

GENERAL NOTES & SPECIFICATIONS

1. DESIGN CRITERIA

- A. CODES
 FLORIDA BUILDING CODE, 2020
 AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (TIMBER CONSTRUCTION MANUAL, LATEST EDITION)
 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (A.C.I. 318).
 BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (A.C.I.531).
 AMERICAN INSTITUTE OF STEEL CONSTRUCTION, LATEST EDITION.

- B. DESIGN LIVE LOADS:
 ROOF 20 PSF
 FLOORS 40 PSF
 LATERAL WIND FORCES 150 MPH ASCE 7-16 EXPOSURE "B"
 1st STORY DESIGN WIND PRESSURE: 35 PSF, -35 PSF

DESIGNED FOR: CLOSED BUILDING
 STRUCTURAL CATEGORY: II
 IMPORTANCE FACTOR: 1.0
 INTERNAL PRESSURE COEFFICIENT +18 to -18
 COMPONENTS & CLADDING SHALL BE DESIGNED AND INSTALLED (BY OTHERS)
 TO COMPLY WITH THE FLORIDA BLDG CODE, 2020
 SHUTTERS OR IMPACT RESISTANT GLAZING ARE REQUIRED.
 SHUTTERS OR IMPACT RESISTANT GLAZING MUST HAVE FLORIDA PRODUCT APPROVAL NUMBERS.

Description	Wind Pressure on Components and Cladding (Ch 30 Part 2)			All pressures shown are based upon STRENGTH Design, with a Load Factor of 1		
	Width Ft	Span Ft	Area Sq Ft	Zone	Max P psf	Min P psf
Roof Infill	42.00	13.68	100.0	1	17.64	-35.93
Roof Edges	42.00	35.00	100.0	2	17.64	-50.58
Roof Corners	3.50	12.3	3	24.43	-100.53	
Wall Infill	40.00	32.67	500.0	4	32.29	-35.93
Wall Corners	3.50	32.67	355.8	5	33.91	-39.20

2. CONCRETE SPECIFICATIONS

- A. ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS.
 B. CONCRETE OPERATIONS SHALL COMPLY WITH A.C.I. STANDARDS.
 C. CONCRETE COMPRESSIVE STRENGTH: 2500 PSI MINIMUM AT 28 DAYS.
 D. REINFORCING BARS: ASTM A615 GRADE 60
 E. WELDED WIRE FABRIC (W.W.F.): ASTM A185.
 F. REINFORCING BARS PLACING ACCESSORIES: IN ACCORDANCE WITH CRSI SPECIFICATIONS.
 G. MINIMUM CONCRETE COVERAGE OF REINFORCEMENT: FOOTINGS: 3" BOTTOM AND 2" SIDES.
 H. EARTH SUPPORTED SLAB: 4 INCHES THICK REINFORCED WITH 6 x 6 x W1.4 x W1.4 W.W.F. THE SLAB SHALL BE PLACED OVER POLYETHYLENE VAPOR BARRIER OF NOT LESS THAN ONE INCH NOMINAL THICKNESS. IN LIEU OF WELDED WIRE FABRIC, CONCRETE SLAB CAN BE TREATED WITH SYNTHETIC REINFORCING FIBERS AS MANUFACTURED BY FIBERMESH COMPANY AND IN ACCORDANCE WITH ASTM STANDARD SPECIFICATION FOR FIBER REINFORCED CONCRETE AND SHOTCRETE C1116. THE DOSAGE SHALL BE ONE AND ONE HALF (1 1/2) POUNDS FIBERS PER CUBIC YARD OF CONCRETE.
 I. ANCHOR BOLTS IN CMU BLOCKS SHALL CONFORM TO ASTM A36 AND SHALL BE 1/2" DIAMETER WITH 7" MIN. DEPTH IN CONCRETE.
 J. DETAIL REINFORCING IN ACCORDANCE WITH A.C.I. 315. REINFORCING SHALL NOT BE WELDED, EXCEPT AS SHOWN WHERE ASTM A708 BARS ARE USED.

3. MASONRY SPECIFICATIONS

- A. HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL CONFORM TO ASTM C90, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI ON THE NET AREA AND 1,000 PSI ON THE GROSS AREA ($f_m = 1,500$ PSI)
 B. ALL MORTAR FOR MASONRY SHALL CONFORM TO ASTM C270, TYPE "M" OR "S". ALL GROUT FOR USE IN MASONRY SHALL CONFORM TO ASTM C478, MINIMUM 2,500 PSI AT 28 DAYS.
 C. CONTINUOUS BARS SHALL HAVE BASIC CLASS "C" TENSION LAPS WITH CORNER BARS AT ALL CORNERS AND END WALL INTERSECTIONS.
 D. ALL VERTICAL REINFORCEMENT IN MASONRY SHALL HAVE CLASS "C" TENSION LAPS.
 E. REINFORCING IN MASONRY WALL FOOTINGS SHALL BE CONTINUOUS.

4. TIMBER SPECIFICATIONS

- A. STRUCTURAL TIMBER SHALL BE #2 SOUTHERN YELLOW PINE (M.C.-19%) OR LODGE POLE OR EQUAL UNLESS OTHERWISE NOTED ON DRAWINGS, WITH ALLOWABLE STRESSES AS FOLLOWS:

BENDING STRESS	1,200 PSI	1,050 PSI
COMPRESSION STRESS PARALLEL TO GRAIN	1,000 PSI	700 PSI
MODULUS OF ELASTICITY	1,600,000 PSI	1,200,000 PSI

 LODGE POLE
 B. STRUCTURAL GLUE LAMINATED TIMBER SHALL BE VISUALLY GRADED SOUTHERN PINE WITH THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

BENDING STRESS	2,400 PSI
SHEAR STRESS	200 PSI
MODULUS OF ELASTICITY	1,800,000 PSI

 C. STRUCTURAL PARALLAM BEAMS ALL SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

BENDING STRESS	2,900 PSI
SHEAR STRESS	200 PSI
MODULUS OF ELASTICITY	2,000,000 PSI

 D. PLYWOOD SHEATHING:
 1. EACH CONSTRUCTION AND INDUSTRIAL PANEL SHALL BE IDENTIFIED WITH THE APPROPRIATE TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION AND SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF U.S. PRODUCT STANDARDS PS 1 OR PRP-180 PERFORMANCE STANDARDS. ALL PANELS WHICH HAVE ANY EDGE OR SURFACE PERMANENTLY EXPOSED TO WEATHER SHALL BE CLASSIFIED EXTERIOR.
 11. PANEL ROOF WALL AND FLOOR SHEATHING SHALL BE 1/2" THICK APA STRUCTURAL 1 RATED SHEATHING EXP 2 (UNLESS OTHERWISE NOTED ON PLANS). SHEATHING PERMANENTLY EXPOSED TO WEATHER SHALL BE CLASSIFIED EXTERIOR.
 111. NAIL PANELS WITH 80 COMMON NAILS AT 3" O.C. ALONG SUPPORTED PANEL EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS, OR AS INDICATED ON PLANS.
 E. ALL BEARING STUD WALLS SHALL HAVE SOLID BLOCKING AT MID-HEIGHT OR AS OTHERWISE NOTED ON BUILDING SECTIONS.
 F. PREFABRICATED WOOD STRUCTURAL MEMBERS, INCLUDING TRUSSES SHALL BE DESIGNED SPECIFICALLY FOR THIS PROJECT FOR A 150 MPH WIND LOAD LAW ASCE 7-16 LATERAL LOAD, AND SEALED BY A FLORIDA REGISTERED ENGINEER.
 G. REVIEW ALL DRAWINGS INCLUDING MECHANICAL, ELECTRICAL, PLUMBING ETC. TO ASCERTAIN LOADS FROM EQUIPMENT, OPENINGS FOR DUCTS ETC. AND PROVIDE MODIFICATION TO TRUSSES IF REQUIRED TO SUPPORT SAME.
 H. TRUSS LAYOUT AS SHOWN ON PLANS IS SCHEMATIC AND MAY BE MODIFIED WITH APPROVAL OF THE ENGINEER
 I. ALTHOUGH WEB LAYOUT MAY BE SHOWN ON PLANS, IT IS THE RESPONSIBILITY OF THE TRUSS DESIGNER TO ACCEPT, APPROVE, OR MODIFY, AS REQUIRED FOR THE DESIGN PURPOSE.
 J. WOOD-TO-WOOD FRAMED CONNECTIONS ARE TO BE MADE WITH BOLTS AND/OR JOIST HANGERS AS SHOWN. TOE-NAILING IS NOT PERMITTED.
 K. MAXIMUM SPANS OF DIMENSIONAL LUMBER USED FOR JACK RAFTERS AT HIPPED ROOF SECTIONS SHALL BE IN ACCORDANCE WITH "SPAN TABLES FOR JOISTS AND RAFTERS" AS PUBLISHED BY THE NATIONAL WOOD PRODUCTS ASSOCIATION.
 L. HIP RAFTERS SHALL BE 2 INCHES DEEPER THAN JACK RAFTERS.
 M. ALL TRUSSES AND RAFTERS SHALL BE STRAPPED OR HURRICANE CLIPPED TO SUPPORTING MEMBERS AT ALL BEARING POINTS.

4. TIMBER SPECIFICATIONS (CONTINUED)

- N. SECURE EACH ROOF TRUSS/RAFTER TO TOP PLATE WITH SIMPSON HURRICANE CLIPS (OR EQUAL) AS INDICATED ON PLANS. CONTRACTOR TO SUBMIT SHOP DRAWINGS OF TRUSSES TO ENGINEER TO VERIFY/MODIFY UP-LIFT CONNECTORS.
 O. ALL EXTERIOR WALL FRAMING SHALL BE 2"x4" OR 2"x6" at 16" O.C. UNLESS NOTED OTHERWISE. 7/16" OSB SHEATHING OR 1/2" CDX PLYWOOD PANELS SHOULD EXTEND TO THE TOP PLATE AND BOTTOM OF EXTERIOR GIRDERS OR SILL PLATE. NAIL PLYWOOD AT 4" O.C. AT ALL EDGES and 6" O.C. AT INTERMEDIATE SUPPORTS OR AS INDICATED PER PLAN.
 P. USE SIMPSON ST18 (OR EQUAL) RIDGE/RAFTER CONNECTORS OR SIMPSON RR STRAPS AT ALL RAFTERS/RIDGE BEAMS OR AS INDICATED PER PLAN.
 Q. USE SIMPSON SP1 & SP2 (OR EQUAL) TO SECURE STUDS TO BOTTOM AND TOP PLATES, OR AS INDICATED ON PLAN.
 R. USE TWO (2) SIMPSON LSTA21 (OR EQUAL) TO SECURE EACH BEAM HEADER BEARING END TO EACH SUPPORT, OR AS INDICATED PER PLAN.
 S. USE SIMPSON LSTA21 STRAP TIES (OR EQUAL) OR SIMPSON SP4 (OR EQUAL) AT TOP OF EACH EXTERIOR WINDOW AND DOOR FRAME OPENING, OR AS INDICATED PER PLANS.
 T. CUTTING, NOTCHING BORED HOLES IN STUD WALLS, RAFTERS, ETC., SHALL BE DONE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2020
 U. RIDGE BOARDS WHERE INDICATED ON FRAMING PLANS SHALL NOT BE LESS THAN 1" IN THICKNESS, AND NOT LESS IN DEPTH THAN CUT END RAFTERS. RAFTERS SHALL BE PLACED DIRECTLY OPPOSITE EACH OTHER AND NAILED TO RIDGE BOARD.
 V. ALL WOOD BUILD-UP GIRDERS, BEAMS, STUDS TO SOLE PLATES, ETC. TO BE CONNECTED AS PER FLORIDA BUILDING CODE 2020
 W. AT OPENING IN EXTERIOR WALLS, A WALL STUD SHALL BE AT EACH SIDE OF THE OPENING WITH THE ENDS OF THE HEADER SUPPORTED AS FOLLOWS (UNLESS OTHERWISE NOTED):
 1. FOR OPENINGS LESS THAN 3 FEET IN WIDTH, EACH SIDE OF HEADER SHALL REST ON A SINGLE HEADER STUD OR MAY BE SUPPORTED BY FRAMING ANCHORS ATTACHED TO WALL STUD.
 11. FOR OPENINGS OVER 3 FEET TO LESS THAN 6'-0", EACH END SHALL BEAR ON A SINGLE HEADER STUD.
 111. FOR OPENINGS MORE THAN 6'-0" AND LESS THAN 12" IN WIDTH, EACH END SHALL BEAR ON A DOUBLE HEADER STUD.
 IV. ANY HEADER SUPPORTING CONCENTRATED LOADS FROM BEAMS ABOVE, EACH END SHALL BEAR ON DOUBLE HEADER STUDS.
 X. WHERE WOOD BEAMS BEAR ON STUD WALLS, PROVIDE MINIMUM DOUBLE OR TRIPLE STUDS, DEPENDING ON BEAM WIDTH AND LOADS, UNDER FOUNDATION.
 Y. AT AREAS WHERE TRUSSES REQUIRE HEADERS TO ADJACENT TRUSSES, PROVIDE HEADERS AS DETERMINED BY ACCEPTABLE ENGINEERING DESIGNS.

NOTE:

1. Bottom of footings must be at least 12" below finished grade.

General Notes:

A.B. 3/4" dia, use 20" embedded length

Every bay shall have a sawn, struck control joint, or keyed joint with every bay having an expansion joint.
 Seal all joints with Dow 890 sl.
 Fibermesh reinforcement may be used in lieu of w.w.f. reinforcement.
 See Golf Course Storage drawings for layout of columns and other components.

WHERE THE BUILDING OFFICIAL REQUIRES FINAL CERTIFICATION OF COMPLETION FROM THE ENGINEER, THE FOLLOWING APPLIES:

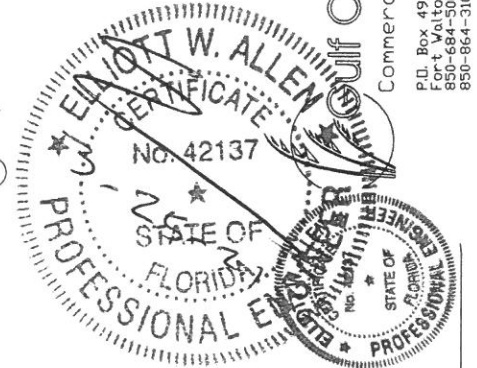
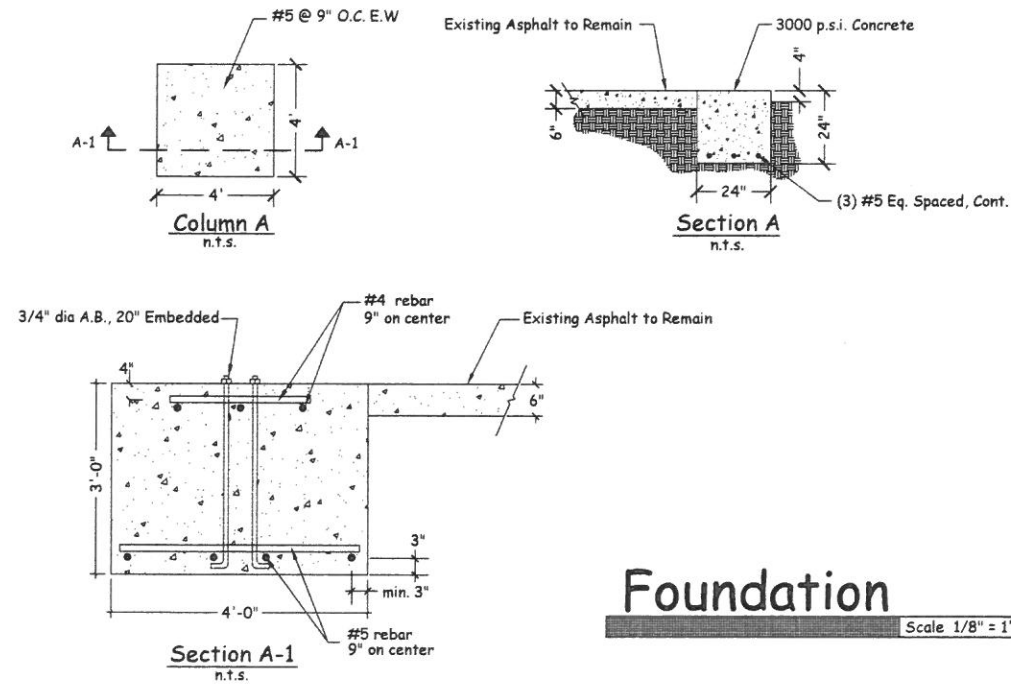
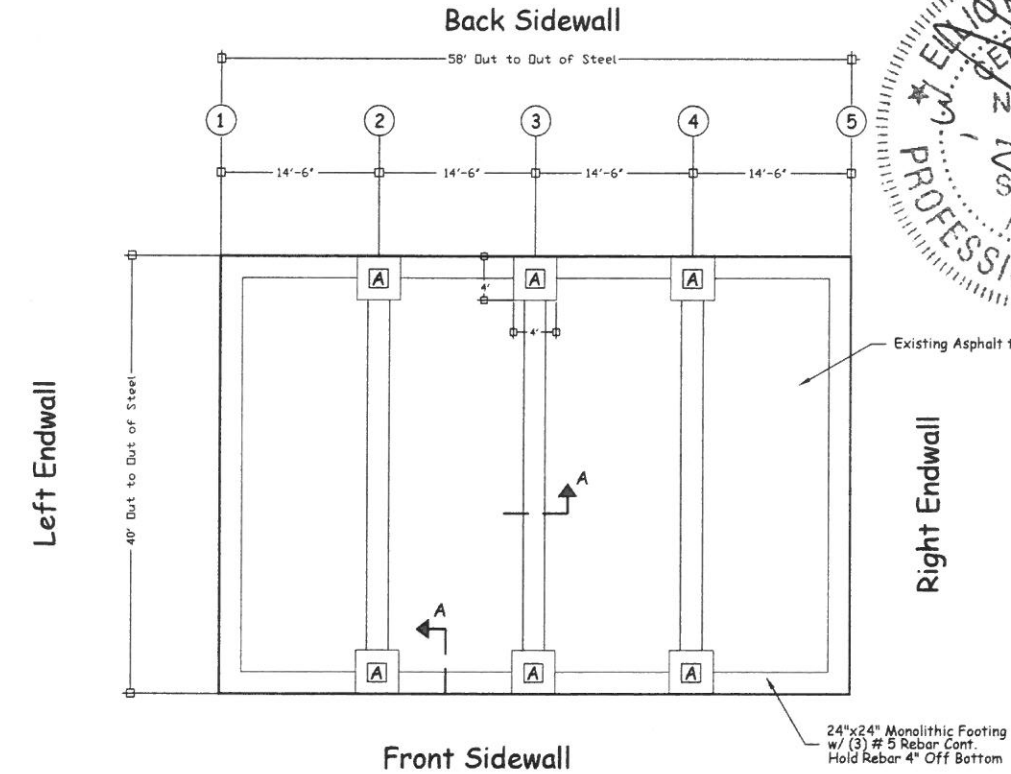
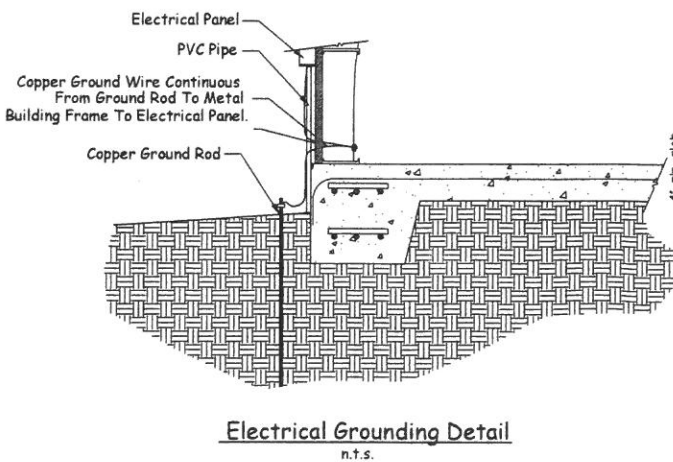
- 3 INSPECTIONS ARE REQUIRED, FOUNDATION/FLOOR FRAMING, NAIL-OFF, AND DRY-IN FRAMING. DRY-IN INSPECTION SHALL OCCUR BEFORE INSULATING & SHEET ROCK INSTALLATION.
- 48 HOURS NOTICE, IN WRITING/VIA FAX OR E-MAIL, SHALL BE GIVEN TO THE ENGINEER.
- THESE INSPECTIONS SHALL BE BILLED AT \$225.00 EACH.

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 Florida CA Number 7066
 mdnewell@wbnet.com

WHILE EVERY ATTEMPT HAS BEEN MADE IN THE PREPARATION OF THIS PLAN TO AVOID MISTAKES, THE AUTHOR CANNOT GUARANTEE AGAINST HUMAN ERROR. THE CONTRACTOR ON THE JOB SITE MUST CHECK ALL DIMENSIONS AND OTHER DETAILS FOR ACCURACY BEFORE AND DURING CONSTRUCTION AND BE RESPONSIBLE FOR SAME.

GULF COAST ENGINEERING
 STRUCTURAL DETAIL SHEETS
 ARE COPYRIGHTED. ANY UNAUTHORIZED
 USE OR REPRODUCTION IS EXPRESSLY
 PROHIBITED, AND SHALL BE AN
 INFRINGEMENT OF THE FEDERAL COPYRIGHT ACT.



Digitally signed by Elliott W. Allen, P.E.
 DN: cn=Elliott W. Allen, P.E., o=Gulf Coast Engineering, email=buckfwb@gmail.com, c=US
 Reason: This item has been electronically sealed by Elliott W. Allen using a digital signature.
 Location: Printed copies are not considered signed and sealed and the signature must be verified on any electronic copies.
 Date: 2021.03.25 06:40:51 -05'00'

NO.	Dwg.	Issue	Date

Prepared For:
Golf Course Storage
 City of Fort Walton Beach

Contractor:
Drawing Description
Foundation
Job # 005728
Drawn By: BCM
Checked By: EWA
Date: 3-20-2021
Scale: 1/8" = 1'-0"
Drawing Number

S-1

Gulf Coast Engineering
 Commercial and Residential Design
 P.O. Box 4915
 Fort Walton Beach, Florida 32549
 850-684-5007
 buckfwb@gmail.com
 mdnewell@cox.net
 850-864-3100
 CA 7066

BUILDING SPECIFICATIONS

The manufacturer is not responsible for the concrete foundation design. The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The manufacturer will assume no responsibility for any loads not indicated.

This manufactured building is designed with the manufacturer's standard design practices which are based on pertinent procedures and recommendations of the following organizations and codes :

- American Institute of Steel Construction "Specification for the design fabrication and erection of structural steel for buildings."
- American Iron and Steel Institute "Specification for the design of cold formed steel structural members" 2007 edition.

-Metal Building Manufacturers Association "Specification for the design fabrication and erection of the structural system" most current edition.

Material properties of steel plate and sheet used in fabrication of primary rigid frames and all primary structural framing members (other than cold-formed sections) conform to ASTM A-529 or A-572 all with a minimum yield point of 55 KSI.

Material properties of cold formed light gage steel members conform to the requirements of ASTM A-653, with a minimum yield point of 55 KSI

High strength bolts and their installation shall conform to ASTM specification A-325 and are designed as bearing type connections with threads included in the shear plane. All high strength bolts are to be installed to the "Snug-Tight" condition as defined by the RCSC Specification for Structural Joints Using A325 or A490 Bolts, Latest Edition, section 8.1, unless noted otherwise. Bolts in standard holes do not require washers per section 6.

Shop and field inspections and associated fees are the responsibility of the contractor, unless stipulated otherwise.

CONTRACTOR RESPONSIBILITIES

The contractor must secure all required approvals and permits from the appropriate agency as required.

Approval of the manufacturer's drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract drawings and specifications. (AISC 303-10 Code of Standard Practice)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Section 3.3 AISC 303-10 Code of Standard Practice)

Design considerations of any materials in the structure which are not furnished by the manufacturer, are the responsibility of the contractor and engineers other than the manufacturer's engineering, unless specifically indicated.

The contractor is responsible for all erection of steel and associated work in compliance with the manufacturer's "For Construction" drawings.

Temporary supports, such as guys, braces, flashwork or other elements required for the erection will be determined and furnished and installed by the erector. (Section 7 AISC 303-10 Code of Standard Practice)

It is the contractors responsibility to apply or observe all pertinent safety rules and regulations, as per OSHA standards as applicable.

The Contractor is responsible for the verification of all shipments received. Any "external" damage or shortages must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim. NO EXCEPTIONS!!!

OLYMPIA STEEL BUILDINGS

DESIGN LOADING

This structure is designed utilizing the loads indicated and applied by the :
FBC 2017 (IBC 15)

It is the contractor's responsibility to confirm that these loads comply with the requirements of the local building department.

Specific loads : (See structural calculations and foundation reactions.)

- 20.00 PSF Live Load
- No LL Reduction Allowed
- 0 PSF Ground Snow Load
- 1.20 Thermal Factor (Ct)
- 1 Snow Exposure Factor (Ce)
- 145 MPH Wind Load Exposure B (If applicable)
- 2.000 PSF Dead Load (Metal Bldg. Weight - Purlins, Panels, Etc.)
- 1 PSF Collateral Load (Ceilings, Sprinklers, Etc.)

II - Normal Risk Category I_s 1.0000 I_e 1.00

DRAWING INDEX

- CS-1 Drawings Cover Sheet
- CS-2W Fastener Schedule
- E1 Anchor Bolt Plan
- E2 Anchor Bolt Details & Reactions
- E3 Rigid Frame Elevation
- E4 Sidewall Framing
- E5 Endwall Framing
- E6 Roof Framing Plan
- E7 Sidewall Sheeting
- E8 Endwall Sheeting
- E9 Detail Drawings
- E10 Detail Drawings
- E11 Trim Drawings

SEISMIC DATA :

- 1) Mapped Spectral Acceleration for Short Period, S_s 0.08
- 2) Site Coefficient, F_a 1.6000
- 3) Seismic Design Category = B
- 4) Seismic Coefficient = 0.09
- 5) Site Class = D
- 6) Basic Structural System and Seismic Resisting System
Ordinary Moment Frame of Steel
- 7) Frames: R = 3.0000
- 8) Analysis Procedure = Equivalent Lateral Force

These Drawings are for :

- Construction Approval *
- Permit Anchor Bolts & Reactions

* Approval orders must be released for fabrication within thirty (30) calendar days after the submittal drawings are issued or they will be subject to any current price increases. Special attention should be given in approving dimensions and/or details. Please verify requested dimensions by indicating 'OK'.

FBC product approval numbers: FL19604 & FL19606

T & Z Consulting Services, LLC
1428 N Shevlin Court Sewickley, PA 15143
Florida CA Lic. No. 32543

DSN: MQZ

DWN: MO

REV:

DET: RB

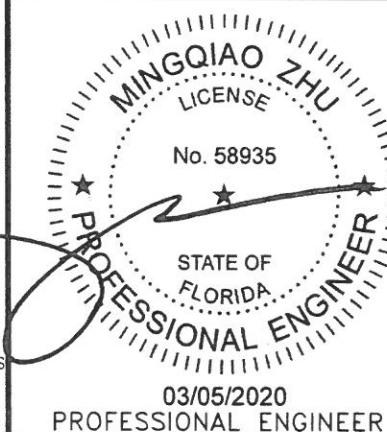
CHK: DP

REVISIONS

NO.	DATE

Engineering Seal

This certification covers parts manufactured and delivered by the manufacturer only, and excludes parts such as doors, windows, foundation design and erection of the building. The buyer is responsible for ensuring all specified loads are in compliance with regulatory authorities.



SCALE:
NOT TO SCALE

DATE:
3/3/20

JOB NO:
005728

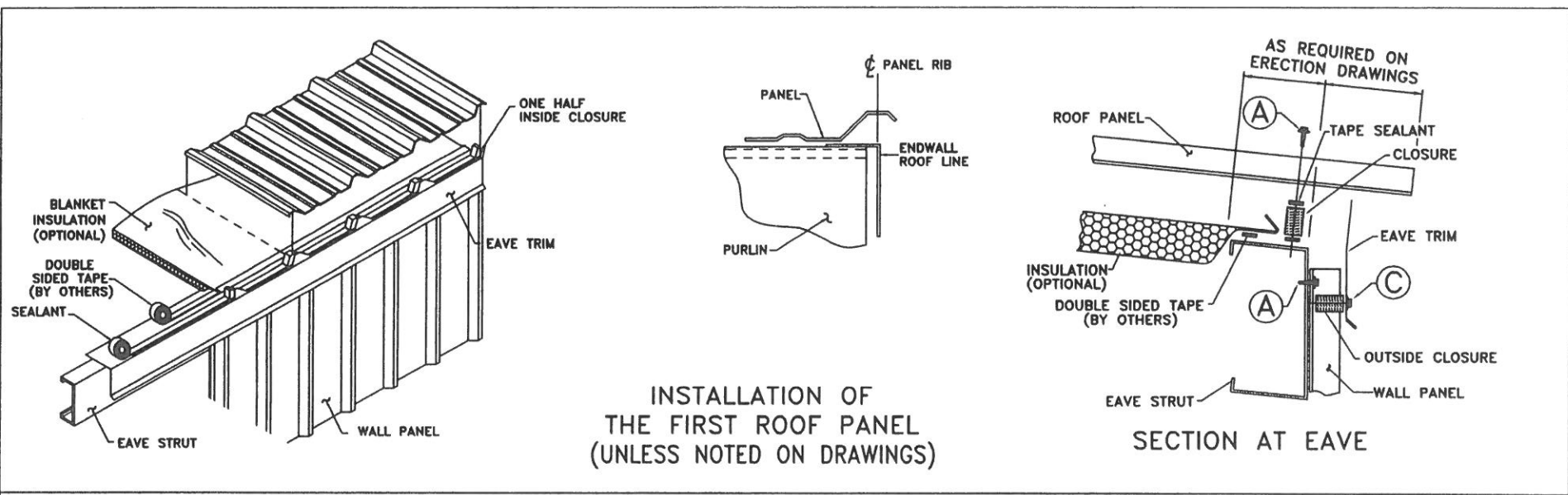
SHT. NO:
CS-1

DRAWINGS COVER SHEET

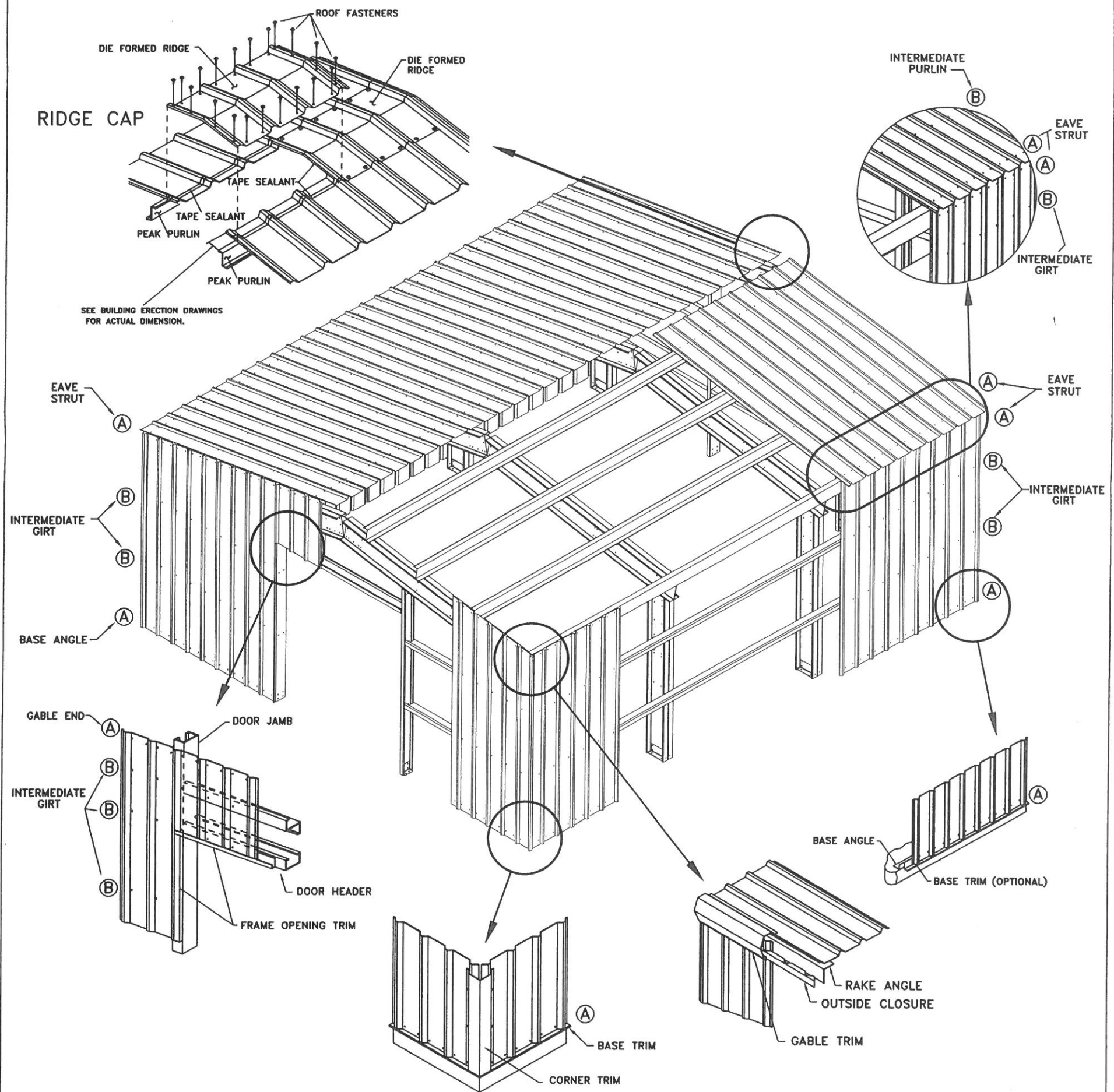
CUSTOMER :

CITY OF FORT WALTON BEACH
1955 LEWIS TURNER BLVD,
FORT WALTON BEACH FL 32548
COUNTY: OKALOOSA COUNTY

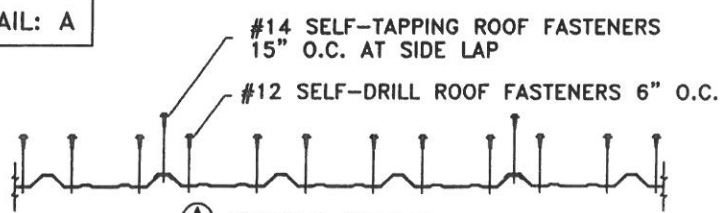
OLYMPIA STEEL BUILDINGS
400 ISLAND AVE
MCKEES ROCKS PA 15136



FASTENER SCHEDULE

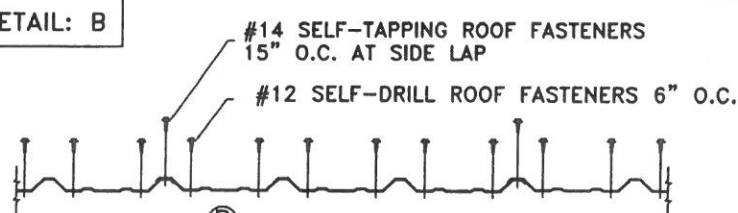


DETAIL: A



(A) FASTENER PATTERN AT BASE ANGLE, EAVE STRUT, RIDGE CAP & INTERMEDIATE PANEL LAPS

DETAIL: B



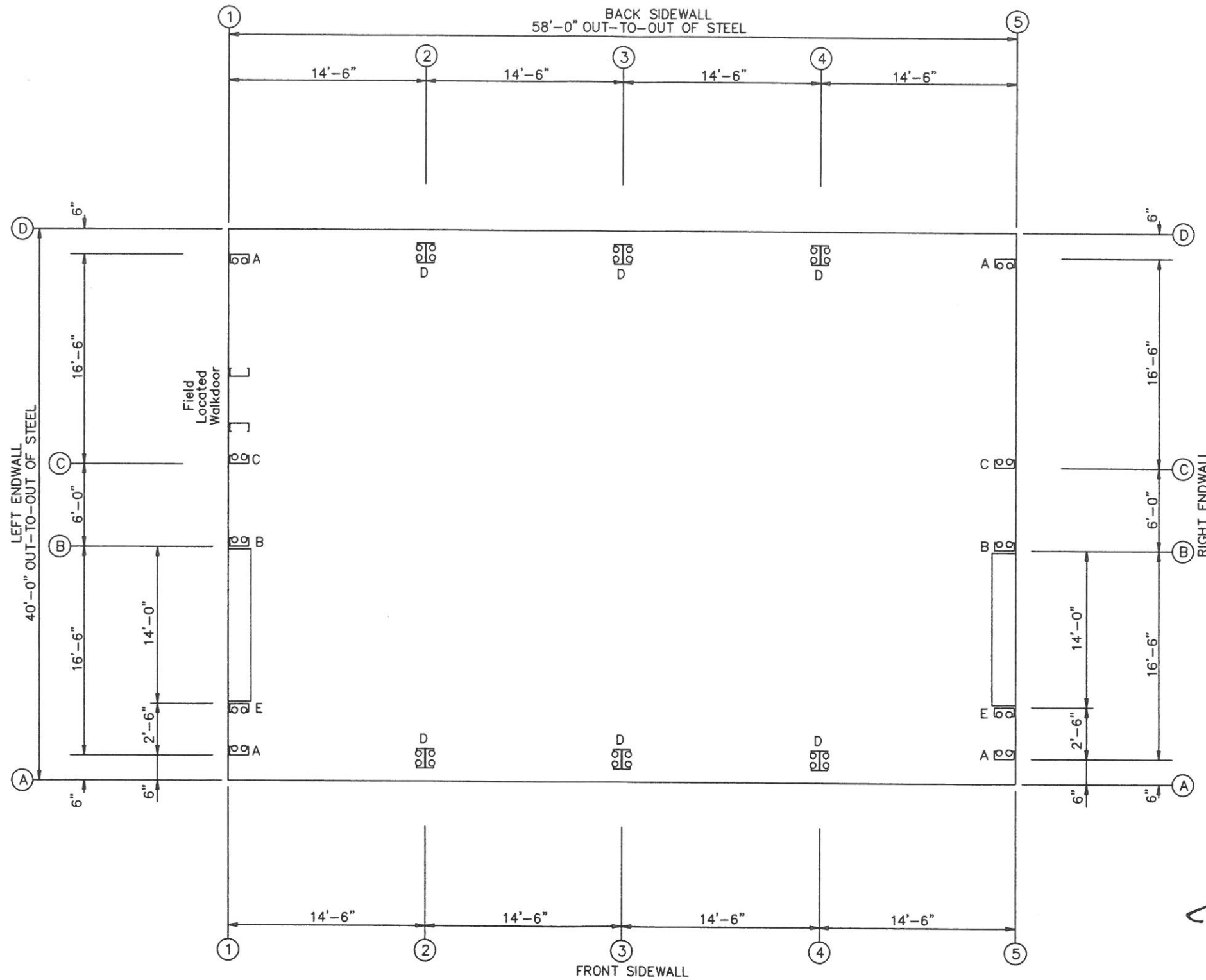
(B) FASTENER PATTERN AT INTERMEDIATE PURLIN OR GIRTS OR LINER PANEL

DETAIL: C

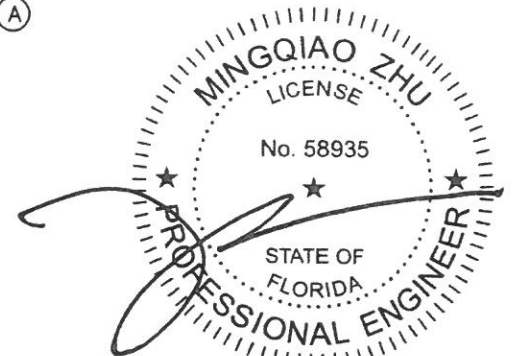
(C) STITCH FASTENER #14x7/8" 12" ON CENTER

DATE: 3/3/20

DWG NO: CS-2W

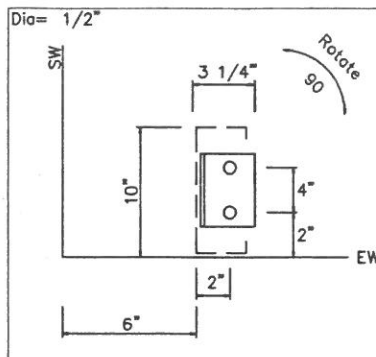


ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)

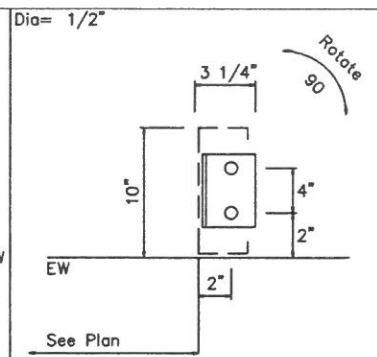


03/05/2020

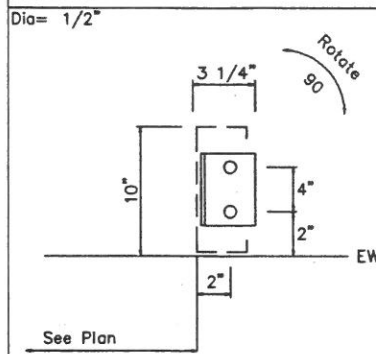
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID: 005728
Checker: DP	Date: 3/3/20		
ANCHOR BOLT PLAN			Sht E1 of 11



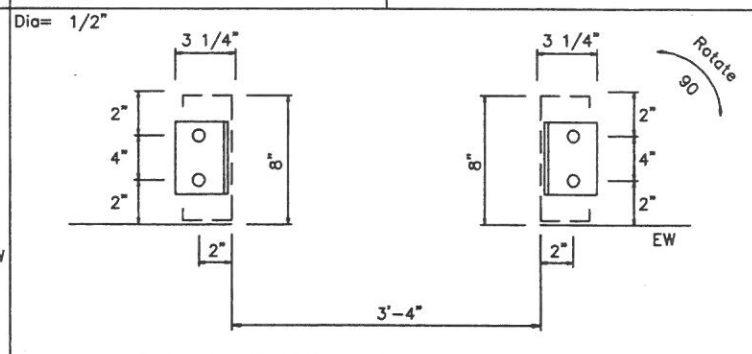
DETAIL A



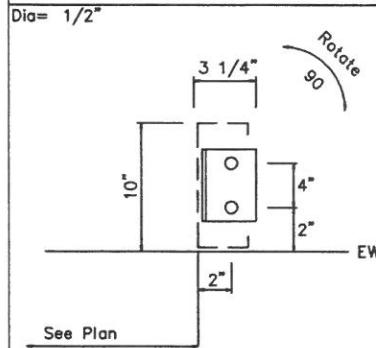
DETAIL E



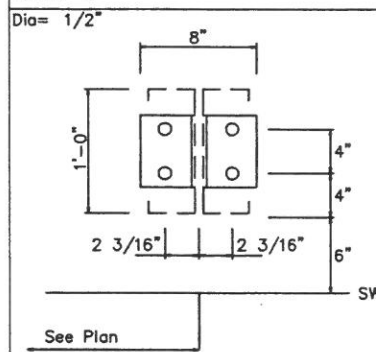
DETAIL B



DETAIL OF FIELD LOCATED WALKDOOR



DETAIL C



DETAIL D

LOAD CASES

WIND_L1 = WIND LOAD FROM LEFT CASE 1
 WIND_R1 = WIND LOAD FROM RIGHT CASE 1
 WIND_LN1 = LONGITUDINAL WIND CASE 1
 SEISMIC_L = SEISMIC LOAD FROM LEFT
 SEISMIC_R = SEISMIC LOAD FROM RIGHT
 SEISMICLN = LONGITUDINAL SEISMIC LOAD
 F1UNB_SL_L = FRAME 1 UNBALANCED SNOW LEFT SIDE
 F1UNB_SL_R = FRAME 1 UNBALANCED SNOW RIGHT SIDE
 F1CRANE 1 = FRAME 1 CRANE LOAD IN POSITION 1
 DRIFT = SNOW DRIFT LOAD
 SLIDE = SLIDE SNOW LOAD

NOTES FOR REACTIONS

Building reactions are based on the following building data:
 Width (ft) = 40
 Length (ft) = 58
 Eave Height (ft) = 14 / 14
 Roof Slope = 2.0:12 / 2.0:12
 Dead Load (psf) = 2.000
 Collateral Load (psf) = 1
 Roof Live Load (psf) = 20.00
 Roof Snow Load (psf) = 0
 Wind Speed (mph) = 145
 Wind Code = FBC 2017 (IBC 15)
 Wind Exposure = B
 Closed/Open = Closed
 Importance - Wind = 1.00
 Importance - Seismic = 1.00
 Seismic Design Category = B
 Seismic Coeff (F_eS_s) = 0.134

Load Combinations
 DL+CL+(LL or SL)
 DL+CL+0.6WL
 DL+CL+0.75(0.6)WL+0.75(LL or SL)
 DL+0.75(0.7SEIS)+0.75(LL or SL)
 0.6DL+0.6WL
 0.6DL+0.7SEIS

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
- THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
- THE SPECIFIED ANCHOR BOLT DIAMETER ASSUMES ASTM A307. ANCHOR BOLT MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER.
- ANCHOR BOLTS TO BE SUPPLIED BY OTHERS. ANCHOR BOLT EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION ENGINEER.
- ANCHOR BOLT PROJECTION ABOVE CONCRETE FINISHED SURFACE TO BE 3" UNLESS OTHERWISE NOTED BY FOUNDATION DESIGNER.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN ELEVATION AND LOCATION.
- THE ANCHOR BOLT LOCATIONS PROVIDED BY THE METAL BUILDING MANUFACTURER MAY NOT SATISFY ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF FOUNDATION DESIGN. IT IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER TO MAKE SURE THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED IN THE FOUNDATION DESIGN.
- MINOR FIELD WORK OF STRUCTURAL SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.
- THIS DRAWING IS NOT TO SCALE.

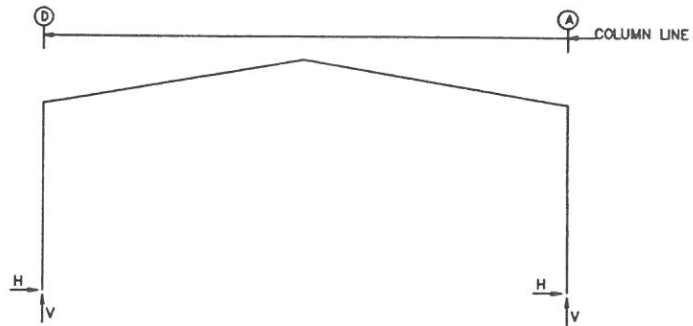
ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in)			Grout (in)
				Width	Length	Thick	
1	D	2	0.500	3.250	6.000	0.250	0.0
1	C	2	0.500	3.250	6.000	0.250	0.0
1	B	2	0.500	3.250	6.000	0.250	0.0
1	A	2	0.500	3.250	6.000	0.250	0.0
5	A	2	0.500	3.250	6.000	0.250	0.0
5	B	2	0.500	3.250	6.000	0.250	0.0
5	C	2	0.500	3.250	6.000	0.250	0.0
5	D	2	0.500	3.250	6.000	0.250	0.0

BUILDING BRACING REACTIONS

Loc	Line	± Reactions (k)				Panel Shear (lb/ft)	
		Wind	Seismic	Wind	Seismic	Wind	Seis
L_EW	1			64	11		
F_SW	A			39	3		
R_EW	S			56	9		
B_SW	D			39	3		

FRAME LINES: 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in)			Grout (in)
				Width	Length	Thick	
2*	D	4	0.500	8.000	9.000	0.250	0.0
2*	A	4	0.500	8.000	9.000	0.250	0.0

2* Frame lines: 2 3 4

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frm Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	0.3	1.1	0.1	0.3	2.1	5.8	-4.2	-8.2	0.3	-5.3	-4.2	-4.8
2*	A	-0.3	1.1	-0.1	0.3	-2.1	5.8	-0.3	-5.3	4.2	-8.2	-0.3	-1.9

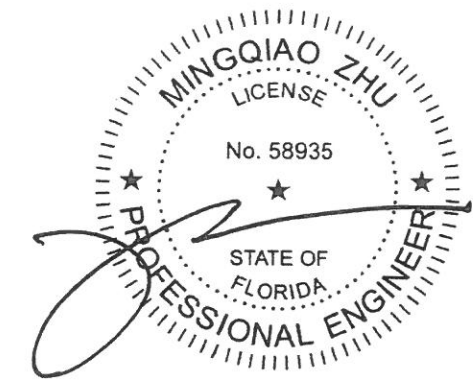
Frm Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	0.3	-1.9	0.0	-7.2	-0.5	-6.0	-0.1	0.0	0.1	0.0
2*	A	4.2	-4.8	0.5	-6.0	0.0	-7.2	-0.1	0.0	0.1	0.0

2* Frame lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Wind		Wind		Wind Press Horz	Wind Suct		Wind		Seis Left Vert
					Left1 Vert	Right1 Vert	Left2 Vert	Right2 Vert		Long1 Vert	Long2 Vert			
1	D	0.2	0.1	1.2	-2.2	-1.5	-1.3	-0.6	-1.6	1.8	-2.3	-1.5	0.0	
1	C	0.3	0.1	1.7	-3.8	-1.5	-2.9	-0.6	-2.4	2.7	-3.8	-1.3	0.0	
1	B	0.3	0.1	1.7	-1.5	-3.8	-0.6	-2.9	-2.4	2.7	-1.3	-3.8	0.0	
1	A	0.2	0.1	1.2	-1.5	-2.2	-0.6	-1.3	-1.6	1.8	-1.5	-2.3	0.0	
5	A	0.2	0.1	1.2	-2.2	-1.5	-1.3	-0.6	-1.6	1.8	-2.3	-1.5	0.0	
5	B	0.3	0.1	1.7	-3.8	-1.5	-2.9	-0.6	-2.4	2.7	-3.8	-1.3	0.0	
5	C	0.3	0.1	1.7	-1.5	-3.8	-0.6	-2.9	-2.4	2.7	-1.3	-3.8	0.0	
5	D	0.2	0.1	1.2	-1.5	-2.2	-0.6	-1.3	-1.6	1.8	-1.5	-2.3	0.0	

Frm Line	Col Line	Seis Right	
		Horz	Vert
1	D	0.0	0.0
1	C	0.0	0.0
1	B	0.0	0.0
1	A	0.0	0.0
5	A	0.0	0.0
5	B	0.0	0.0
5	C	0.0	0.0
5	D	0.0	0.0



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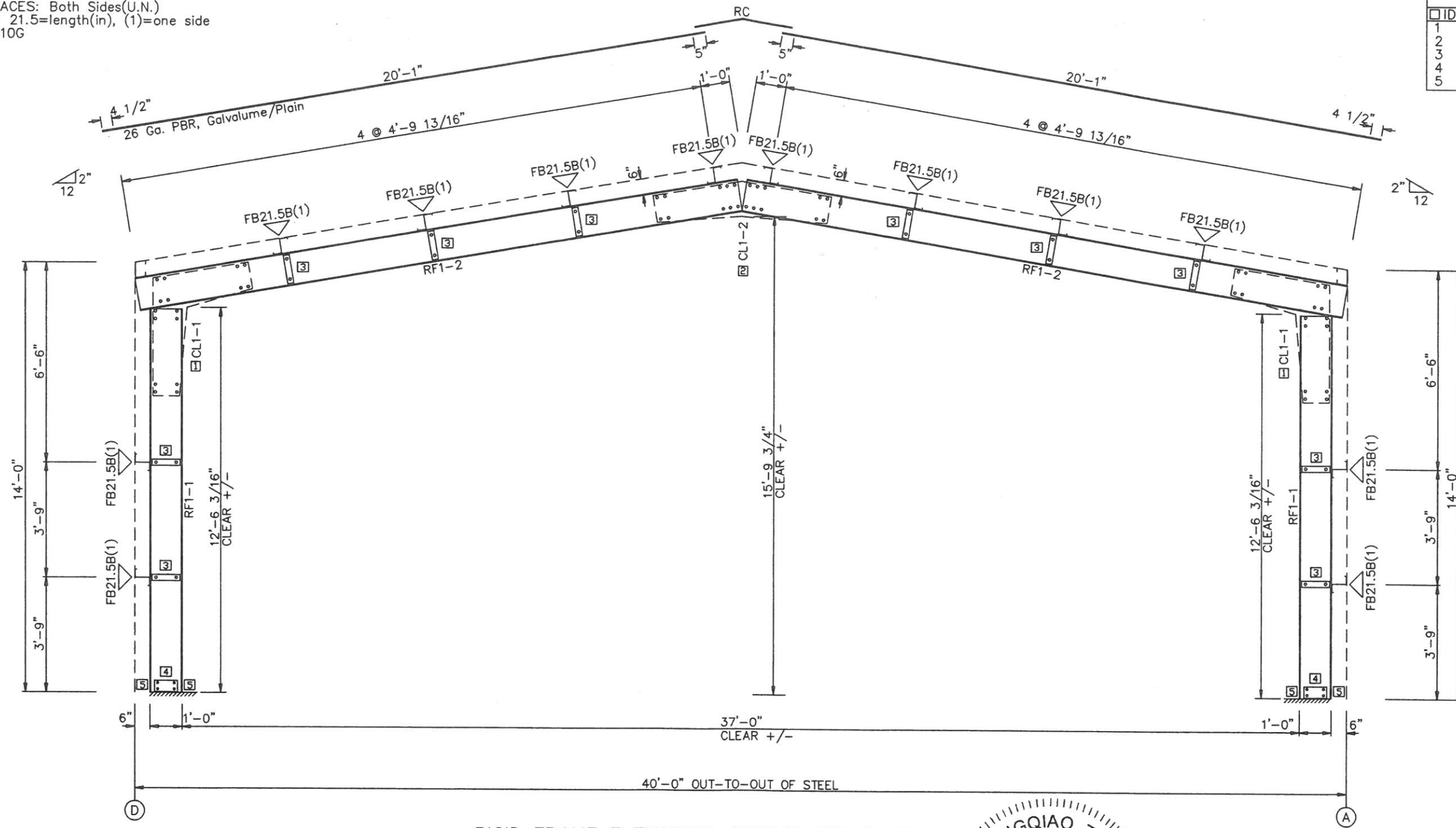
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
ANCHOR BOLT DETAILS & REACTIONS			Sht E2 of 11

SPLICE BOLT TABLE						
Mark	Qty		Type	Dia	Length	
	Top	Bot				
CL1-1	8	8	0	A325	0.500	2.00
CL1-2	10	10	0	A325	0.500	2.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	12x75D12	12'-6 3/4"
RF1-2	12x75D12	20'-1 5/16"

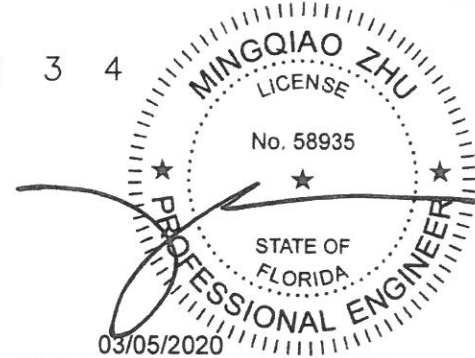
CONNECTION PLATES	
ID	Mark/Part
1	CL1-1
2	CL1-2
3	CLC103
4	CLC102
5	CLC001

▽ FLANGE BRACES: Both Sides(U.N.)
 FB21.5B(1): 21.5=length(in), (1)=one side
 B - L2X2X10G



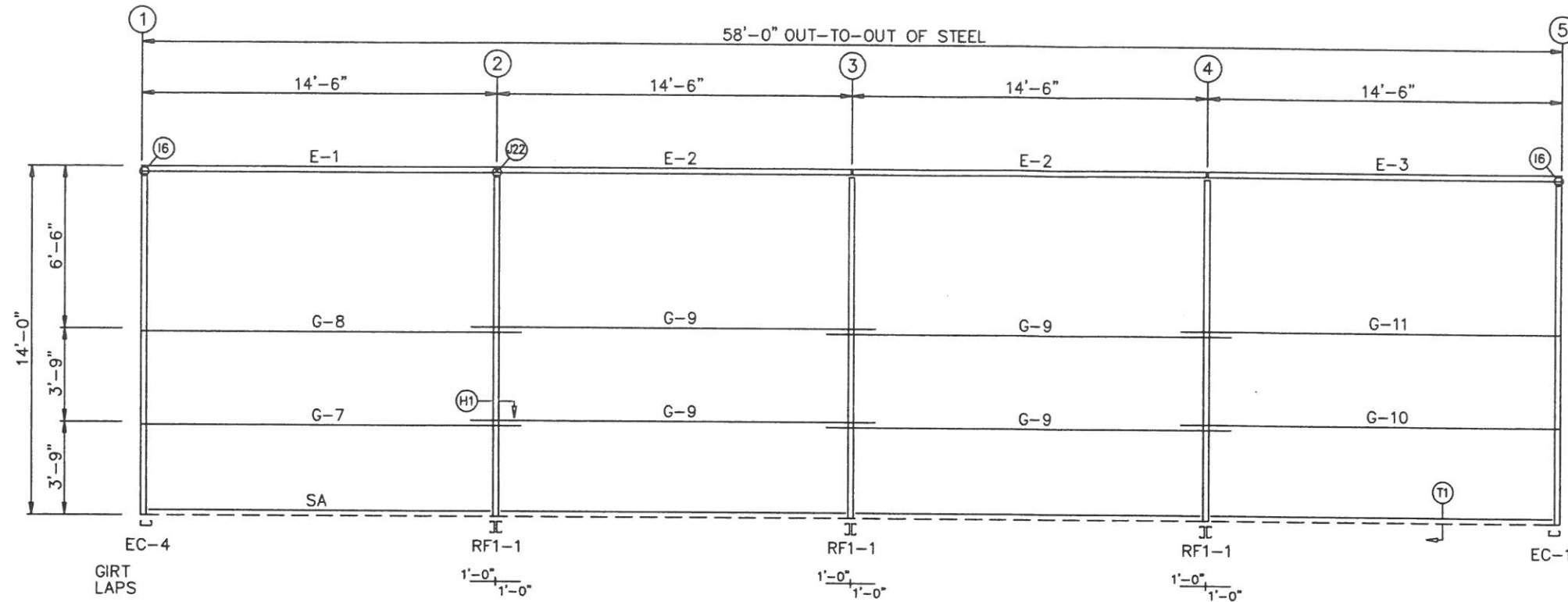
RIGID FRAME ELEVATION: FRAME LINE 2 3 4

GENERAL NOTES:
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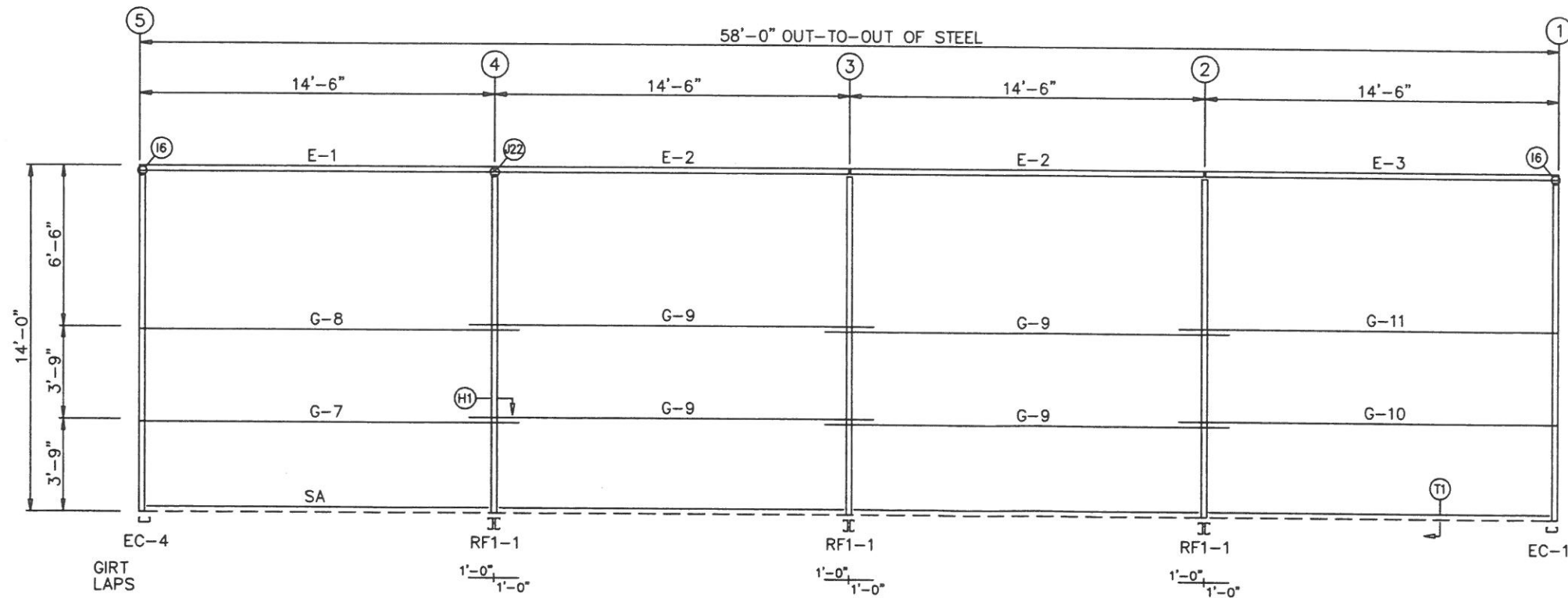


OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID: 005728
Checker: DP	Date: 3/3/20		
RIGID FRAME ELEVATION			Sht E3 of 11

MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
E-1	6x354E16	14'-5 1/4"
E-2	6x354E16	14'-5 1/2"
E-3	6x354E16	14'-5 1/4"
G-7	6x25Z18	15'-5 1/2"
G-8	6x25Z16	15'-5 1/2"
G-9	6x25Z18	16'-6"
G-10	6x25Z18	15'-5 1/2"
G-11	6x25Z16	15'-5 1/2"

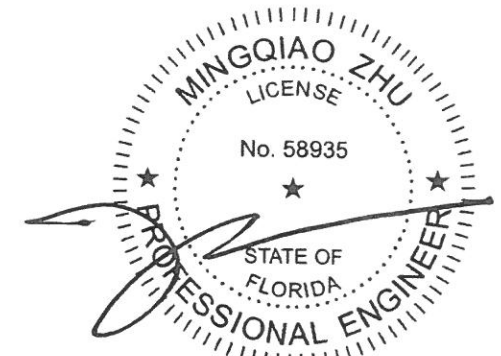


SIDEWALL FRAMING: FRAME LINE A



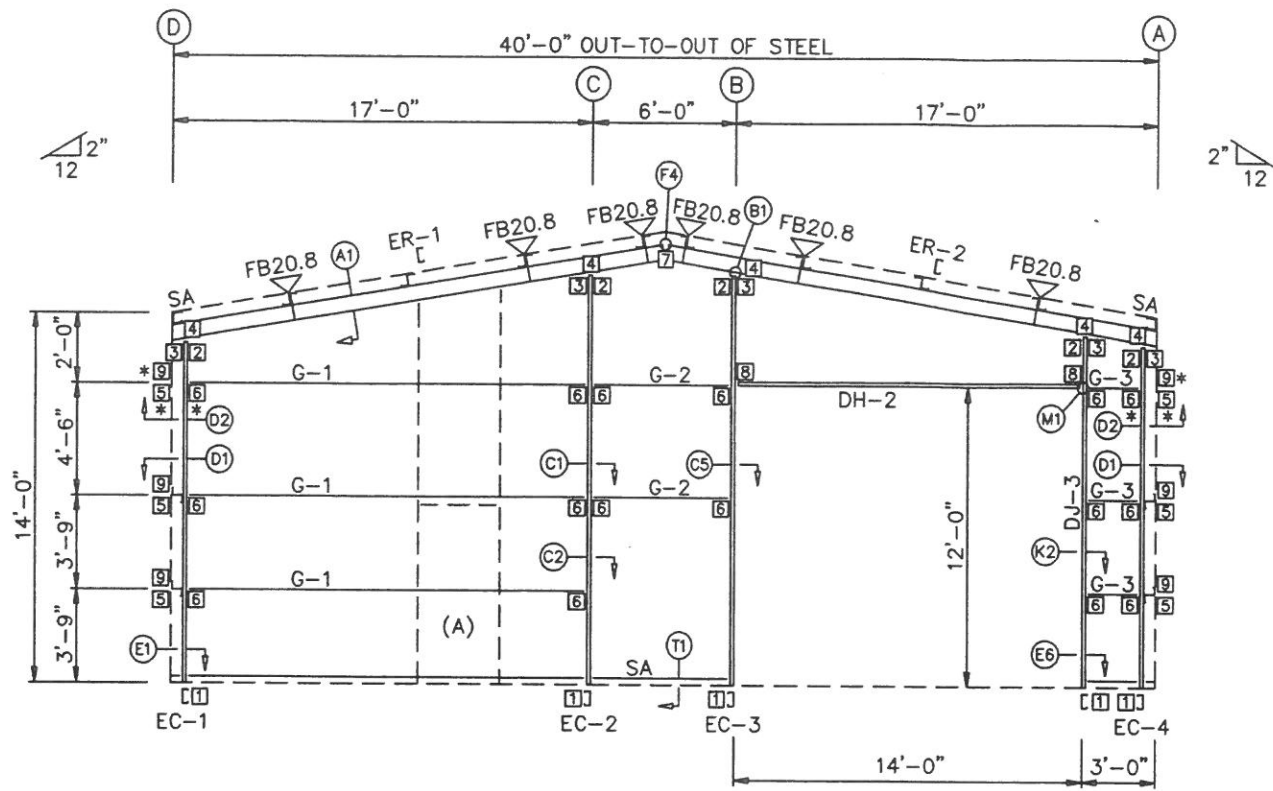
SIDEWALL FRAMING: FRAME LINE D

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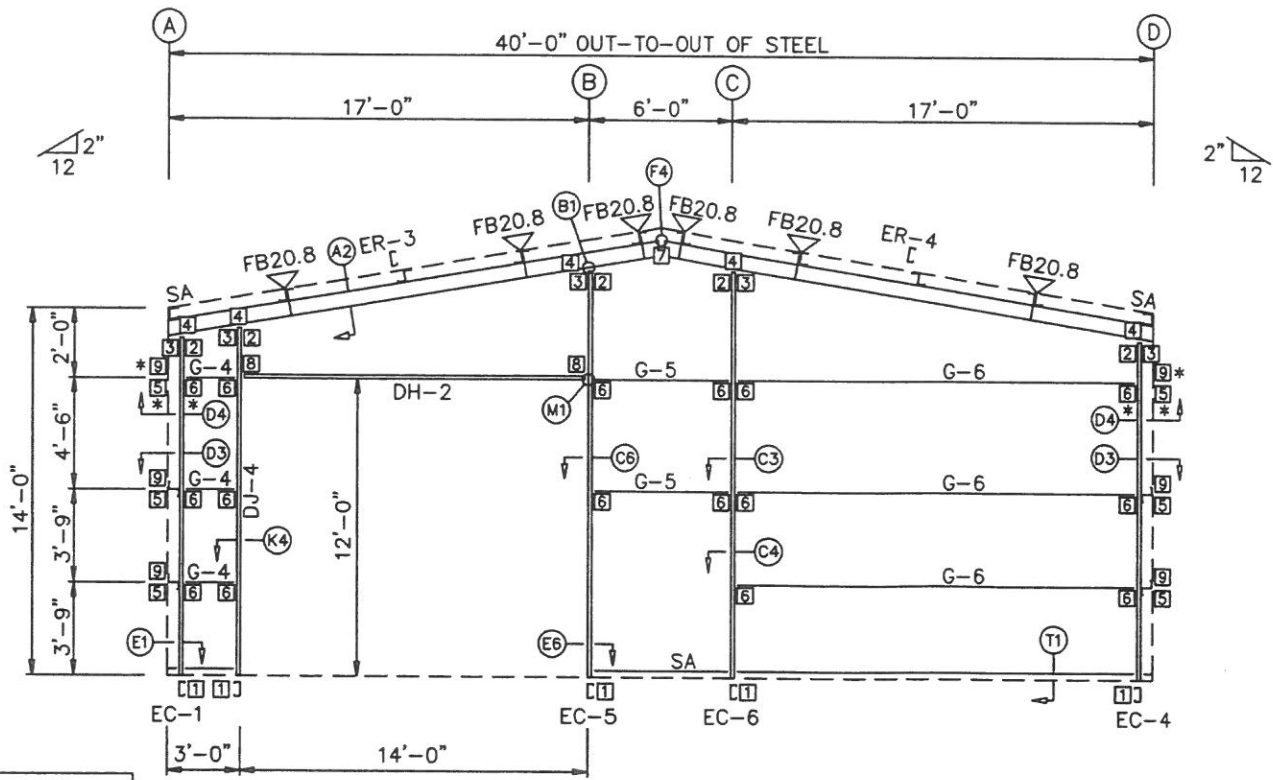


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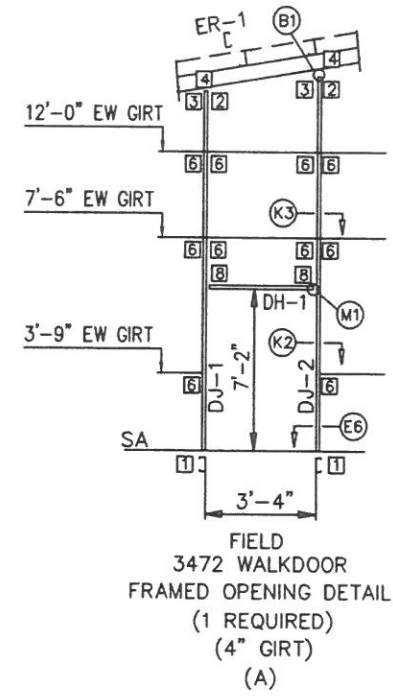
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
SIDEWALL FRAMING			Sht E4 of 11



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 5



FIELD
3472 WALKDOOR
FRAMED OPENING DETAIL
(1 REQUIRED)
(4" GIRT)
(A)

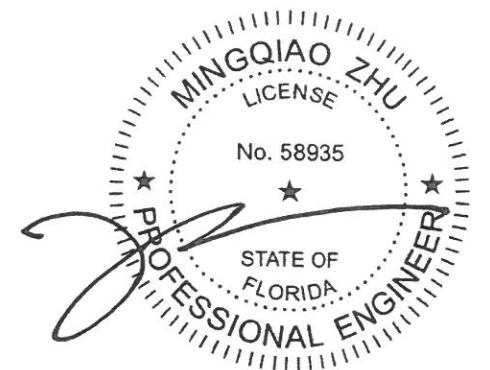
BOLT TABLE FRAME LINE 1 & 5			
LOCATION	QUAN	TYPE DIA	LENGTH
ER-1/ER-2	4	A325T 1/2"	2"
ER-3/ER-4	4	A325T 1/2"	2"
Columns/Raf	8	A325T 1/2"	2"
Jamb/Raf	8	A325T 1/2"	2"

MEMBER TABLE FRAME LINE 1 & 5		
MARK	PART	LENGTH
EC-1	10x28C15	12'-7 3/4"
EC-2	10x28C15	15'-4 3/4"
EC-3	10x28C15	15'-4 3/4"
EC-4	10x28C15	12'-7 3/4"
EC-5	10x28C15	15'-4 3/4"
EC-6	10x28C15	15'-4 3/4"
ER-1	10x28C15	20'-1 1/2"
ER-2	10x28C15	20'-1 1/2"
ER-3	10x28C15	20'-1 1/2"
ER-4	10x28C15	20'-1 1/2"
DJ-1	8x275C16	14'-2 3/4"
DJ-2	8x275C16	14'-9 7/16"
DJ-3	10x28C15	13'-0 3/4"
DJ-4	10x28C15	13'-0 3/4"
DH-1	8x275C16	3'-3 1/2"
DH-2	10x28C15	13'-11 1/2"
G-1	4x25Z18	15'-9"
G-2	4x25Z18	5'-7"
G-3	4x25Z18	1'-9"
G-4	8x25Z16	1'-9"
G-5	8x25Z16	5'-7"
G-6	8x25Z16	15'-9"

FLANGE BRACE TABLE FRAME LINE 1 & 5			
VID	QUAN	MARK	LENGTH
1	12	FB20.8	1'-8 3/4"

CONNECTION PLATES FRAME LINE 1 & 5	
ID	MARK/PART
1	CLC008
2	CLC081
3	CLC083
4	CLC089
5	CLC061
6	CLC062
7	CLC122
8	CLC025
9	CLC063

* REVERSE CLIP



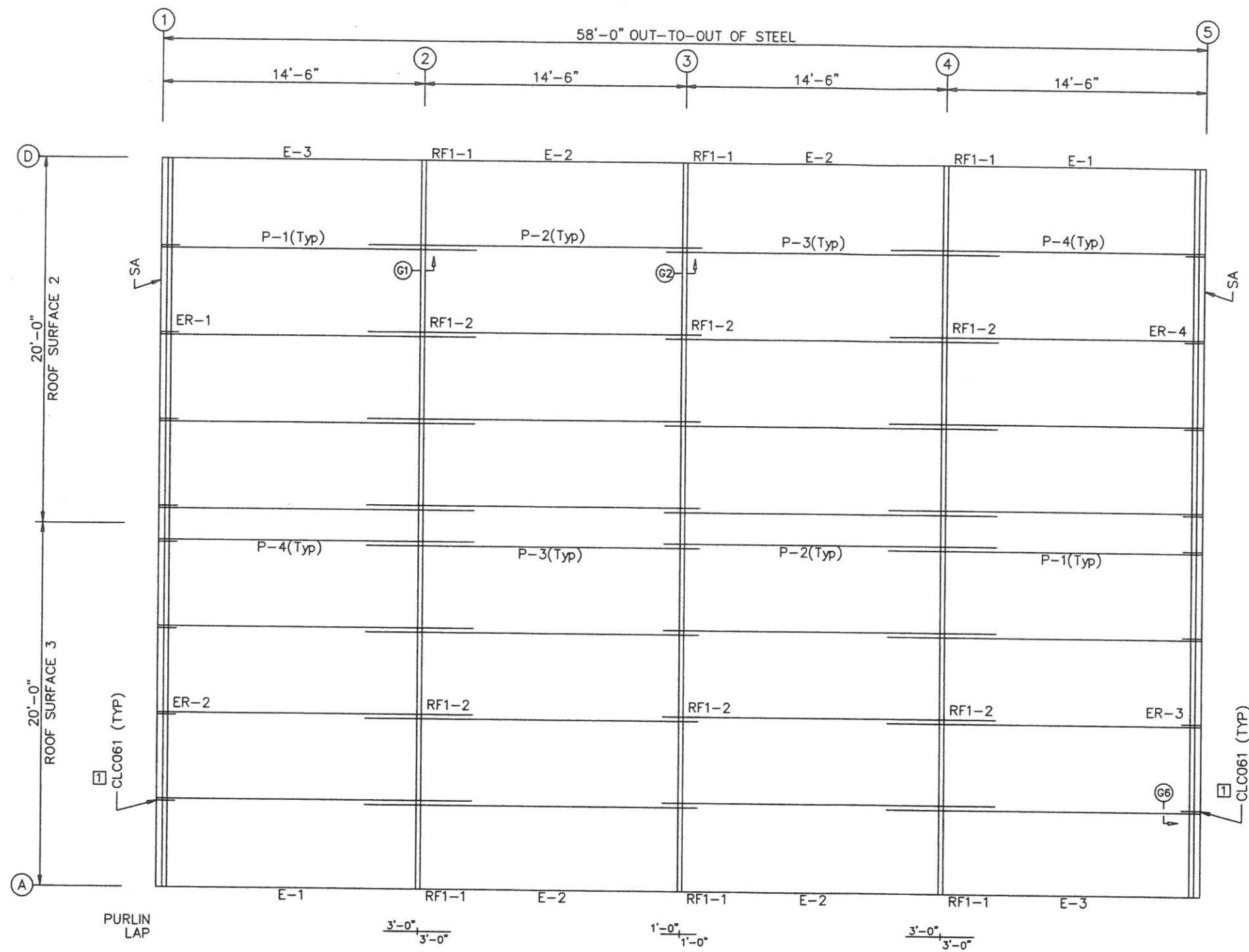
03/05/2020

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OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
ENDWALL FRAMING			Sht E5 of 11

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	6x25Z14	17'-5 1/2"
P-2	6x25Z18	18'-6"
P-3	6x25Z18	18'-6"
P-4	6x25Z14	17'-5 1/2"
E-1	6x354E16	14'-5 1/4"
E-2	6x354E16	14'-5 1/2"
E-3	6x354E16	14'-5 1/4"

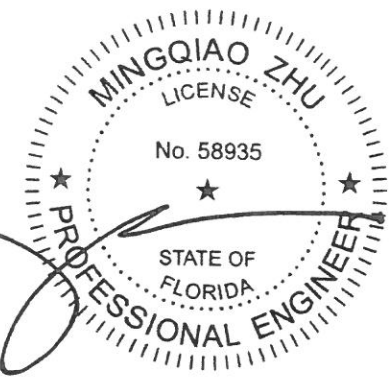
CONNECTION PLATES	
ROOF PLAN	
ID	MARK/PART
1	CLC061



ROOF FRAMING PLAN

RC
(21)

20'-1" (20)
20'-1" (20)

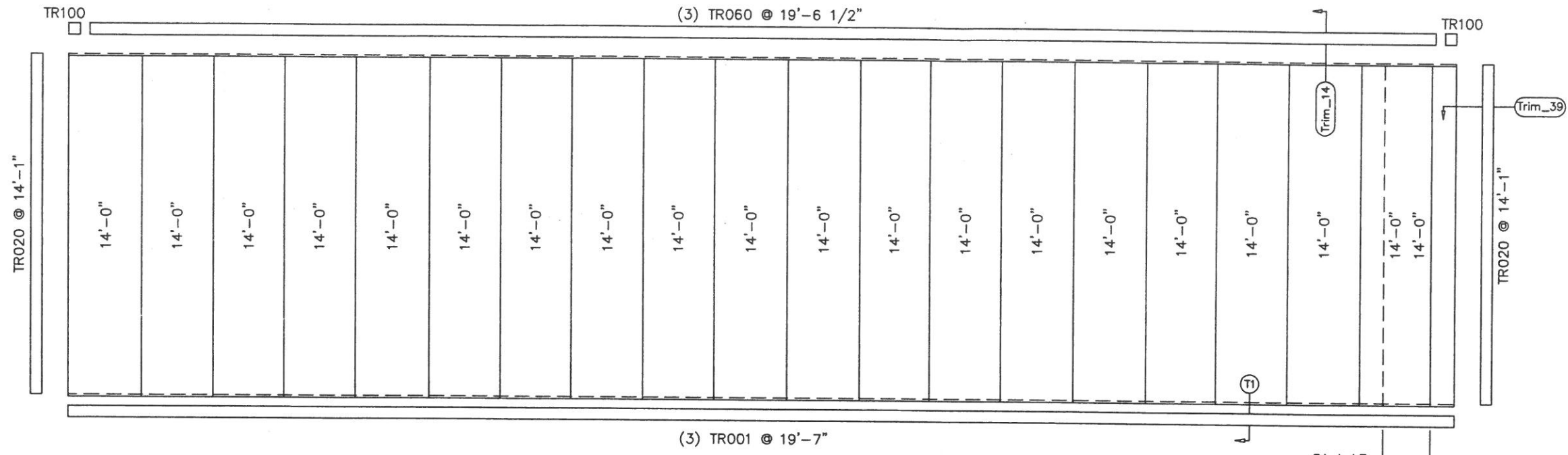


ROOF SHEETING
PANELS: 26 Ga. PBR Galvalume/Plain

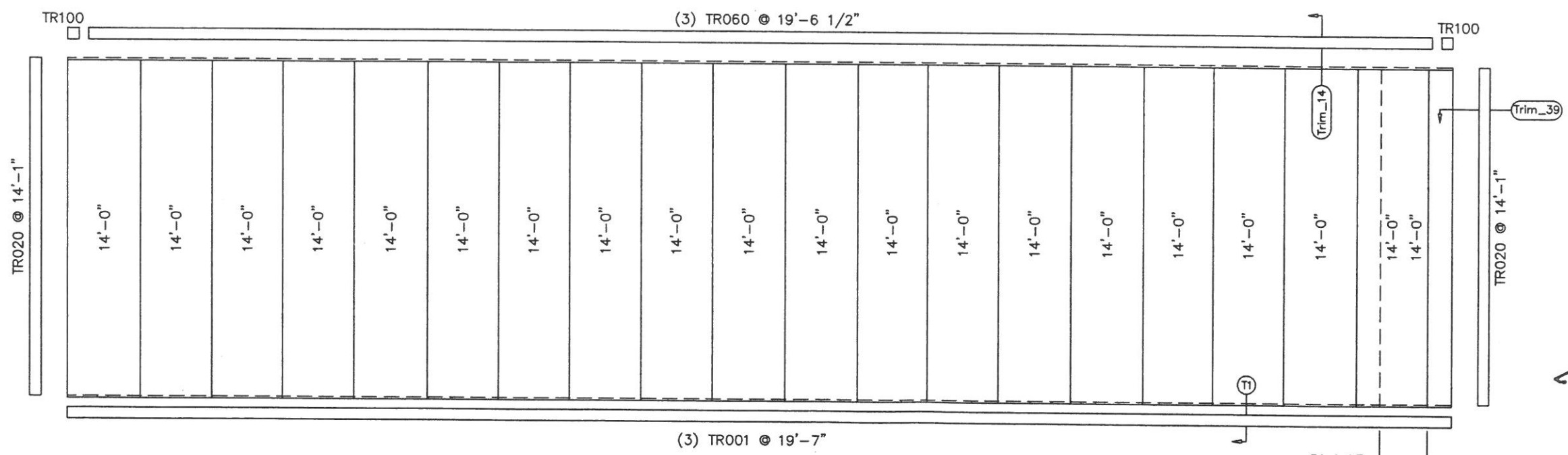
03/05/2020

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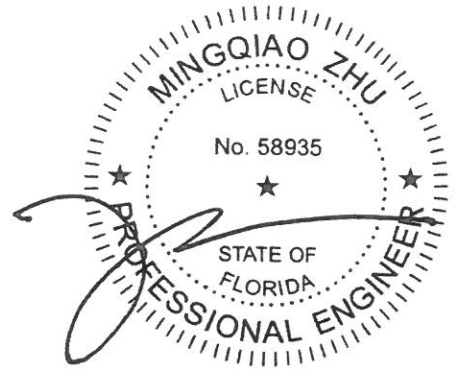
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
ROOF FRAMING PLAN			Sht E6 of 11



SIDEWALL SHEETING & TRIM: FRAME LINE A
 PANELS: 26 Ga. PBR - Light Stone



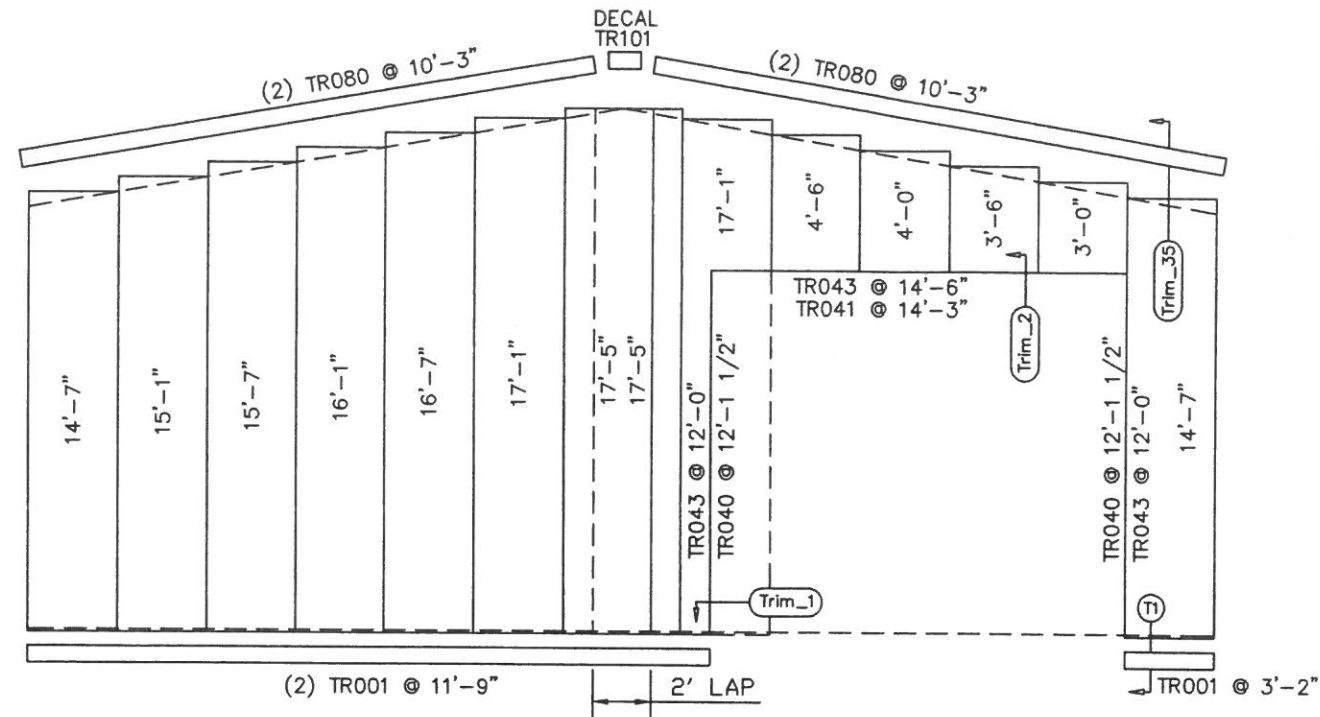
SIDEWALL SHEETING & TRIM: FRAME LINE D
 PANELS: 26 Ga. PBR - Light Stone



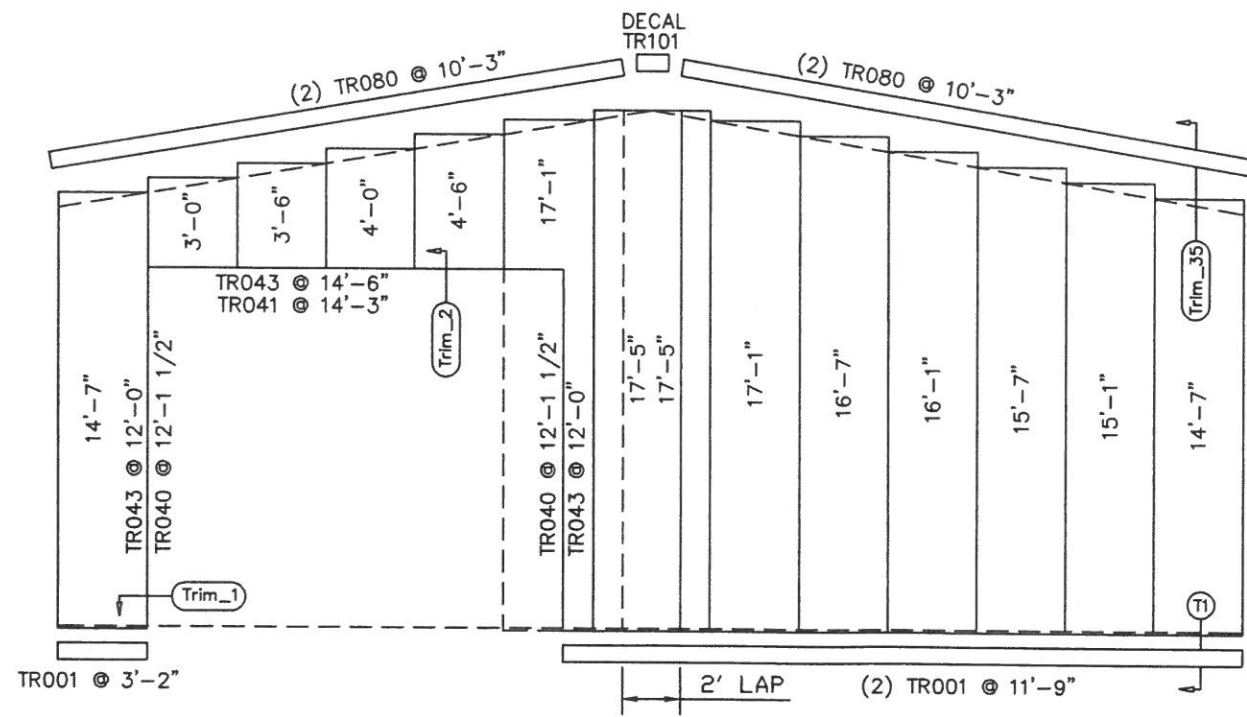
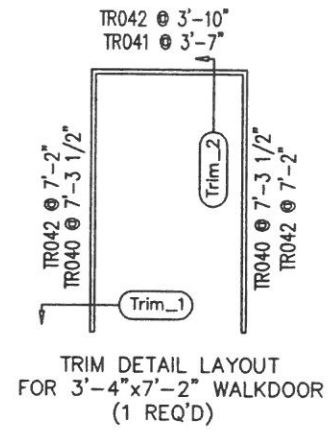
03/05/2020

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OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
SIDEWALL SHEETING			Sht E7 of 11

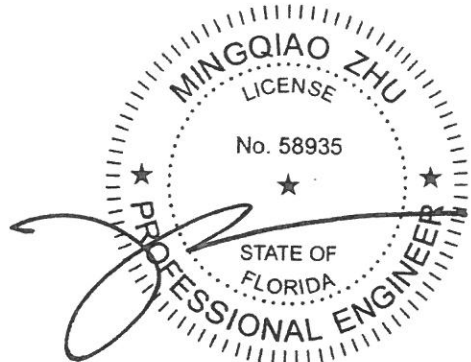


ENDWALL SHEETING & TRIM: FRAME LINE 1
 PANELS: 26 Ga. PBR - Light Stone



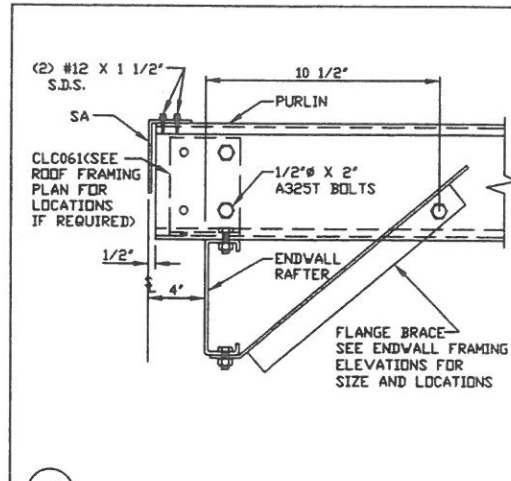
ENDWALL SHEETING & TRIM: FRAME LINE 5
 PANELS: 26 Ga. PBR - Light Stone

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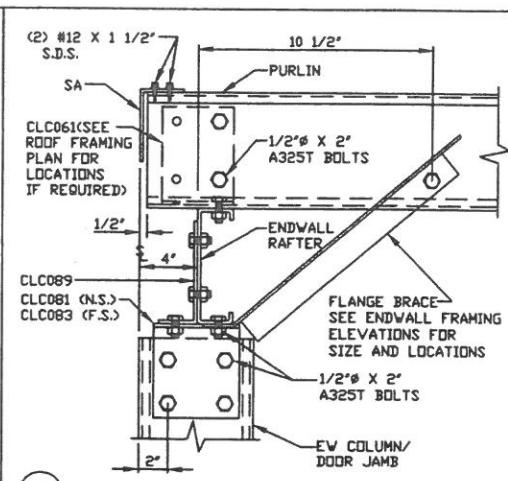


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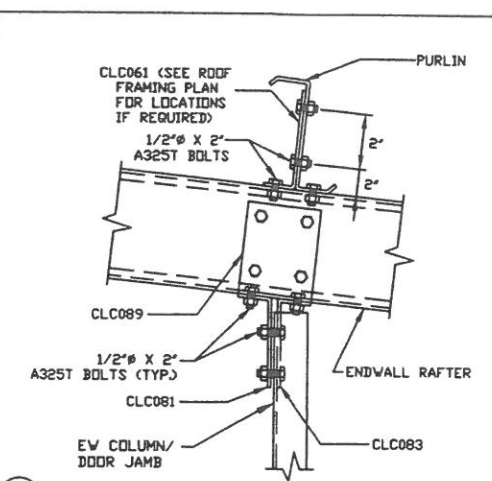
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID: 005728
Checker: DP	Date: 3/3/20		
ENDWALL SHEETING			Sht E8 of 11



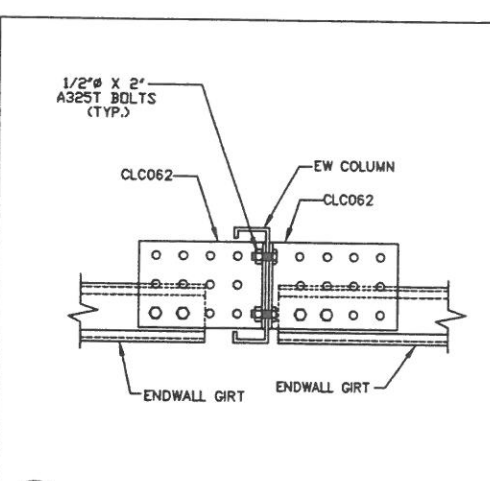
A1 PURLIN TO ENDWALL RAFTER



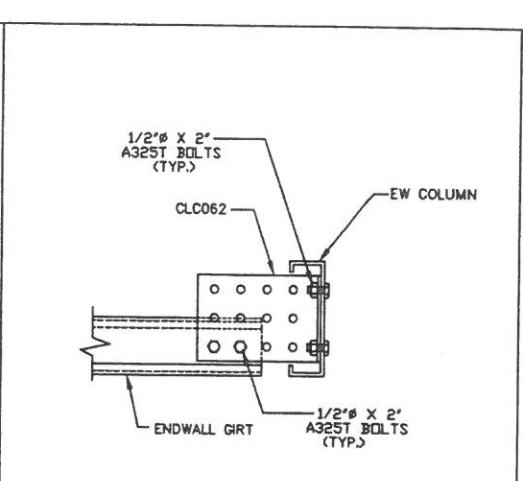
A2 PURLIN TO ENDWALL RAFTER



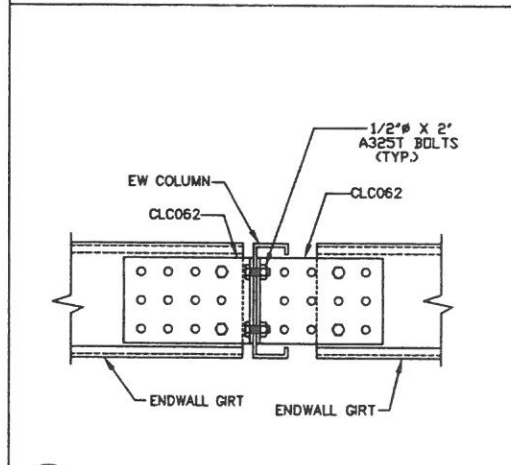
B1 EW COLUMN/DOOR JAMB TO ENDWALL RAFTER



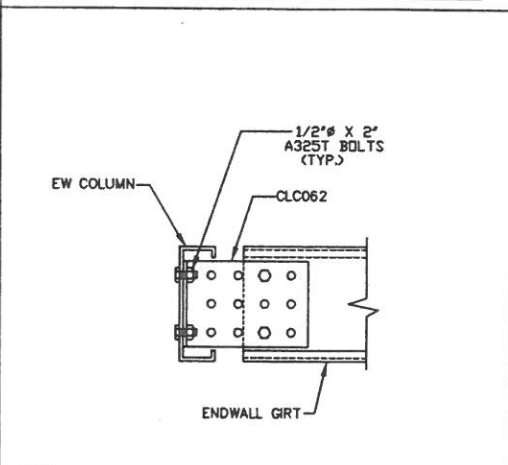
C1 ENDWALL COLUMN GIRT CONNECTION



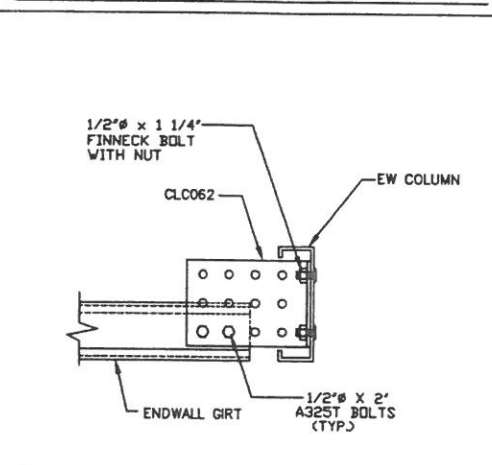
C2 ENDWALL COLUMN GIRT CONNECTION



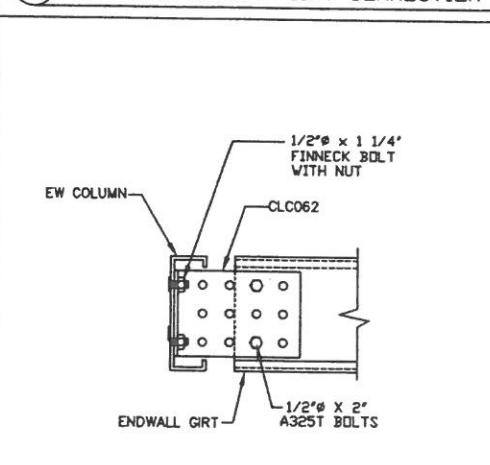
C3 ENDWALL COLUMN GIRT CONNECTION



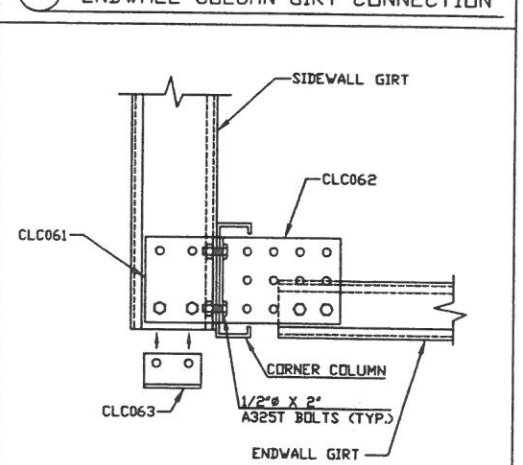
C4 ENDWALL COLUMN GIRT CONNECTION



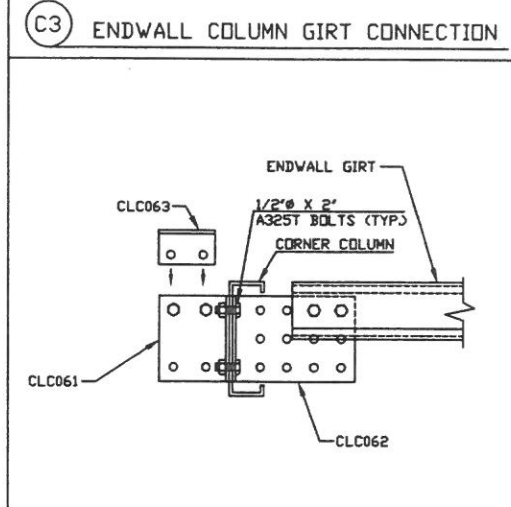
C5 ENDWALL COLUMN GIRT CONNECTION



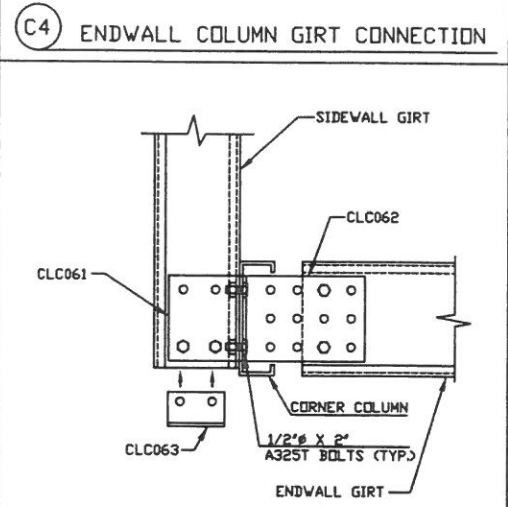
C6 ENDWALL COLUMN GIRT CONNECTION



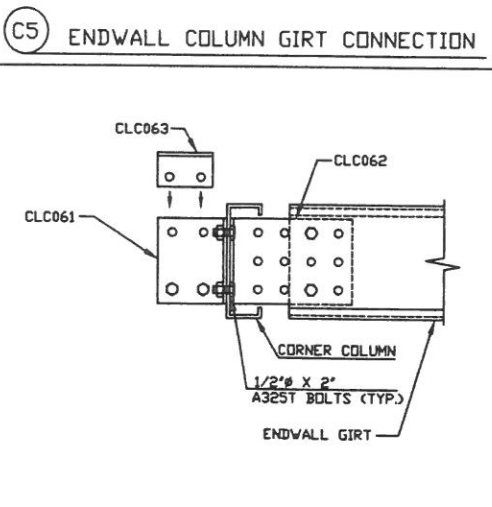
D1 CORNER COLUMN GIRT CONNECTION



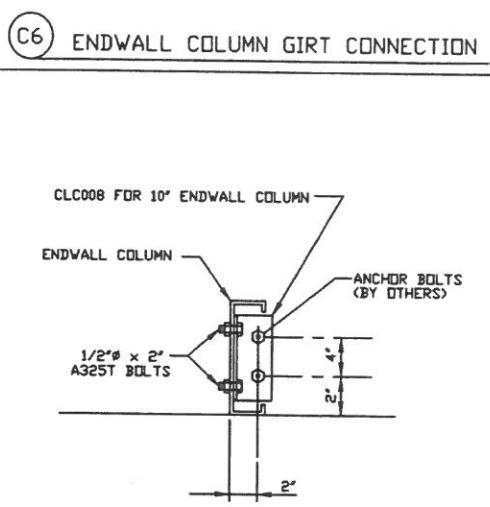
D2 CORNER COLUMN GIRT CONNECTION



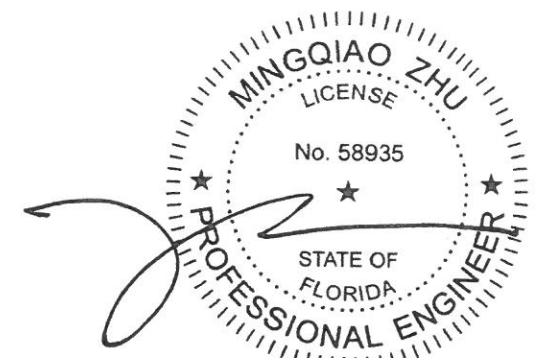
D3 CORNER COLUMN GIRT CONNECTION



D4 CORNER COLUMN GIRT CONNECTION



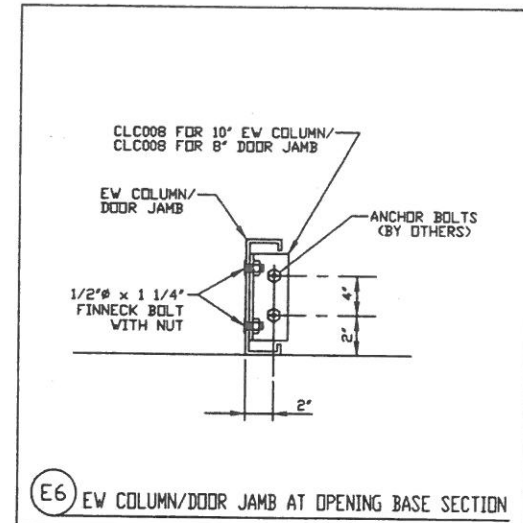
E1 ENDWALL COLUMN BASE SECTION



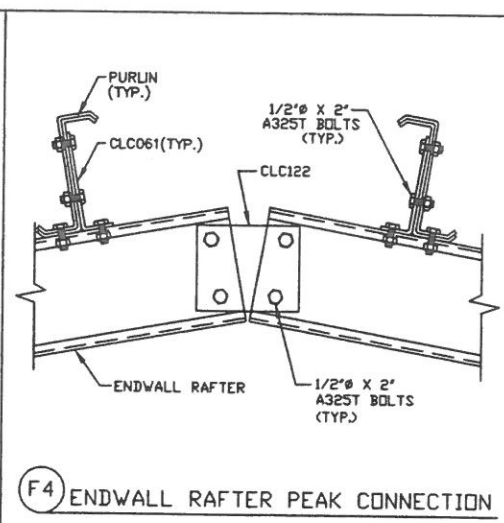
03/05/2020

NOTES: 1.) ALL SCREWS ARE WITH WASHERS UNLESS NOTED.

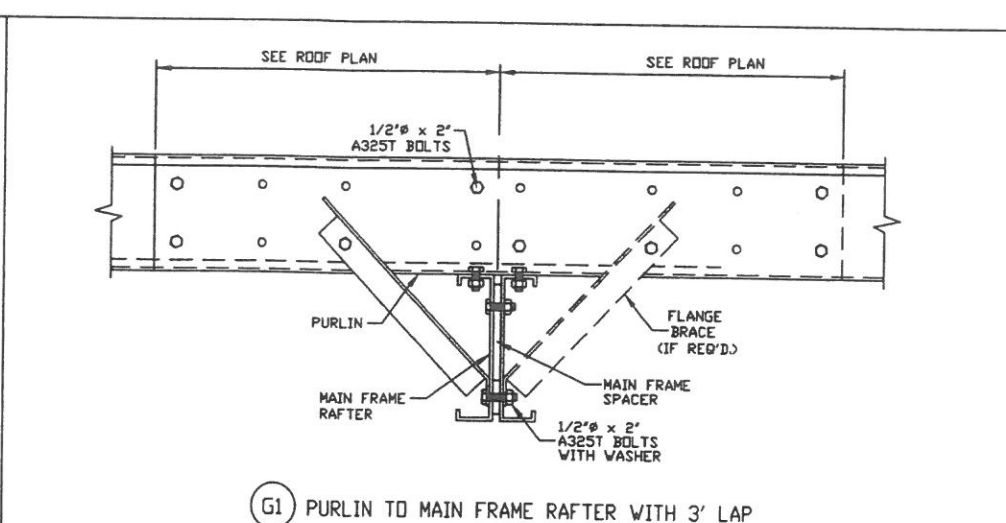
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
DETAIL DRAWINGS			Sht E9 of 11



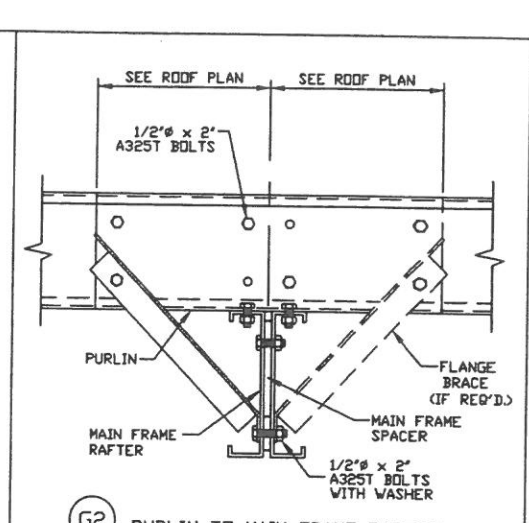
E6 EW COLUMN/DOOR JAMB AT OPENING BASE SECTION



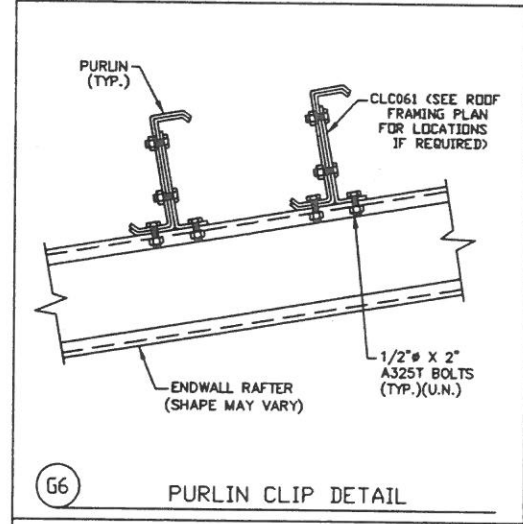
F4 ENDWALL RAFTER PEAK CONNECTION



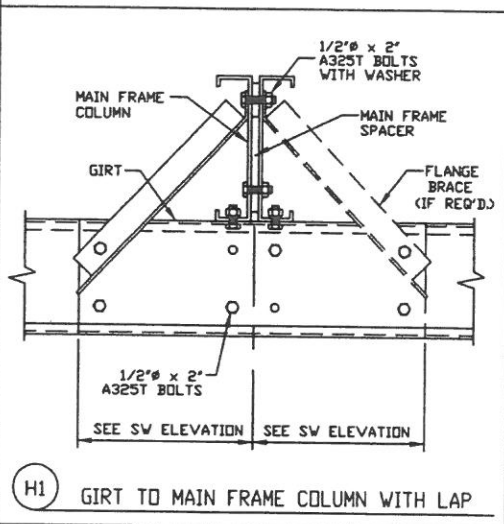
G1 PURLIN TO MAIN FRAME RAFTER WITH 3' LAP



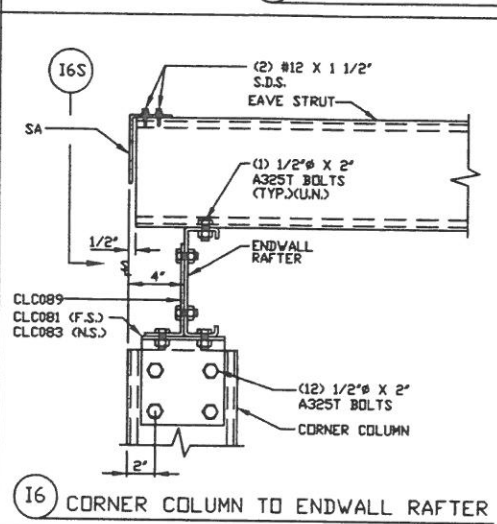
G2 PURLIN TO MAIN FRAME RAFTER



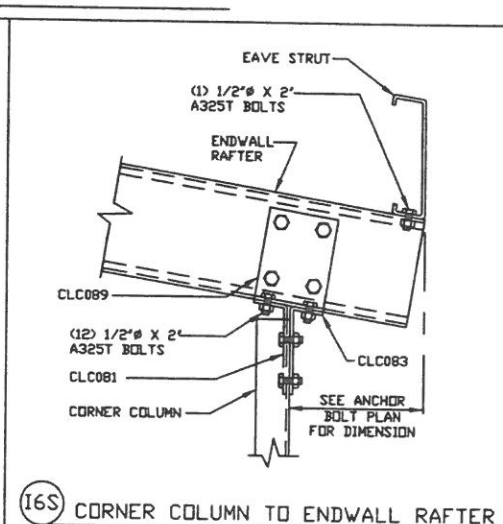
G6 PURLIN CLIP DETAIL



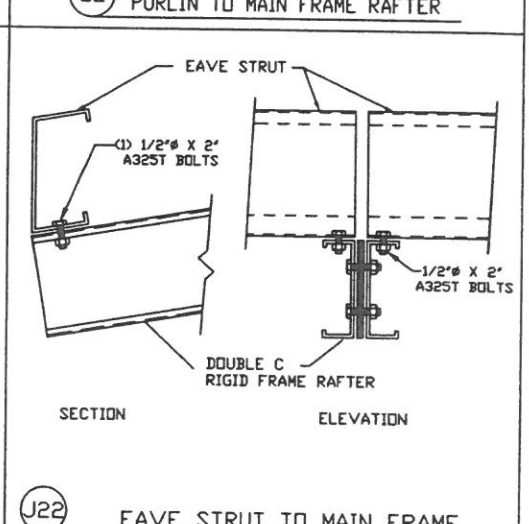
H1 GIRT TO MAIN FRAME COLUMN WITH LAP



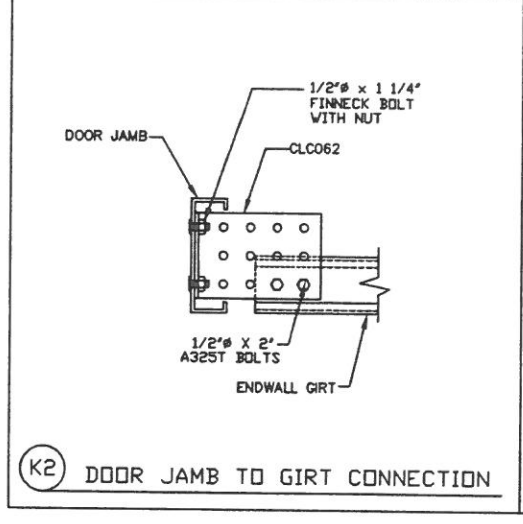
I6 CORNER COLUMN TO ENDWALL RAFTER



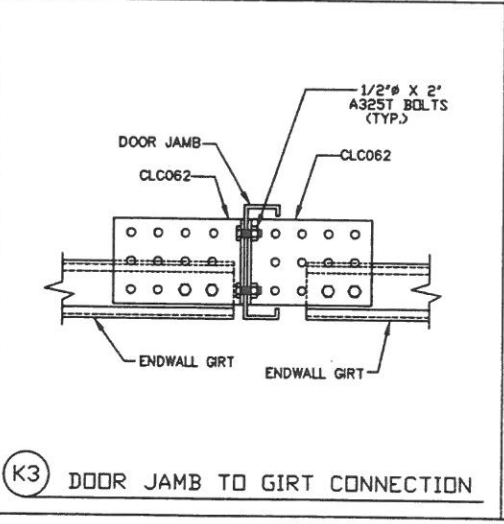
I6S CORNER COLUMN TO ENDWALL RAFTER



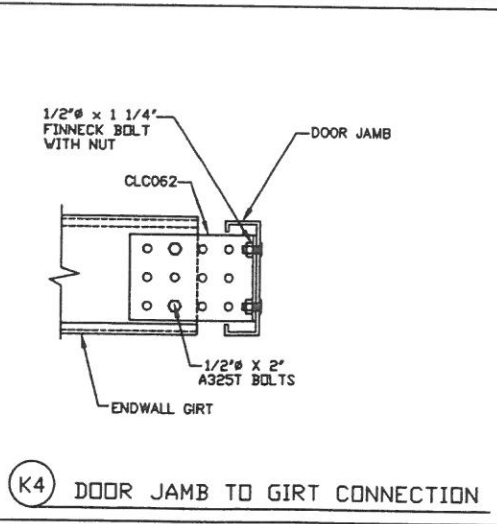
J22 EAVE STRUT TO MAIN FRAME



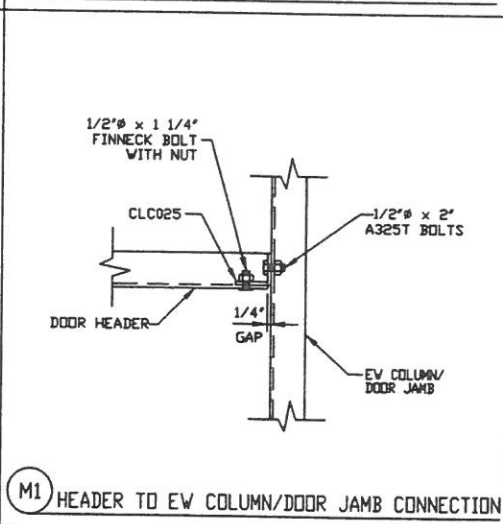
K2 DOOR JAMB TO GIRT CONNECTION



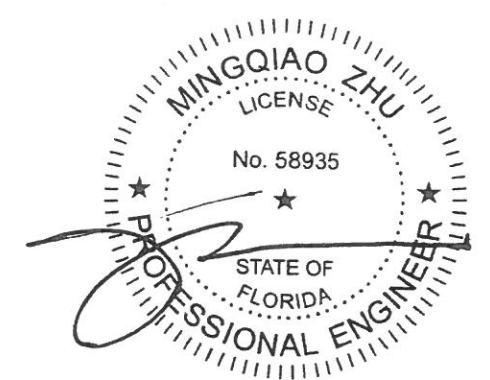
K3 DOOR JAMB TO GIRT CONNECTION



K4 DOOR JAMB TO GIRT CONNECTION

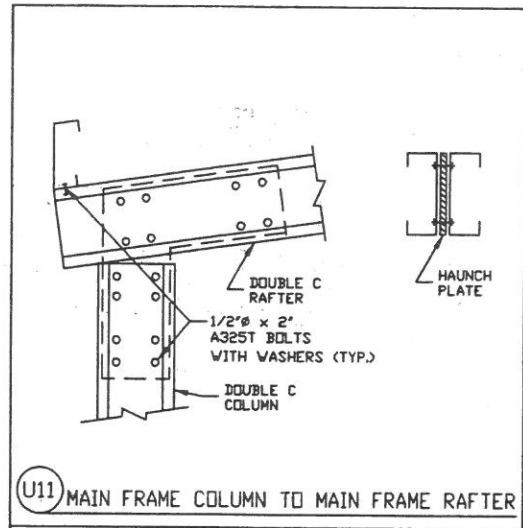


M1 HEADER TO EW COLUMN/DOOR JAMB CONNECTION

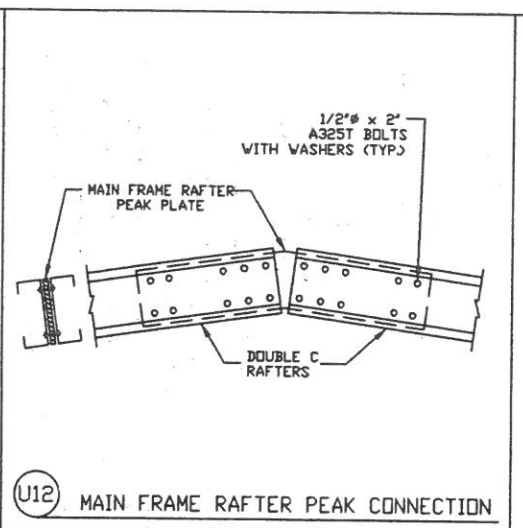


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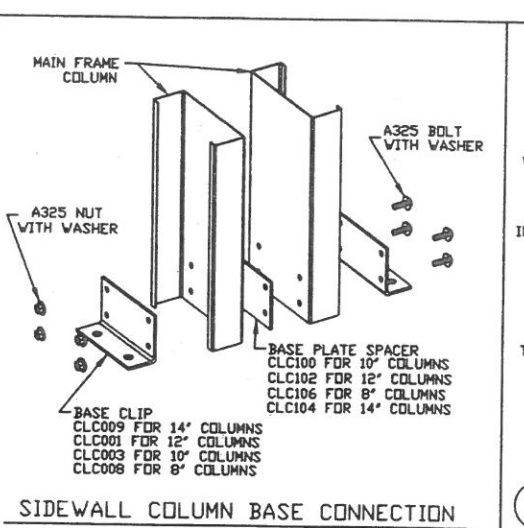
OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detaller: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
DETAIL DRAWINGS			Sht E10 of 11



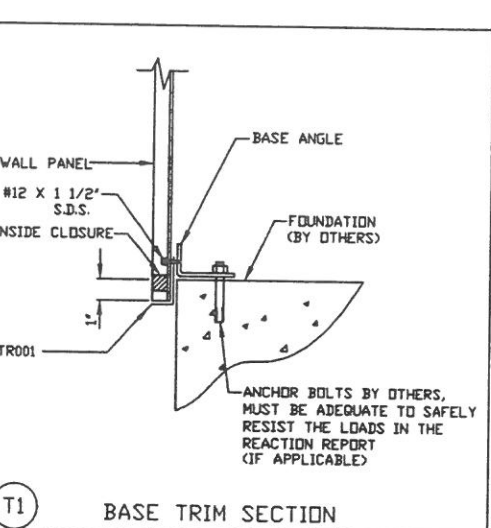
U11 MAIN FRAME COLUMN TO MAIN FRAME RAFTER



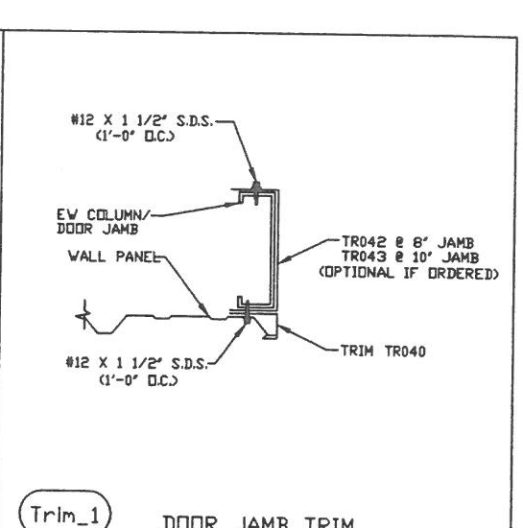
U12 MAIN FRAME RAFTER PEAK CONNECTION



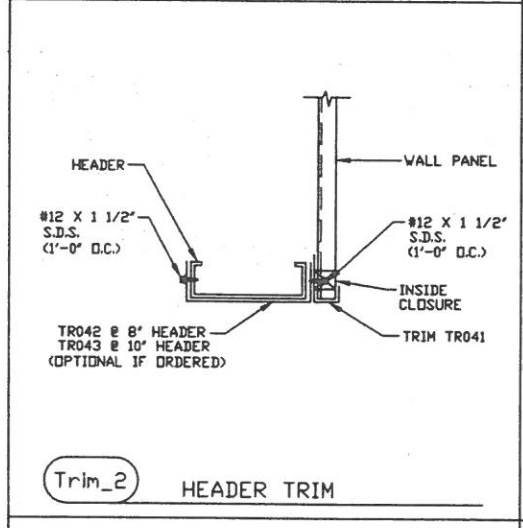
T1 SIDEWALL COLUMN BASE CONNECTION



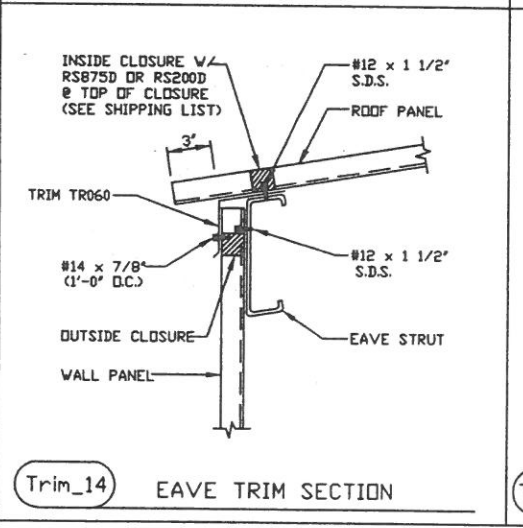
Trim_1 BASE TRIM SECTION



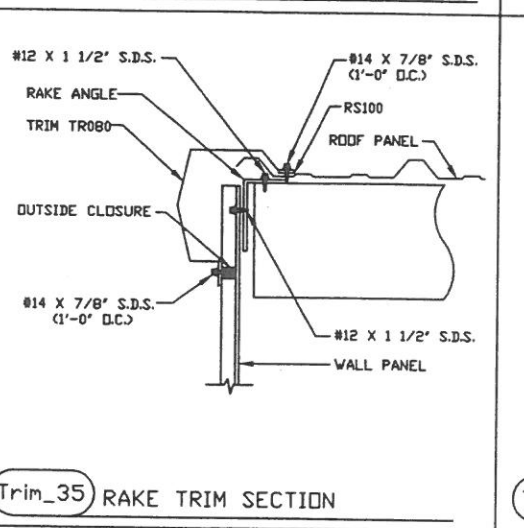
Trim_2 DOOR JAMB TRIM



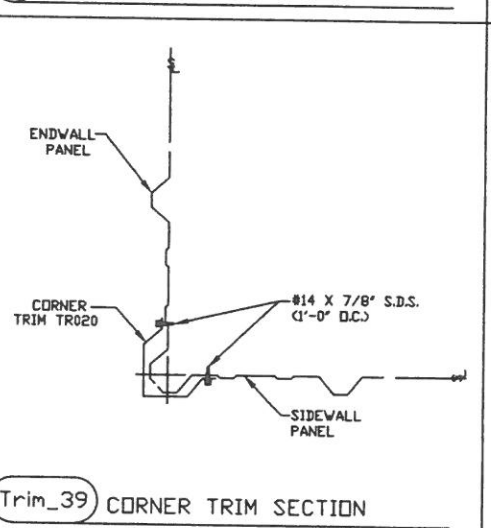
Trim_3 HEADER TRIM



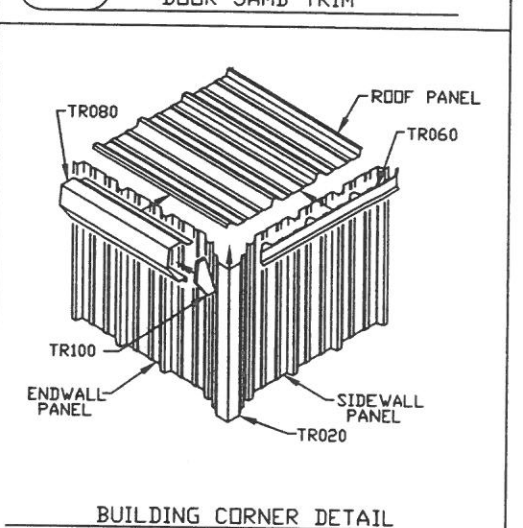
Trim_14 EAVE TRIM SECTION



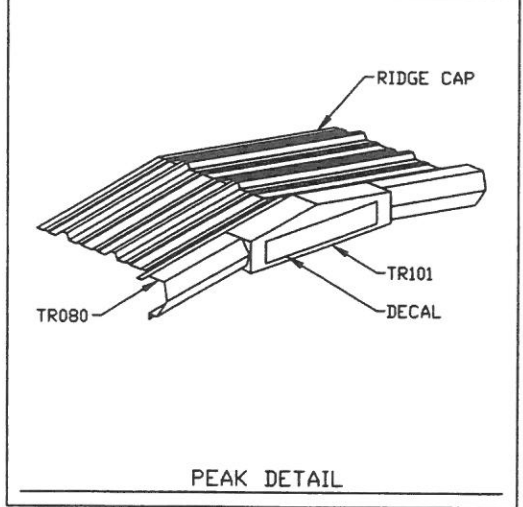
Trim_35 RAKE TRIM SECTION



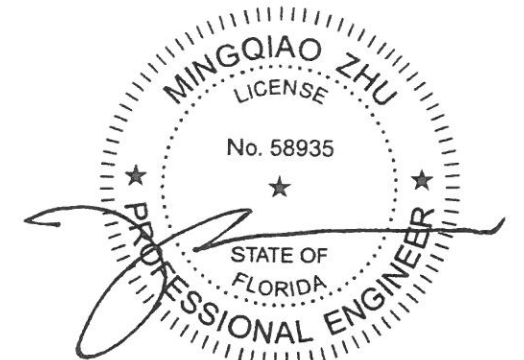
Trim_39 CORNER TRIM SECTION



PEAK DETAIL



PEAK DETAIL



03/05/2020

OLYMPIA STEEL BUILDINGS		Customer: CITY OF FORT WALTON BEACH	
MCKEES ROCKS PA 15136		FORT WALTON BEACH FL 32548	
Drafter: MO	Date: 3/3/20	Designer: MQZ	Date: 3/3/20
Detailer: RB	Date: 3/3/20	Sales ID:	Factory ID:
Checker: DP	Date: 3/3/20		005728
TRIM DRAWINGS			Sht E11 of 11