof. Cnl is pressure on leeward half of roof.
3) Positive pressures act toward the roof. Negative pressures act away from the roof.

Roof pressures - Wind Parallel to Ridge, γ = 90 deg

Table for Roof pressures - Wind Parallel to Ridge. Columns: Wind Flow, Load Case, Horizontal Distance from Windward Edge, Cn, p. Values for Clear Wind Flow cases A and B.

Fascia Panels -Horizontal pressures

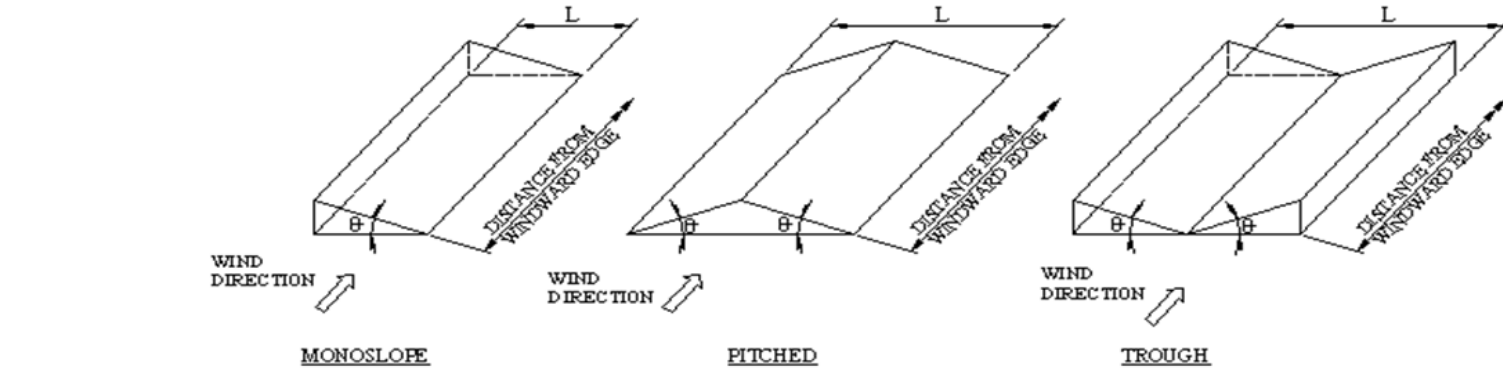
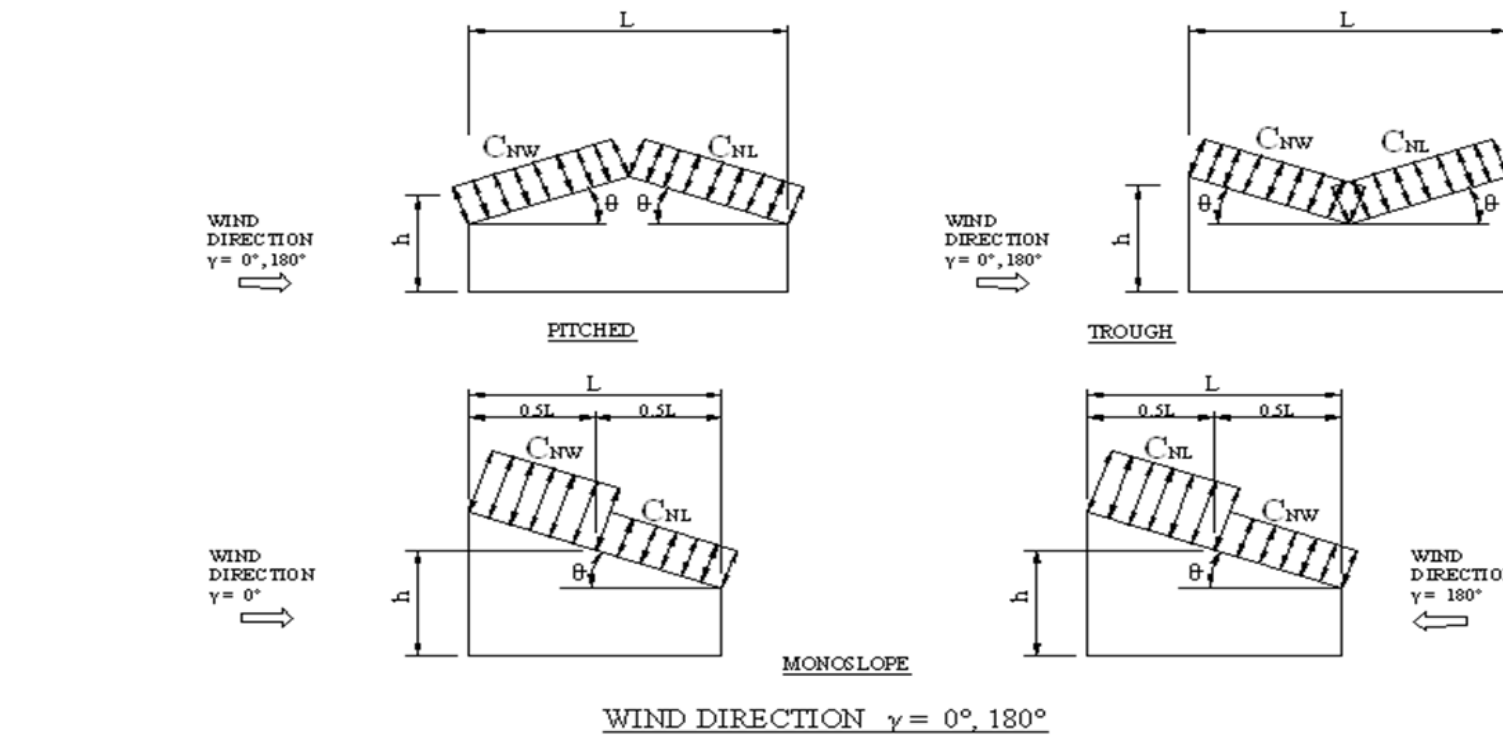
qp = 45.5 psf
Windward fascia: 68.3 psf (GCpn = +1.5)
Leeward fascia: -45.5 psf (GCpn = -1.0)

Components & Cladding - roof pressures

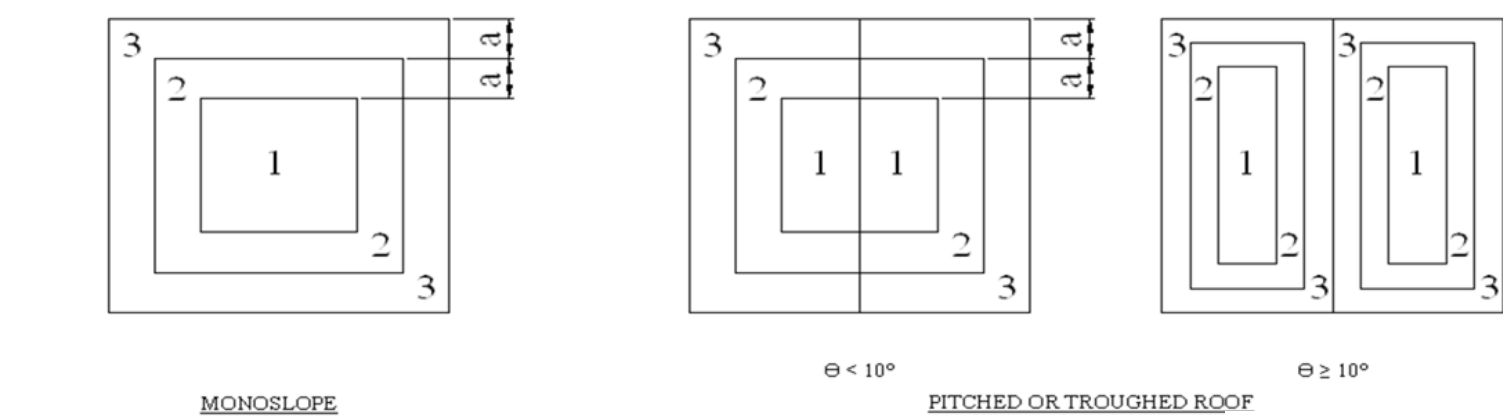
Kz = Kh (case 1) = 1.01
Base pressure (qh) = 45.5 psf
G = 0.85
a = 5.0 ft
a² = 25.0 sf
4a² = 100.0 sf

Table for Components & Cladding - roof pressures. Columns: Effective Wind Area, zone 3 positive/negative, zone 2 positive/negative, zone 1 positive/negative. Values for Cn and Wind pressure.

Location of Wind Pressure Zones



MAIN WIND FORCE RESISTING SYSTEM



COMPONENTS AND CLADDING

Ultimate Wind Pressures

NOTE: The code requires the MWFRS be designed for a minimum pressure of 16 psf.



129 HIGHPOINT DRIVE, GULF BREEZE, FL 32561 800-733-8295

AVCON, ENGINEERS & PLANNERS

320 BAYSHORE DRIVE, SUITE A NICEVILLE, FL 32578-2425 OFFICE: (850) 678-0050 FAX: (850) 678-0040 CORPORATE CERTIFICATE OF AUTHORIZATION NUMBER: 0057 www.avconinc.com

TRANSFORMING TODAY'S IDEAS INTO TOMORROW'S REALITY

JDF ARCHITECTURE, LLC 201 HOLLYWOOD BLVD, NE FT WALTON BEACH, FLORIDA 32548 (850) 496-2166

FL CA # 29804

Table with columns: NO., DATE, REVISION, BY

RELEASE FOR BID

FORT WALTON BEACH LANDING IMPROVEMENTS PREPARED FOR CITY OF FORT WALTON BEACH

DESIGNED BY: T.D.N.
DRAWN BY: C.A.P.
CHECKED BY: V.C.L.
APPROVED BY: V.C.L.
PROJECT NO: 2015.238.03
DATE: 04/08/2022

SHEET NUMBER S.0

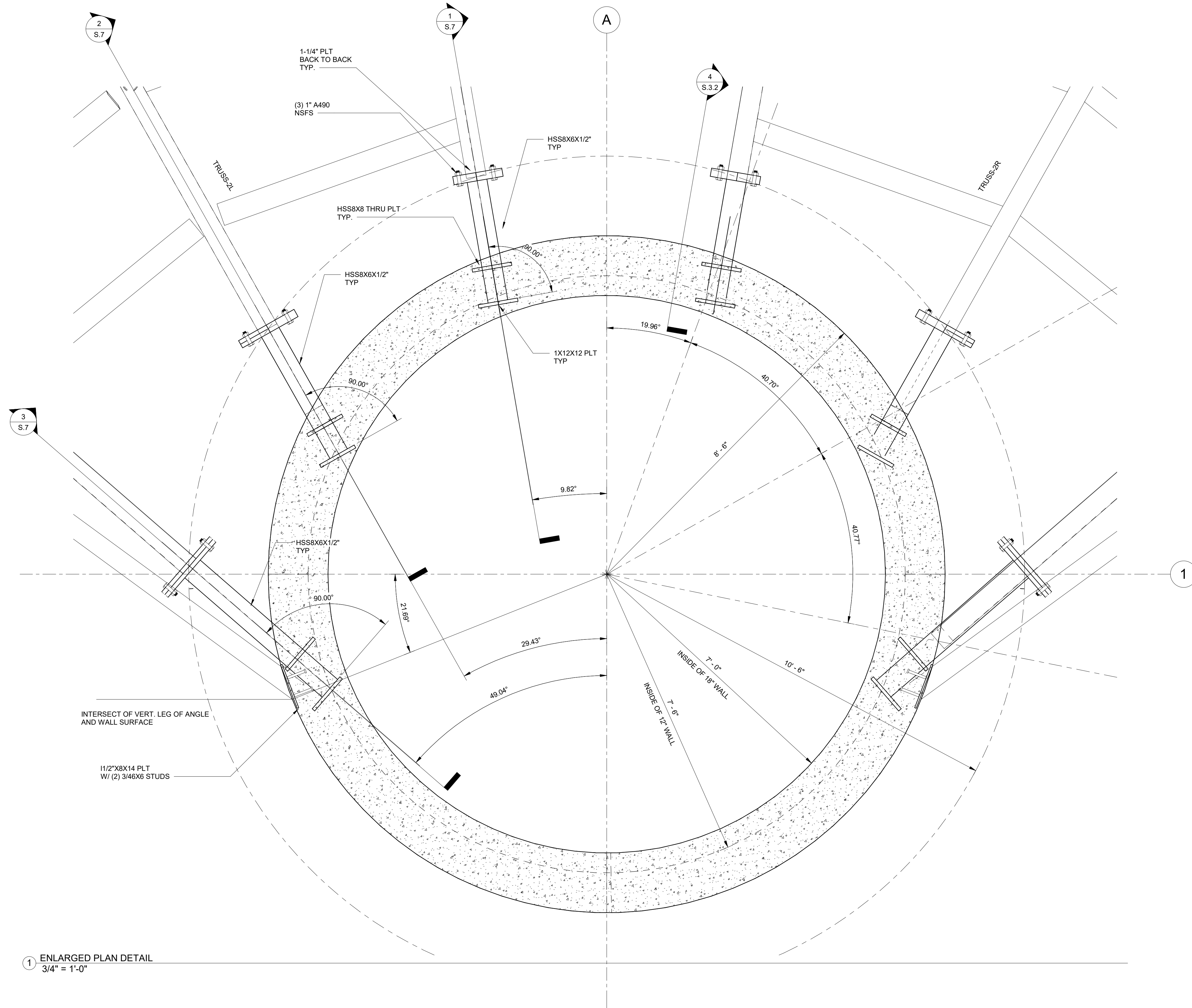








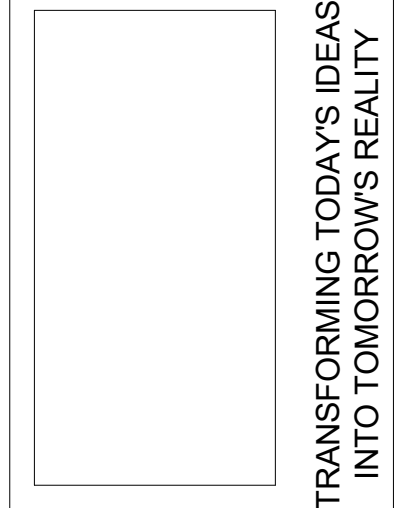
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Lenovo



1 ENLARGED PLAN DETAIL  
3/4" = 1'-0"

This item has been electronically signed and sealed by Roger A. Craft using digital signature and data. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

**AVCON,**  
**ENGINEERS & PLANNERS**  
320 BAYSHORE DRIVE, SUITE A  
NICEVILLE, FL 32576-2425  
OFFICE: (850) 678-0050  
FAX: (850) 678-0040  
CORPORATE CERTIFICATE OF AUTHORIZATION NUMBER: 5057  
WWW.AVCONINC.COM



JDF ARCHITECTURE, LLC  
201 HOLLYWOOD BLVD, NE  
FT WALTON BEACH, FLORIDA 32548  
(850) 496-2166

NO.	DATE	REVISION	BY

**FORT WALTON BEACH LANDING IMPROVEMENTS**  
PREPARED FOR  
**CITY OF FORT WALTON BEACH**

DESIGNED BY: T.D.N.  
DRAWN BY: C.A.P.  
CHECKED BY: V.C.L.  
APPROVED BY: V.C.L.  
PROJECT NO: 2015.238.03  
DATE: 12/28/2023

**SHEET NUMBER**  
S.3.3





NO.	DATE	REVISION	BY

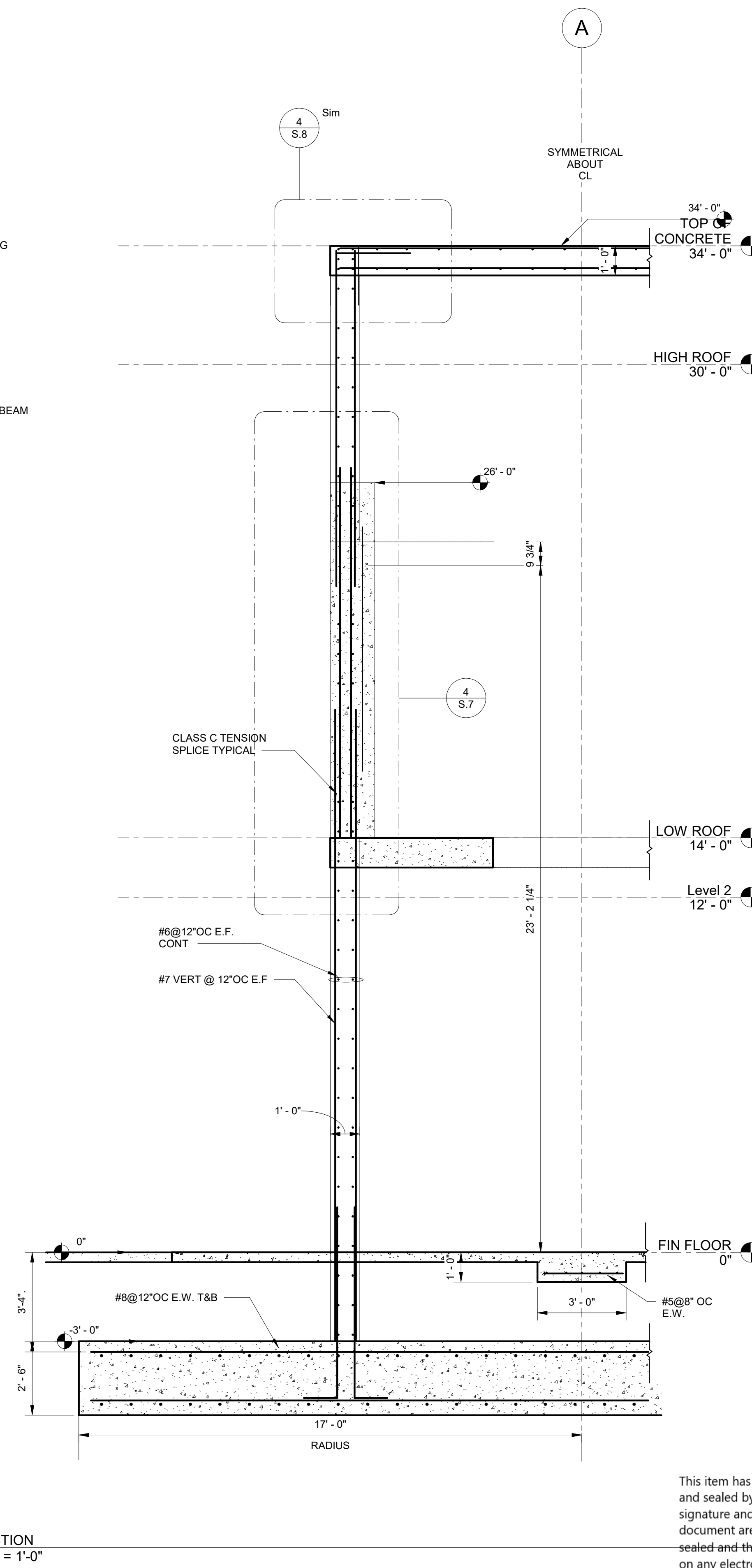
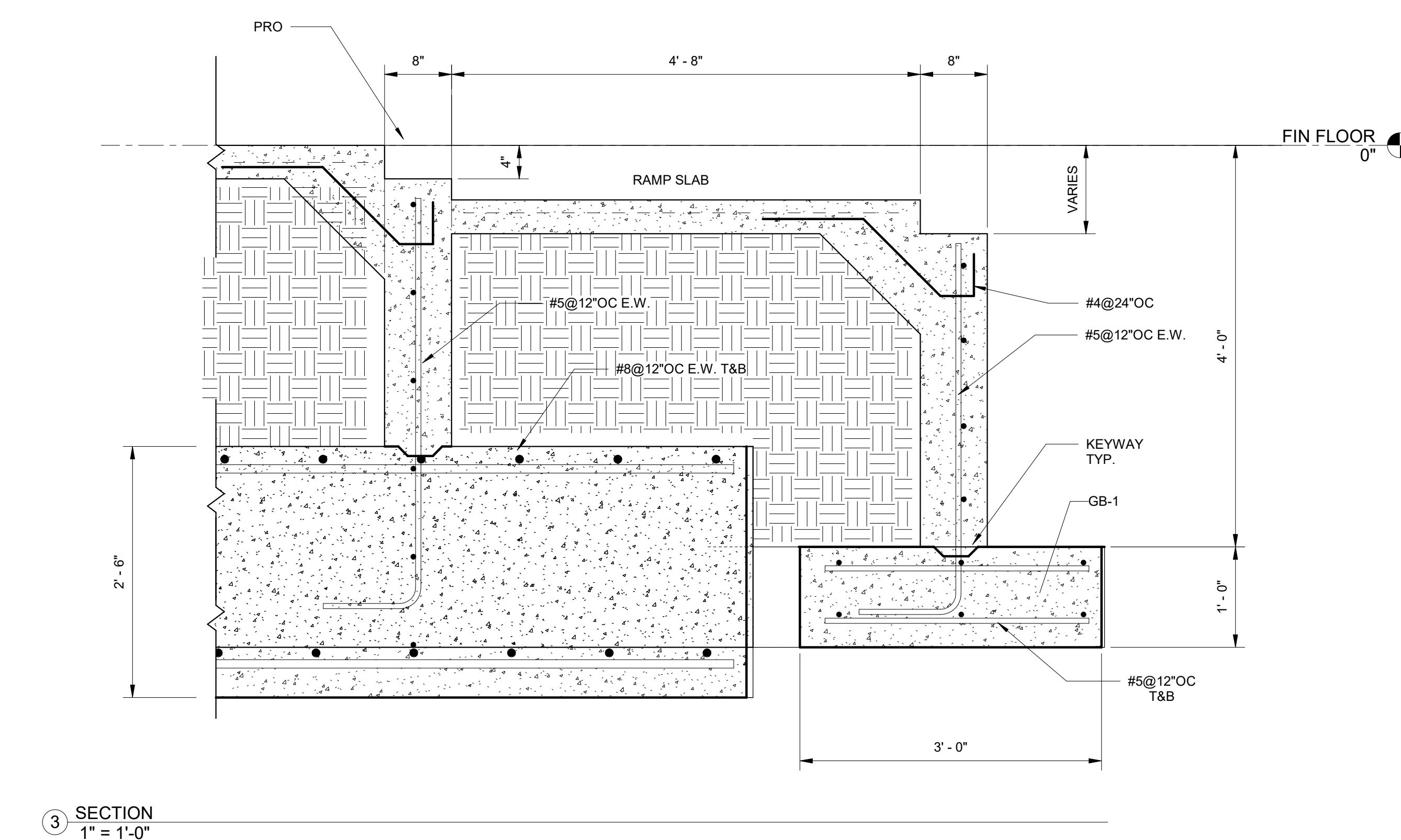
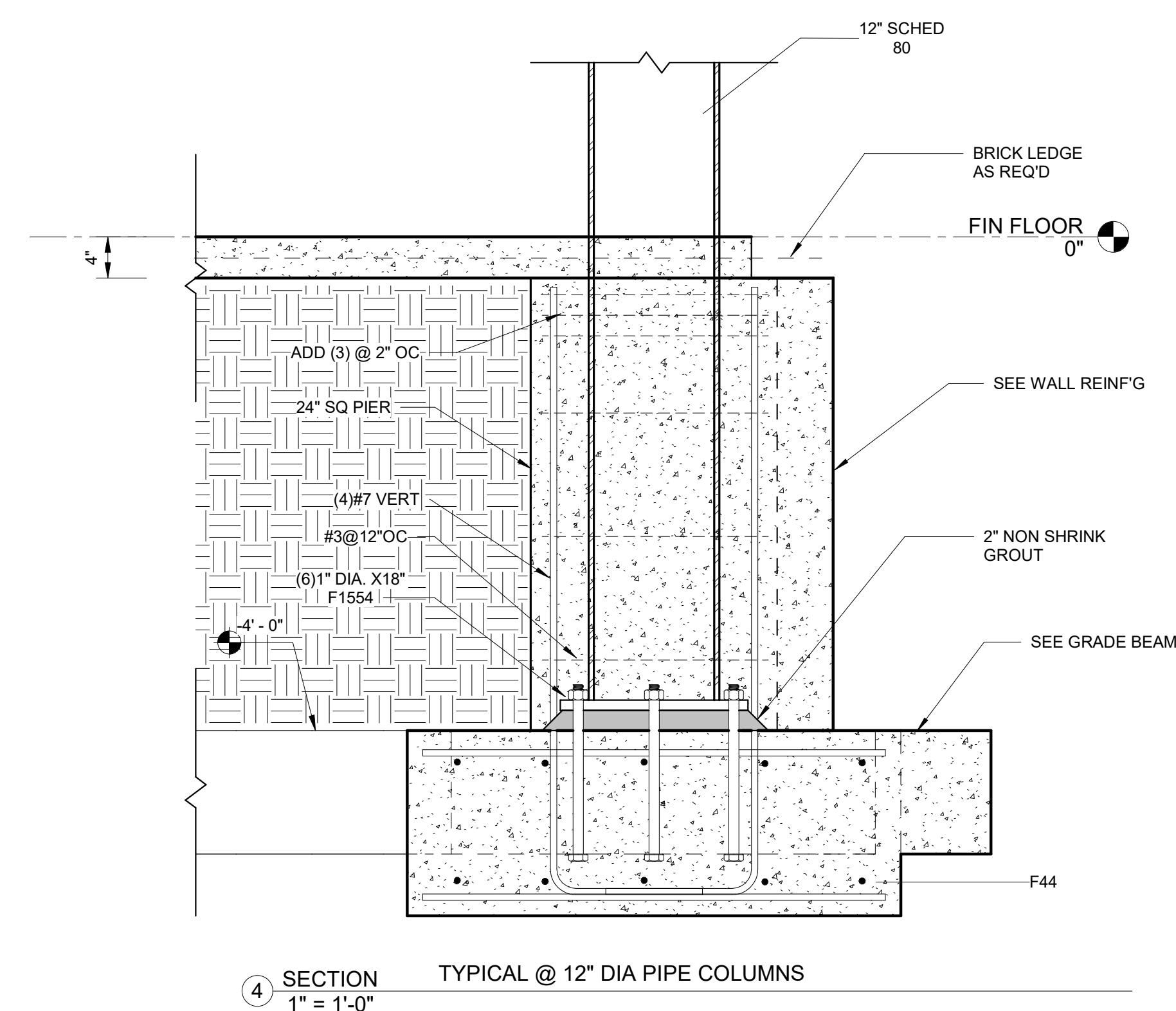
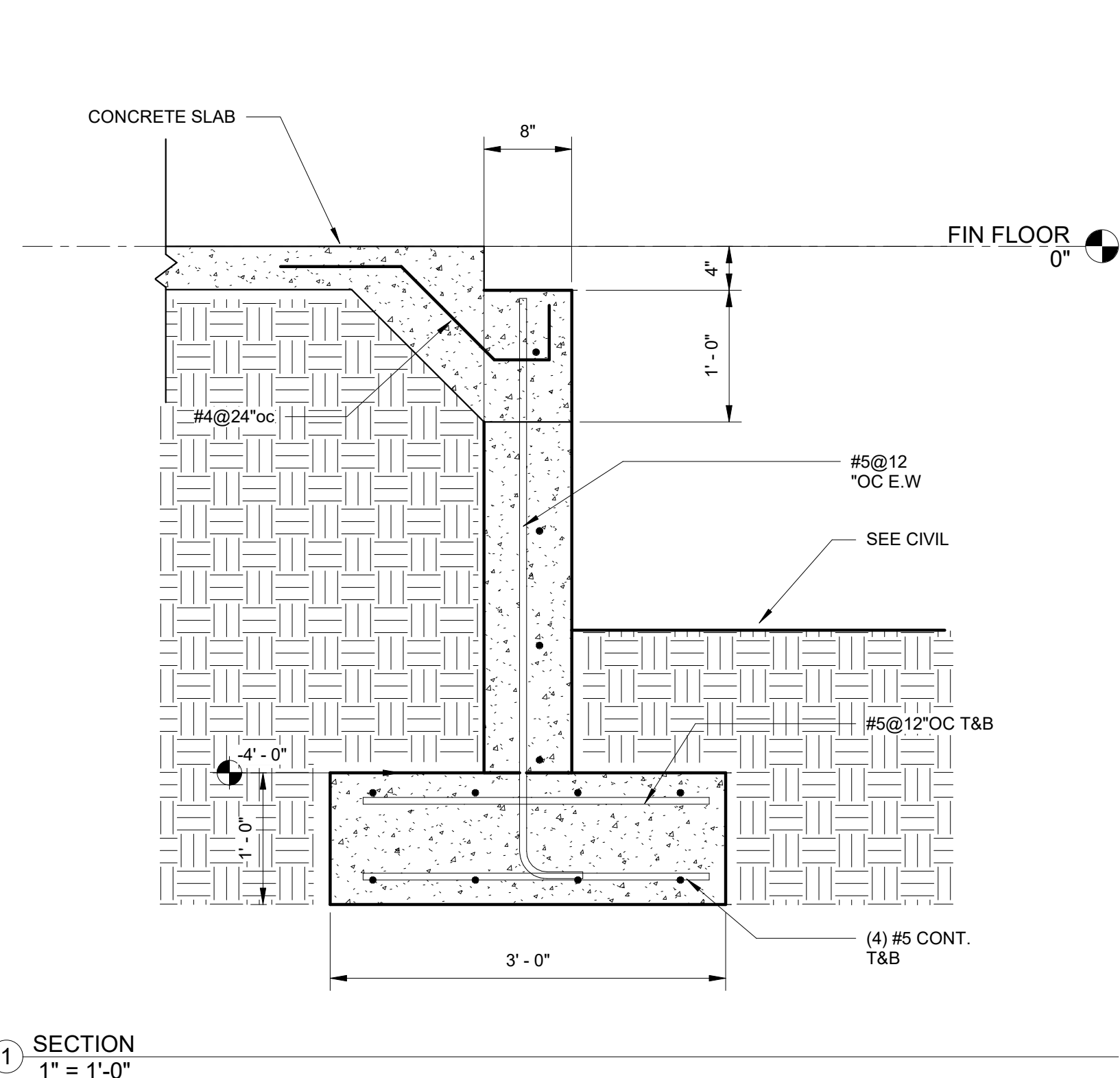
RELEASE FOR BID

**FORT WALTON BEACH  
LANDING  
IMPROVEMENTS**  
PREPARED FOR  
**CITY OF FORT WALTON  
BEACH**

DESIGNED BY: T.D.N.  
DRAWN BY: C.A.P.  
CHECKED BY: V.C.L.  
APPROVED BY: V.C.L.  
PROJECT NO: 2015.238.03  
DATE: 04/08/2022

**SHEET NUMBER**

**S.6**



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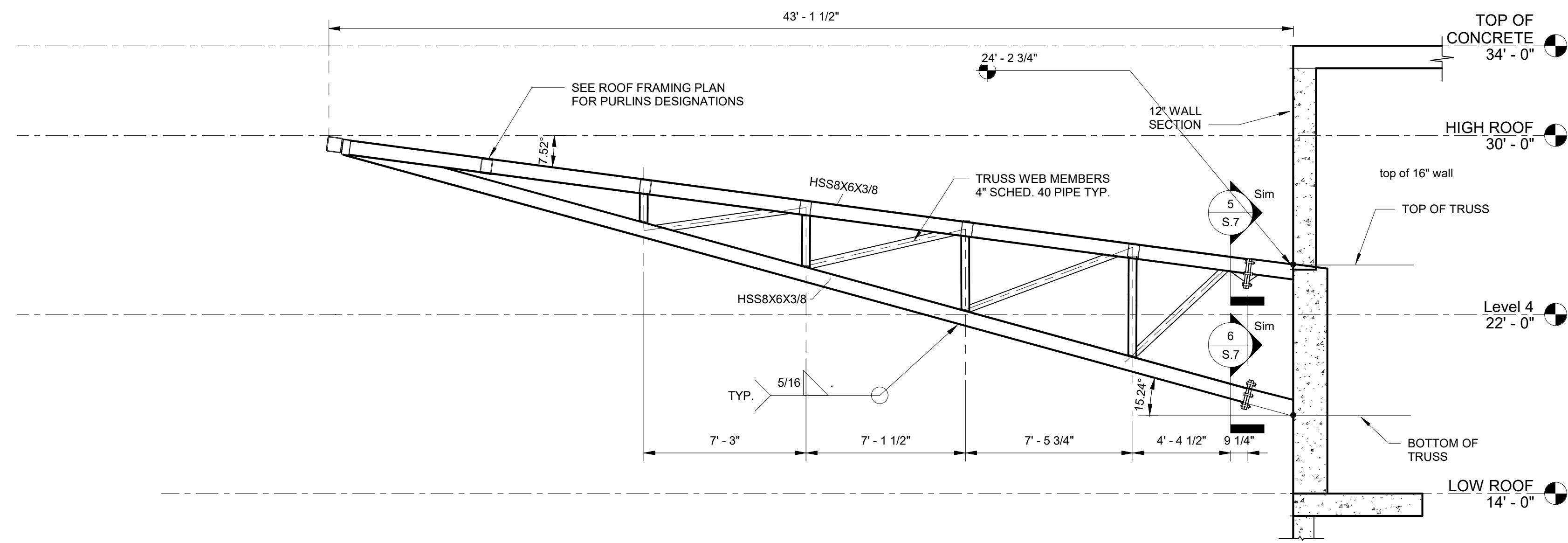
NO.	DATE	REVISION	BY

**RELEASE FOR BID**

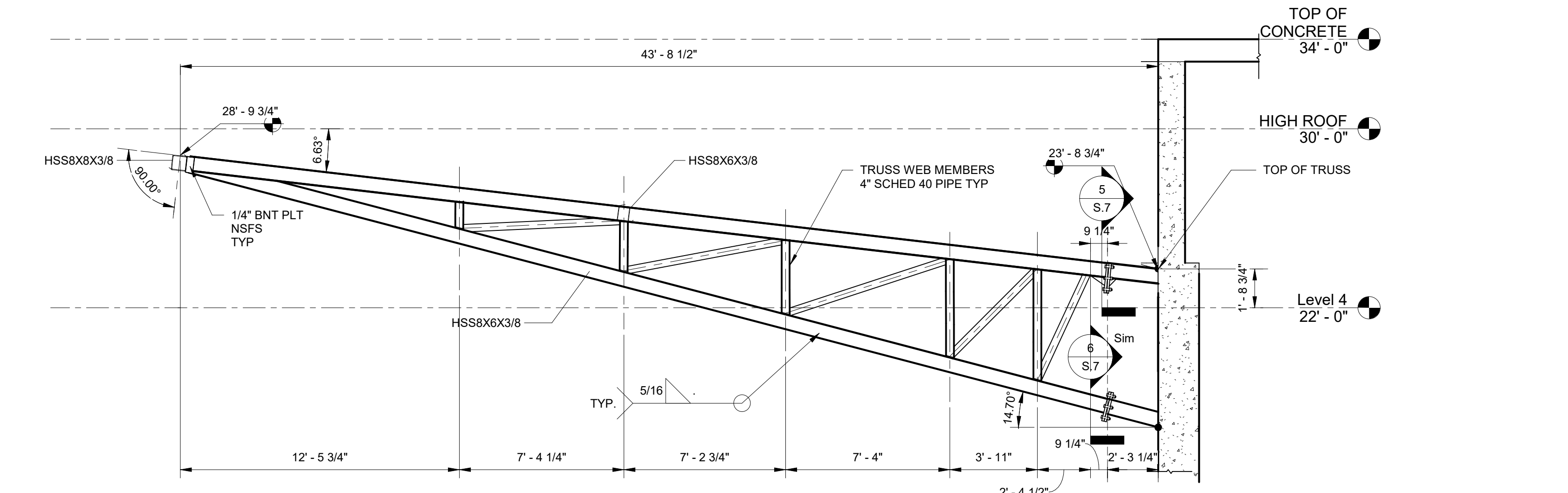
**FORT WALTON BEACH  
LANDING  
IMPROVEMENTS**  
PREPARED FOR  
**CITY OF FORT WALTON  
BEACH**

DESIGNED BY: T.D.N.  
DRAWN BY: C.A.P.  
CHECKED BY: V.C.L.  
APPROVED BY: V.C.L.  
PROJECT NO: 2015.238.03  
DATE: 04/08/2022

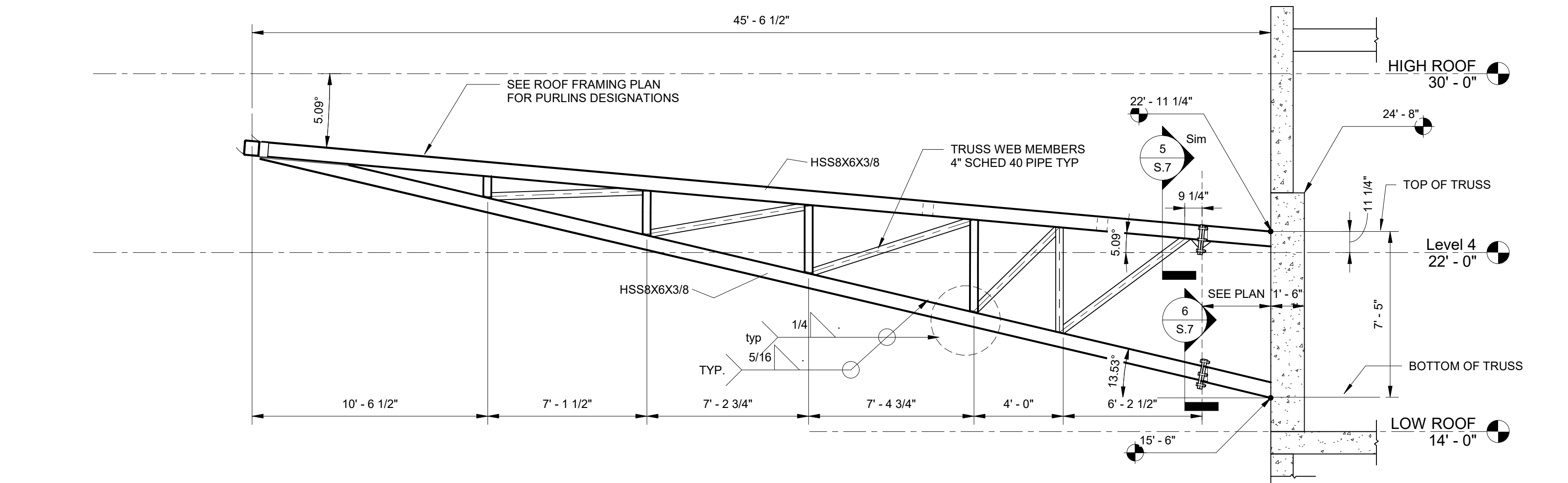
**SHEET NUMBER**  
**S.7**



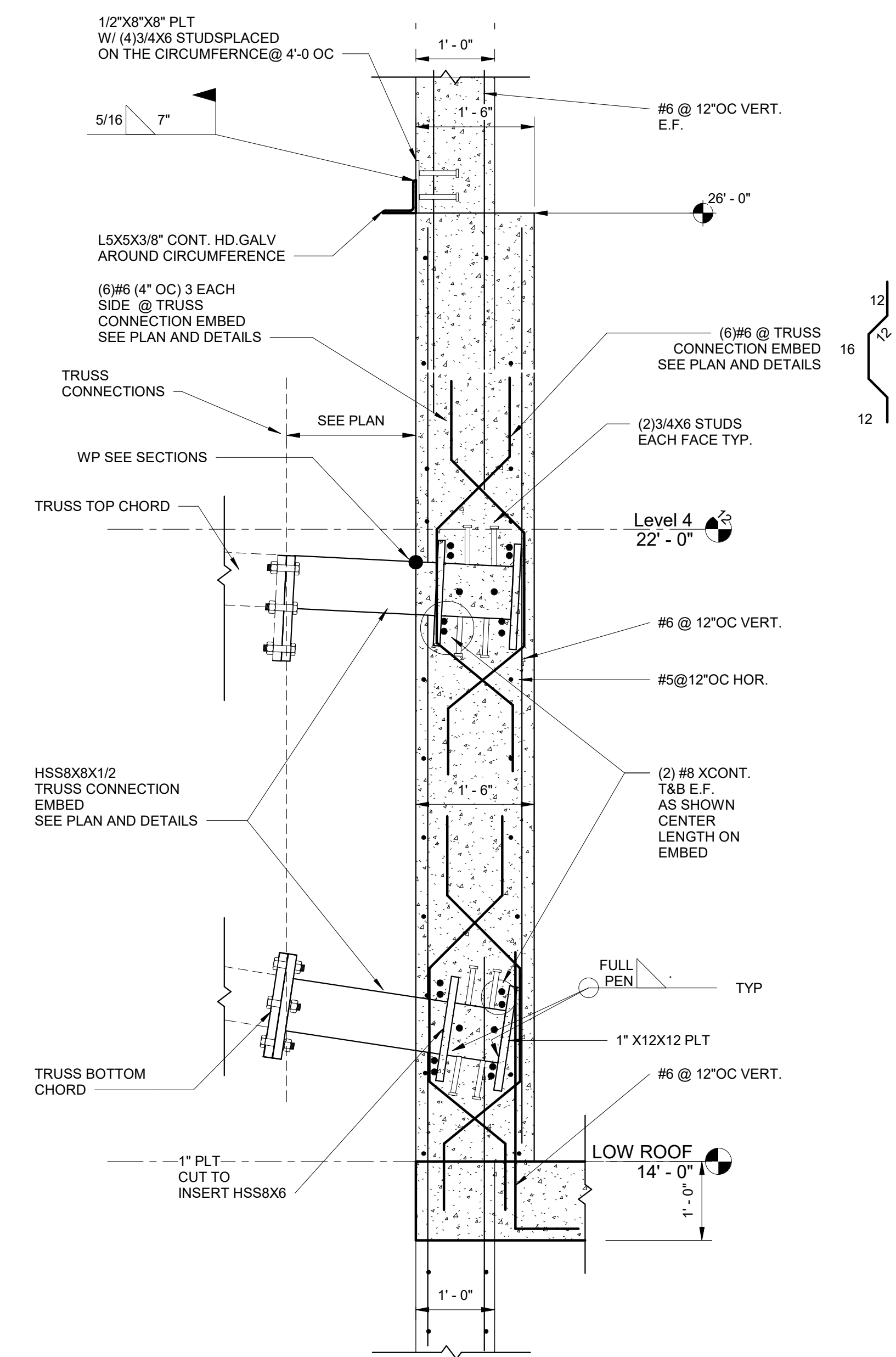
1 SECTION (TRUSS-3L)  
1/4" = 1'-0"



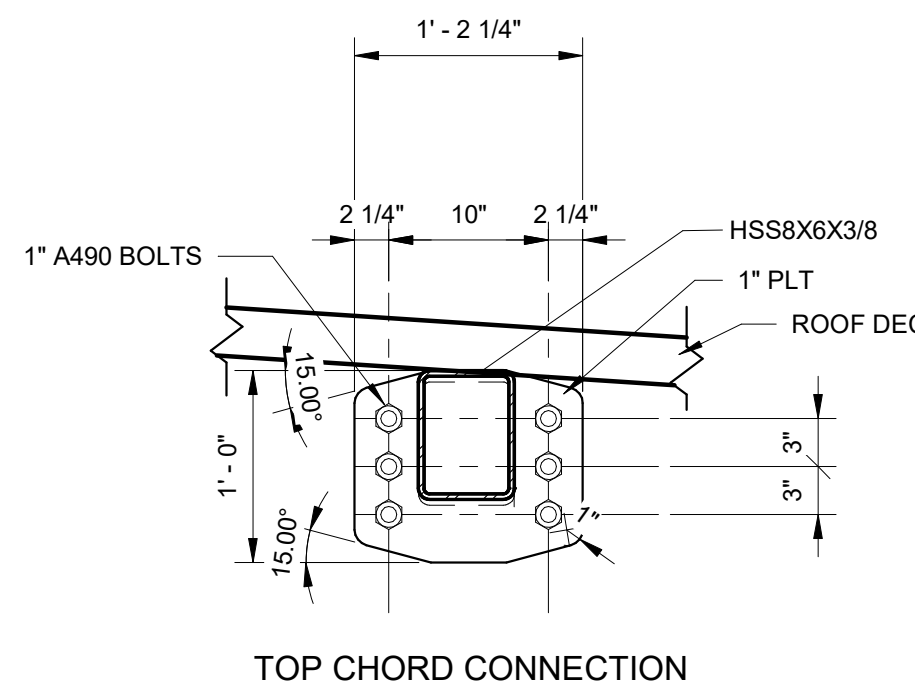
2 SECTION (TRUSS-2L)  
1/4" = 1'-0"



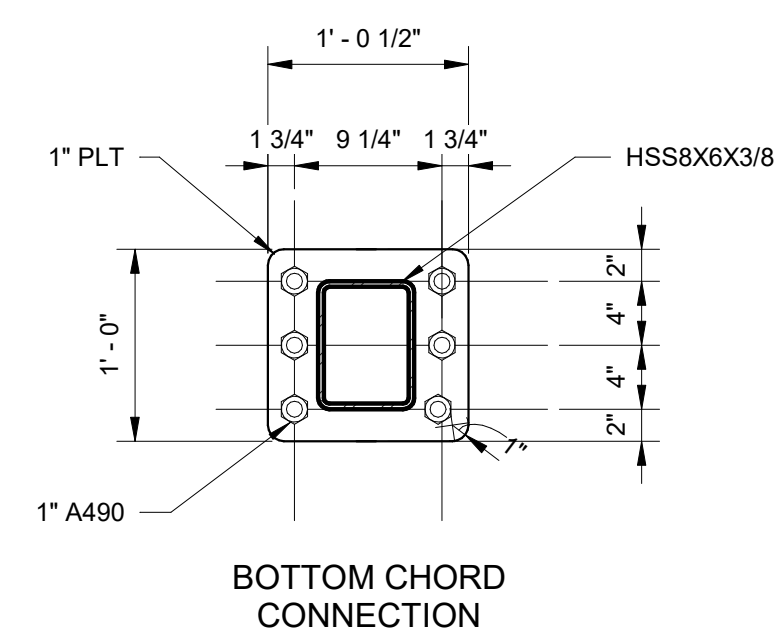
3 SECTION (TRUSS-1L)  
1/4" = 1'-0"



4 DETAIL  
3/4" = 1'-0"



5 DETAIL  
1" = 1'-0"



6 DETAIL  
1" = 1'-0"

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