GEOTECHNICAL ENGINEERING REPORT

for

Army Navy Drive Complete Streets Project Arlington, Virginia

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24 April 2020 Revised 7 July 2020

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1.0 EXECUTIVE SUMMARY

This report presents the findings of our geotechnical engineering study for the proposed Army Navy Drive Complete Streets Project located in Arlington, Virginia. This executive summary does not represent a complete summary of our project understanding and recommendations. The report is one cohesive document and should be read in its entirety.

The studied portion of Army Navy Drive borders the west and north boundaries of Pentagon City and is aligned generally parallel with Interstate 395. Army Navy Drive starts at the corner of South Adams Street and 25th Street South and ends at the intersection of 12th Street South. The studied portion for the Complete Streets Project includes the north section of the Army Navy Drive roadway located between the intersections of South Joyce Street and 12th Street South. The project will consist of reconfiguring Army Navy Drive into a multimodal roadway that includes increased bicycle, pedestrian and public transit spaces and incorporates new stormwater management facilities.

Our geotechnical investigation consisted of drilling 14 borings at accessible roadway areas and performing laboratory testing of collected soil samples.

A summary of pertinent information and our recommendations further discussed in this report are as follows:

- Subsurface conditions within the studied alignment generally consist of a layer asphalt pavement overlying successive strata of fill and granular (sand and gravel) terrace deposits.
- Asphalt pavement was observed to range from about 5 to 11 inches thick. Concrete pavement
 was encountered below asphalt in 7 of 14 borings and ranged in thickness from about 7.5 to
 11 inches.
- Groundwater was first encountered in LB-3 at a depth of 13.5 feet below existing grade, or at approximate el 28.6. Upon completion, groundwater was observed at 15.1 feet below grade, or at approximate el 27. Groundwater was not encountered in any other borings performed for this study.
- Existing fill soil (Stratum A) are the pavement subgrade and are generally suitable for the support of new pavement areas associated with the improvements.
- Signal pole foundations can be designed as drilled shafts deriving their capacities from frictional and end-bearing resistance. Recommended axial capacities are given in Section 7.2. Lateral resistance can be developed along the length of the pier foundation. Lateral resistances should exclude the upper 5 feet of the pier to account for seasonal variations (shrink-swell and freeze-thaw) and future utility work. We recommend ultimate lateral resistance below this exclusion zone be designed using 2,000 and 2,800 pounds per linear foot for 3 and 4-foot-diemeter piers, respectively.

A complete summary of our field investigation and our geotechnical recommendations for the proposed Army Navy Drive Complete Streets Project are given in this report.

2.0 INTRODUCTION

This report presents the results of our geotechnical engineering study for the proposed Army Navy Drive Complete Streets Project in Arlington, Virginia. The purposes of this study were to:

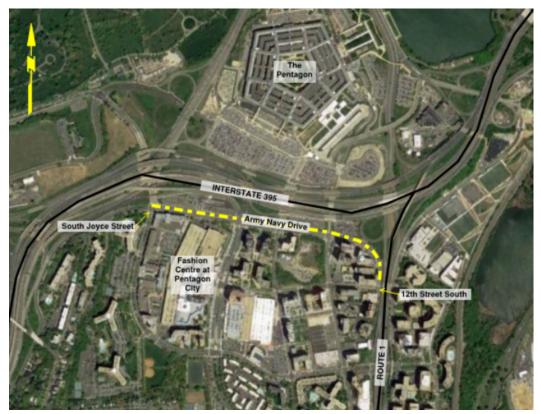
- 1) Research and review available site information;
- 2) Obtain subsurface and groundwater information by drilling borings within accessible roadway areas and completing specialty laboratory testing;
- 3) Provide recommendations for pavement design, signal pole foundation design, site preparation, site grading, and other geotechnical aspects of the proposed realignment;

Unless otherwise noted, all elevations given in this report are referenced to the North American Vertical Datum of 1988 (NAVD88). Surface elevations at investigation locations were determined by our surveyors during the investigation stakeout. Existing grades for the alignment beyond the investigation locations have not been provided at the time of this report. No environmental sampling or testing was completed as part of our investigation.

3.0 PROJECT DESCRIPTION

3.1 Existing Conditions

The Army Navy Drive Complete Streets project site is located in the Pentagon City neighborhood of Arlington County, Virginia. The west side of Army Navy Drive starts at the corner of South Adams Street and 25th Street South and ends on the east side at 12th Street South. The roadway alignment generally parallels US Interstate 395. The project study area consists of the north section of the Army Navy Drive located between the intersections of South Joyce Street and 12th Street South; see Figure 1 and Inset 1.



Inset 1

The studied portion of Army Navy Drive is a divided asphalt roadway with east and westbound lanes. Both the east and westbound lane consist of two-to-three lanes that are owned and operated by Arlington County. The studied portion of the road has intersections at South Joyce Street, South Hayes Street, South Fern Street, South Eads Street, and 12th Street South. The road provides access to three Pentagon parking lots located on the north side of the road. Arlington Transit bus stops are located along the westbound lane near the intersections of Joyce, Hayes, Fern, and Eads Street. Bus stops along the eastbound lane are located near the same intersections, except no bus stop is located near the Fern Street intersection. An underground WMATA tunnel for the blue and yellow lines is located beneath Hayes Street. Based on conversations with Arlington County, we understand that the existing roadway was milled and re-resurfaced in 2015.

3.2 Proposed Construction

According to the 6 November 2019 Army Navy Drive 30% Construction Drawings prepared by Arlington County, project consists of improvements to an approximate 4,200-foot-long section of Army Navy Drive between South Joyce Street and 12th Street South. The project will reconfigure Army Navy Drive into a multimodal roadway and provide increased accessibility for bicycle and pedestrian traffic, public transit, and establish more environmental (stormwater management) facilities including decreasing the impervious area along the corridor. Details of the proposed construction are as follows:

- <u>Protected bike lane</u> A new bicycle lane will be located along the eastbound side of Army Navy
 Drive and will consist of a two-way path separated from vehicle traffic.
- <u>Dedicated transit lanes</u> The curbside lanes in both directions between South Joyce Street and South Hayes Street will be re-purposed as a dedicated transit lane.
- Roadway improvements The roadway will be will be reconfigured to reduce the number of travel lanes and narrow vehicle travel lanes. Improved and shorter pedestrian crossings will be constructed in an effort to provide a more accessible pedestrian area.
- <u>Traffic signal installation</u> New traffic signals will be installed at intersections.

It is our understanding that a majority of existing medians, curbs, traffic signals, and pavement will be removed and replaced with the following:

- Mainline left turn lanes where they are currently missing
- Mainline protected left turn lane phasing
- Updated traffic signals at each intersection
- ADA accessible curb ramps
- Landscaping
- On-street tour bus parking

We understand that major utility re-routing is not planned at this time.

4.0 REVIEW OF AVAILABLE INFORMATION

We reviewed available historic maps, regional geologic information, and FEMA flood maps for the site and vicinity. Information obtained from these documents is summarized in the following sections:

4.1 Historic Maps

We reviewed the Washington West Quadrangle dated 1885 and the Military Map of Northern Virginia dated 1865 provided on the Arlington Virginia GIS system. The 1865 map indicates that the east portion of the project between South Eads and 12th Street South is aligned through the former Fort Runyon area. The 1885 map indicates that the former Chesapeake and Ohio Canal is aligned north-south and intersects Army Navy Drive between South Fern and South Eads Street.

We also reviewed the Alexandria County map dated 1900. This map indicates that a former stream crossed the roadway alignment between South Hayes and South Fern Streets. Former railways are noted on this plan that cross Army Navy Drive at two locations; the first between South Fern and South Eads Street (the approximate alignment of the former Chesapeake and Ohio Canal) and at the approximate center of the roadway turn between South Eads and 12th Street South.

4.2 Regional Geologic Map

We reviewed the 2006 Simplified Geologic Map of Arlington County, Virginia, and Vicinity prepared by the United States Geologic Survey; see Figure 3. Based on this map, the surficial geology at the site area and generally consists of terrace deposits, alluvium, and artificial fill. Terrace deposits generally consist of gray brown to medium orange gravel, sand, silt, and clay. These soils are generally found below el 50 and the thickness of this stratum may be up to 35 feet. Alluvium consists of river deposited soils that are similar to the terrace deposits but are typically less weathered and contains well-rounded pebbles and cobbles.

We also reviewed the Soils Map for Arlington County, Virginia on the Arlington County GIS system. This map indicates the surficial soil along the subject alignment is expected to consist of Udorthents Complex soils and Urban Land made from Udorthents Complex.

4.3 FEMA Flood Map

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM), Panel Number 51013C0081C, dated 19 August 2013, the site is located outside of the mapped 100-year and 500-year floodplains; see Figure 4.

4.4 Arlington County Traffic Counts

Various sources for traffic counts in Average Daily Traffic (ADT) and Annual Average Daily Traffic (AADT) were provided by Arlington County for specific sections of Army Navy Drive and are summarized in Table 1. Original traffic count data is given in Appendix A. Our interpretation of the information given below is summarized in Section 7.1.

Average Daily Traffic

From	To	ADT¹ from Traffic	VDOT Counts					
From	То	Counts	Current AADT ²	2020 AADT	2040 AADT			
S. Joyce Street	Pentagon City Mall Garage Access	11,713³	14,061	16,369	17,234			
Pentagon City Mall Garage Access	S. Hayes Street	11,/13°	20,202	21,234	21,455			
S. Hayes Street	S. Fern Street	S. Fern Street 19,913 ⁴		27,334	27,714			
S. Fern Street	S. Eads Street	_	17,029	19,744	19,665			
S. Eads Street	12 th Street S.	8,158⁵	6,572	7,068	7,620			

Table 1

Notes:

- 1. ADT = Average Daily Traffic performed by third-party firm, Quality Counts, LLC
- 2. AADT = Annual Average Daily Traffic from Virginia Department of Transportation (VDOT) provided by Arlington County
- 3. ADT shown is from South Joyce Street to South Hayes Street from November 2019 traffic count data
- 4. ADT from October 2014 traffic count data
- 5. ADT from October 2019 traffic count data
- 6. " " = Traffic Count Not Provided

5.0 SUBSURFACE INVESTIGATION

Our geotechnical investigation consisted of drilling borings and conducting laboratory tests on collected soil samples. Permits for work within the right-of-way were obtained from Arlington County before starting the work.

A Miss Utility ticket was completed before the start of the fieldwork. Our team noticed multiple utilities on the Arlington County plans that were not marked by the Miss Utility ticket and therefore each boring location was investigated for detectable utilities by our private utility markout firm prior to performing the investigation within the right-of-way.

5.1 Borings

Fourteen borings, identified as LB-1 through LB-14 were drilled by Free State Drilling between 2 and 6 April 2020, see Figure 2 for locations. Surface elevations at the boring locations were obtained from our field surveyors who staked out the investigation locations under the direction of our engineer.

The borings were drilled using a CME-55 track-mounted drill rig. Before drilling, the asphalt pavement at each boring location was cored using a 6-inch-diameter bit. General pavement core conditions were observed and recorded by our field engineer and photo-documented. The borings were advanced through the soil to depths of 10 feet (for pavement borings) and 25 feet (for traffic signal borings LB-3, LB-6, LB-8 and LB-10) below existing grade using hollow-stem augers. A standard 2-inch-outer-diameter split-spoon sampler was used to obtain samples of the underlying soil strata. The Standard Penetration Test (SPT)¹ was accomplished as part of the sampling procedure (in accordance with ASTM D1586) and the results were recorded by our inspecting engineer. An automatic hammer was used as part of the SPT test for all sampling in the borings. Our field engineer used a field pocket penetrometer to measure unconfined compressive strengths of disturbed cohesive soil samples.

Groundwater levels were recorded when first encountered and upon completion of each boring. All boreholes were backfilled with soil cuttings and surface patched with cold-patch asphalt upon completion.

The borings were completed under the full-time observation of an engineer from our office. Our engineer maintained logs of all explorations, classified encountered soil, and obtained representative material samples.

The individual boring logs are given in Appendix B. Pavement core photographs are given in Appendix C.

5.2 Laboratory Testing

During our investigation, soil samples were visually examined in the field, and classifications were confirmed by re-examination after completion of the investigation. Select samples were sent to a specialty testing laboratory where the following tests were performed:

- Grain Size Analysis (ASTM D422)
- Grain Size Analysis with Hydrometer (ASTM D422)
- Fines Content (ASTM D1140)

¹ The Standard Penetration Test (SPT) is a measure of the soil density and consistency. The SPT N-value is defined as the number of blows required to drive a 2-inch O.D. split-barrel sampler 12 inches, after an initial penetration of 6 inches using a 140 pound hammer falling freely for 30 inches.

- Water Content (ASTM D2216)
- Atterberg Limits (ASTM D-4318)
- California Bearing Ratio (CBR) Test (VTM-8)
- Standard Proctor Test (VTM-1)

The results of the laboratory testing are discussed together with the soil strata descriptions in the following sections. The laboratory test results are given in Appendix D.

6.0 SUBSURFACE CONDITIONS

The subsurface conditions at the site generally consist of a layer pavement overlying successive strata of fill and terrace deposits. The following sections describe the encountered subsurface conditions.

Since the automatic hammer was used in conjunction with the Standard Penetration Testing in the borings, the SPT N-values presented in the text of the following strata description sections are corrected (N_{60}) SPT N-values. SPT N-values presented in the boring logs are uncorrected N-values.

6.1 Pavement

Asphalt pavement was encountered at the surface of all boring locations. The asphalt ranged from about 5 to 14.5 inches thick and generally consisted of one or more surface course layers and a base course. One or more layers of concrete were encountered at boring locations LB-1, LB-2, and LB-5 through LB-9 below the asphalt. The concrete ranged from about 5 to 11 inches thick.

A summary of existing pavement thicknesses is given in Table 2 and photographs of the individual cores are given in Appendix C. Gravel subbase of variable thickness was encountered below the pavement in all borings.

Existing Pavement Summary

	Roadway	Asp	Asphalt Thickness (inches)				
Boring	Station (Approx.)	Surface Course ¹	Base Course ¹	Total Asphalt Thickness	Thickness (inches)		
LB-1	135+86	3	2	5	13		
LB-2	138+89	3.5	NE	3.5	9		
LB-3	140+50	5	8	13	NE		
LB-4	142+87	3	6.5	9.5	NE		
LB-5	145+12	2	3.5	5.5	7.5		
LB-6	147+44	2	3	5	9.5		
LB-7	150+37	3	3.5	3.5	8.5		
LB-8	152+82	3.5	2.5	6	11		
LB-9	156+17	2	3	5	9		
LB-10	159+52	1	4	5	NE		
LB-11	159+71	5	7	12	NE		
LB-12	3+50	3	7	10	NE		
LB-13	6+35	3	11.5	14.5	NE		
LB-14	8+71	3	9	12	NE		

Table 2

Notes:

- 1. Surface and Base course thickness is based on general observations of the asphalt core and observed aggregate size.
- NE = Not Encountered
- 3. A detailed breakdown of asphalt and concrete core observations is given in the boring logs in Appendix B and in the Photographic Logs in Appendix C.

6.2 Stratum A - Fill

An approximate 0.5 to 12.2-foot-thick layer of fill was encountered beneath the pavement and subbase in all borings. This layer typically consisted of brown and gray sand, clayey sand, clay, sandy silt, and sandy gravel (typical USCS classifications of SW, SP, SC, CL, SM, and GW). Varying amounts of brick and asphalt and trace amounts of mica, cinders, concrete and wood were also present within the fill. The fill extended to depths ranging from approximately 3 to 13.5 feet below existing grade, or el 27.8 to 35.2. Borings LB-1, LB-2, and LB-5 were terminated in this stratum

Thick zones of debris were encountered in some borings that consisted of:

- Approximate 1 to 16-inch-thick layers of brick were encountered in borings LB-7, LB-8, and LB-9 between 3 and 7 feet below grade, or at about el 32.2 to 36.7.
- Approximate 1 to 11-inch-thick layers of buried asphalt were encountered in borings LB-7, LB-12, and LB-13 between 3.5 and 6.7 feet below grade, or at about el 28.4 to 39.9.
- Six inches of concrete was encountered 2.5 feet below grade or at el 28.5 in LB-14.

Stratum A was observed to be very loose to very dense as evidenced by SPT N_{60} -values ranging from 1 to 52 blows/foot (average SPT N_{60} -value of 17 blows/foot). Higher blow counts are likely attributed to gravel and cobbles-sized obstructions (rock, brick, and asphalt) within the fill. Laboratory Test results from Stratum A are given in Table 3.

Test	Minimum	Maximum	Average
Moisture Content (%)	4.5	21.3	13.6
Gravel Content (%)	5.2	44.7	17.2
Sand Content (%)	31	63.8	47.2
Fines Content (%)	24.2	51.3	35.6
Liquid Limit (%) ¹	29	36	-
Plastic Limit (%) ¹	17	24	-
Plasticity Index (%) ¹	12	12	-
Optimum Moisture Content (%) ²	7.5	13.7	10
Maximum Dry Unit Weight (pcf) ²	117.6	134.9	125.8
California Bearing Ratio ³	6.6	58	27.8

Table 3

Notes:

- 1. Two samples were tested from this stratum; therefore, an average value is not given.
- Standard proctor compaction testing was performed in accordance with Virginia Department of Transportation (VDOT) testing standard VTM-1.
- 3. California Bearing Ratio was performed in accordance with VDOT testing standard VTM-8.

6.3 Stratum B – Terrace Deposits

Stratum B was encountered beneath Stratum A and consisted of primarily of sand and gravel. This stratum was observed to extend to the termination depth of all borings that did not terminate in Stratum A.

The sand portion of this stratum generally consisted of light brown, brown, and red-brown fine to coarse sand with varying amounts of silt and clay and trace amounts of fine gravel (typical USCS classifications of SW, SP, SC, and SM). The sand portion was observed to be very loose to medium dense as evidenced by SPT N₆₀-values ranging from 2 to 21 blows/foot (average of 10 blows/foot).

Fine gravel (typical USCS classification of GW) was encountered beneath the sand near the termination depth of LB-3, LB-8, and LB-10, at 23.5 feet below existing grade, or at about el 15.7 to 18.6. The gravel was observed to be dense as evidenced by SPT N_{60} -values of 30, 38, and 44 blows/foot. The thickness of this gravel layer is not determined.

Laboratory test results for Stratum B are given in Table 4.

Stratum B Laboratory Test Results

Test	Minimum	Maximum	Average				
Moisture Content (%)	4	27	14.7				
Gravel Content (%)	0.2	56.5	19.6				
Sand Content (%)	32.4	92.6	54.2				
Fines Content (%)	5.1	62.1	38				
Liquid Limit (%) ¹		28					
Plastic Limit (%) ¹		19					
Plasticity Index (%) ¹		9					

Table 4

Notes:

1. One sample was tested from this stratum; therefore one value is reported.

6.4 Groundwater

Groundwater encountered in LB-3 at a depth of 13.5 feet below existing grade, or at approximate el 28.6, as inferred from the moisture content of the soil samples obtained in the split spoon sampler. Groundwater was measured in boring LB-3 at 15.1 feet below existing grade, or at approximate el 27 upon completion of drilling within the augers.

Groundwater was not encountered in any other borings performed at the site.

7.0 DESIGN RECOMMENDATIONS

Our geotechnical design recommendations for pavement are given in the following sections.

7.1 Asphalt Pavement Design

Army Navy Drive will be reconfigured to include a new bike lane, designated bus lanes, full-depth pavement areas, and landscaped areas. Our recommendations for pavement design are given in the following sections.

7.1.1 Army Navy Drive Full-Depth Pavement Design

Portions of the new roadway alignment will include new full-depth pavement. As such, we have designed asphalt pavement section for the proposed roadway improvements following the August 2018, Virginia Department of Transportation Materials Division Guidelines for 1993 AASHTO Pavement Design. Our pavement recommendations given herein are contingent upon the construction recommendations given in Section 8 of this report.

We anticipate that Stratum A soil will be encountered at the proposed pavement subgrades. California Bearing Ratio (CBR) values within Stratum A ranged from about 6.6 to 58 indicating poor to good subgrade material. Given the wide range of CBR values, we recommend using different design CBRs along the roadway to account for subgrade variability. A summary of our full-depth pavement design assumptions are given below and in Table 5.

Initial Serviceability:4.2Terminal Serviceability:2.8Design Life:30 yearsReliability:90 percentStandard Deviation:0.49

Pavement Design Recommendations

Roadway Section	Average Daily Traffic (2050) ¹	Vehicula (Million	r Loading ESALs) ¹	Lane Distribution Factor	Design CBR	
		Typical	Bus Lane	1 40101		
Joyce to Hayes	21,555	5.590	24.597	0.9 (2-lanes) 1.0 (Bus Lane)	7	
Hayes to Fern	27,914	23.740	-	0.9 (2-lanes)	20	
Fern to Eads	19,744	10.465	-	0.9 (2-lanes)	20	
Eads to 12 th Street	9,141	5.381	-	0.9 (2-lanes)	10	

Table 5

Notes:

- Average Daily Traffic (ADT) was determined based on traffic counts provided by Arlington County and applying a growth factor based on the AADT values determined by VDOT; see Appendix A.
- 2. In calculating the value of ESALs, we assumed future quantities of cars, single unit trucks, and tractor-trailers to be consistent with current traffic count percentages.

Given the design assumptions listed above, our recommendations for new full depth pavement design of the Army Navy Drive improvements are given in Table 6.

Vehicular Pavement Design Recommendations

Material	Minimum Pavement Design Thickness							
iviateriai	Typical Lanes	Bus Lanes						
Bituminous Surface Course	2 inches	2 inches						
Bituminous Base Course	8 inches	10 inches						
Aggregate Base Course ^{1,2}	6 inches typically; 8 inches when adjacent to bus lane	6 inches						

Table 6

Notes:

- 1. Aggregate base course should consist of VDOT 21B aggregate connected to a standard UD-4 edgedrain.
- 2. The bottom of aggregate base course should match between lanes, therefore a thicker section is recommended for typical lanes adjacent to bus lanes.

Flexible pavement sections should follow the Arlington County guidelines outlined in Section 02600 of the Construction Standards and Specifications Manual. In conformance with these guidelines, the following sections of the Virginia Department of Transportation (VDOT) Road and Bridge Specifications should be used to guide asphalt material selection, mix design, and construction execution:

- A tack coat consisting of asphalt cement of viscosity grade CM-2 or CRS-2 in conformance with VDOT Section 210 should be applied between asphalt surfaces and each asphalt surface placed thereafter in conformance with VDOT Sections 310 and 315.
- Bituminous surface course should consist of Type SM-12.5D (formally Marshall VDOT Mix Design Type SM-2) in conformance with VDOT Section 211.
- Bituminous base course should consist of Type BM-25A (formally Marshall VDOT Mix Design Type BM-2) in conformance with VDOT Section 211.
- Application of asphalt concrete should conform to VDOT Section 315 that states pavement courses shall not be placed in lifts exceeding 4 times the nominal maximum size aggregate and designates a minimum lift thickness of 2.5 times the nominal maximum aggregate size.

7.1.2 Bicycle Lane Pavement

Bicycle lane pavement sections are typically designed in accordance with the Arlington County Construction Standards and Specifications Manual Section 02601 which assumes vehicular traffic loading is prohibited. However, we anticipate that bike lanes will be subject to occasional vehicular traffic and the minimum pavement section shown in Table 7 per Arlington County Standard R-1.4 should be used.

Bike Lane Pavement Design Recommendations

Material	Minimum Thickness
Bituminous Surface Course	2 inches
Bituminous Base Course	6 inches
Aggregate Base Course	6 inches

Table 7

7.2 Signal and Light Pole Foundation Design

We understand that new signal and light poles will be installed as part of the project. Signal and light pole foundations should be designed in accordance with Virginia Department of Transportation standards.

Very loose soils associated with the fill (Stratum A) were encountered in borings LB-3, LB-8, and LB-10. In addition, brick and deleterious material was encountered in boring LB-8 between approximately 5 to 5.5 feet below-grade. We recommend that signal and light pole foundations bear below loose soils and extend to a minimum depth of 10 feet below grade. The pole foundations should consist of properly reinforced concrete piers deriving their capacity from frictional and end-bearing resistance in soil. We recommend that the piers be designed as 3- or 4-foot-diameter shafts with a 28-day concrete compressive strength of 4,000. The allowable geotechnical axial capacities for the piers is given in Table 8.

Diameter (feet)	Pier Length (feet)	Allowable Axial Capacity (kips)
3	10	20
3	15	30
4	10	35
4	15	45

Table 8

Notes:

- 1. An allowable end-bearing capacity of 2,000 psf was used for design.
- 2. Allowable capacities include a factor of safety of 2 for frictional resistance and 3 for end bearing.
- 3. Frictional resistance has been excluded in the upper 5 feet of the pier.
- 4. Pier depth can be increased if additional axial capacity is required. We recommend that an allowable unit friction capacity of 200 psf be used for design.

Wing heights and lengths for the shafts and reinforcement should be designed by the project structural engineer to resist torsional forces induced on the signal pole foundation.

Passive lateral resistance can be developed along the depth of the pier foundation; however, resistance should be excluded in the upper 5 feet of each pier to account for seasonal variations (shrink-swell and freeze-thaw). The ultimate lateral resistance per foot below the 5 foot exclusion zone is given below. The structural engineer should apply proper safety factors or resistance factors to the lateral resistance parameters per applicable building codes or VDOT standards:

- 3-foot diameter 2,000 pounds per linear foot (of pier embedment below 5 feet)
- 4-foot diameter 2,800 pounds per linear foot (of pier embedment below 5 feet)

8.0 DESIGN RECOMMENDATIONS

8.1 Site Preparation

Prior to commencement of grading or fill placement, any miscellaneous trash, debris, or other unsuitable materials should be removed from the site. In existing vegetated areas, clearing and grubbing of all trees (including removal of any associated root systems) and vegetation designated for removal should be performed. All debris and trees/vegetation should be properly disposed off-site in accordance with applicable Federal, Virginia, and Arlington County regulations.

All site clearing work should be performed in accordance with any environmental regulations and requirements established for the project. In addition, all construction work should be performed so as not to adversely impact the neighboring buildings, the WMATA tunnel located below South Hayes Street, the existing access drive east of the realignment site, or off-site structures or utilities. Protection and monitoring of these elements should be provided as necessary during the curse of all construction activities at the site.

Topsoil and associated root-mat and organic material should be stripped from the proposed pavement areas. The reuse of the topsoil should be evaluated by a qualified Landscape Architect concerning nutrient levels, grain size, pH, etc. Topsoil deemed unsuitable for reuse should be properly disposed of off-site. All clearing activities should be performed in strict accordance with the approved soil erosion and sediment control plan prepared for the project.

Existing concrete pavement at the surface denoted for demolition should be completely removed. Concrete placement beneath the asphalt within the roadway should also be demolished, however, this concrete can alternatively be left in place if the Contractor verifies proper subbase thickness is present beneath the concrete. Demolished concrete should be removed from the site and properly disposed.

All utilities designated for abandonment must be either removed or completely filled using flowable fill or grout. Excavations made to remove utilities below proposed roadway subgrade elevations should be backfilled with approved compacted fill as discussed in Section 8.3 of this report.

8.2 Subgrade Preparation Procedures

Exposed subgrade soils should be scarified to a minimum depth of 6 inches at least 3 feet beyond the proposed edges of the pavement and compacted per VDOT standards after performing the aforementioned site preparation work. This should be completed prior to raising grades or placing subbase. The subgrade should be compacted to 100 percent of the maximum dry density as determined by VTM-1 within 2 percentage points of the optimum moisture content. Following density testing, the subgrade should be proofrolled with at least two passes of a fully loaded 10-wheel dump truck prior to placing fill to at least 3 feet beyond the outer edge of the shoulders. The proofroll should be completed in overlapping manner for the entire roadway width.

All proofroll activities and compaction testing should be inspected by a qualified geotechnical engineer certified by VDOT. Additional proofrolling coverages should be performed in any areas deemed necessary based on observations made by the geotechnical inspector. Soft subgrade areas identified during proofrolling should be undercut and replaced with VDOT Type I select material having a minimum CBR of 30.

Care should be taken to prevent disturbance of the proofrolled areas and softening of these materials prior to finished construction. At a minimum, all subgrade areas should be temporarily sloped and sealed with a smooth drum roller at the end of each working day, as necessary, so as to maximize surface water runoff, and minimize potential ponding and infiltration.

8.3 Engineered Fill

8.3.1 Fill Designation

Structural fill should be used to backfill beneath proposed pavements and bike lanes as required. We recommend that structural fill have no more than 30 percent fines (silt and clay), a liquid limit not more than 35, and a plasticity index not more than 10. Structural fill should be classified as GW, GP, GM, SW, SP, SM, or a combination of these in accordance with USCS classifications. Structural fill should not contain man-made debris or organics. The maximum particle size should not exceed 3 inches.

Subgrade areas determined by the geotechnical inspector to be unsuitable should be over-excavated and replaced with VDOT Type I select material.

Aggregate subbase for proposed new full-depth vehicular pavement areas and the bicycle path should meet the gradation of VDOT 21B dense-graded aggregate.

Drainage fill (if required) for use as subsurface drainage should consist of washed AASHTO No. 57 stone. The drainage fill should be separated from existing site soils using drainage geotextile fabric.

Non-structural fill should be only be used in landscaped areas of the site (planter areas). Non-structural fill should not have particles exceeding 3 inches within the uppermost foot of final grade and 6 inches below. Soils classified as CH, MH, OL, and PT in accordance with USCS should not be used at the site as backfill.

Grain size distribution, Atterberg Limits, maximum dry density, and the optimum water content determinations should be made on representative samples of the fill materials (on-site and imported) proposed by the Contractor. Additional subbase material testing should include standard proctor and CBR testing in accordance with VDOT standards VTM-1 and VTM-8, respectively.

8.3.2 Reuse of Existing On-site Soils

Existing on-site soil meeting the requirements of structural fill given in Section 8.3.1 can be reused as compacted fill to raise grades within pavement subgrade areas. Any deleterious materials should be separated from the fill prior to use. Stratum A soils not meeting the designation for structural fill can be reused in landscaped areas.

The Contractor's ability to successfully work the site soils, combined with the weather conditions and the time of year during the site preparation and filling phases of construction, will have a significant impact on timely project completion.

During periods of wet weather, the Contractor should make provisions to dry portions of the excavated material such as by discing/air drying prior to compaction to an acceptable moisture content as determined by the project Geotechnical Engineer.

Soils classified as expansive should not be used as backfill material at the site. Expansive soils are defined as soils with the following properties: a plasticity index of 15 or greater per ASTM D4318, greater than 10 percent finer than the No. 200 sieve, and more than 10 percent of soil particles less than five micrometers in size per ASTM D 422. Additionally, soils with an expansion index greater than 20 per ASTM D 4829 are

classified as expansive. The geotechnical engineer observing the construction should regularly test the Stratum A soils if they are to be used during site grading operations to confirm if this criteria is met.

Any encountered environmentally impacted soils should be addressed in accordance with the environmental requirements established for the site. If any environmentally impacted soils are encountered, the Contractor should immediately notify Arlington County. Further coordination with the Contractor and Arlington County will be necessary to finalize recommendations regarding off-site disposal or on-site reuse of any environmentally impacted on-site soils.

8.3.3 Imported Fill

Imported fill to be used as subgrade soils beneath pavements should meet the structural fill requirements given in Section 8.3.1. Imported fill to be used as roadway subbase should meet the VDOT requirements described in Section 8.3.1. Suitable fill should be free of organics and other deleterious materials. Imported fill intended for use as a roadway subbase should meet the maximum aggregate size of the required VDOT gradation.

Any approved imported fill should be "certified clean" free of hazardous substances and meeting all Arlington County, Virginia, and Federal regulations. The Contractor should provide documentation of compliance prior to delivery of any fill to the site. Grain size distribution, Atterberg limits, maximum dry density (determined using VTM-1), and the optimum water content determinations should be made on representative samples of the backfill and fill materials proposed by the Contractor.

8.3.4 Fill Placement and Compaction

All fill should be placed in uniform lifts and compacted to at least 100 percent of the material's maximum dry density as determined by the Standard Proctor Compaction Test (VTM-1). The water content at the time of compaction should be within two percentage points of the optimum water content. For use as pavement subgrades, on-site granular soils and imported select fill can be placed in maximum 8-inch-thick loose lifts. Aggregate subbase materials should be placed in maximum 6-inch-thick lifts in accordance with VDOT standards.

Each lift should compacted using a roller having a minimum static drum weight of 15 tons. Typically, sand and silt soils should be compacted with a smooth drum roller and clay soils (if encountered) should be compacted using a sheeps-foot roller. Smaller compaction equipment (i.e. walk-behind trench roller or jumping jack compactor) may be necessary in areas of limited maneuverability. All fill placement should be subject to inspection and testing by a qualified geotechnical engineer.

No fill material should be placed on areas where free water is standing, on a subgrade covered with snow, on frozen subgrade areas, or on surfaces that have not been approved by the geotechnical testing agency.

Fill placed in landscaped areas away from retaining walls and engineered slopes should be compacted to at least 92 percent of the material's maximum dry density as determined by VTM-1.

24 April 2020 Revised 7 July 2020 Page 18 of 18

9.0 CLOSURE/LIMITATIONS

The conclusions and recommendations given in this report are based on subsurface conditions inferred from our investigation and our review of the available design documents and traffic studies provided by Arlington County. The recommendations given herein are contingent upon one another and no recommendation should be followed independent of the others. This report has been prepared to assist Arlington County for their design of the Army Navy Drive Complete Streets project and is only applicable to the design and construction of the specific project identified. Changes to proposed roadway layout, grading, or traffic loading should be provided to us so that we can review our recommendations and modify if necessary. Langan Engineering and Environmental Services, Inc. cannot assume responsibility for the use of this report to generate recommendations other than for the specific site and structures addressed in this report.

Information on subsurface strata and groundwater levels shown on the logs represent conditions encountered only at the locations indicated and at the time of investigation. Actual subsurface conditions may vary. If different conditions are encountered during construction, they should immediately be brought to Langan's attention for evaluation, as they may affect our recommendations.

Environmental issues (such as permitting or potentially contaminated soil and groundwater) are outside the scope of this study and, if required, should be addressed in a separate evaluation.

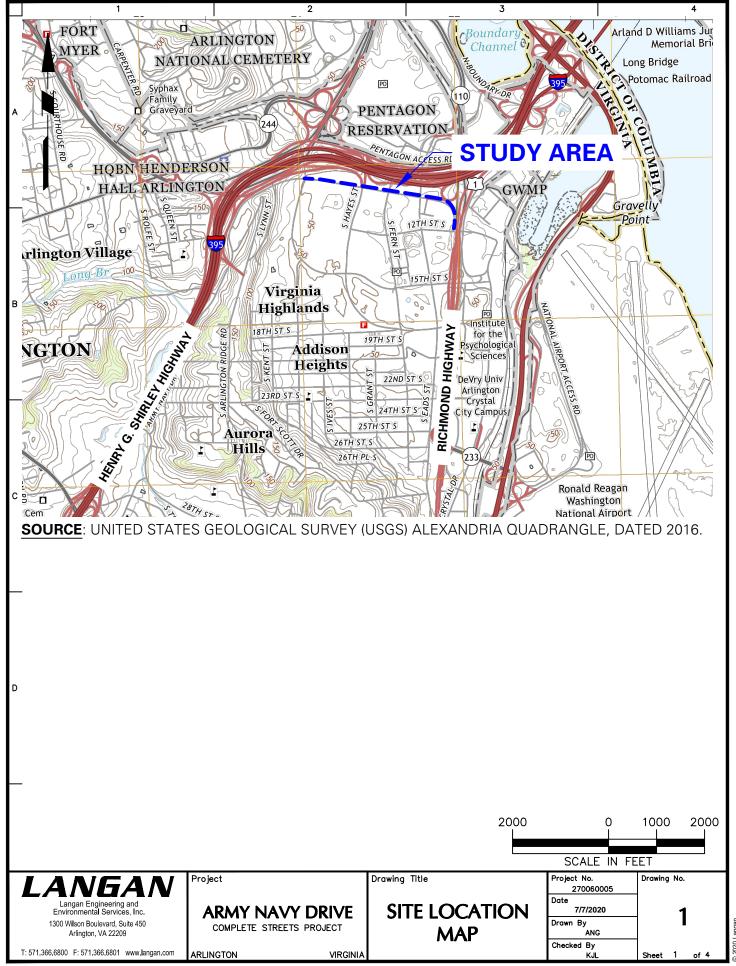
FIGURES

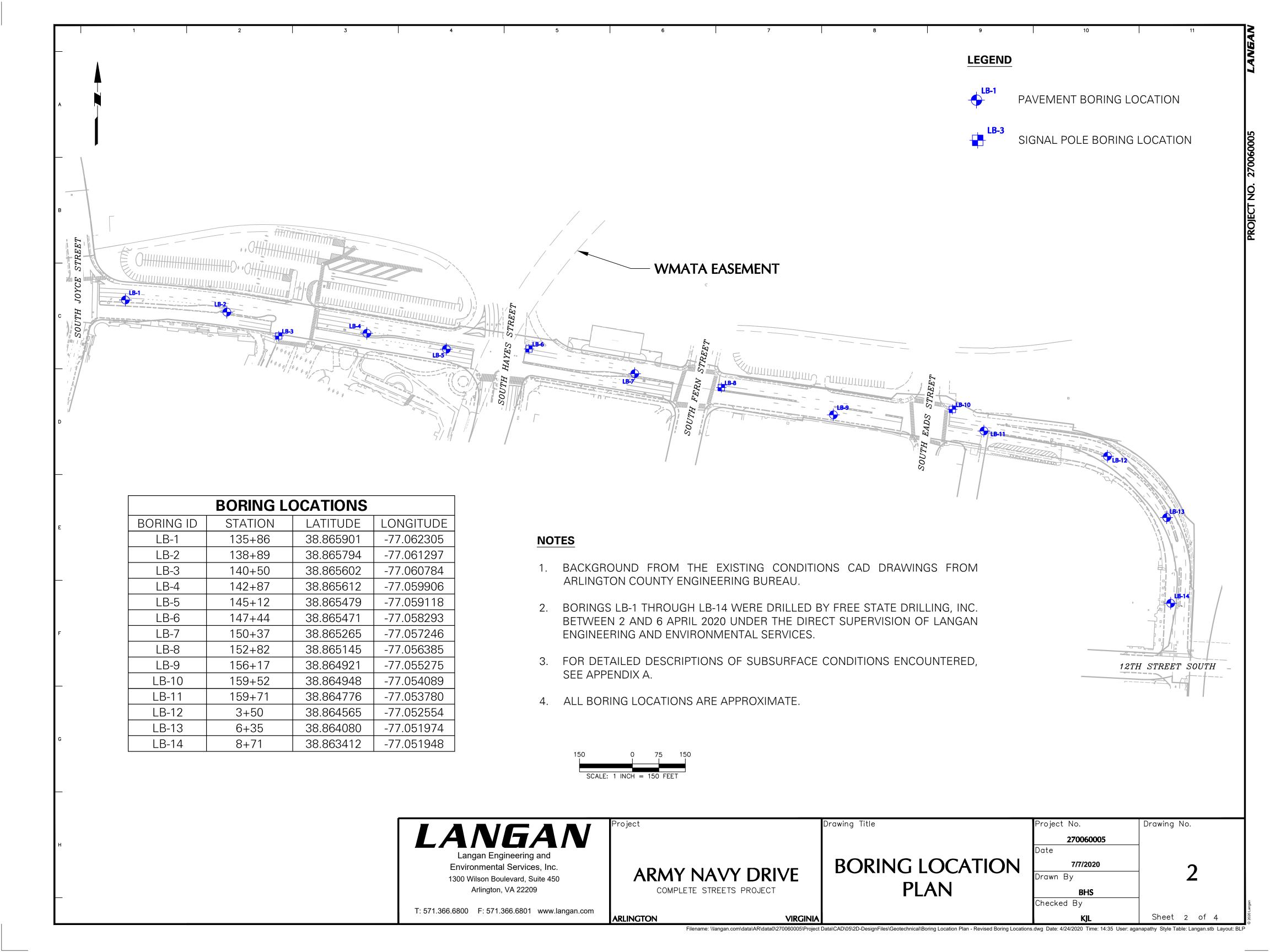
Figure 1 Site Location Map

Figure 2 Boring Location Plan

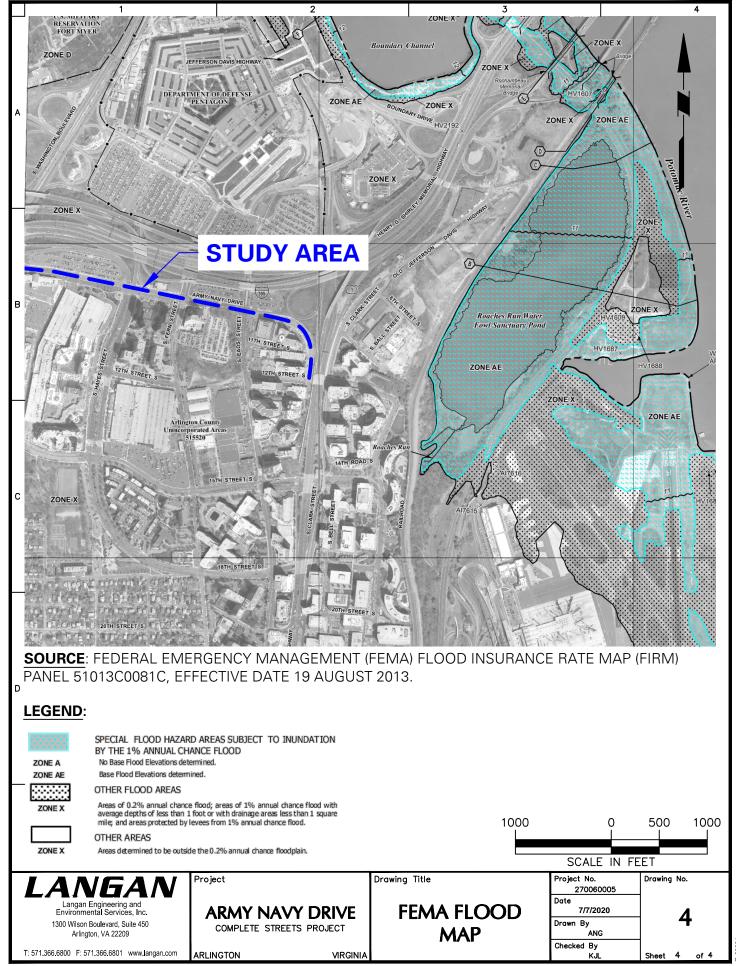
Figure 3 Regional Geology Map

Figure 4 FEMA Flood Map









Appendix A

Provided Traffic Count Data

SPECIFIC LOCATION:

Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:00 AM	0	9	2	0	0	0	0	0	0	1	0	0	0	3	15
12:15 AM	1	15	2	0	0	0	0	0	0	0	0	0	0	0	18
12:30 AM	0	13	1	2	0	0	0	0	0	0	0	0	0	0	16
12:45 AM	0	10	3	1	0	0	0	0	0	0	0	0	0	1	15
01:00 AM	0	5	3	1	5	1	0	0	0	0	0	0	0	0	15
01:15 AM	0	7	0	0	2	0	0	0	0	0	0	0	0	0	9
01:30 AM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
01:45 AM	0	4	2	0	0	0	0	0	0	0	0	0	0	0	6
02:00 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
02:15 AM	0	4	2	0	0	0	0	0	0	0	0	0	0	0	6
02:30 AM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
02:45 AM	0	5	0	1	0	0	0	0	0	0	0	0	0	0	6
03:00 AM	0	10	2	0	0	0	0	0	0	0	0	0	0	0	12
03:15 AM	0	13	0	0	0	0	0	0	0	0	0	0	0	0	13
03:30 AM	0	21	2	0	0	0	0	0	1	0	0	0	0	1	25
03:45 AM	0	23	7	0	0	0	0	0	0	0	0	0	0	0	30
04:00 AM	0	19	6	2	0	0	0	0	1	0	0	0	0	2	30
04:15 AM	0	27	7	1	0	0	0	0	0	0	0	1	0	0	36
04:30 AM	0	28	4	2	2	0	0	1	2	0	0	0	0	0	39
04:45 AM	0	32	6	1	0	0	0	0	1	0	0	0	0	0	40
05:00 AM	0	38	4	1	0	0	0	0	0	0	0	0	0	1	44
05:15 AM	1	66	10	4	2	1	0	2	0	0	0	0	0	1	87
05:30 AM	1	54	4	4	1	0	0	1	1	0	0	0	0	2	68
05:45 AM	0	64	15	5	2	1	0	0	0	0	0	0	0	2	89
Day Total Percent				DATA	THA	TDI	DIV/E	SCC	1111	דוואו ז	TIES				
ADT 11715				JAJA		<u> </u>	VIVE	3.00	IVIIVI	UNII	163				
AM Peak 15-min Vol PM Peak															
15-min Vol															

SPECIFIC LOCATION: CITY/STATE: Arlington VA

CITY/STATE: A	Arlington, VA													DATE: I	Nov 6 2019
Start Time	Motorcycles	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Not	Total
Start Time	iviotorcycles	Trailer	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classified	Total
06:00 AM	0	55	11	6	3	0	0	0	2	0	0	0	0	3	80
06:15 AM	2	74	17	6	6	1	0	0	0	0	0	0	1	5	112
06:30 AM	1	89	11	10	6	0	0	3	0	0	0	0	0	9	129
06:45 AM	2	104	13	11	4	1	0	1	0	0	0	0	0	6	142
07:00 AM	0	101	8	8	9	1	0	1	0	0	0	0	0	3	131
07:15 AM	4	133	10	9	9	0	1	1	0	0	0	0	0	16	183
07:30 AM	4	127	15	7	11	1	0	3	1	0	0	0	0	19	188
07:45 AM	3	148	17	10	5	1	0	1	1	0	0	0	0	7	193
MA 00:80	2	141	11	9	7	3	0	1	0	0	0	0	0	9	183
08:15 AM	1	141	12	14	7	1	0	3	0	0	0	0	0	15	194
08:30 AM	1	152	17	9	12	0	0	0	1	0	0	0	0	18	210
08:45 AM	1	151	13	9	7	1	0	2	1	0	0	0	0	8	193
09:00 AM	2	109	16	7	7	2	0	0	0	0	0	0	0	10	153
09:15 AM	2	99	6	6	5	2	0	2	0	0	0	0	0	4	126
09:30 AM	0	92	9	5	2	0	0	2	0	0	1	0	0	7	118
09:45 AM	0	89	5	7	4	1	0	1	0	0	0	0	0	10	117
10:00 AM	1	92	9	8	5	1	0	0	0	0	0	0	0	6	122
10:15 AM	0	93	12	3	4	0	0	2	0	1	0	0	0	3	118
10:30 AM	2	95	14	6	7	0	0	1	0	0	0	0	0	3	128
10:45 AM	1	80	6	3	6	0	0	1	0	0	0	0	0	7	104
11:00 AM	1	100	11	6	7	1	0	0	_ 1	0	1	0	0	1	129
11:15 AM	1	105	11	7	3	0	0	1	1	0	0	1	0	4	134
11:30 AM	1	109	14	4	8	1	0	1	0	1	0	0	0	2	141
11:45 AM	0	100	12	8	4	0	0	2	0	1	0	0	0	9	136
Day Total															
Percent				$)\Delta TA$	THA	T D	RIVE	SCC	MM	UNIT	11-5				
ADT															
11715															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
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SPECIFIC LOCATION:

CITY/STATE: A	ırlington, VA														Nov 6 201
Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:00 PM	0	133	9	6	6	0	0	1	0	0	0	0	0	6	161
12:15 PM	3	118	9	2	5	0	0	0	0	0	1	0	0	12	150
12:30 PM	0	118	18	8	5	0	0	2	0	0	0	0	0	9	160
12:45 PM	1	143	9	5	5	0	0	1	0	1	0	0	1	10	176
01:00 PM	1	112	10	5	1	3	0	0	0	1	0	0	0	11	144
01:15 PM	1	131	14	7	6	1	0	1	2	0	0	0	0	4	167
01:30 PM	2	119	17	5	9	0	0	3	0	0	0	0	0	11	166
01:45 PM	3	127	13	4	7	0	0	2	1	1	0	0	0	12	170
02:00 PM	0	125	11	6	6	0	0	0	0	0	0	0	0	3	151
02:15 PM	1	122	14	5	3	0	0	0	0	0	0	0	0	4	149
02:30 PM	2	126	12	5	4	1	0	0	0	0	0	0	0	7	157
02:45 PM	1	126	11	4	1	0	0	1	0	0	0	1	0	5	150
03:00 PM	3	126	18	6	3	0	0	0	0	0	0	0	0	14	170
03:15 PM	3	137	9	5	2	0	0	0	0	0	0	0	0	6	162
03:30 PM	1	158	15	7	7	1	0	2	0	0	0	0	0	8	199
03:45 PM	1	170	15	8	5	0	0	0	1	0	1	0	0	6	207
04:00 PM	1	154	14	7	2	1	1	3	0	1	0	0	0	6	190
04:15 PM	0	179	20	8	9	0	0	1	0	0	0	1	0	7	225
04:30 PM	2	202	14	10	4	2	0	2	1	0	0	0	0	3	240
04:45 PM	2	229	20	9	3	_1	0	1	2	0	0	0	0	21	288
05:00 PM	4	232	16	9	7	1	0	3	0	0	0	0	1	16	289
05:15 PM	14	279	34	6	8	1	0	2	2	1	0	0	0	28	375
05:30 PM	10	258	20	13	11	1	0	1	0	0	0	0	0	24	338
05:45 PM	6	210	20	7	7	2	0	1	0	0	0	0	0	22	275
Day Total															
Percent				DATA	THA	AT D	RIVE	SCC	MM	UNIT	TES				
ADT 11715															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	d == 11/11/201	O F 33 AAA									COLIDCE O	l'aCa	- IIC/I	. / /	

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109304 DIRECTION: EB, WB

DATE: Nov 6 2019

Start Time	Motorcycles	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Not	Total
Start Time	Wiotor Cycles	Trailer	Long	Duses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classified	iotai
06:00 PM	0	233	10	12	3	0	0	1	0	1	0	0	0	19	279
06:15 PM	1	223	12	9	4	2	1	3	0	1	0	1	0	11	268
06:30 PM	4	175	22	12	3	0	0	1	0	0	0	0	0	16	233
06:45 PM	3	172	9	7	4	1	0	1	1	0	0	0	0	6	204
07:00 PM	2	144	4	10	5	0	0	0	2	0	1	0	0	14	182
07:15 PM	1	152	8	5	4	0	0	0	0	0	0	0	0	9	179
07:30 PM	0	131	8	6	2	0	0	1	1	0	0	0	1	7	157
07:45 PM	0	116	3	4	2	0	0	0	0	0	0	0	0	6	131
08:00 PM	1	114	5	3	2	1	0	0	0	2	0	0	0	4	132
08:15 PM	1	98	11	5	2	0	0	1	0	0	0	0	0	5	123
08:30 PM	0	92	3	5	2	0	0	0	0	0	0	0	0	4	106
08:45 PM	0	83	8	5	1	1	0	1	0	0	0	0	0	8	107
09:00 PM	0	60	15	6	0	0	0	1	0	0	0	0	0	5	87
09:15 PM	0	74	4	4	2	0	0	1	0	0	0	0	0	4	89
09:30 PM	0	74	6	7	0	0	0	0	0	0	0	0	0	6	93
09:45 PM	0	54	1	4	0	0	0	1	0	0	0	0	0	2	62
10:00 PM	0	54	2	5	1	0	0	0	0	0	0	0	0	3	65
10:15 PM	0	58	4	3	2	0	0	1	0	0	0	0	0	2	70
10:30 PM	1	34	5	4	0	0	0	3	0	0	0	0	0	0	47
10:45 PM	0	32	5	2	0	0	0	0	0	0	0	0	0	2	41
11:00 PM	0	25	0	3	1	0	0	0	0	0	0	0	0	7	36
11:15 PM	0	26	0	3	1	0	0	0	0	0	0	0	0	2	32
11:30 PM	1	15	1	1	1	0	0	0	0	0	0	0	0	1	20
11:45 PM	0	7	1	2	0	0	0	0	0	0	0	0	0	4	14
Day Total	112	9140	873	487	330	41	3	76	27	13	5	5	4	599	11715
Percent	1%	78%	7.5%	4.2%	2.8%	0.3%	0%	0.6%	0.2%	0.1%	0%	0%	0%	5.1%	11715
ADT 11715															
AM Peak	7:15 AM	8:30 AM	6:15 AM	8:15 AM	8:30 AM	8:00 AM	7:15 AM	6:30 AM	4:30 AM	12:00 AM	9:30 AM	4:15 AM	6:15 AM	7:30 AM	8:30 AN
15-min Vol	4	152	17	14	12	3	1	3	2	1	1	1	1	19	210
PM Peak 15-min Vol	5:15 PM	5:15 PM	5:15 PM	5:30 PM	5:30 PM	1:00 PM	4:00 PM	1:30 PM	1:15 PM	8:00 PM	12:15 PM	2:45 PM	12:45 PM	5:15 PM	5:15 PI
	14	279	34	13	11	3	1	3	2	2	1	1	1	28	375

SPECIFIC LOCATION:

Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Nov 7 2019 Total
12:00 AM	2	14	1	0	0	0	0	0	0	0	0	0	0	3	20
12:15 AM	0	6	0	1	0	0	0	0	0	0	0	0	0	2	9
12:30 AM	0	12	0	1	0	0	0	0	0	0	0	0	0	0	13
12:45 AM	0	10	2	1	0	0	0	0	0	0	0	0	0	0	13
01:00 AM	0	1	3	1	0	0	0	0	0	0	0	0	0	0	5
01:15 AM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
01:30 AM	0	4	0	0	0	0	0	1	0	0	0	0	0	0	5
01:45 AM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
02:00 AM	0	4	0	1	1	0	0	0	0	0	0	0	0	0	6
02:15 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
02:30 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
02:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
03:00 AM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
03:15 AM	0	4	0	0	0	0	0	0	1	0	0	0	0	0	5
03:30 AM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
03:45 AM	0	10	2	0	0	0	0	0	0	0	0	0	0	0	12
04:00 AM	0	7	1	0	0	0	0	0	0	0	0	0	0	0	8
04:15 AM	0	3	2	0	0	0	0	0	0	0	0	0	0	1	6
04:30 AM	0	4	0	0	0	0	0	0	1	0	0	0	0	1	6
04:45 AM	0	13	1	0	1	0	0	0	0	0	0	0	0	1	16
05:00 AM	0	15	1	3	0	0	0	0	0	0	0	0	0	0	19
05:15 AM	1	22	3	4	1	1	0	0	0	0	0	0	0	3	35
05:30 AM	0	29	4	6	1	0	0	0	0	0	0	0	0	1	41
05:45 AM	0	41	2	2	1	0	0	1	0	0	0	0	0	5	52
Day Total Percent				DATA	7117	TEN	D// / [SCO	N // N //	1 1K 113	TIEC				
ADT 11712				<i>J</i> A [A		<u> </u>	UVE.	3.00	uvuvi		153				
AM Peak 15-min Vol PM Peak 15-min Vol															
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ort generated (11/11/201	0 5.22 414									COLIDOE: O:	l:4 C 4	- 110/644	//www.nuali	

SPECIFIC LOCATION:

Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
06:00 AM	0	49	2	8	4	2	0	1	2	0	0	0	0	2	70
06:15 AM	1	64	6	7	3	0	0	1	0	0	0	0	0	7	89
06:30 AM	0	71	5	8	7	0	0	0	0	0	0	0	0	11	102
06:45 AM	1	73	7	12	10	1	0	0	1	0	0	0	0	13	118
07:00 AM	1	95	7	11	6	0	0	2	0	0	0	0	0	8	130
07:15 AM	1	118	7	9	6	0	0	1	0	0	1	0	0	11	154
07:30 AM	3	121	13	12	10	0	1	6	0	0	0	0	0	21	187
07:45 AM	1	104	10	7	5	0	0	3	0	0	0	0	0	25	155
08:00 AM	2	113	12	10	8	1	0	4	0	0	0	0	0	25	175
08:15 AM	2	130	7	10	7	0	0	3	1	1	0	0	0	20	181
08:30 AM	0	127	6	9	7	2	0	0	0	1	0	0	0	16	168
08:45 AM	2	119	13	5	10	2	0	1	0	0	0	0	0	9	161
09:00 AM	1	136	10	7	9	2	0	2	0	0	0	0	0	10	177
09:15 AM	0	89	12	7	9	1	1	0	0	0	0	0	0	6	125
09:30 AM	2	96	14	7	7	0	0	0	0	0	0	0	0	2	128
09:45 AM	1	95	12	6	3	1	0	0	0	0	0	0	0	4	122
10:00 AM	1	81	13	4	8	3	0	2	2	0	0	0	0	6	120
10:15 AM	1	90	11	3	8	1	0	1	0	0	0	0	0	5	120
10:30 AM	0	112	16	7	6	1	0	2	0	0	0	0	0	5	149
10:45 AM	1	107	15	2	4	1	1	1	0	1	1	0	0	1	135
11:00 AM	0	99	20	4	3	1	0	0	0	0	0	0	0	2	129
11:15 AM	2	104	5	5	4	1	0	1	0	0	0	0	0	6	128
11:30 AM	1	118	17	5	5	0	0	0	0	0	0	0	0	4	150
11:45 AM	1	108	12	6	1	0	0	0	0	0	0	0	0	6	134
Day Total				DATA	THI	TDI	PIVE	000		HMIT	TIFE				
Percent			-	JAIA		AL DI	RIVE	500	IIVIIVI	UNII	IES				
ADT 11712															
AM Peak 15-min Vol															
PM Peak 15-min Vol															

SPECIFIC LOCATION:

CITY/STATE: A	rlington, VA													DATE: I	Nov 7 2019
Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:00 PM	1	124	15	4	2	0	0	2	0	0	0	0	0	3	151
12:15 PM	1	142	9	6	5	0	0	0	0	0	0	0	0	10	173
12:30 PM	1	137	15	7	6	0	0	0	0	0	0	0	0	2	168
12:45 PM	1	113	14	5	4	0	0	0	0	1	0	0	0	4	142
01:00 PM	5	144	9	5	3	1	1	1	0	0	0	0	0	6	175
01:15 PM	1	117	24	4	7	4	0	1	0	0	0	0	0	7	165
01:30 PM	3	121	12	5	2	0	0	3	0	0	0	0	0	6	152
01:45 PM	0	142	6	4	0	0	0	1	0	0	0	0	0	2	155
02:00 PM	0	128	8	9	4	0	0	0	0	0	0	0	0	2	151
02:15 PM	0	113	11	3	4	1	0	0	0	0	0	0	0	1	133
02:30 PM	0	146	15	7	5	0	0	1	0	0	0	0	0	4	178
02:45 PM	2	118	15	2	6	1	0	0	0	0	0	0	0	11	155
03:00 PM	1	128	16	5	4	2	0	0	0	0	0	0	0	5	161
03:15 PM	2	159	15	2	7	0	0	0	0	0	1	0	0	6	192
03:30 PM	0	150	11	7	2	0	0	1	0	0	0	0	0	10	181
03:45 PM	2	155	15	4	6	1	0	3	0	0	0	0	0	8	194
04:00 PM	2	207	16	9	5	1	0	1	1	1	0	0	0	8	251
04:15 PM	4	208	17	7	9	0	0	1	0	0	0	0	0	24	270
04:30 PM	2	194	11	6	8	1	0	2	0	0	3	0	0	29	256
04:45 PM	5	218	24	14	4	4	1	1	0	0	1	0	0	22	294
05:00 PM	4	274	27	7	4	2	2	1	0	1	0	0	1	25	348
05:15 PM	7	259	28	6	10	2	0	1	1	1	2	0	0	34	351
05:30 PM	4	282	18	8	9	3	0	1	0	0	1	0	1	33	360
05:45 PM	10	266	24	19	4	2	0	1	2	0	0	0	0	22	350
Day Total				3 A T A			D0 /E		NANA	HMI					
Percent				JAIA	LIHA	AL DI	RIVE	5 ((IVIIVI	UNII	IES				
ADT 11712															
AM Peak 15-min Vol				_											
PM Peak 15-min Vol															
omments:															
ort <u> </u>	d on 11/11/201	0 E.22 ANA						•		•	COLIDCE, O.	ality Count	o IIC/b++n	·//www.auali	+,,,,,,,,,,

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109304 DIRECTION: EB, WB

DATE: Nov 7 2019

Start Time	Motorcycles	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Not	Total
Start Time	Wiotorcycles	Trailer	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classified	Total
06:00 PM	5	231	18	19	6	1	0	1	0	0	0	0	0	24	305
06:15 PM	3	175	15	8	3	2	0	1	1	0	0	0	0	19	227
06:30 PM	5	215	17	16	4	1	0	2	0	0	1	0	0	14	275
06:45 PM	2	166	13	9	7	0	0	1	2	0	0	0	0	11	211
07:00 PM	1	189	9	11	4	0	1	0	0	0	0	0	0	5	220
07:15 PM	0	137	6	9	3	0	1	1	0	0	0	0	0	5	162
07:30 PM	0	123	12	9	4	0	0	1	0	0	0	0	0	10	159
07:45 PM	0	141	6	7	4	0	0	0	1	0	0	0	0	6	165
08:00 PM	0	122	6	6	3	0	0	0	0	1	0	0	0	4	142
08:15 PM	0	104	8	6	1	0	0	0	0	1	0	0	0	1	121
08:30 PM	1	99	5	4	2	0	0	0	0	0	0	0	0	3	114
08:45 PM	0	95	9	3	2	0	0	0	0	0	0	0	0	0	109
09:00 PM	0	110	11	6	0	2	1	0	0	0	0	0	0	6	136
09:15 PM	0	93	5	5	1	0	0	0	0	0	0	0	0	2	106
09:30 PM	0	78	4	5	2	0	0	0	0	0	0	0	0	1	90
09:45 PM	1	71	2	2	3	0	0	1	0	0	1	0	0	0	81
10:00 PM	0	59	6	6	0	0	1	0	0	1	0	0	0	2	75
10:15 PM	0	42	1	3	2	0	0	0	0	0	0	0	0	0	48
10:30 PM	0	49	2	2	1	1	0	0	0	1	0	0	0	0	56
10:45 PM	0	46	3	1	1	0	0	0	0	0	0	0	0	3	54
11:00 PM	0	52	3	4	0	0	0	2	0	0	0	0	0	1	62
11:15 PM	0	26	3	2	3	0	0	0	0	0	0	0	0	2	36
11:30 PM	0	21	4	2	0	0	0	0	0	0	0	0	0	1	28
11:45 PM	0	17	3	1	1	0	0	0	0	0	0	0	0	0	22
Day Total	104	9143	823	492	334	53	11	65	16	11	12	0	2	646	11717
Percent	0.9%	78.1%	7%	4.2%	2.9%	0.5%	0.1%	0.6%	0.1%	0.1%	0.1%	0%	0%	5.5%	11712
ADT 11712															
AM Peak	7:30 AM	9:00 AM	11:00 AM	6:45 AM	6:45 AM	10:00 AM	7:30 AM	7:30 AM	6:00 AM	8:15 AM	7:15 AM	12:00 AM	12:00 AM	7:45 AM	7:30 AN
15-min Vol	3	136	20	12	10	3	1	6	2	1	1	0	0	25	187
	5:45 PM	5:30 PM	5:15 PM	5:45 PM	5:15 PM	1:15 PM	5:00 PM	1:30 PM	5:45 PM	12:45 PM	4:30 PM	12:00 PM	5:00 PM	5:15 PM	5:30 PN
PM Peak 15-min Vol	10	282	28	19	10		2	3	2	1	3	0	1	34	360

LOCATION: Army Navy Dr btwn S Joyce St & S Hayes St

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

DATE: Nov 6 2019

	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
Grand Total Percent	216 0.9%	18283 78%	1696 7.2%	979 4.2%	664 2.8%	94 0.4%	14 0.1%	141 0.6%	43 0.2%	24 0.1%	17 0.1%	5 0%	6 0%	1245 5.3%	23427
ADT 11713															

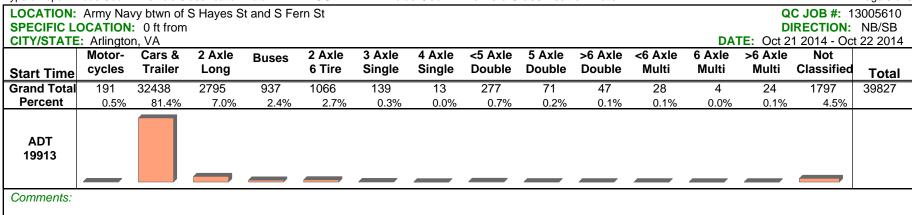
Report generated on 11/14/2019 5:22 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



LOCATION: Army Navy btwn of S Hayes St and S Fern St QC JOB #: 13005610 SPECIFIC LOCATION: 0 ft from **DIRECTION: NB/SB** CITY/STATE: Arlington, VA **DATE:** Oct 21 2014 Motor-Cars & 2 Axle 2 Axle 3 Axle 4 Axle <5 Axle 5 Axle >6 Axle <6 Axle 6 Axle >6 Axle Not **Buses Trailer** 6 Tire Single Double **Double** Double Multi Multi Classified cycles Long Single Multi **Start Time** Total 12:00 AM 1:00 AM 2:00 AM 3:00 AM 4:00 AM 5:00 AM 6:00 AM 7:00 AM 8:00 AM 9:00 AM O 10:00 AM 11:00 AM 12:00 PM 1:00 PM 2:00 PM 3:00 PM 4:00 PM 5:00 PM 6:00 PM 7:00 PM 8:00 PM 9:00 PM 10:00 PM 11:00 PM **Day Total** 0.0% Percent 0.5% 81.1% 7.1% 2.3% 2.7% 0.4% 0.0% 0.7% 0.2% 0.1% 0.1% 0.1% 4.8% ADT AM Peak 8:00 AM 9:00 AM 9:00 AM 10:00 AM 8:00 AM 12:00 AM 8:00 AM 7:00 AM 9:00 AM 7:00 AM 7:00 AM 11:00 AM 4:00 AM 8:00 AM 7:00 AM Volume PM Peak 5:00 PM 4:00 PM 6:00 PM 5:00 PM 2:00 PM 12:00 PM 4:00 PM 12:00 PM 3:00 PM 12:00 PM 4:00 PM 2:00 PM 4:00 PM 5:00 PM 5:00 PM Volume Comments:

LOCATION: Army Navy btwn of S Hayes St and S Fern St QC JOB #: 13005610 SPECIFIC LOCATION: 0 ft from **DIRECTION: NB/SB** CITY/STATE: Arlington, VA **DATE:** Oct 22 2014 Motor-Cars & 2 Axle 2 Axle 3 Axle 4 Axle <5 Axle 5 Axle >6 Axle <6 Axle 6 Axle >6 Axle Not **Buses Trailer** 6 Tire Single Double **Double Double** Multi Multi Classified cycles Long Single Multi **Start Time** Total 12:00 AM 1:00 AM 2:00 AM 3:00 AM 4:00 AM 5:00 AM 6:00 AM 7:00 AM 8:00 AM 9:00 AM O 10:00 AM 11:00 AM 12:00 PM 1:00 PM 2:00 PM 3:00 PM 4:00 PM 5:00 PM 6:00 PM 7:00 PM 8:00 PM 9:00 PM 10:00 PM 11:00 PM **Day Total** 0.4% Percent 81.8% 7.0% 2.4% 2.7% 0.3% 0.0% 0.6% 0.2% 0.1% 0.1% 0.0% 0.1% 4.3% ADT AM Peak 8:00 AM 8:00 AM 7:00 AM 8:00 AM 8:00 AM 8:00 AM 6:00 AM 6:00 AM 11:00 AM 7:00 AM 8:00 AM 8:00 AM 7:00 AM 8:00 AM 8:00 AM Volume PM Peak 5:00 PM 5:00 PM 1:00 PM 5:00 PM 4:00 PM 12:00 PM 1:00 PM 4:00 PM 12:00 PM 2:00 PM 2:00 PM 4:00 PM 5:00 PM 5:00 PM Volume Comments:



Report generated on 10/31/2014 11:10 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109305 DIRECTION: EB, WB

DATE: Nov 6 2019

12:15 AM	Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:30 AM	12:00 AM	0	17	0	0	0	0	0	1	0	0	0	0	0	1	19
12:45 AM	12:15 AM	0	10	2	0	0	0	0	0	0	0	0	0	0	1	13
01:00 AM	12:30 AM	0	7	0	0	1	0	0	0	0	0	0	0	0	0	8
01:15 AM	12:45 AM	0	6	2	0	1	1	0	0	0	0	0	0	0	0	10
01:30 AM	01:00 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
01:45 AM	01:15 AM	0	3	3	0	0	0	0	0	0	0	0	0	0	0	6
02:00 AM	01:30 AM	0	3	3	0	1	0	0	0	0	0	0	0	0	0	7
02:15 AM	01:45 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
02:30 AM	02:00 AM	0	9	0	0	1	0	0	1	0	0	0	0	0	0	11
02:45 AM	02:15 AM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	5
03:00 AM	02:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:15 AM	02:45 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
03:30 AM	03:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
03:45 AM	03:15 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
04:00 AM	03:30 AM	0	7	3	0	0	0	0	0	0	0	0	0	0	0	10
04:15 AM	03:45 AM	0	10	0	0	0	0	0	0	0	0	0	0	0	0	10
04:30 AM	04:00 AM	0	9	0	1	0	0	0	0	0	0	0	0	0	0	10
04:45 AM	04:15 AM	0	9	2	0	1	0	0	0	0	0	0	0	0	1	13
05:00 AM	04:30 AM	0	5	3	1	1	0	0	0	0	0	0	0	0	0	10
05:15 AM	04:45 AM	0	7	1	0	0	0	0	0	0	0	0	0	0	0	8
05:30 AM	05:00 AM	0	16	4	0	1	0	0	0	0	0	0	0	0	0	21
05:45 AM	05:15 AM	0	20	2	1	0	0	0	0	0	0	0	0	0	0	23
05:45 AM	05:30 AM	1	30	4	0	1	0	0	0	0	0	0	0	0	1	37
ADT 7991 AM Peak 15-min Vol PM Peak	05:45 AM	1	46	5	1		0	0	0	0	0	0	0	0	2	57
ADT 7991 AM Peak 15-min Vol PM Peak	Day Total															
AM Peak 15-min Vol PM Peak	Percent				DATA	THA	TD	RIVE	SCO	MM	UMIT	IES				
15-min Vol PM Peak																

SPECIFIC LOCATION:

CITY/STATE: Arlington VA

QC JOB #: 15109305 DIRECTION: EB, WB DATE: Nov 6 2019

Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
06:00 AM	2	53	3	0	2	0	0	1	0	0	0	0	0	2	63
06:15 AM	2	60	3	1	5	0	0	1	0	0	0	0	0	1	73
06:30 AM	0	65	8	0	5	0	0	0	0	0	0	0	0	0	78
06:45 AM	1	77	7	2	3	1	0	0	0	0	0	0	0	4	95
07:00 AM	5	120	15	0	4	0	0	0	0	0	0	0	0	3	147
07:15 AM	5	110	12	1	4	1	0	0	0	0	0	0	0	2	135
07:30 AM	2	143	9	1	2	0	0	0	1	0	0	0	0	0	158
07:45 AM	1	126	8	0	4	0	1	0	0	0	0	0	0	0	140
MA 00:80	0	118	11	0	4	1	0	0	0	0	0	0	0	2	136
08:15 AM	0	123	9	2	3	1	0	1	0	0	0	0	0	5	144
08:30 AM	1	117	13	3	3	1	1	1	0	0	0	0	0	5	145
08:45 AM	1	119	15	4	4	3	1	0	0	0	0	0	1	6	154
09:00 AM	2	103	14	2	3	0	1	0	0	0	0	0	0	2	127
09:15 AM	1	92	10	3	3	1	0	0	0	0	0	0	0	3	113
09:30 AM	1	84	4	3	1	1	0	1	0	0	0	0	0	3	98
09:45 AM	1	79	7	2	4	0	1	1	0	0	0	0	0	4	99
10:00 AM	1	95	9	2	2	0	0	0	0	0	0	0	0	0	109
10:15 AM	0	71	11	3	5	0	0	1	0	0	0	0	0	1	92
10:30 AM	0	63	10	3	3	1	0	0	0	0	0	0	0	1	81
10:45 AM	0	46	5	0	4	0	1	0	0	0	0	0	0	1	57
11:00 AM	1	61	9	4	4	0	0	1	0	0	0	0	0	1	81
11:15 AM	1	66	10	1	6	0	0	0	0	1	0	0	0	2	87
11:30 AM	1	65	7	3	1	0	0	0	0	0	0	0	0	0	77
11:45 AM	0	88	4	3	2	1	0	0	0	0	0	0	0	0	98
Day Total Percent															
ADT 7991															
AM Peak 15-min Vol															
PM Peak 15-min Vol															

SPECIFIC LOCATION:

QC JOB #: 15109305 DIRECTION: EB, WB

CITY/STATE: A	rlington, VA													DATE: I	Nov 6 2019
Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:00 PM	0	78	12	0	5	1	0	0	0	0	0	0	0	3	99
12:15 PM	2	88	11	1	2	0	0	2	0	0	0	0	0	2	108
12:30 PM	1	80	5	6	2	0	1	0	0	0	0	0	0	0	95
12:45 PM	1	67	8	3	6	1	0	0	0	0	0	0	0	2	88
01:00 PM	3	73	17	2	1	0	0	0	0	0	0	0	0	0	96
01:15 PM	0	65	9	2	3	0	0	1	0	0	0	0	0	1	81
01:30 PM	0	71	11	4	3	0	0	0	0	0	0	0	0	1	90
01:45 PM	0	91	16	2	4	0	0	1	0	0	1	0	0	1	116
02:00 PM	1	89	9	1	4	0	0	1	0	0	0	0	0	1	106
02:15 PM	0	96	10	1	3	0	0	0	0	0	0	0	0	1	111
02:30 PM	0	90	12	4	3	1	0	0	0	0	0	0	0	4	114
02:45 PM	0	101	1	1	1	0	0	1	0	0	0	0	0	0	105
03:00 PM	2	144	15	4	3	0	0	0	0	0	0	0	0	5	173
03:15 PM	2	119	11	2	6	0	0	1	0	0	0	0	0	10	151
03:30 PM	4	112	15	2	4	0	0	1	0	0	0	0	0	7	145
03:45 PM	2	108	15	2	6	2	0	0	0	0	0	0	0	14	149
04:00 PM	6	136	7	1	6	3	0	0	0	0	0	0	0	24	183
04:15 PM	0	146	6	3	1	0	0	0	0	0	0	0	0	26	182
04:30 PM	8	125	13	1	1	0	0	2	0	0	0	0	0	31	181
04:45 PM	2	111	6	2	3	0	1	3	0	0	0	0	0	22	150
05:00 PM	3	101	10	3	2	2	0	0	0	0	0	0	0	25	146
05:15 PM	2	106	8	4	2	0	0	1	0	0	0	0	0	21	144
05:30 PM	2	159	14	1	4	1	0	0	0	0	0	0	0	15	196
05:45 PM	1	113	11	2	2	1	0	0	0	0	0	0	0	14	144
Day Total															
Percent				DATA	THA	AT D	RIVF	SCC	MM	UNIT	IFS				
ADT 7991															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
Comments:															
nort gonorato	d on 11/11/201	0 E.22 ANA									COLIDCE, O.	ality Carret	a IIC/b++n	//www.	+,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109305 DIRECTION: EB, WB DATE: Nov 6 2019

C 0	lington, VA	2 4 - 1 -		2 A.d. C	2 4-1-	4 4 - 1 -	4F AI-	C A.da	> C AI-	aC A.da	C AI-	> C AI-		Nov 6 201
Cars & Trailer	Motorcycles		Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
	1		4											152
116	1	9	4	2	0	0	2	0	0	1	0	0	18	153
121	2	10	2	1	0	0	1	0	0	0	0	0	19	156
134	2	12	0	1	0	0	1	0	0	0	0	0	9	159
115	1	6	0	2	0	0	0	0	0	0	0	0	5	129
109	1	15	·	0	0	0	0	0	0	0	0	0	5	130
111	1	3	0	1	0	0	0	0	0	0	0	0	1	117
91	3	7	0	2	0	0	0	0	0	0	0	0	3	106
93	0	9	1	1	0	0	0	0	0	0	0	0	0	104
76	1	3	0	2	1	0	1	0	0	0	0	0	1	85
52	0	4	0	0	0	0	0	0	0	0	0	0	0	56
62	0	10	0	1	0	0	0	0	0	0	0	0	0	73
83	0	4	0	1	1	1	0	1	0	0	0	0	1	92
71	2	4	0	1	0	0	0	0	0	0	0	0	0	78
86	1	5	3	2	0	0	0	0	0	0	0	0	3	100
81	0	5	0	1	0	0	0	0	0	0	0	0	1	88
50	2	3	0	0	0	0	0	0	0	0	0	0	2	57
54	0	2	0	0	0	0	0	0	0	0	0	0	0	56
53	0	6	0	0	0	0	0	0	0	0	0	0	0	59
40	0	8	0	0	0	0	0	0	0	0	0	0	0	48
33	0	1	0	0	0	0	0	0	0	0	0	0	0	34
19	0	1	0	0	0	0	0	0	0	0	0	0	1	21
11	0	3	0	1	0	0	0	0	0	0	0	0	0	15
20	0	2	0	0	0	0	0	0	0	0	0	0	0	22
15	0	4	0	1	0	0	0	0	0	0	0	0	0	20
6537	90	640	111	189	27	9	29	2	1	2	0	1	353	7991
81.8%	1.1%	6 8%	1.4%	2.4%	0.3%	0.1%	0.4%	0%	0%	0%	0%	0%	4.4%	7991
7:30 AM			8:45 AM	11:15 AM	8:45 AM	7:45 AM	12:00 AM	7:30 AM				8:45 AM	8:45 AM	7:30 A
143														158
5:30 PM 159	4:30 PM 8	M 1:00 PM 17	12:30 PM 6	12:45 PM 6	4:00 PM 3	12:30 PM 1	4:45 PM 3	8:45 PM 1	12:00 PM 0	1:45 PM 1	12:00 PM 0	12:00 PM 0	4:30 PM 31	5:30 P 196
5:30 1	5 4:30 PM	43) Pi 59	43 15 0 PM 1:00 PM 59 17	43 15 4 0 PM 1:00 PM 12:30 PM 59 17 6	43 15 4 6 0 PM 1:00 PM 12:30 PM 12:45 PM 59 17 6 6	43 15 4 6 3 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 59 17 6 6 3	43 15 4 6 3 1 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 59 17 6 6 3 1	43 15 4 6 3 1 1 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 59 17 6 6 3 1 3	43 15 4 6 3 1 1 1 1 D PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 59 17 6 6 3 1 3 1	43 15 4 6 3 1 1 1 1 1 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 12:00 PM 59 17 6 6 3 1 3 1 0	43 15 4 6 3 1 1 1 1 1 0 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 12:00 PM 1:45 PM 59 17 6 6 3 1 3 1 0 1	43 15 4 6 3 1 1 1 1 1 0 0 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 12:00 PM 1:45 PM 12:00 PM 59 17 6 6 3 1 3 1 0 1 0	43 15 4 6 3 1 1 1 1 1 0 0 1 1 1 1 0 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 12:00 PM 1:45 PM 12:00 PM 12:0	43 15 4 6 3 1 1 1 1 1 0 0 1 1 6 0 PM 1:00 PM 12:30 PM 12:45 PM 4:00 PM 12:30 PM 4:45 PM 8:45 PM 12:00 PM 1:45 PM 12:00 PM 12:00 PM 4:30 PM 59 17 6 6 3 1 3 1 0 1 0 0 31

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109305 DIRECTION: EB, WB

DATE: Nov 7 2019

CITY/STATE: A	T .	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Not	Nov / 2019
Start Time	Motorcycles	Trailer	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classified	Total
12:00 AM	0	15	0	0	1	0	0	0	0	0	0	0	0	0	16
12:15 AM	0	8	0	0	0	0	0	0	0	0	0	0	0	0	8
12:30 AM	0	9	3	0	0	0	0	0	0	0	0	0	0	0	12
12:45 AM	0	6	1	0	1	1	0	0	0	0	0	0	0	0	9
01:00 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
01:15 AM	0	7	1	0	0	0	0	0	0	0	0	0	0	0	8
01:30 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
01:45 AM	0	7	0	0	0	0	0	0	0	0	0	0	0	0	7
02:00 AM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
02:15 AM	0	3	0	0	1	0	0	0	0	0	0	0	0	0	4
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
03:00 AM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
03:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:30 AM	0	5	0	1	0	0	0	0	0	0	0	0	0	0	6
03:45 AM	0	8	5	0	0	0	0	0	0	0	0	0	0	0	13
04:00 AM	0	11	0	0	1	0	0	0	0	0	0	0	0	0	12
04:15 AM	0	6	4	0	1	0	0	0	0	0	0	0	0	0	11
04:30 AM	0	8	2	0	0	0	0	0	0	0	0	0	0	1	11
04:45 AM	0	14	4	1	2	0	0	0	0	0	0	0	0	0	21
05:00 AM	0	16	4	0	0	0	0	0	0	0	0	0	0	0	20
05:15 AM	0	28	5	0	0	0	0	0	0	0	0	0	0	0	33
05:30 AM	0	37	2	0	2	1	0	0	0	0	0	0	0	0	42
05:45 AM	0	61	2	0	1	0	0	0	0	0	0	0	0	0	64
Day Total Percent															
reiteilt			-	JAIA	11116	1111	RIVE	5.00	IIVIIVI	UIVIII	IES				
ADT 8326															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
art ganarata	d on 11/11/201	0 E.22 ANA									COLIDOT, O.	olity Count	c IIC (b++n	·//www.auali	t

SPECIFIC LOCATION:

QC JOB #: 15109305 **DIRECTION:** EB, WB

CITY/STATE: A	rlington, VA													DATE: I	Nov 7 201
Start Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
06:00 AM	3	54	5	1	2	0	0	0	0	0	0	0	0	2	67
06:15 AM	2	71	19	1	2	0	0	0	0	0	0	0	0	1	96
06:30 AM	1	59	8	0	2	0	0	0	0	1	0	0	0	2	73
06:45 AM	1	80	10	1	3	0	0	0	0	0	0	0	0	3	98
07:00 AM	5	87	16	1	5	0	0	2	0	0	0	0	0	4	120
07:15 AM	0	118	9	2	0	0	0	0	0	0	0	0	0	2	131
07:30 AM	0	114	12	1	7	0	0	0	0	0	0	0	0	1	135
07:45 AM	0	121	14	1	3	0	0	0	0	0	0	0	0	3	142
08:00 AM	3	121	19	8	1	0	0	0	0	0	0	0	0	5	157
08:15 AM	3	128	12	0	2	2	0	1	0	0	0	0	0	2	150
08:30 AM	1	149	13	1	4	0	0	0	0	0	0	0	0	13	181
08:45 AM	2	118	17	4	5	1	0	0	0	0	0	0	0	21	168
09:00 AM	2	107	13	1	1	1	0	1	0	0	0	0	0	3	129
09:15 AM	1	89	15	3	2	2	0	0	0	0	0	0	0	3	115
09:30 AM	0	126	14	2	3	1	0	2	0	0	0	0	0	3	151
09:45 AM	0	99	7	3	4	0	0	2	0	0	0	0	0	0	115
10:00 AM	0	81	8	3	2	0	0	0	0	0	0	0	0	3	97
10:15 AM	1	60	22	3	2	0	1	0	0	0	0	0	0	1	90
10:30 AM	0	74	4	2	4	0	0	1	0	0	0	0	0	1	86
10:45 AM	1	73	9	1	2	_1	0	0	0	0	0	0	0	3	90
11:00 AM	0	82	8	2	1	1	0	3	_1	0	0	0	0	0	98
11:15 AM	0	78	10	3	3	0	0	1	0	1	0	0	0	0	96
11:30 AM	0	83	5	3	3	0	0	0	0	0	0	0	0	1	95
11:45 AM	0	64	5	3	2	0	0	1	0	0	0	0	0	0	75
Day Total															
Percent				DATA	THA	TD.	RIVE	SCC	MM	UNIT	11-5				
ADT 8326															
AM Peak															
15-min Vol															
PM Peak 15-min Vol															
omments:															

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109305 DIRECTION: EB, WB

DATE: Nov 7 2019

12:30 PM	t Time	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
12:30 PM	00 PM	0	76	16	4	2	1	0	0	0	0	0	0	0	1	100
12:45 PM	15 PM	0	74	8	1	3	0	0	2	0	0	0	0	0	0	88
01:00 PM	30 PM	0	104	5	4	4	1	0	1	0	0	0	0	0	3	122
01:15 PM	45 PM	1	85	14	2	3	0	0	0	0	0	0	0	0	2	107
01:30 PM	00 PM	1	82	6	2	3	0	0	1	0	0	0	0	0	1	96
01:45 PM	15 PM	2	78	16	1	3	0	0	0	0	0	0	0	0	1	101
02:00 PM	30 PM	1	85	18	4	6	0	0	0	0	0	0	0	0	2	116
02:15 PM	45 PM	1	94	13	2	1	0	0	3	0	0	0	0	0	0	114
02:30 PM	00 PM	2	99	14	2	1	0	0	0	0	0	0	0	0	5	123
02:45 PM	15 PM	1	95	10	0	6	0	0	1	0	0	0	0	0	5	118
03:00 PM	30 PM	0	111	9	4	2	0	0	1	0	0	0	0	0	2	129
03:15 PM	45 PM	1	112	9	1	4	0	0	0	0	0	0	0	0	1	128
03:30 PM	00 PM	1	117	14	5	2	0	0	0	0	0	0	0	0	14	153
03:45 PM	15 PM	2	123	17	5	3	1	0	1	0	0	0	0	0	17	169
04:00 PM	30 PM	1	111	8	4	4	0	0	0	0	0	0	0	0	19	147
04:15 PM	45 PM	2	128	14	4	2	2	0	0	0	0	0	0	1	10	163
04:30 PM	00 PM	5	156	11	3	4	0	0	1	0	0	0	0	0	9	189
04:45 PM	15 PM	6	138	8	6	3	0	0	0	0	0	0	0	0	16	177
05:00 PM	30 PM	2	114	11	2	2	0	0	0	0	0	0	0	0	25	156
05:15 PM	45 PM	4	79	18	6	4	0	0	0	0	0	0	0	0	35	146
05:30 PM	00 PM	5	48	12	4	0	1	0	1	0	0	0	0	0	35	106
10 88 10 2 2 0 0 1 0 0 0 0 0 29	15 PM	6	58	4	2	2	0	0	0	1	0	0	0	1	31	105
10 88 10 2 2 0 0 1 0 0 0 0 0 29	30 PM	1	53	5	0	1	0	0	0	0	0	0	0	0	26	86
ADT 8326 AM Peak 15-min Vol	45 PM	10	88	10	2		0	0	1	0	0	0	0	0	29	142
ADT 8326 AM Peak 15-min Vol	/ Total															
AM Peak L5-min Vol	rcent				DATA	THA	T D	RIVE	SCO	MM	UNIT	IFS				
15-min Vol																
PM Peak																
15-min Vol																

SPECIFIC LOCATION:

CITY/STATE: Arlington, VA

QC JOB #: 15109305 DIRECTION: EB, WB

DATE: Nov 7 2019

	72 60 107 113 113 106 116	5 4 7 12 4 7	1 4 3 0 0	7 Tire 2 2 4 1 1	0 0 1	Single 0 0	O 0 0	Double 0	Double	Multi 0	Multi 0	Multi 0	Classified 41	126
	60 107 113 113 106	4 7 12 4	4 3 0	2 4	0 1	0			0	0	0	0	<i>A</i> 1	176
	107 113 113 106	7 12 4	3 0	4	1		0							
	113 113 106	12 4	0	•		0		0	0	0	0	0	27	101
	113 106	4	-	1	^	•	1	0	0	0	0	0	27	151
	106	•	0		0	0	0	0	1	0	0	0	10	137
		7		1	0	0	0	0	0	0	0	0	6	125
	116		1	1	0	0	0	1	0	0	0	0	1	117
		3	0	0	0	0	1	0	0	0	0	0	4	126
	108	3	3	1	0	0	0	1	0	0	0	0	2	119
	91	8	0	1	0	0	0	0	0	0	0	0	1	101
	107	3	0	5	0	0	0	0	0	0	0	0	0	115
	115	4	0	2	1	0	1	1	0	0	0	0	1	128
	89	8	3	2	0	0	0	0	0	0	0	0	1	104
	102	2	1	1	0	0	1	0	0	0	0	0	2	109
	76	9	3	2	0	0	0	0	0	0	0	0	6	97
	81	4	0	0	0	0	0	0	0	0	0	0	0	85
	64	3	2	1	0	0	0	0	0	0	0	0	1	71
	71	4	0	1	0	0	0	0	0	0	0	0	1	77
	48	6	0	2	0	0	0	0	0	0	0	0	0	56
	47	5	1	0	0	0	0	0	0	0	0	0	0	53
	45	7	0	2	0	0	0	0	0	0	0	0	0	54
	37	1	0	0	1	0	0	0	0	0	0	0	0	39
	21	3	0	0	1	0	3	0	0	0	0	0	4	32
	17	2	0	0	1	0	0	0	0	0	0	0	0	20
	21	3	0	2	1	0	0	0	0	0	0	0	2	29
) (6637	698	144	175	23	1	34	5	3	0	0	2	504	8326
% 7	79.7%	8.4%	1.7%	2.1%	0.3%	0%	0.4%	0.1%	0%	0%	0%	0%	6.1%	0320
			8:00 AM	7:30 AM	8:15 AM			11:00 AM	6:30 AM				8:45 AM	8:30 Al
			8	7	2	1	3	1	1	0	0	0		181
	00 PM 156	1:30 PM 18	4:15 PM 6	1:30 PM 6	3:45 PM 2	12:00 PM 0	1:45 PM 3	5:15 PM 1	6:45 PM 1	12:00 PM 0	12:00 PM 0	3:45 PM 1	6:00 PM 41	4:00 PI 189
		149 4:00 PM	149 22 4:00 PM 1:30 PM	149 22 8 4:00 PM 1:30 PM 4:15 PM	149 22 8 7 4:00 PM 1:30 PM 4:15 PM 1:30 PM	149 22 8 7 2 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM	149 22 8 7 2 1 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM	149 22 8 7 2 1 3 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM	149 22 8 7 2 1 3 1 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM	149 22 8 7 2 1 3 1 1 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM 6:45 PM	149 22 8 7 2 1 3 1 1 0 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM 6:45 PM 12:00 PM	149 22 8 7 2 1 3 1 1 0 0 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM 6:45 PM 12:00 PM 12:00 PM	149 22 8 7 2 1 3 1 1 0 0 0 4:00 PM 1:30 PM 4:15 PM 1:30 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM 6:45 PM 12:00 PM 12:00 PM 3:45 PM	149 22 8 7 2 1 3 1 1 0 0 0 21 4:00 PM 1:30 PM 4:15 PM 3:45 PM 12:00 PM 1:45 PM 5:15 PM 6:45 PM 12:00 PM 12:00 PM 3:45 PM 6:00 PM

CITT/STATE: A	lington, vA													DATE:	NOV 6 2019
	Motorcycles	Cars & Trailer	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Not Classified	Total
Grand Total	190	13174	1338	255	364	50	10	63	7	4	2	0	3	857	16317
Percent	1.2%	80.7%	8.2%	1.6%	2.2%	0.3%	0.1%	0.4%	0%	0%	0%	0%	0%	5.3%	16317
ADT 8158	[
Comments:															

Report generated on 11/14/2019 5:22 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



Appendix B

Boring Logs

	<u>LA</u>	/VU/	1/V	Log		Boring			LE	B-1				Sheet	1	of	1	
	Project	Army Navy Drive Cor	mplete Streets Project		Pro	oject No.			270	06000	5							
	Location	Arlington, Virginia			Ele	evation an	d Da		Δnn	rox. el	146	(NIA	WD8	18)				
	Drilling Compan	у			Da	te Started	I							Finished				
	Drilling Equipme	Free State Drilling ent			Со	mpletion	Dept	h		4/6/20)	ı	Rock I	Depth		4/6/20		
	Size and Type o	CME-55 Track-Moun	ted Drill Rig		ļ.,					10.5 ft urbed	t		Un	Not Er		ntered Core		
	Casing Diamete	2-1/4-inch Hollow-Ster (in)	em Augers	Casing Depth (ft)	-	mber of S		ies	First			5	Coi	mpletion		24 HR.	-	
	Casing Hammer	<u>- ` ` </u>	Weight (lbs)	Drop (in)		ater Level	` ′		∇		N	IE_	Ţ	<u> </u>	<u> </u>	<u>Ā</u>	-	
242	Sampler	2-inch OD Split Spoo		-	Fie	eld Engine	or	R	onal	d Stidl	han	n						
<u> </u>	Sampler Hamme		Weight (lbs) 140	Drop (in) 30		nu Engine	-	Α		r Gana		thy						
Report: Log	MATERIAL SYMBOL (tt) +46.0		Sample Description			Depth Scale	Number	Type		Penetr. resist aldm BL/6in Q	(N-Val Blows	s/ft)	(Drilling Fl Fluid Loss, D	Rema uid, De Orilling I		ng, , etc.)	
#8: ID PIM ::		ASPHALT (13 inch	es thick)			- 0 - 						20 0	0 40	Cored thr with a 6 ii Began dr	nch c	ore bit.		
24/2020 3:4	+44.9		hes thick) <i>BASE</i> , Brown and gray FILL, trace clay, mediu			- 1 - 				15	-			SS 1.5 to			e B-1	
VF.GT2 4/		(GW)	TEE, trace day, media	m dense, moist		- 2 - 3 -	S1	SS	16	9	:	23•		from 2 to				
אם ויאאו וואה	+42.5		o coarse, CLAYEY SAI s layers of fine to coar			- 4 -	S2	SS	22	3 4	10			Augered SS 3.5 to				
- COOOGO / 7/COO		fine gravel, contain	o coarse, CLAYEY SAI s cinders and layer of um dense, moist (SC)	fine to coarse		- 5 - - 5 - - 6 -	S3	SS	24	6 6 5 5	11+			Augered SS 5.5 to				
		No Recovery				- 7 - - 7 - 	S4 8	SS	0	6 7 5 8	15			Augered SS 7.5 to		5 ft.		
DISCIPLINE	+37.0	FILL, Brown, CLAY stiff, moist (CL)	FILL, trace fine grave	I, contains brick,	- —	9 -	S5	SS	12	7 6 7	-			Augered SS 9 to 1	0.5 ft	t		
NGAIN. COIMIDA I AMAINDA I AUIZ I OUGOUGO ITROSEO I DA I AIL	+35.5	End of Boring at 10 Boring Location Co Lat = 38.865901 Long = -77.062305	oordinates:			- 10 - - 11 - - 12 - - 13 - - 14 - - 14 -				7	14			Complete Borehole auger ren not encou drilling. B with soil o patched v completic Pavemen Surface C good con Surface C fair condi Base Cou fair condi Concrete good con Concrete	cave noval untercoreho cuttin with a on. It Sur Cours ditior Cours tion (urse: tion (Laye ditior	ed at 8.5 d. Grounded during ole backings and seasphalt ummary: See Layer (1.5 in) (2 in) (2 in) (7.5 in)	ft afte dwater g filled surface pon 1:	er r

			T/W	Log of	В	oring			LB	-10			Sheet	1	of	2
Project					Pro	ject No.										
Lagarita		Army Navy Drive Cor	mplete Streets Project			untin :	- d D		270	06000	5					
Location		Arlington Virginia			⊨ıe\	vation ar	id Da		۸nn	rov ol	20.2	/NIA\/F	700)			
Drilling C	ompar	Arlington, Virginia			Dat	e Started	d		√hb	iox. el	39.2		Finished			
		Free State Drilling								4/2/20					4/2/20	
Drilling E	quipm		A. J. D. III D.		Cor	mpletion	Depti	า		05.5		Rock	Depth	_		
Size and	Туре	CME-55 Track-Moun of Bit	ited Drill Rig	+				. 1	Dist	25 ft urbed		Un	Not disturbed	Encou	untered Core	
		2-1/4-inch Hollow-Ste	em Augers		Nur	mber of S	Samp	les			8			-		-
Casing D	iamete	er (in) -		Casing Depth (ft)	Wa	ter Level	l (ft.)		First		NE		mpletion	NE	24 HR.	_
Casing F	lamme	r_	Weight (lbs)	Drop (in)	Drill	ling Fore	man		_				= -			
Sampler		2-inch OD Split Spoo	nn		Eial	ld Engine	nor	R	onal	d Stidl	nam					
Sampler	Hamm		Weight (lbs) 140	Dron (in)	ı ıel	u Liigii16	- C I	Δr	nhe	r Gan	apathy	,				
4-		, tuto	140					/1		mple D				Dor	orko	
MATERIAL	Elev. (ft)		Sample Description	l		Depth Scale	Number	Type	in)	Penetr. resist BL/6in		alue vs/ft)	(Drilling	Rem g Fluid, D	epth of Casing	l,
M _S ⊗	+39.2					— 0 —	N	۲	Re	Pa a H	,	30 40			Resistance, e	
	+38.8	ASPHALT (5 inche		CAND	_		1						Cored	throug	h pavemen core bit.	nt
			BBASE, Brown, fine to EL FILL, trace silt, cla		þ	- - - 1 -	1						Began	drilling	g at 12:55.	
S.V.	+38.0	_ medium dense, mo	oist (GW)		7	- ' -				8			SS 1 to	3 ft		
		FILL, Brown, fine to gravel, medium de	o medium, SAND FILL nse, moist (SP)	, trace silt and fine	F		╡_	SS		6						
		,	, , ,		F	- 2 -	S	SS	8	6	12•					
					E		3			9						
					þ	- 3 -				9			Augere		ft.	
					þ	- :	1	SS		8			SS 3 to	5 ft		
					E	- 4 -	S2	SS	24		15					
					F		1	l		7						
		Ell Drawn fine to	a coorea CAND FILL	trace oilt and alov	þ	- 5 -	1	H		6			Augere	ed to 5	ft	
		contains brick, loos	o coarse, SAND FILL, se, moist (SW)	trace siit and clay,	Ė		3	l		4			SS 5 to	7 ft	16.	
					E	- 6 -	S3	SS	24	4	8					
					þ		"	"	(1	4						
	+32.2					 				3						
		No Recovery	ate brown sand with s	ilt von looso		- 7 - -				1			Augere SS 7 to			
		- oon cuttings indic	ale biowii sailu Willi S	iii, very ioose	þ		8	SS	0	1_				J.J 10		
					F	- 8 -	1			1	†					
		TERRACE DEPOS	TT, Brown to light brow	n, fine to	F		\vdash			1			Augere	ed to 8	.5 ft.	
		medium, SAND, lo	ose, moist (SP)		þ	- 9 -	S5	SS	91	2			SS 8.5	ιο 10	IL	
					þ]"		ν-	2	+					
					E	10 -		F		_			Augere	ed to 1	3.5 ft.	
					þ		1									
					þ	- - 11 -	1									
					E											
					F	- 12 -	1									
					þ	: '- :	1									
					ŀ	- - 13 -	1									
					F	. is -	1									
			IT, Brown, fine to med	ium, SAND, trace	þ					2			SS 13.	5 to 15	5 ft	
		silt, loose, moist (S	PF)		þ	- 14 -	Se Se	SS	18	3						
					E]			3	6					
					F	_ 15 _	1	┝╘		-			Augere	ed to 1	8.5 ft.	
					þ		1									



NLANGAN.COM/DATANAR/DATA0/270060005/PROJECT DATA_DISCIPLINE\GEOTECHNICAL\GINTLOGS\270060005 - ARMY NAVY DRIVE.GPJ

LB-10 Log of Boring Sheet of 2 Project Project No. Army Navy Drive Complete Streets Project 270060005 Location Elevation and Datum Approx. el 39.2 (NAVD88) Arlington, Virginia Sample Data Remarks Elev Depth N-Value (Blows/ft) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale 10 20 30 40 +23.2 16 17 Report: Log - LANGAN 18 SS 18.5 to 20 ft TERRACE DEPOSIT, Red-brown, fine to medium, SAND, SS trace silt, loose, moist (SP) 9 S7 2 2 20 Augered to 23.5 ft. 21 22 23 TERRACE DEPOSIT, Red-brown, fine to coarse, SAND SS 23.5 to 25 ft SS 6 WITH FINE GRAVEL, trace silt, dense, moist (SW) 88 12 14 17 25 Completed drilling at 13:30. End of Boring at 25 ft Borehole caved at 23.5 ft Boring Location Coordinates: Lat = 38.864948 after auger removal. Groundwater not 26 Long = -77.054089encountered during drilling. Borehole backfilled with soil cuttings and surface patched 27 with asphalt upon completion. 28 Pavement Summary: Surface Course good condition (1 in) Base Course 29 good condition (4 in) 30 31 32 33 34 35

			NUA	L/V	Log	of E	Boring			LB	-11		_	S	Sheet	1	of	1
	Project		AN	outsta Otas eta Dusta et		Pr	oject No.			070	00000	_						
	Location		Army Navy Drive Com	iplete Streets Project		Ele	evation ar	nd Da	itum	270	06000	5						
	D :::: 0		Arlington, Virginia			Ļ				App	rox. e	38.2						
	Drilling C	ompai	าy Free State Drilling			Da	ate Starte	d			4/2/20	1	Dat	e Fir	nished		4/2/20	
	Drilling E	quipm				Co	mpletion	Dept	h		112120	·	Roo	ck De	epth		4/2/20	
	Size and	Type	CME-55 Track-Mounte	ed Drill Rig							10.5 fl urbed	t	Ц,	Indi	Not sturbed	Encou	Intered Core	
		•	2-1/4-inch Hollow-Ster	m Augers	0 1 0 11 (6)	Nu	umber of S	Samp	les			5				-		-
	Casing D	iamet	er (in) -		Casing Depth (ft)		ater Leve	` '		First		NE		Com <u> </u>	pletion I	ΝE	24 HR.	-
,	Casing H	amme	er_	Weight (lbs)	Drop (in)	Dr	illing Fore	man			-1 Ot: -1							
NGA.	Sampler		2-inch OD Split Spoon			_ Fie	eld Engine	eer	K	onal	d Stid	nam						
9 - 17	Sampler	Hamm	ner Auto	Weight (lbs) 140	Drop (in) 30				A		r Gan		у					
JI. LO	MATERIAL SYMBOL	Elev.		Samuela Dagawintian			Depth	- Je	0		mple D		Value			Rem		
Jeby.	MATE	(ft) +38.2	3	Sample Description			Scale	Number	Type	Reco (in)	Penetr. resist BL/6in	(Bk	ows/ft) 0 30 4				epth of Casing Resistance, e	
:		- 00.2	ASPHALT (12 inche	es thick)			- 0 - -							Ì	Cored t	hroug	h pavemei	nt
49.29		+37.2					:									drilling	core bit. j at 09:00.	
.50 02		+37.0 +36.8	AGGREGATE SUBE	BASE, Gray, fine, GRA	VEL FILL, trace		- 1 - - :				12				SS 1 to Collecte		k Sample	B-1
77/47			BRICK (2 inches this	ck)		_//	- 2 -	S	SS		5	9+			from 1 t			
4			FILL, Brown, fine to gravel, loose, moist	coarse, SAND, trace (SW)	clay and fine			S	S		4	9						
5		+35.2	•	, ,			3 -				5					0	<i>c.</i>	
2				T, Orange-brown, fine medium dense, moist				1			5				Augered SS 3 to		π.	
7			,	,	,		- 4 -	S2	l _S	8	5	10+						
							-	97			5							
Z - C		+33.2					5 -				4				Augere	d to 5	f	
2000			SAND, trace silt, loc	T, Orange-brown, fine ose, moist (SP)	to medium,		<u> </u>				3				SS 5 to		IL.	
0/2/0							- 6 -	S3	SS	12	4	9						
L C C] "			5							
200			TERRACE DEPOSIT	T, Light brown, fine to	modium CAND		7 -	_	SS		4				Augere	d to 7	ft	
				avel, loose, moist (SP							2				SS 7 to			
							- 8 -	8	SS	16	5	8•						
											3							
			TERRACE DEPOSIT	T, Light brown, fine to	medium SAND		9 -	_	H		3				Augere	d to 9	ft.	
100				avel, loose, moist (SP			- :	S5	s	16	2 4				SS 9 to	10.51	ft	
ב ב			TERRACE DEPOSIT	T , Red-brown, fine to α	coarse, SAND,		10 -	S	SS	-	6	10						
5		+27.7	trace silt, medium de End of Boring at 10.	ense, moist (SW)			上 :	+	F		0						illing at 09	
			•				- 11 -	}									ed at 8 ft a al. Ground	
ווי			Boring Location Coc Lat = 38.864776	ordinates:			-								not enc	ountei	red during nole backfi	
20000			Long = -77.053780				_ 12 -								with soi	l cuttir	າgs and ຣເ	ırface
2/00								1							complet		asphalt up	oon
2							_ 13 -	1							Paveme	ent Su	ımmary:	
בְּי								1							Surface fair con	Cours	se 1:	
2							<u> </u>	1							Surface	Cours	se 2:	
							<u> </u>	1							fair con Base Co	ourse:	,	
JAIN.							- 15 -	1							fair con	dition	(7 in)	
ź							ļ .	4										

L	_/		/VG/	1/V	Log	of E	Boring			LB.	-12		_	Shee	et 1		of	1
Proj	ect		Army Novy Drivo Co	mploto Strooto Project		Pr	oject No.			2700	າຣດດດ	5						
Loca	ation		Army Navy Drive Col	mplete Streets Project		Ele	evation ar	nd Da		2700	06000	5						
Drill	ina C	ompai	Arlington, Virginia			Da	ate Starte	4		App	rox. el	36.3		VD88) e Finishe	d			
	Ū	·	Free State Drilling				ate otarte	4		4	4/2/20	ı	Dak	o i illiono	u	4/2/	20	
Drill	ing E	quipm		stad Daill Dia		Co	ompletion	Dept	h		10 #		Roc	k Depth	Not Engl			
Size	and	Туре		<u> </u>		Nı	umber of S	Samn	les l	Distu	10 ft urbed		Τ.	Jndisturb	Not Enco ed	Core		
Cas	ing D	iamet	2-1/4-inch Hollow-Ste er (in)	em Augers	Casing Depth (ft)	-			103	First		5		Completic	- on	24 H		-
Cas	ina H	lamme	- or	Weight (lbs)	Drop (in)		ater Leve	` ′		∇		NE		Ţ	NE	Ī		-
	npler	iaiiiiii	-	-						onal	d Stidl	ham						
San	npler	Hamm	2-inch OD Split Spoo ner Auto	Weight (lbs)	Drop (in) 30	_ Fie	eld Engine	eer	Δr	mhai	r Gana	anath	.,					
			Auto	140	30					Sai	mple D		у		Poi	marks		
ATERI	SYMBOL	Elev. (ft)		Sample Description			Depth Scale	Number	Туре	ecov.	Penetr. resist BL/6in		/alue ws/ft)		Drilling Fluid, Loss, Drilli	Depth o	f Casing,	•)
Z ≥	0,	+36.3	ASPHALT (10 inch	nes thick)			<u> </u>	ž		œ	<u> </u>	10 20	30 40	ו	red throu			,
¥.		+35.5	() ()	,			- :							with	n a 6 incl gan drillir	core	bit.	
	\bigcirc	. 00.0		BBASE, Brown, fine to c ce clay, contains brick,			- 1 -				7			SS	1 to 3 ft	•		
	$\widetilde{\rho}$		moist (GW)	, ,,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			_	SS SS	_	12				llected B n 1 to 5		mpie B	-1
	\sim						_ 2 -	S	SS	4	14	26						
	(.). 0.,,	+33.3					- :				16							
	\bigcirc			o coarse, SAND WITH ledium dense, moist (G			3 -				9				gered to 3 to 5 ft	3 ft.		
	$\widetilde{\rho}$		r izz, trado diay, iii	iodiam donos, moist (c	,		- 4 -	S2	S	12	17	2						
	$\mathcal{C}_{C}}}}}}}}}}$							9	"		12		"					
	0,/~		FILL Down for A	CAND WITH	FINE ODAVE		5 -				8		$ \cdot $	Λ	gered to	5 ft		
	$\mathcal{S}_{\mathcal{S}}}}}}}}}}$		FILL, Brown, fine to FILL, trace clay, de	o coarse, SAND WITH ense, moist (GW)	FINE GRAVEL						43				5 to 7 ft			
	نن						- 6 -	S3	SS	18	16		35					
	0 .	+29.8	ASPHALT (6 inche	es thick)			£ :		SS SS		19		/					
		+29.3	,	o coarse, SAND FILL,	trace clay and fin	е	7 -		H		26 6				gered to			
			gravel, contains br	ick, medium dense, mo	oist (SW)			84	SS	∞	9		/	SS	7 to 8.5	ft		
		+28.1	TERRA OF DEROS	NT Linkshows for to			8 -	"			10	19						
		.07.0	trace silt, medium					H			4				gered to 8.5 to 10			
	77	+27.3	↑ TERRACE DEPOS ↑ trace silt, loose, me ↑ trace silt, loose, me	SIT, Red-brown, fine to object (SW)	coarse, SAND,	/	9 -	SS	SS	92	3				0.5 10 10	<i>J</i> 10		
		+26.3	TERRACE DEPOS SAND, trace silt, lo	SIT, Red-brown, fine to	coarse, CLAYEY	_	10		SS		3	6 •						
<u>{</u>			End of Boring at 10				10 -								mpleted ehole ca			
2			Boring Location Co	oordinates:			11 -								ger remo			ater
			Lat = 38.864565 Long = -77.052554	1			E '' :							dril	ling. Bore	ehole b	oackfille	
600			-				- - 12 -							pat	ched witl	h asph		
7,000								1							npletion.			
							13							Sui	vement S face Coι	ırse:	•	
4							-	1							conditio se Cours)	
Ž							14								or conditi		ipped (7 in)
2																		
SAN.							- 15 -											
							F :	1										

			/VG/	1/V	Log c		Boring			LB-	-13		_1	Sheet	1	of	1
	Project		Army Novy Drive Cor	mplete Streets Project		Pro	oject No.			2700	06000	5					
	Location		Almy Navy Drive Cor	implete Streets Project		Εle	evation an	d Da		2700	00000	ວ					
	Drilling C	omnai	Arlington, Virginia			Da	ate Started	1		App	rox. el	35.1		D88) Finished			
			Free State Drilling				ato otarto			4	4/2/20		Date	Timorioa		4/2/20	
	Drilling E	quipm		to d Duill Dia		Co	ompletion	Dept	h		1004		Rock	Depth	-4 -		
	Size and	Туре	CME-55 Track-Moun of Bit	ited Drill Rig		Nu	ımber of S	Samn	lec		10.2 ft ırbed		Uı	N ndisturbed	ot Enco	Core	
	Casing D	iamet	2-1/4-inch Hollow-Ste er (in)	em Augers	Casing Depth (ft)	┢			103	First		5	Co	ompletion	-	24 HR.	-
	Casing H	amme	-	Weight (lbs)	Drop (in)		ater Level	` ′		∇		NE		<u>Y</u>	NE	Ā	-
Z Z	Sampler	annine	-	-		L			R	onal	d Stidl	nam					
5	Sampler	Hamm	2-inch OD Split Spoo ner Auto	Weight (lbs)	Drop (in) 30	Fie	eld Engine	er	Δ	mhai	r Gana	anathi	,				
- Fog .	7 A		Auto	140		I	<u> </u>	_		Saı	mple D	ata			Ram	narks	
ie Geboir	MATERIAL SYMBOL	Elev. (ft)	:	Sample Description			Depth Scale	Number	Type	(in)	Penetr. resist BL/6in		/alue ws/ft)	(Dril	lina Fluid. D	ומו הט Depth of Casing Resistance, e	j, etc.)
:	2"	+35.1	ASPHALT (14.5 inc	ches thick)			0 -	Ž	Ė	œ	С-п	10 20	30 40			h pavemer	
7.09.7			,	,										with	a 6 inch	core bit. at 10:50.	
.C O.	h	+33.9	ACCRECATE SUR	BBASE, Red-brown, fine	o to coareo		- 1 -							SS 1	.2 to 3.2	ft	D 4
24/207	$^{\circ}$ \bigcirc° $\stackrel{<}{\sim}$		SAND WITH FINE	GRAVEL FILL, trace of							10				1.2 to 5	lk Sample ft.	D- I
			(GW)				- 2 -	S	SS	16	22		42 •				
ב ב							- 3 -		SS		17						
2	ŹŻ	+31.9		o coarse, CLAYEY SAN		_					7				ered to 3 .2 to 5.2		
7			GRAVEL FILL, me	edium dense, moist (SC	()		4 -		SS	_	8		$^{\prime} \mid$				
2								S2	SS	18	10	18					
2 - 20		+29.9					- - 5 -				11			Auge	ered to 5	.2 ft; grindi	na
0000		- 20.0	FILL, Brown, fine to fine gravel, loose, r	o coarse, SAND WITH	CLAY FILL, trace	_					8			and r	rig chatte 5.2 to 7.2	er at 4 ft.	9
20/2/			mie gravei, ieeee, i	moior (CC)			6 -	S3	SS	18	5	9			.2 10 7.2	. 11	
2		+28.4	AODUALT (44 : 1	(1:1)				0)	SS		4						
5			ASPHALT (11 inch	nes thick)			- 7 -				39	$ \cdot $			ered to 7	.2 ft; grindi	ng at
2	////	+27.5	FILL, Brown, fine to	o medium, SAND WITH	H CLAY, medium			4	6	_	27			7 ft. SS 7	.2 to 8.7	ft	
			dense, moist (SC)				- 8 -	S	SS	Ψ	9	18					
		+26.4	TERRACE DEPOS	SIT, Red-brown, fine, SA	AND WITH		 - 9 -								ered to 8 .7 to 10.		
			CLAY, loose, moist	t (SC)			- 9 -	S5	SS	12	3						
ב ב		.04.0					- 10 -	,		12	3	6 €		Com	plotod d	rilling of 11	.20
=	<i>y.</i> ; <i>y y</i> , <i>y</i> :	+24.9	End of Boring at 10	0.2 ft										Bore	hole cav	rilling at 11 red at 7.9 ft	t after
2			Boring Location Co	oordinates:			11 -							not e	encounte	al. Ground\ red during	
Š			Lat = 38.864080 Long = -77.051974	ļ												nole backfil ngs and su	
2000							12 -							patch	ned with pletion.	asphalt up	on
Z / 000							<u> </u>							'		ımmary:	
							13 -	-						Surfa	ace Cour	se:	
Į,														Base	condition Course	•	
A A							- 14 -							good	conditio	on (11.5 in)	
5							- 45	1									
SAIN.							- 15 -										
$\stackrel{\checkmark}{}$	1						F -	1	1	1				1			

1		ANUAN						Boring			LB	-14		_		Sheet	1	of	1
Γ	Project						Pro	oject No.					_						
h	Location		Army Navy Drive Co	mplete Streets Pro	oject		Ele	evation ar	nd Da	atum	270	06000)5						
			Arlington, Virginia								App	rox. e	I 31 (
	Drilling C	ompar	•				Da	ite Starte	d			410100			ate F	inished		4/0/00	
h	Drilling E	quipm	Free State Drilling ent				Со	mpletion	Dept	:h		4/2/20	,	F	Rock E	Depth		4/2/20	
L			CME-55 Track-Mour	nted Drill Rig								10 f	t				ot Enco	untered	
	Size and	Туре	of Bit 2-1/4-inch Hollow-St	em Augers			Nu	mber of	Samp	oles	Dist	urbed	į	5	Und	disturbed	_	Core	_
1	Casing D	iamete			C	asing Depth (ft)	Wa	ater Leve	l (ft.)		First	İ	NE	=	Cor	npletion	NE	24 HR.	
1	Casing H	amme	- er_	Weight (lbs)		Drop (in)	Dri	illing Fore	eman		<u> </u>	-	INE	=	<u> </u>	_	INE	<u> </u>	-
	Sampler		2-inch OD Split Spoo				_	Id Familia		R	onal	d Stid	ham						
	Sampler	Hamm		Weight (lhs)	140	Drop (in) 30	FIE	eld Engine	eer	А	mbe	r Gan	anatl	าง					
	۲Ą		71010		110				L .		Sa	mple D	ata				Par	narks	
200	MATERIAL SYMBOL	Elev. (ft)		Sample Descript	tion			Depth Scale	Number	ype	Recov.	Penetr. resist BL/6in	N (B	-Valı lows			ng Fluid, [Depth of Casir	
:	≥″	+31.0	ASPHALT (12 inch	hes thick)				<u> </u>	ž	┞_	ď	g = 0	10	20 30	40			g Resistance,	
2			ASI TIALI (IZ IIIGI	ies trion)					1							with a	6 inch	th paveme core bit.	
0. 5 9. 5	$\mathcal{O}(\cdot)$	+30.0	AGGREGATE SUE	BBASE, Brown, fine	e to co:	arse SANDY		1 -		╁							n drillin to 3 ft	g at 10:50	-
202	$\bigcirc \bigcirc \bigcirc$		GRAVEL FILL, tra-	ice clay, contains b				<u> </u>				11				Collec	cted Bu	lk Sample	B-1
4/24	o.∪ O. ().		(GW)					_ 2 -	S	SS	9	22			46,•	110111	1 10 5 11		
2	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	+28.5	CONCRETE (6 inc	ches thick)				<u> </u>				24 16							
		+28.0	•	SIT, Brown, fine, SA	AND W	/ITH CLAY.		3 -		H		2				Augei	red to 3	ft.	
			loose, moist (SC)	,		- ,		- :	1	SS		3				SS 3	to 5 ft		
<u> </u>								4 -	SS	SS	92	3	6 🕈						
								- :	1			5							
			TERRACE DEPOS	SIT, Brown, fine, SA	AND W	/ITH CLAY.		5 -				7	┨			Augei	red to 5	ft.	
			medium dense, mo			- ,		<u> </u>				5				SS 5	to 7 ft		
אַכָּכָּ		+25.0		SIT, Brown, fine to r	mediur	m, SAND, trace	_	6 -	S3	SS SS	92	5	10						
			silt, medium dense	e, moist (SP)				- :	1			6							
5		+24.0	TERRACE DEPOS	SIT, Brown, fine, SA	AND W	/ITH SILT,	_	7 -		H		22	1				red to 7		
			medium dense, mo	oist (SM)					S4	SS	12	6				SS 7	to 8.5 f	t	
<u>.</u>								8 -	"			4	10						
				SIT, Red-brown, fine	e, SAN	ND WITH SILT,		Ė :		H			1				red to 8		
			medium dense, mo	oist (SM)				- 9 -	SS	SS	12	9	$ \rangle$			SS 8.	5 to 10	π.	
]"		<u> </u>	6	15						
	<u>.1.1.4</u>	+21.0	End of Boring at 10	0 ft				10 -								Comp	oleted d	rilling at 1	1:20.
5			Boring Location Co	oordinates:												auger	remov	ed at 7.9 al. Ground	lwater
			Lat = 38.863412 Long = -77.051948					<u> </u>	†									ered during hole backt	
100			Long = -11.001040	,]							with s	oil cutti	ngs and s asphalt u	urface
								<u> </u>	†								letion.	aspriait u	pori
72/0								[:	}							Paver	ment Sı	ummary:	
<u> </u>								- 13 -	=							Surfac	ce Cour	rse: on (3 in)	
1								ļ , .								Base	Course	:	
2								<u> </u>								good	CONCIN	on (9 in)	
								<u> </u>											
2								<u> </u>											
31								F .	4			1	1 [

L	4	NL	1/V	Log	of E	Boring			LE	3-2		_	5	Sheet	1	of		1
Project		A Nana Daina Car	manufactor Other ata Durahant		Pro	oject No.			070	00000	\							
Location	l	Army Navy Drive Cor Arlington, Virginia	mplete Streets Project		Ele	evation ar	id Da			06000 rox. e		2 (NI/	WDs	88/				
Drilling (Compar				Da	te Started	ł		App	IOX. C	142.			nished				
Drilling E	-auinme	Free State Drilling			Co	mpletion	Dent	h		4/6/20)	Ro	ck D	enth		4/6/20		
Driming E	-quipini	CME-55 Track-Moun	nted Drill Rig			inplotion	Борі			10 f	t		OK D	•	Encou	ıntered		
Size and	Type o	of Bit 2-1/4-inch Hollow-Ste	em Augers		Nu	ımber of S	Samp	les	Dist	urbed		5	Undi	isturbed	_	Core		_
Casing I	Diamete		om ragoro	Casing Depth (ft)		ater Level	. ,		First	İ	NE		Com	pletion	NE	24 HR.		_
Casing I		r_	Weight (lbs)	Drop (in)	Dri	illing Fore	man	_	1	-1 O4:-1	l							
Sampler		2-inch OD Split Spoo		1= "	Fie	eld Engine	er	K	onai	d Stid	nam							
ട്ട് Sampler	Hamm	er Auto	Weight (lbs) 140	Drop (in) 30		ı		Α		r Gan		ny						
Report: Log - LANGAN MATERIAL SYMBOL SYMBOL	Elev. (ft) +42.2		Sample Description			Depth Scale	Number	Type		Penetr. resist aldw BL/6in Q	N (B	-Value lows/fl)	(Drilling Fluid Loss	Rema Fluid, Do Drilling		asing, ce, etc.	.)
	+41.9	ASPHALT (3.5 inch	hes thick)			0 -								Cored t				
49.48	+41.2	CONCRETE (9 inc	cnes tnick)											with a 6 Began	drilling			
			BBASE, Brown and grayce clay, dense, moist (38				SS 1 to	ed Bull		ole B-	-1
4 ///	+40.4		ine to medium, SAND			- - 2 -	S1	SS	16	11	19	,		from 2 t	o 5 ft.			
		contains mica, med	dium dense, moist (SC)				SS		8	$ \cdot $	$^{\prime}$						
		FILL Gray fine to	medium, CLAYEY SAN	ND FILL trace		3 -		H		8	1 /			Augere	d to 3	ft.		
			d, loose, moist (SP)	1 1 122, 11400		-				3 4	/			SS 3 to				
\$ ////						4 -	S	SS	9	2	6							
ARW////						-				4								
9////			ray, fine to coarse, CLA			5 -	_			4	┨			Augere		ft.		
		trace fine gravel, m	nedium dense, moist (S	SC)						5				SS 5 to	/ ft			
Z /////						6 -	S3	SS	8	7	12							
								SS		8	Ш							
\			o coarse, SAND FILL, t			- 7 -		S		2	1			Augered SS 7 to				
		gravei, contains wo	ood, medium dense, m	oist (SVV)			84	SS	4	5				33 / 10	0.5 11			
	.					- 8 - -		SS		10	15							
DISCIPLINE/GEOTECH			o coarse, CLAYEY SAN el, contains brick, loose			- 9 -			10	3	1 /			Augered SS 8.5				
		into to course grave	oi, comaine briok, loco.	5, 1110101 (00)		_ 9 -	S5	SS	9	5	9							
	+32.2					10 -				4				Cl-	امیدا	:!!:	40.5	-0
PATA)		End of Boring at 10	Ο ft			- :								Comple Borehol	e cave	ed at 8	.4 ft a	after
		Boring Location Co Lat = 38.865794	oordinates:			- 11 -								auger re				ater
ROI		Long = -77.061297	,											drilling. with soi	Boreh	ole ba	ckfille Lsurf	d ace
NLANGAN COMIDATANARIDATAO/270060005/PROJECT DATAN						12 -								patched	l with a			
0/270(- 1								Paveme			' :	
DATA						- 13 -								Surface fair con				
AAR							1							Concret	e Laye	èr. ´		
INDAT						<u> </u>								5554 00		(5 111)		
SON I						_ _ 15 _	1											
NGAN						<u> </u>												
<u> </u>						L ₁₆ -												

	\V <i>U</i>	4/V	Log o					LE	3-3			Sheet 1	of	2
Project	Army Navy Drive Complete Streets Project				ect No.			270	06000	5				
Location	Arlington, Virginia				ation an			Арр	rox. el	42.1				
Drilling Comp	Free State Drilling				Started				4/6/20			Finished	4/6/20	
Drilling Equip	CME-55 Track-Mour	nted Drill Rig		Com	pletion I	Deptl			25 ft		Rock I	Not Enco		
Size and Type Casing Diame	2-1/4-inch Hollow-St	em Augers	Casing Depth (ft)	Num	nber of S	amp	les	Dist First	urbed	8		disturbed - mpletion	Core 24 HR.	-
Casing Hamn	- ` `	Weight (lbs)	Drop (in)		er Level ng Forei	` ′		∇		13.5	Ţ		<u>Ā</u>	-
_	2-inch OD Split Spoo		-	Field	d Engine	er	Ro	nal	d Stidl	nam				
Sampler Ham	mer Auto	Weight (lbs) 140	Drop (in) 30	<u> </u>			An		r Gana			1		
Sampler Ham Sampler Ham TVINGENIA TVINGEN		Sample Description			Depth Scale	Number	Туре	Recov. (in)	Penetr. resist BL/6in	N-V (Blov	alue vs/ft) 30 40	Ren (Drilling Fluid, Fluid Loss, Drillin	narks Depth of Casing g Resistance, e	, tc.)
142	ASPHALT (13 inch	hes thick)		-	- 0 <u> </u>					10 20	1	Cored throug	core bit.	nt
+41. +40.	8 AGGREGATE SUB FILL, trace fine to (GC) FILL, Brown, CLA	BBASE, Gray, CLAY W medium sand, medium Y FILL, trace fine to coa	n dense, moist	<u> </u>	- 1 - - 1 - - 2 -	S1	SS	16	12 5	10+		Began drillin SS 1 to 3 ft	g at 10:15.	
+39	FILL, Brown, SILT	Y SAND FILL, trace fin ica and wood, loose, m		- - - - - - -	- 3 - - 3 - - 4 -	S2	SS	18	3 3	7•		Augered to 3 SS 3 to 5 ft	3 ft.	
		Y SAND FILL, trace fin rick, mica, and wood, lo		- - - - - - - - - - - - - - - - - - -	- 5 - - 5 - - 6 -	S3	SS	18	4 5 3	6+		Augered to 5 SS 5 to 7 ft	oft.	
	FILL, Brown, SILT moist (SM)	Y SAND FILL, contains	s mica, very loose,	- - - - - -	- 7 - - 7 - - 8 -	S4	SS	10	1 1 2	•		Augered to 7 SS 7 to 9 ft	ft.	
	FILL, Brown, SILT fragments, very loc	Y SAND FILL, contains ose, moist (SM)	s mica and rock	- - - - - -	- 9 - 	SS	B	10	1 2 1 2	•		Augered to 8 SS 8.5 to 10	3.5 ft. ft	
				-	- 10 - - - 11 - - - 12 - -				1			Augered to 1	3.5 ft.	
+28.	6 TERRACE DEPOS soft, wet (CL)	SIT, Brown, CLAY WITI	H FINE SAND,	<u> </u>	- 13 - - - - 14 -	98	SS	18	1 2	.•		SS 13.5 to 1	5 ft	
+28				Y	- 15 - - 15 - - 16 -				2			Augered to 1	8.5 ft.	



LB-3 Log of Boring Sheet of 2 Project Project No. Army Navy Drive Complete Streets Project 270060005 Location Elevation and Datum Approx. el 42.1 (NAVD88) Arlington, Virginia Sample Data Remarks Elev Depth N-Value (Blows/ft) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale 10 20 30 40 +26. 16 17 18 SS 18.5 to 20 ft TERRACE DEPOSIT, Brown, fine to medium, SAND, trace SS silt and fine gravel, loose, wet (SP) 9 S7 2 3 20 Augered to 23.5 ft. 21 22 /ILANGAN.COM/DATA/AR/DATA0/270060005/PROJECT DATA_DISCIPLINE\GEOTECHNICAL\GINTLOGS\270060005 - ARMY NAVY DRIVE.GPJ 23 TERRACE DEPOSIT, Brown, fine to coarse, GRAVEL SS 23.5 to 25 ft SS 6 WITH SAND AND SILT, medium dense, wet (GW) 88 9 12 9 25 Completed drilling at 11:25. End of Boring at 25 ft Groundwater observed at Boring Location Coordinates: Lat = 38.865602 15.9 ft inside augers. Borehole caved at 18 ft after 26 Long = -77.060784auger removal. Groundwater observed at 15.1 ft. Borehole backfilled with soil cuttings 27 and surface patched with asphalt upon completion. 28 Pavement Summary: Surface Course: good condition (5 in) Base Course: 29 good condition (8 in) 30 31 32 33 34 35

4		1/VU/A	v	Log o					LB	-4			Sheet	1	of	1
	Project	Army Navy Drive Complete		Proje	ect No.			2700	60005							
ł	Location	Army Navy Drive Complete	e Sifeets Project		Eleva	ation an	d Da		2700	00000)					
	Drilling Comp	Arlington, Virginia			Date	Started	I		Appr	ox. el	44.7 (D88) Finished			
		Free State Drilling							4	1/3/20					4/3/20	
	Drilling Equip	•	nrill Dia		Com	pletion I	Depth	1		10 ft		Rock	Depth	Encou	ntorod	
ŀ	Size and Typ	•			Num	ber of S	Sampl	es	Distu	10 ft rbed		Un	disturbed	Encou	ntered Core	
ŀ	Casing Diam	2-1/4-inch Hollow-Stem Aumeter (in)		sing Denth (ft)		er Level			First		5	Co	mpletion	-	24 HR.	-
ŀ	Casing Hami	- omer Weig	ıht (lbs)	-		ng Forei	` ′		$\overline{\Delta}$		NE	1		NE	Ā	-
PAIN	Sampler	2-inch OD Split Spoon	<u>-</u>	- ' ' -				Ro	nalo	l Stidh	am					
Ž Z	Sampler Han		ght (lbs)	Drop (in) 30	Field	Engine	er	An	nber	Gana	pathy					
Log	1 P P		140	00		D 11	_		San	nple Da	ita			Rema	arks	
report	MATERIAL SYMBOL (tt	ft) Sam	ple Description			Depth Scale	Number	Туре	(in)	Penetr. resist BL/6in	N-Va (Blow	vs/ft)	(Drilling	Fluid, De	epth of Casing Resistance, et	tc.)
:	+44	ASPHALT (9.5 inches th	nick)		+	- 0 —	z			ш ш	10 20	30 40			n pavemen	-
0.00	+43	3.9			F	=									ore bit. at 13:15.	
20.02	500	AGGREGATE SUBBASE clay, medium dense, mo		EL FILL, trace	E	· 1 —		Ħ		12			SS 1 to	3 ft	s Sample E	R_1
24/20	7.7.7.43	FILL, Brown, fine to coar	rse SAND WITH CI	AY FILL trace	+	_	_	SS	16	9	45.		from 2		(Campic L	5 -1
ر 4		fine gravel, contains bric			F	2 -	S	Š	-	6	15•					
- - - - -		5". D	CAND WITH O	A)/ = 11 1 /	Ē	3 -				6			Augoro	d to 2 :	f4	
2		FILL, Brown, fine to coar fine gravel, loose, moist		.AY FILL, trace	Ė	=				3			Augere SS 3 to		it.	
Ž.					F	4 -	S2	ss	18	. 4	8					
Y INIX					F	=				4						
/ - con		FILL, Brown, fine to coar	rse, SAND WITH CL	AY FILL, trace	F	5 -				4			Augere		ft.	
00007		fine gravel, loose, moist	(SC)		Ė	-				4			SS 5 to	7 ft		
אפטר					-	6 –	S3	SS	18	3	7					
	//// 	7.7			F	_ =		SS		3						
7AL 6		TERRACE DEPOSIT, Br moist (SC)	rown, fine, SANDY C	CLAY, soft,	F	7 - -		Ī		1			Augere SS 7 to		ft.	
		moiot (GG)			Ė	8 -	\$	SS	16	1			q _u =1.5		sf	
		TERRACE DEPOSIT D	OANDY OLAY		Ė					2			Augere	d to 8	5 ft	
ואבו		TERRACE DEPOSIT, Br gravel, soft, moist (SC)	own, SANDY CLAY	, trace fine	F	9 -		SS	_	1			SS 8.5	to 10 f	t t	
7					F	=	SS	SS	16	2			q _u =0.5	ısı		
Ξ {	<u>////</u> +34	End of Boring at 10 ft			+	10 -		月		2			Comple	eted dr	illing at 13:	45.
5		Boring Location Coordin	nates:		Ė	=									ed at 8.5 ft I. Groundv	
OUEC		Lat = 38.865612 Long = -77.059906			E	· 11 –									ed during ole backfill	ed
77.00					F	-							with so	il cuttir	igs and sui asphalt upo	rface
nnonn					F	· 12 —							comple			
AUZ/					Ė	- - 13 -							Pavem Surface		mmary:	
NOA!					E	-							good co	onditio		
AA					-	14 -							Base C good co		n (6.5 in)	
JAIN.					F	=										
7.0.					F	15 -										
Ć N					þ	=										

	4	/VU/	4/V	Log		Boring	-		LB	-5		_	S	heet	1	of	1
Project		Army Navy Drivo Co	emplete Streets Project		Pr	oject No.			2700	06000	5						
Location	1	Allily Navy Drive Co	implete Streets Project		Ele	evation an	nd Da		2700	0000	<u>J</u>						
Drilling	Compa	Arlington, Virginia			Da	ate Started	1		Appı	ox. el	45.9			ished			
		Free State Drilling				ato otal to	•		2	1/6/20		Dat	O 1 III	ioriod		4/6/20	
Drilling	Equipm				Co	mpletion	Dept	h		40.5		Roc	k De	•	_		
Size an	д Туре	CME-55 Track-Mour of Bit	nted Drill Rig		Ni	umber of S	Comp	loo	Distu	10 ft irbed		<u> </u>	Jndis	Not sturbed	Encou	Intered Core	
Casing	Diamet	2-1/4-inch Hollow-Ster (in)	em Augers	Casing Depth (ft)	+			ies	First		5		Comr	oletion	-	24 HR.	-
		-	Weight (lbs)		\perp	ater Level	. ,		∇		NE		Ţ		ΝE	Ā	-
Casing Sample		er	vveignt (ibs)	Drop (in)	-	illing Fore	IIIaII	R	onalo	d Stidl	ham						
Sample		2-inch OD Split Spoo	Weight (lbs)	Drop (in)	Fie	eld Engine	eer										
5	I	Auto	140	30				Ar		Gana		у			_		
MATERIAL SYMBOL	Elev. (ft) +45.9		Sample Description			Depth Scale	Number	Туре	Recov. (in)	Penetr. resist BL/6in	N- (Bl	Value ows/ft)	0		Rem Fluid, D , Drilling	arks epth of Casir Resistance,	ng, etc.)
: E	+45.9	ASPHALT (5.5 inc	ches thick)			- 0 -	-				10 2	0 30 4				h paveme	nt
		CONCRETE (7.5 i	inches thick)			[]										core bit. at 11:00	
	+44.8		BBASE, Gray, fine, GRA			1 -		I		32				SS 1 to		k Sample	B-1
	+44.2		d and clay, very dense, fine to coarse, SAND W			- 2 -	S	SS	18	8	16.			from 2 t			
			gravel, contains mica,				S	SS	_	8	16•						
	+42.9		5			3 -				9				Augoro	d to 0	4	
	,		fine to coarse, SANDY lay, medium dense, mo							8				Augered SS 3 to		IL.	
						4 -	S2	SS	8	7	14						
										7_							
	+40.9	FILL Red-brown f	fine to coarse, SAND F	II I trace clay and	4	5 -		H		7 6				Augere	d to 5	ft.	
			ns mica, medium dense			-				6				SS 5 to	7 ft		
99						6 -	S3	SS	10	10	16•						
										6							
	+38.9		fine to coarse, SAND V			7 -				2				Augered			
		trace fine to coarse	e gravel, medium dens	e, moist (SC)			84	SS	16	10				33 / 10	0.5 11		
						- 8 - 				8	18						
	+36.9					9 -				3				Augered SS 8.5	to 10		
		FILL, Brown, CLAN gravel, stiff, moist	Y WITH FINE SAND FI (SC)	LL, trace fine			SS	SS	18	6	13			q _u =1.0 t	sf		
	+35.9	End of Boring at 10	O ft			10 -		LĒ		7				Comple	ted di	rilling at 1	2·15
<u> </u>		•					1							Borehol	e cav	ed at 8.1	ft after
		Boring Location Co Lat = 38.865479				11 -								not enc	ounte	al. Ground red during	1
Ž L		Long = -77.05911	8											with soi	l cuttir	nole backt ngs and s	urface
000						12 -								patched		asphalt u	pon
ANGAN', CUMILA I AARIDA I AUZ TOGOGOGOFRADECI						<u> </u>	1							•		ımmary:	
3						_ 13 _	1							Surface good co	Cour	se:	
						Ė :	-							Base Co	ourse:	, ,	
						- 14 -	1							Concret	te Lay		
5						15 -								good co	onditio	n (7.5 in)	
200						13 -	1										
₹I	1					Г -	1										

_		7	/VG/	Log		oring			LE	B-6				Sheet	1	of	2		
Pr	oject		Army Navy Drive Co	mnlete Streets Pro	niect		Pro	oject No.			270	06000	5						
Lc	cation		Almy Navy Blive Co.	inpicte directs i re	oject		Ele	vation an	d Da		210	00000	<u> </u>						
Dr	illing C	omnar	Arlington, Virginia				Da	te Started	1		App	rox. e	45			D88) Finished			
	iiiiig O	ompai	Free State Drilling					to otal to	•			4/3/20)		Julio I	i ii iioi iou		4/3/20	
Dr	rilling E	quipm					Co	mpletion	Depth	1		05.0		F	Rock	Depth			
Si	ze and	Туре					Nu	mber of S	comp	loc	Dist	25 ft urbed			Un	NC disturbed	t Encou	Core	
Cá	asing D	iamete	2-1/4-inch Hollow-Sto	em Augers		Casing Depth (ft)				162	First			8	Со	mpletion	-	24 HR.	-
				Weight (lbs)		Drop (in)		ater Level	` ′		∇		N	E		<u>Z</u>	NE	$ar{ar{ar{\Lambda}}}$	-
_	asing H ampler	iamme	-		-	- Diop (III)	-	iiiig i oic	man	R	onal	d Stid	ham	1					
ÿL_	ampler	Hamm	2-inch OD Split Spoo	Weight (lhs)		Drop (in)	Fie	ld Engine	er										
Log -			Auto		140	Brop (III) 30	1			A		r Gana mple D		thy					
10 PM Report: Log	MATERIAL SYMBOL	Elev. (ft) +45.6		Sample Descrip	tion			Depth Scale	Number	Type	Recov. (in)	Penetr. resist BL/6in	(I 10	N-Vali Blows	/ft)	(Drillin Fluid Lo	Rem ng Fluid, D ss, Drilling	arks lepth of Casing Resistance, e	itc.)
: Z D		+45.2	ASPHALT (5 inche	·				— 0 —						Ť				h pavemer	nt
50:10	4		CONCRETE (9.5 i	nches thick)				 - 1 -										core bit. gat 12:00.	
020 3	ؙۣؽٙڹ	+44.4	AGGREGATE SUE					- ' -								00.4			
124/2	///	+43.9	FILL, trace silt, me					- 2 -				21				SS 1.	5 to 3.5	π	
<u>.</u>			fine gravel, contair moist (SC)						S1	SS	9	2	•						
DRIVE.GP			moier (00)					- - 3 -				2	$ \cdot $						
											_	2	-			Auger	ed to 3	.5 ft.	
<u>`</u>								_ 4 _				2 5					5 to 5.5		
ARM.									S2	SS	18	5	10						
96/								_ 5 -				4							
9002			FILL, Brown, fine to									3	1				ed to 5.5 to 7 ft		
368/			fine gravel, contair	ns glass, medium o	aense	, moist (SC)		- 6 -	S3	SS	18	5				33 3.	<i>J</i> 10 <i>T</i> 11		
		+38.6										7	12						
CALIG			FILL, Light brown, medium dense, mo		ND F	ILL, trace clay,		- 7 - 		SS		5		$\ \ $			ed to 7 to 8.5 ft		
		+37.9	FILL, Brown, CLAY	Y FILL, trace fine to	o coa	rse sand, very		_ 8 -	S ₄	SS	19	10	18			q _u =1.5	5 to 2.0	tsf	
		+37.1	FILL, Brown, fine,	SAND FILL, trace	clay a	and cinders,						3	-				ed to 8		
			medium dense, mo	oist (SP)				_ 9 -	SS	SS	16	10				SS 8.	5 to 10	ft	
SISCI									,			9	19	•					
[- 10 -								Auger	ed to 1	3.5 ft.	
CT D																			
ROJE								- 11 -											
005\P								_ _ 12 _											
20060								- ' - -											
TA0\2								_ _ 13 _											
ANDA A	///	+32.1	TERRACE DEPOS	SIT. Orange-brown	. fine	to coarse. SAND						2				SS 13	5.5 to 15	5 ft	
ĕ[//		+31.3	WITH CLAY, medi	ium dense, moist ((SC)			14	S6A	S	8	3 4							
COM/IDATAVAR/DATA0/270060005/PROJECT DATA_DISCIPLINE/GEOTECH	7//	31.5	TERRACE DEPOS medium dense, mo		WITH	FINE SAND,		- -	S6B	⊨	-	10	14			q _u =5.0) tsf		
NA NA NA NA NA NA NA NA NA NA NA NA NA N								_ 15 _	JUB			-				Auger	ed to 1	8.5 ft.	
Y Y																			



/\LANGAN.COM\DATA\AR\DATAO\Z70060005\PROJECT DATA_DISCIPLINE\GEOTECHNICAL\GINTLOGS\Z70060005 -

LB-6 Log of Boring Sheet of 2 Project Project No. Army Navy Drive Complete Streets Project 270060005 Location Elevation and Datum Approx. el 45.6 (NAVD88) Arlington, Virginia Sample Data Remarks Elev Depth N-Value (Blows/ft) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale 10 20 30 40 +29.6 16 17 18 SS 18.5 to 20 ft TERRACE DEPOSIT, Brown, fine, CLAYEY SAND, SS medium dense, moist (SC) 9 S7 5 20 Augered to 23.5 ft. 21 22 23 SS 23.5 to 25 ft TERRACE DEPOSIT, Brown, fine to medium, CLAYEY SS 2 SAND, medium dense, moist (SC) 88 9 3 8 +20.6 25 Completed drilling at 13:00. End of Boring at 25 ft Borehole caved at 23.5 ft Boring Location Coordinates: Lat = 38.865471 after auger removal. Groundwater not 26 Long = -77.058293encountered during drilling. Borehole backfilled with soil cuttings and surface patched 27 with asphalt upon completion. Pavement Summary: Surface Course: 28 good condition (2 in) Base Course 29 fair condition (3 in) Concrete Layer good condition (9.5 in) 30 31 32 33 34 35

L	ANGAN					oring			LE	B-7		-	Sheet	1	of	1
Project					Pro	ject No.										
Location		, ,	mplete Streets Project		Ele	vation an	d Da	itum		06000						
Drilling C	`omna	Arlington, Virginia			Da	te Started	1		App	rox. el	43.5		D88) Finished			
Drilling E		Free State Drilling				mpletion		h		4/3/20					4/3/20	
Drilling	quipm	ent CME-55 Track-Moun	nted Drill Ria			mpletion	рері	n		10 ft		ROCK	Depth Not E	Encour	ntered	
Size and	Туре	of Bit 2-1/4-inch Hollow-Sto			Nu	mber of S	Samp	les	Dist	urbed	5	Un	ndisturbed		Core	-
Casing D	Diamet		elli Augels	Casing Depth (ft)	Wa	ater Level	(ft.)		First				mpletion		24 HR.	
Casing H	łamme	_ - er_	Weight (lbs)	Drop (in)		lling Fore	` ′		<u> </u>		NE		<u> </u>	IE	<u> </u>	
Sampler		2-inch OD Split Spoo			- Eio	ld Engine	or	R	onal	d Stidl	nam					
Sampler Sampler	Hamn		Weight (lbs) 140	Drop (in) 30	Trie	na Engine	er	Aı	mbe	r Gana	apathy	,				
.: Log					•	Б 11	_		Sa	mple D	ata		-	Rema		
Report: Log MATERIAL SYMBOL	Elev. (ft) +43.5		Sample Description			Depth Scale	Number	Type	Recov.	Penetr. resist BL/6in	(Blo	alue ws/ft) 30 40			pth of Casing Resistance, e	tc.)
18 PM	+43.0	ASPHALT (6.5 inc	hes thick)			— 0 —							Cored the		pavemen	ıt
3:50:18	+42.3	CONCRETE (8.5 i	nches thick)			- 1 -									at 11:10.	
	+42.0		BBASE, Brown, fine, GF					F		18			SS 1.5 t			
42////	SAND FILL, trace clay, medium dense, moist (GW) FILL, Brown, fine to coarse, CLAYEY SAND FILL, tra fine gravel, contains asphalt and brick, medium dense					_ 2 _		SS		6			Collecte		Sample E	3-1
B ////	fine gravel, contains asphalt and brick, medium dense moist (SC)						S	SS	14	11	17 •					
	. 40.0					- 3 -				8						
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	+40.0 +39.9 +39.8	(_ :		I		6			Augered SS 3.5 t			
Z / / / / / / / / / / / / / / / / / / /		FILL, Brown, fine to	o coarse, SAND WITH	CLAY FILL, trace	/ e	- 4 - 	S2	SS	12	4			000.00	0.01	•	
8////		fine gravel, contair moist (SC)	ns asphalt and brick, m	edium dense,		 - 5 -	S	S	_	10						
	+38.0 +37.8	BRICK (3 inches the	nick)							6			Augered	to 5.5	i ft.	
28/2/	-07.0	FILL, Brown, fine to	o coarse, SAND WITH			- 6 -	S3	SS	_	5 4			SS 5.5 t			
Ŏ <i>ĺ////</i>		GRAVEL FILL, COR	ntains brick, loose, moi	isi (SC)			S	S	~	3	7 🕴					
	+36.5	FILL, Gray, CLAY,	medium stiff, moist (C	H)		_ 7 _				2			Augered SS 7 to		t.	
	+36.0		coarse, CLAYEY SAN	ND, medium			S4	SS	18	5			q _u =3.0 ts		ft	
GEOTECH /////	+35.0	dense, moist (SC)				- 8 - 				10	15					
		TERRACE DEPOS medium dense, mo	SIT, Brown, fine, CLAYED ist (SC)	EY SAND,		- - - 9 -				4			Augered SS 8.5 t			
□		,	(- /				SS	SS	16	9	19					
	+33.5	End of Doring at 10) #			_ _ 10 _				10			Complet	ed drii	lling at 11:	·45
DATA		End of Boring at 10											Borehole	e cave	d at 8 ft a	fter
JECT		Boring Location Co Lat = 38.865265				- - 11 -							not enco	ountere	. Groundved during	
ON PRO		Long = -77.057246	5										with soil	cutting	ole backfil gs and su	rface
90009						_ 12 _							patched complet		sphalt up	on
0/2700													Paveme			
DATA						- 13 - - -							Surface fair cond			
AVAR						 - 14 -							Base Co	ourse:`	,	
MDAT						- ' - -	-						Concrete good co	e Laye	r.	
Z.CON						_ 15							9500 00	ididoll	(0.0 111)	
\\LANGAN.COMIDATA\AR\DATA\)\Z70060006\PROJECT DATA\.																
 						- 16 -										

	4	/VU/	1/V	Log c		Boring			LE	8-8			Sheet	1	of	2
Project		Army Navy Drive Co	mplete Streets Project		Pro	oject No.			270	06000	5					
Location	l		impiete Gaedis i Toject		Ele	evation ar	id Da	tum								
Drilling (Compa	Arlington, Virginia			Da	ite Started	ł		App	rox. el	41.3 (088) Finished			
Drilling E	auinm	Free State Drilling			Co	mpletion	Dont	<u> </u>		4/3/20		Rock I	Donth		4/3/20	
Drilling	quipm	ent CME-55 Track-Moun	nted Drill Rig		0	mpletion	Depti	1		25 ft		ROCK I	•	Encou	ntered	
Size and	Туре		-		Nu	ımber of S	Samp	les	Dist	urbed	8	Un	disturbed	_	Core	_
Casing [Diamet			Casing Depth (ft)	Wa	ater Level	(ft.)		First		NE	Col	mpletion	NE	24 HR.	
Casing I	lamme	- er_	Weight (lbs)	Drop (in)	Dri	illing Fore	man		_				<u></u>	NL	<u>-¥</u> -	
Sampler		2-inch OD Split Spoo			Fie	eld Engine	er	R	onal	d Stidl	ham					
Sampler	Hamn	ner Auto	Weight (lbs) 140	Drop (in) 30				Ar			apathy		1			
MATERIAL SYMBOL	Elev. (ft) +41.3		Sample Description			Depth Scale	Number	Type		Penetr. resist aldw BL/6in G		vs/ft)	(Drilling Fluid Loss	Rema Fluid, De , Drilling	arks epth of Casing Resistance, e	tc.)
: 	+40.8	ASPHALT (6 inche	es thick)			0 -							Cored t	hrough	n pavemen	nt
2.00.5		CONCRETE (11 in	iches thick)			- 1 -									at 09:05.	
	+39.9	AGGREGATE SUB	BBASE, Gray and browr	n, fine, SANDY . moist (GW)						13			SS 1.5	to 3.5	ft	
2 4 D. V.	+39.1	FILL, Red-brown, fi	ine to medium, SAND F			- 2 - - :	S1	SS	10	6	12 †					
Z		medium dense, mo	JIST (SP)			3 -		SS SS		6 5						
_ } ¥						- 4 -				2			Augere			
						- 4	S2	SS	20	10	21					
	+36.3	BRICK (8 inches th	nick)			5 -				11 8						
	+35.6	,								4			Augere		5 ft.	
						- 6 -	S3	SS	9	3	5		00 0.0	10 7 11		
		No Recovery				7 -				2			Augere	d to 7 1	ft.	
			cate clayey sand and br	ick fill, very loose			82	SS	0	1 WOH			SS 7 to	8.5 ft		
	+32.8					- 8 -	,			1						
	132.0		/ FILL, trace fine to coand brick, very soft, mois			- - - 9 -		SS		1			Augere SS 8.5	to 10 f	5 ft. t	
		·	•	, ,			SS	SS	14	1 2			q _u =0.75	tsf		
						10 -				1			Augere	d to 13	3.5 ft.	
						<u> </u>										
						- 11 - 										
						12 -										
NA CONTRACTOR OF THE PROPERTY						13 -										
	+27.8	TERRACE DEPOS sand, very soft, mo	SIT, Brown, CLAY, trace	fine to coarse						WOH			SS 13.5	5 to 15	ft	
	1	Sand, very son, inc	7.5t (OL)			- 14 - -	Se	SS	12	WOH			q _u =0.5 t	sf		
	1					15 -				1			Augere	d to 18	8.5 ft.	
SI ///	1					<u> </u>									- **	

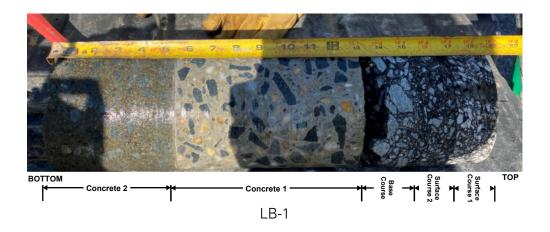


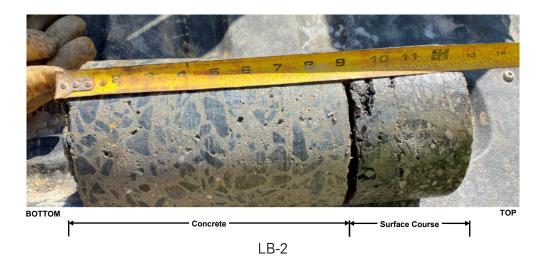
LB-8 Log of Boring Sheet of 2 2 Project Project No. Army Navy Drive Complete Streets Project 270060005 Location Elevation and Datum Approx. el 41.3 (NAVD88) Arlington, Virginia Sample Data Remarks Elev Depth N-Value (Blows/ft) Sample Description (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.) (ft) Scale 10 20 30 40 +25. 16 17 18 +22.8 SS 18.5 to 20 ft TERRACE DEPOSIT, Brown, fine to medium, SAND, trace SS fine to coarse gravel and clay, medium dense, moist (SP) 12 S7 6 20 Augered to 23.5 ft. 21 22 NLANGAN.COM/DATANAR/DATA0/270060005/PROJECT DATA_DISCIPLINE\GEOTECHNICAL\GINTLOGS\270060005 - ARMY NAVY DRIVE.GPJ 23 SS 23.5 to 25 ft TERRACE DEPOSIT, Red-brown, fine to coarse, SS GRAVELLY SAND, trace clay, medium dense, moist (GW) 88 12 10 17 +16.3 25 Completed drilling at 10:15. End of Boring at 25 ft Borehole caved at 23.2 ft Boring Location Coordinates: Lat = 38.865145 after auger removal. Groundwater not 26 Long = -77.056385encountered during drilling. Borehole backfilled with soil cuttings and surface patched 27 with asphalt upon completion. Pavement Summary: Surface Course: 28 good condition (3.5 in) Base Course: 29 good condition (2.5 in) Concrete Layer good condition (11 in) 30 31 32 33 34 35

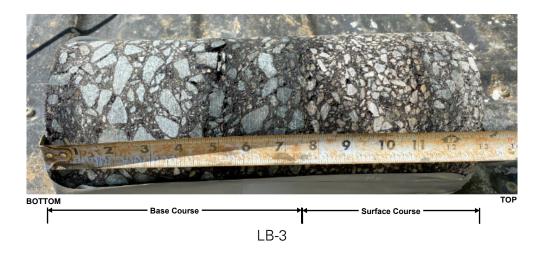
1		4		1/V	Log		Boring			LE	3-9		_	Sheet	1	of	1
	Project		Army Navy Drive Cor		Pr	oject No.			270	06000	5						
İ	Location		,	<u></u>		Ele	evation ar	nd Da	tum				/NIAN	(D00)			
ŀ	Drilling C	ompar	Arlington, Virginia			Da	ate Started	d		Арр	rox. el	39.7		Finished			
ŀ	Drilling E	quipm	Free State Drilling ent			Co	ompletion	Dept	h		4/6/20	<u> </u>	Rock	c Depth		4/6/20	
ļ	Size and	Time	CME-55 Track-Moun	ited Drill Rig							10.5 ft urbed	:	<u> </u>	Not Er			
		,,	2-1/4-inch Hollow-Ste	em Augers	0 : 0 !! (6)	Nι	umber of S	Samp	les			5		ndisturbed .	-	Core	-
ļ	Casing D		-		Casing Depth (ft)		ater Level	. ,		First		NE		ompletion NE		24 HR. <u>V</u>	-
۶L	Casing F Sampler	lamme	-	Weight (lbs)	Drop (in)	- Dr	illing Fore	man		onal	d Stidl	ham					
2	Sampler	Hamm	2-inch OD Split Spoo	Weight (lbs)	Drop (in) 30	_ Fie	eld Engine	eer	٨	mbo	r Can	an ath					
- 601			Auto	140	30			L	A	Sa	r Gana	ata		Ь	ema		
Report	MATERIAL SYMBOL	Elev. (ft) +39.7	:	Sample Description			Depth Scale	Number	Туре	Recov.	Penetr. resist BL/6in	N-\ (Blo 10 20	/alue ws/ft) 30 40	(Drilling Flu			g, etc.)
MM	ACCONDICTOR	+39.2	ASPHALT (5.5 inch	*			<u> </u>							Cored throwith a 6 in			nt
3.30.3		+38.5	CONCRETE (9 inc	nes trick)			_ 1 -							Began dri			
0707/+	$^{\circ}$ \bigcirc $^{\circ}$ \bigcirc $^{\circ}$			BBASE, Gray, fine to co FILL, medium dense,							20			SS 1.3 to Collected			B-1
4/2	\$.0. <i>.</i> >						_ 2 -	S	SS	16	15	27	,	from 2 to		·	
10.0		+36.7					3 -]"	SS		12						
<u></u>			BRICK (16 inches t	thick)							10			Augered t	o 3.5	5 ft; grindi	ng at
NA I		+35.4					- 4 -				12 18			3 ft. SS 3.5 to	5.5 f	ft	
MY - CC		100.4.		o coarse, SAND WITH ns brick and cinders, me		е	5 -	S2	SS	16	10	28	3				
7,000007			FILL, Brown, fine to	o medium, SAND WITH	H CLAY FILL,			_			7			Augered t			
200			medium dense, mo	JIST (SC)			- 6 -	S3	SS	20	8	18 +		00 0.0 to	7.01		
							- 7 -]"		"	10						
		+32.2	BRICK (6 inches th	nick)							10 5			Augered t		5 ft.	
		+31.7		SIT, Light brown, fine to			8 -	84	SS	16	6			SS 7.5 to	9 ft		
			•	edium dense, moist (S	,		9 -		ΙĒ		9	15					
				SIT, Light brown, fine to edium dense, moist (SP					SS		5			Augered t			ter at
							10	SS	SS	16	10	19		SS 9 to 10	J.ວ II		
<u> </u>		+29.2	End of Boring at 10).5 ft							10			Complete Borehole			
			Boring Location Co	ordinates:			- 11 -							auger rem	noval	I. Ground	water
LI COO			Lat = 38.864921 Long = -77.055275	,			12 -							drilling. Bo	oreho	ole backfi	lled
727000C														patched w	vith a		
¥ E							- 13 -	1						Pavemen			
AAR							- - 14 -							Surface C	dition		
MUA								1						Base Cou good cond Concrete	dition		
AN.C							15							good cond			
ANG							Ē :	1									

Appendix C

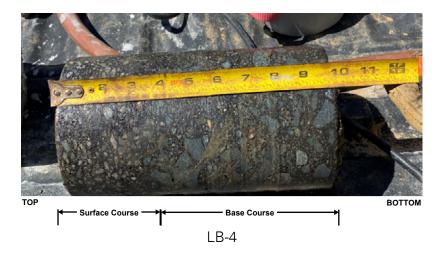
Pavement Core Photographic Log

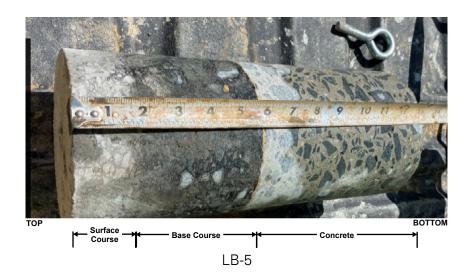


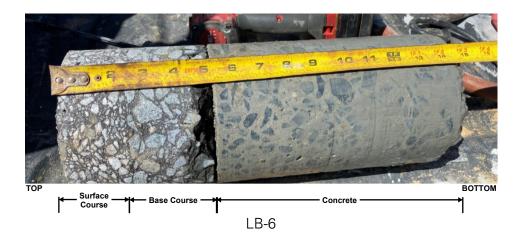




Langan Engineering
Langan Project No.: 270060005 Page 1 of 5 Appendix C

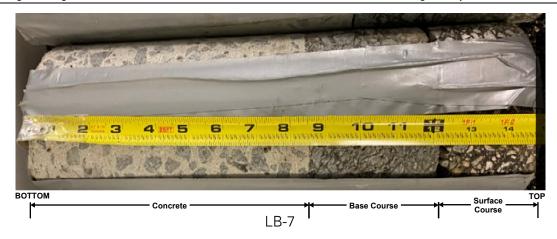


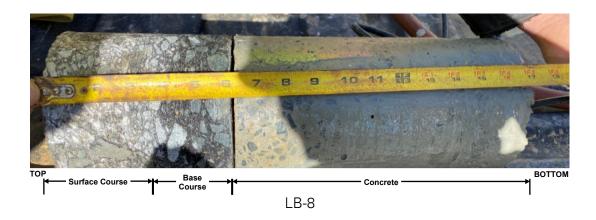


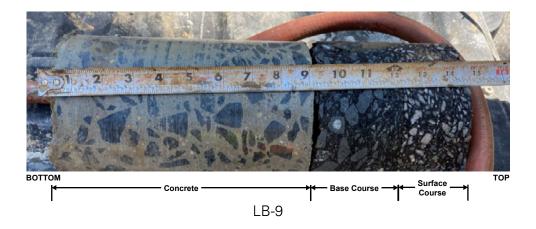


Langan Engineering
Langan Project No.: 270060005 Page 2 of 5 Appendix C

Langan Project No.: 270060005

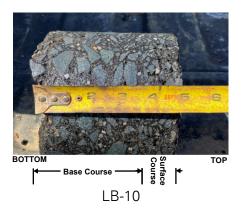


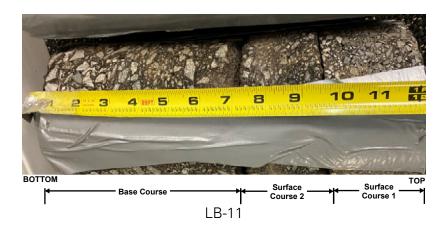


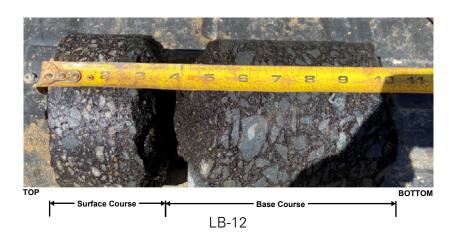


Langan Engineering
Langan Project No.: 270060005 Page 3 of 5

Langan Project No.: 270060005

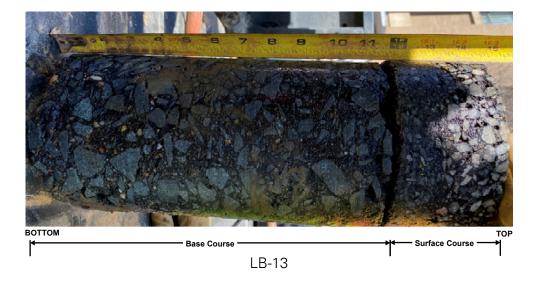


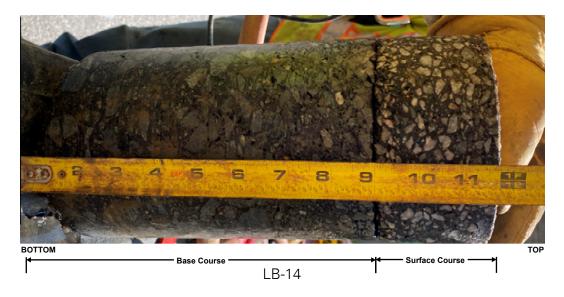




Langan Engineering
Langan Project No.: 270060005 Page 4 of 5 Appendix C

Langan Project No.: 270060005





Langan Engineering
Langan Project No.: 270060005 Page 5 of 5 Appendix C Appendix D

Laboratory Test Data

SUMMARY OF LABORATORY TESTING

ARMY NAVY DRIVE COMPLETE STREETS PROJECT

PROJECT NO. 270060005

04/22/20

SAMPLE

SAMPLE DATE

MC %

PL

ы

% FINES

SAMPLES: 90

BORING

LB-5

LB-5

LB-6

LB-6

LB-6

LB-6

S-4

S-5

S-1

S-2

S-3

S-4

7-8.5

8.5-10

1.5-3.5

3.5-5.5

5.5-7

7-8.5

REPORT:

LOCATION: REMARKS:

DEPTH

Arlington, VA

ОМ %

5233 Lehman Road, Suite 110

Spring Grove, PA 17362 Phone: (814) 404-9283

USCS

JAY KAY TESTING, INC.

DOMING	JAIM EL	DEI III	IVIC /U	OIII /0				/01 II1E3	0363
LB-1	Bulk	2-5	13.5	-	-	-	-	26.6	-
LB-1	S-1	1.5-3.5	18.4	-	-	-	-	-	-
LB-1	S-2	3.5-5.5	15.1	-	-	-	-	-	-
LB-1	S-3	5.5-7.5	17.6	-	-	-	-	-	-
LB-1	S-5	9-10.5	18.6	-	29	17	12	-	-
LB-2	Bulk	2-5	15.0	-	-	-	-	-	-
LB-2	S-1	1-3	16.9	-	-	-	-	-	-
LB-2	S-2	3-5	19.4	-	-	-	-	-	-
LB-2	S-3	5-7	16.1	-	-	-	-	38.6	-
LB-2	S-4	7-8.5	13.5	-	-	-	-	-	-
LB-2	S-5	8.5-10	14.8	-	-	-	-	-	-
LB-3	S-1	1-3	17.2	-	36	24	12	-	-
LB-3	S-2	3-5	14.7	-	-	-	-	-	-
LB-3	S-3	5-7	17.0	-	-	-	-	43.1	-
LB-3	S-4	7-8.5	15.8	-	-	-	-	-	-
LB-3	S-5	8.5-10	20.2	-	-	-	-	-	-
LB-3	S-6	13.5-15	26.1	-	-	-	-	-	-
LB-3	S-7	18.5-20	24.3	-	-	-	-	-	-
LB-3	S-8	23.5-25	11.1	-	-	-	-	11.1	-
LB-4	Bulk	2-5	9.4	-	-	-	-	44.3	-
LB-4	S-1	1-3	14.0	-	-	-	-	-	-
LB-4	S-2	3-5	13.9	-	-	-	-	-	-
LB-4	S-3	5-7	14.5	-	-	-	-	-	-
LB-4	S-4	7-8.5	18.5	-	-	-	-	62.1	-
LB-4	S-5	8.5-10	18.9	-	-	-	-	-	-
LB-5	Bulk	2-5	9.4	-	-	-	-	-	-
LB-5	S-1	1-3	16.4	-	-	-	-	-	-
LB-5	S-2	3-5	7.1	-	-	-	-	-	-
LB-5	S-3	5-7	16.2	-	-	-	-	-	-

Jay Kay Testing, Inc. (AASHTO-Accredited)

17.7

20.2

7.4

17.4

17.1

14.9

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 1 OF 37

SUMMARY OF LABORATORY TESTING

ARMY NAVY DRIVE COMPLETE STREETS PROJECT

PROJECT NO. 270060005

SAMPLE DATE

REMARKS:

JAY KAY TESTING, INC. 5233 Lehman Road, Suite 110

SAMPLES: 9

REPORT:

90

04/22/20

LOCATION: Arlington, VA

Spring Grove, PA 17362

Phone: (814) 404-9283

BORING	SAMPLE	DEPTH	MC %	ОМ %	LL	PL	PI	% FINES	USCS
LB-6	S-5	8.5-10	19.7	-	-	-	-	-	-
LB-6	S-6 B	13.5-15	15.4	-	-	-	-	-	-
LB-6	S-6 T	13.5-15	21.1	-	-	-	-	-	-
LB-6	S-7	18.5-20	18.5	-	-	-	-	56.5	-
LB-6	S-8	23.5-25	19.0	-	-	-	-	-	-
LB-7	Bulk	2-5	9.3	-	-	-	-	32.3	-
LB-7	S-1	1.5-3.5	13.7	-	-	-	-	44.8	-
LB-7	S-2	3.5-5.5	21.3	-	-	-	-	-	-
LB-7	S-3	5.5-7	12.5	-	-	-	-	-	-
LB-7	S-4	7-8.5	18.6	-	-	-	-	-	-
LB-7	S-5	8.5-10	14.6	-	-	-	-	-	-
LB-8	S-1	1.5-3.5	12.5	-	-	-	-	-	-
LB-8	S-2	3.5-5.5	18.5	-	-	-	-	-	-
LB-8	S-3	5.5-7	10.5	-	-	-	-	-	-
LB-8	S-5	8.5-10	19.2	-	-	-	-	-	-
LB-8	S-6	13.5-15	27.0	-	28	19	9	-	-
LB-8	S-7	18.5-20	15.9	-	-	-	-	-	-
LB-8	S-8	23.5-25	11.4	-	-	-	-	-	-
LB-9	Bulk	2-5	9.8	-	-	-	-	-	-
LB-9	S-1	1.3-3.3	13.3	-	-	-	-	-	-
LB-9	S-2	3.5-5.5	15.1	-	-	-	-	-	-
LB-9	S-3	5.5-7.5	10.3	-	-	-	-	33.7	-
LB-9	S-4	7-8.5	6.9	-	-	-	-	-	-
LB-9	S-5	8.5-10	7.8	-	-	-	-	-	-
LB-10	S-1	1-3	13.0	-	-	-	-	-	-
LB-10	S-2	3-5	8.5	-	-	-	-	-	-
LB-10	S-3	5-7	8.0	-	-	-	-	-	-
LB-10	S-5	8.5-10	4.5	-	-	-	-	-	-
LB-10	S-6	13.5-15	13.9	-	-	-	-	-	-
LB-10	S-7	18.5-20	19.0	-	-	-	-	-	-
LB-10	S-8	23.5-25	6.0	-	-	-	-	-	-
LB-11	Bulk	1-3	14.7	-	-	-	-	51.3	-
LB-11	S-1	1-3	11.6	-	-	-	-	24.3	-
LB-11	S-2	3-5	10.6	-	-	-	-	-	-
LB-11	S-3	5-7	7.4	-	-	-	-	-	-

Jay Kay Testing, Inc. (AASHTO-Accredited)

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 2 OF 37

SUMMARY OF LABORATORY TESTING

ARMY NAVY DRIVE COMPLETE STREETS PROJECT

PROJECT NO. 270060005 SAMPLE DATE - JAY KAY TESTING, INC.

SAMPLES: 90 LOCATION: Arlington, VA 5233 Lehman Road, Suite 110
Spring Grove, PA 17362

REPORT: 04/22/20 REMARKS: - Spring Grove, PA 1/362
Phone: (814) 404-9283

BORING	SAMPLE	DEPTH	MC %	ОМ %	LL	PL	PI	% FINES	USCS
LB-11	S-4	7-9	4.0	-	-	-	-	5.1	-
LB-11	S-5	9-10.5	6.1	-	-	-	-	-	-
LB-12	Bulk	1-5	4.5	-	-	-	-	-	-
LB-12	S-1	1-3	8.2	-	-	-	-	-	-
LB-12	S-2	3-5	10.9	-	-	-	-	-	-
LB-12	S-3	5-7	8.7	-	-	-	-	-	-
LB-12	S-4	7-8.5	10.4	-	-	-	-	-	-
LB-12	S-5	8.5-10	14.0	-	-	-	-	-	-
LB-13	Bulk	1-5	5.7	-	-	-	-	28.3	-
LB-13	S-1	1-3	8.4	-	-	-	-	-	-
LB-13	S-2	3-5	11.0	-	-	-	-	-	-
LB-13	S-3	5-7	15.0	-	-	-	-	-	-
LB-13	S-4	7-8.5	14.7	-	-	-	-	-	-
LB-13	S-5	8.5-10	23.7	-	-	-	-	-	-
LB-14	Bulk	1-5	4.5	-	-	-	-	24.2	-
LB-14	S-1	1-3	9.4	-	-	-	-	-	-
LB-14	S-2	3-5	17.2	-	-	-	-	55.1	-
LB-14	S-3	5-7	13.3	-	-	-	-	-	-
LB-14	S-4	7-8.5	15.9	-	-	-	-	-	-
LB-14	S-5	8.5-10	15.4	-	-	-	-	-	-

Jay Kay Testing, Inc. (AASHTO-Accredited)

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 3 OF 37

Boring: LB-1 Project No.: 270060005

Sample: Bulk

2-5' Depth:

Arlington, VA

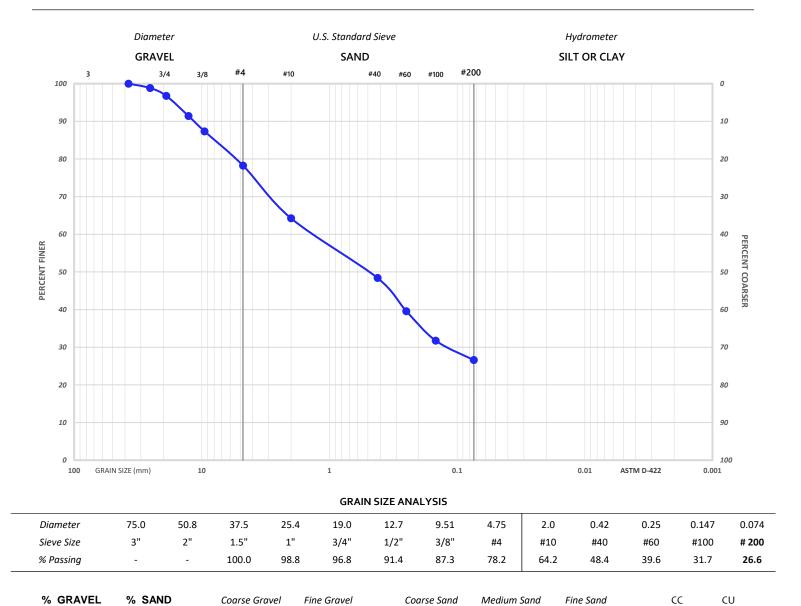
Sample Date:

Location:

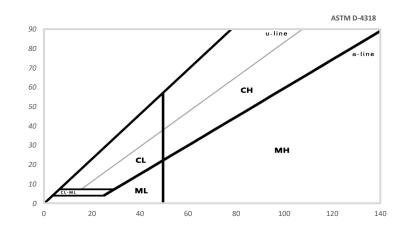
5233 Lehman Road, Suite 110 Spring Grove, PA 17362

Phone: (814) 404-9283

JAY KAY TESTING, INC.



70 01171122	,0 O,D	course oraver	inic Graver
21.8	51.6	3.2	18.6
Moisture Content	13.5	Organic Content	-
рН	-	Other	-
ATTERBERG LIMITS	S	CLASSIFICATION	l
Liquid Limit	-	AASHTO	-
Plastic Limit	-	USCS	-
Plasticity Index	-		
VISUAL SOIL DESC	RIPTION		
Brown clayey SAND v	vith gravel		



21.8

14.0

Boring: **LB-2** Project No.: 270060005

Sample: S-3

Depth:

5-7'

Sample Date: -

Location:

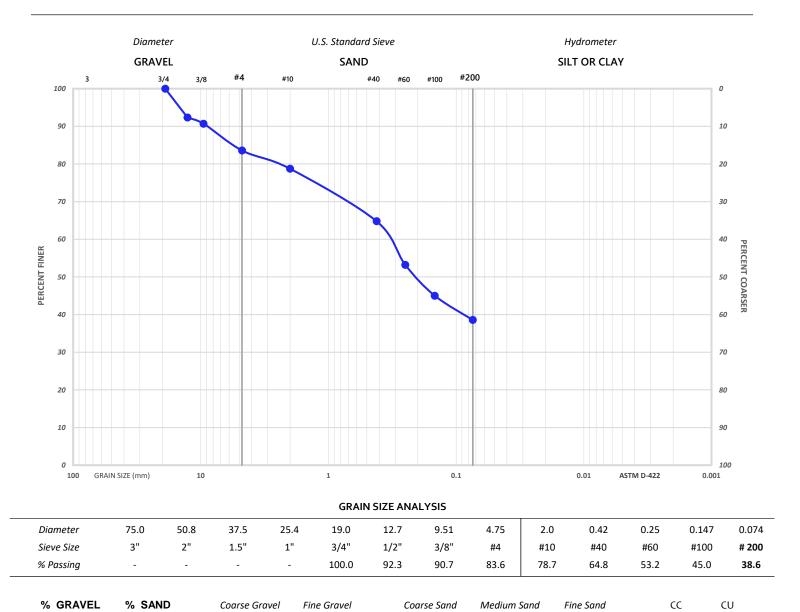
_

Arlington, VA

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16.4	45.0	-	16.4				
Moisture Content	16.1	Organic Content	-				
рН	-	Other	-				
ATTERBERG LIMITS	S	CLASSIFICATION					
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Brown clayey SAND v	Brown clayey SAND with gravel						

						ASTM D	0-4318
90				/	u-line /		
80							ı-line
70							
60		,		СН			
50							
40			/ <i>/</i>				
30					мн		
20		CL	ſ				
10	CL-ML	ML					
0) 20	40	60	80	100	120	140
,			-0	0	_50		1.0

26.2

4.9

Boring: LB-3 Project No.: 270060005

Sample: S-3

Depth:

5-7'

Location:

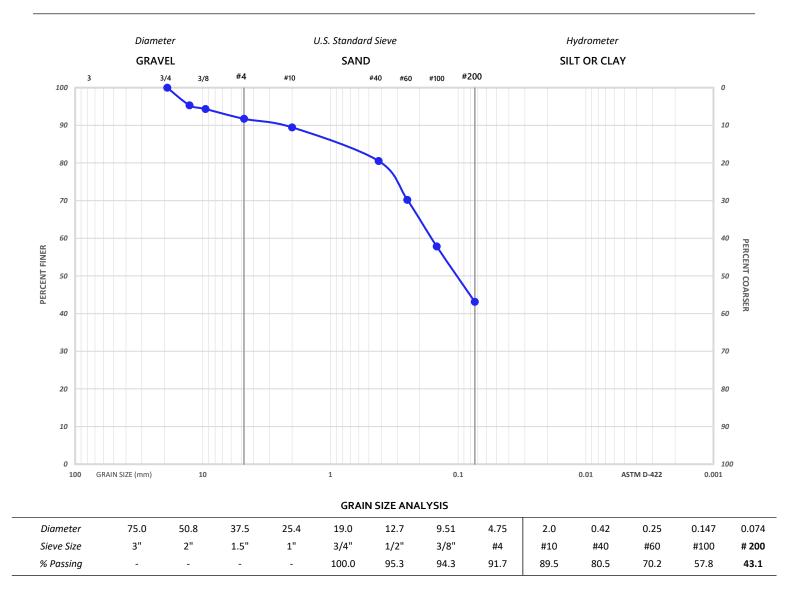
Sample Date:

Arlington, VA

JAY KAY TESTING, INC.

5233 Lehman Road, Suite 110 Spring Grove, PA 17362

Phone: (814) 404-9283



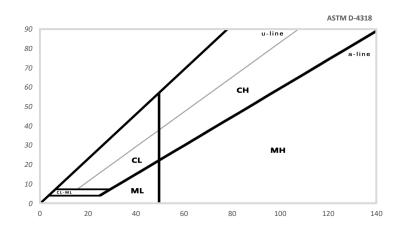
Coarse Sand

2.2

Medium Sand

9.0

% GRAVEL	% SAND	Coarse Gravel	Fine Gravel
8.3	48.6	-	8.3
Moisture Content	17.0	Organic Content	_
Wioistore Content	17.0	Organic Content	_
рН	-	Other	-
ATTERBERG LIMIT	rs	CLASSIFICATION	
1 invital timete		AACUTO	
Liquid Limit	-	AASHTO	-
Plastic Limit	-	USCS	-
Plasticity Index	-		
VISUAL SOIL DESC	CRIPTION		
Brown silty SAND			



Fine Sand

37.4

CC

CU

Boring: LB-3 Project No.:

Sample: S-8

Sample

Depth: 23.5-25'

Sample Date: -

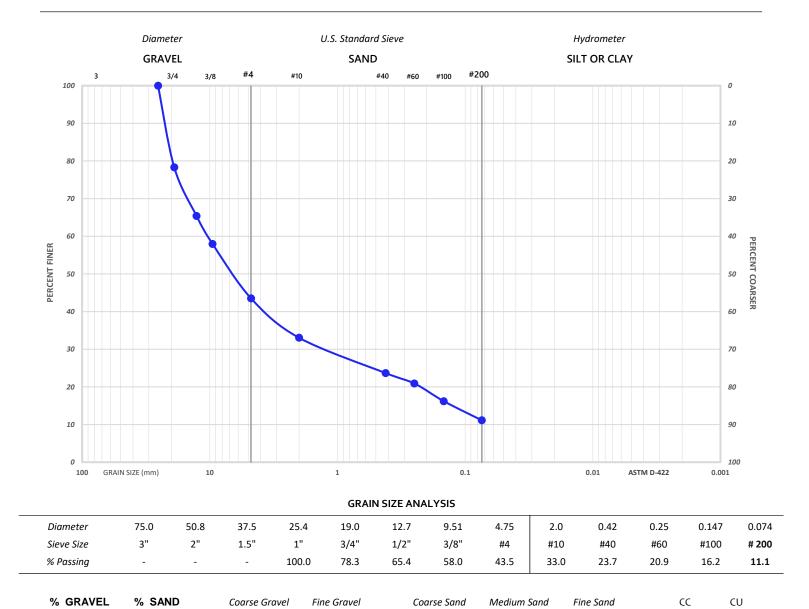
Location: Arlington, VA

270060005

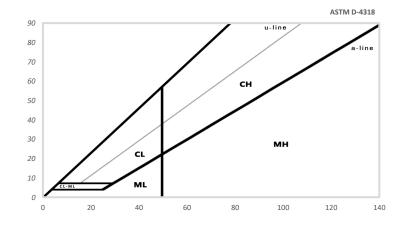
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% GRAVEL	% SAND	Coarse Gravei	Fine Gravei			
56.5	32.4	21.7	34.8			
Moisture Content	11.1	Organic Content Other	-			
ATTERBERG LIMI	rs	CLASSIFICATION				
Liquid Limit	-	AASHTO	-			
Plastic Limit	-	USCS	-			
Plasticity Index	-					
VISUAL SOIL DES	CRIPTION					
Brown well graded GRAVEL with silt and sand						



12.6

2.72

166.94

10.5

Boring: **LB-4** Project No.: 270060005

Sample: Bulk

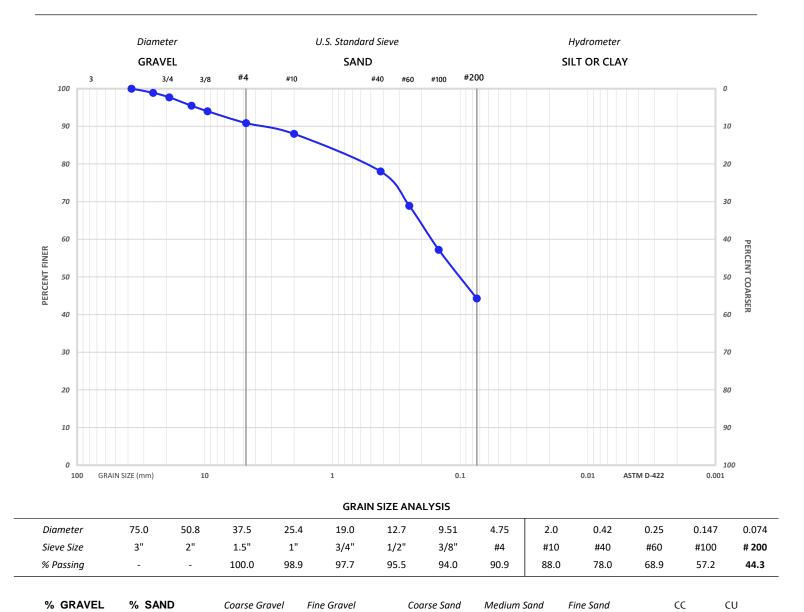
Depth: 2-5' Location: Arlington, VA

Sample Date:

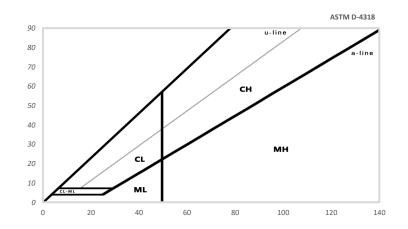
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			6.8
Moisture Content	9.4	Organic Content	-
рН	-	Other	-
ATTERBERG LIMITS		CLASSIFICATION	
Liquid Limit	-	AASHTO	-
Plastic Limit	-	USCS	-
Plasticity Index	-		
VISUAL SOIL DESCRI	PTION		



33.7

2.9

Boring: LB-4 Project No.:

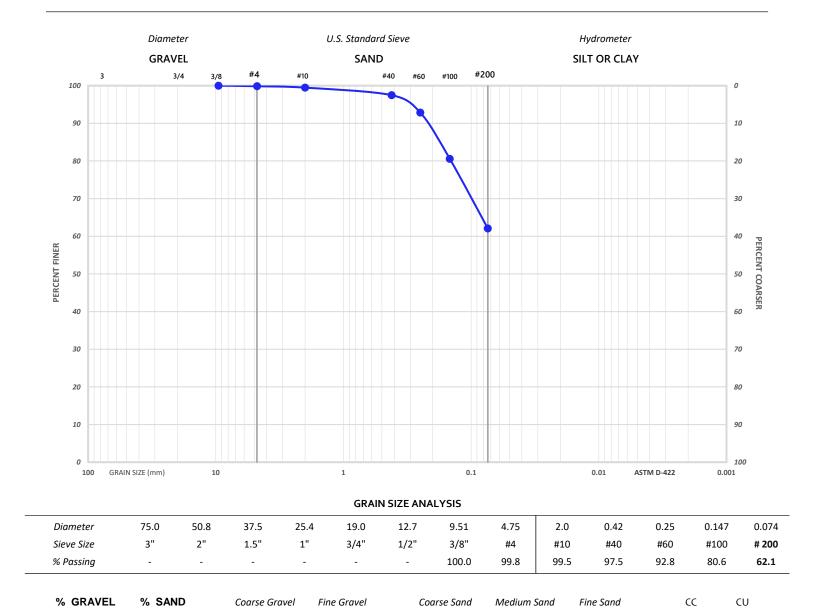
Sample: **S-4**

Depth: 7-8.5 270060005

Sample Date:

Location: Arlington, VA JAY KAY TESTING, INC.

5233 Lehman Road, Suite 110 Spring Grove, PA 17362 Phone: (814) 404-9283



0.2	37.7	-	0.2
Moisture Content	18.5	Organic Content	-
pH	-	Other	-
ATTERBERG LIMITS	5	CLASSIFICATION	
Liquid Limit	-	AASHTO	-
Plastic Limit	-	USCS	-
Plasticity Index	-		
VISUAL SOIL DESC	RIPTION		
Brown sandy silt			

						ASTM D	4318
90				/	u-line		
80				_		a	line
70							
60		1		СН			
50							
40		/ /					
30					мн		
20		/ ¹					
10	CL-ML	ML					
0 0	20	40	60	80	100	120	140
U	20	40	00	00	100	120	140

35.4

2.0

Boring: **LB-7** Project No.: 270060005

Sample: Bulk

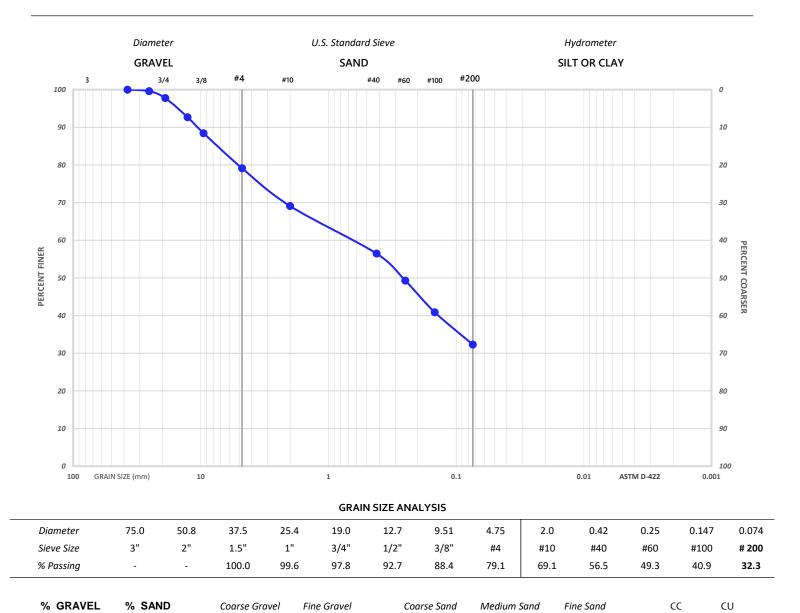
Depth: 2-5' Location: Arlington, VA

Sample Date:

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20.9	46.8	2.2	18.7			
Moisture Content	9.3	Organic Content	-			
pН	_	Other	_			
ριι	-	Other	_			
ATTERBERG LIMITS		CLASSIFICATION				
Liquid Limit	-	AASHTO	-			
Plastic Limit	-	USCS	-			
Plasticity Index	-					
VISUAL SOIL DESCRIPTION						
Dark brown silty SANE) with gravel					

						ASTM D	-4318
90					u-line /		
80						a	-line
70							
60		1		СН			
50							
40			/ /				
30		/ /			мн		
20		/ a					
10	CL-ML	ML					
0	CL-ML						
(20	40	60	80	100	120	140

24.2

10.0

Boring: LB-7 Project No.: 270060005

Sample: S-1

Depth:

1.5-3.5'

Sample Date:

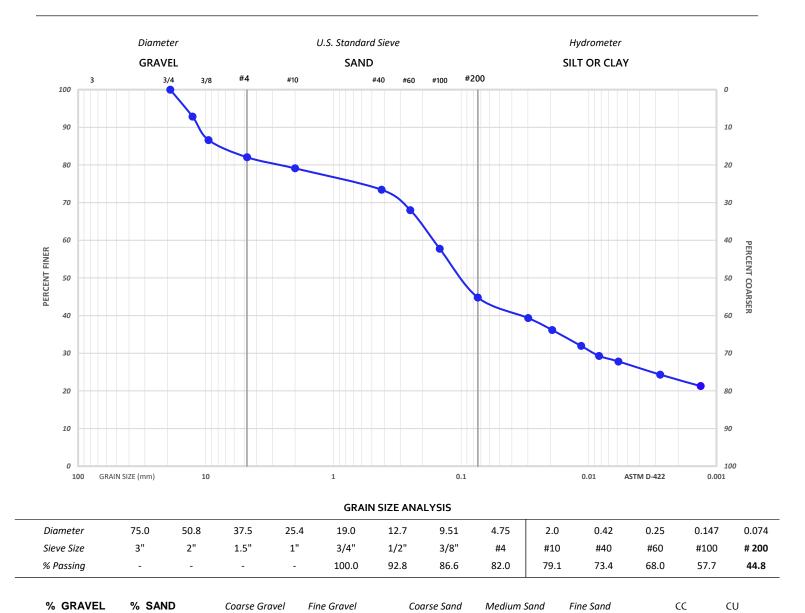
Location:

Arlington, VA

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18.0	37.2	-	18.0				
Moisture Content	13.7	Organic Content	-				
рН	-	Other	-				
ATTERBERG LIMITS		CLASSIFICATION					
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Brown clayey SAND with gravel							

						ASTM D	-4318
90					u-line /		
80						a	-line
70							
60		/		СН			
50							
40		/	_				
30					мн		
20		"					
10	CL-ML	ML					
0							
0	20	40	60	80	100	120	140

28.6

2.9

Boring: LB-9 Project No.: 27006

Sample: S-3

Depth:

5.5-7.5'

Sample Date:

270060005

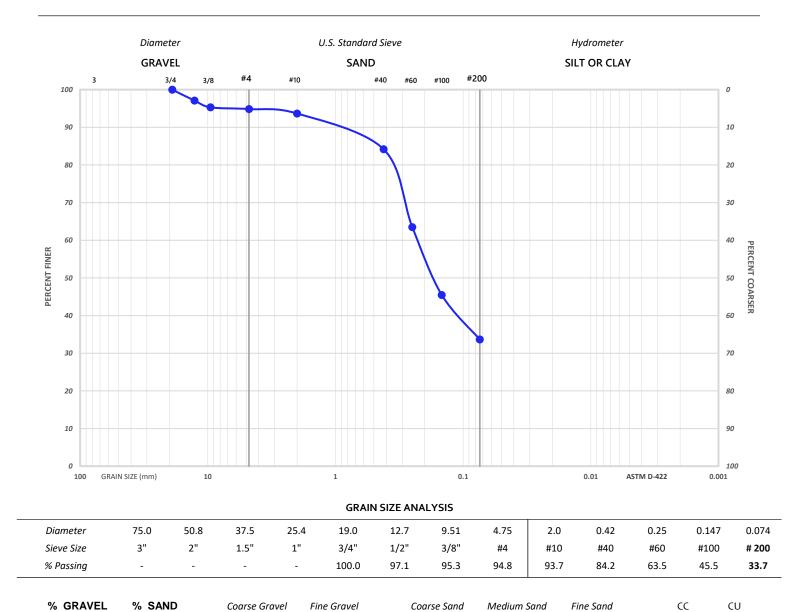
Location:

Arlington, VA

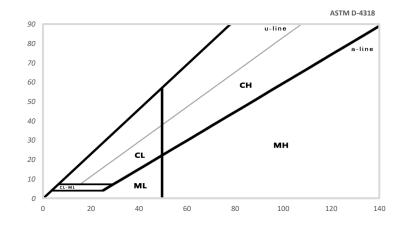
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5.2	61.1	-	5.2				
Moisture Content	10.3	Organic Content	-				
рН	-	Other	-				
ATTERBERG LIMITS	5	CLASSIFICATION					
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Brown silty SAND							



50.5

1.1

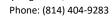
Boring: LB-11 Project No.: 270060005

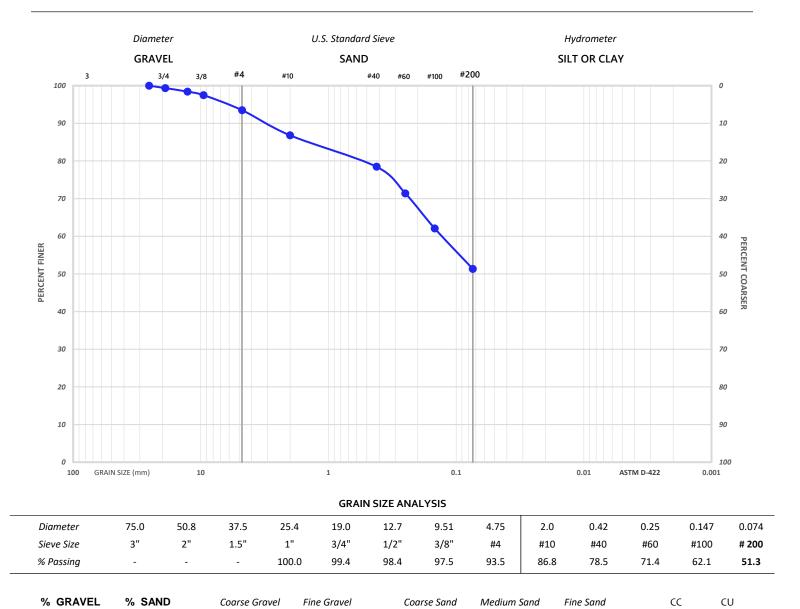
Sample: Bulk Sample Date:

1-3' Location: Arlington, VA Depth:

JAY KAY TESTING, INC.

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Coarse Sand

6.7

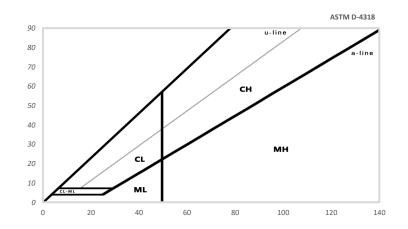
Medium Sand

8.3

6.5	42.2	0.6	5.9			
Moisture Content	14.7	Organic Content	-			
рН	-	Other	-			
ATTERBERG LIMITS	5	CLASSIFICATION				
Liquid Limit	-	AASHTO	-			
Plastic Limit	-	USCS	-			
Plasticity Index	-					
VISUAL SOIL DESCRIPTION						
Dark brown sandy clay						

Coarse Gravel

Fine Gravel



27.2

CC

CU

Boring: **LB-11** Project No.: 270060005

Sample: S-1

Depth:

1-3'

Sample Date:

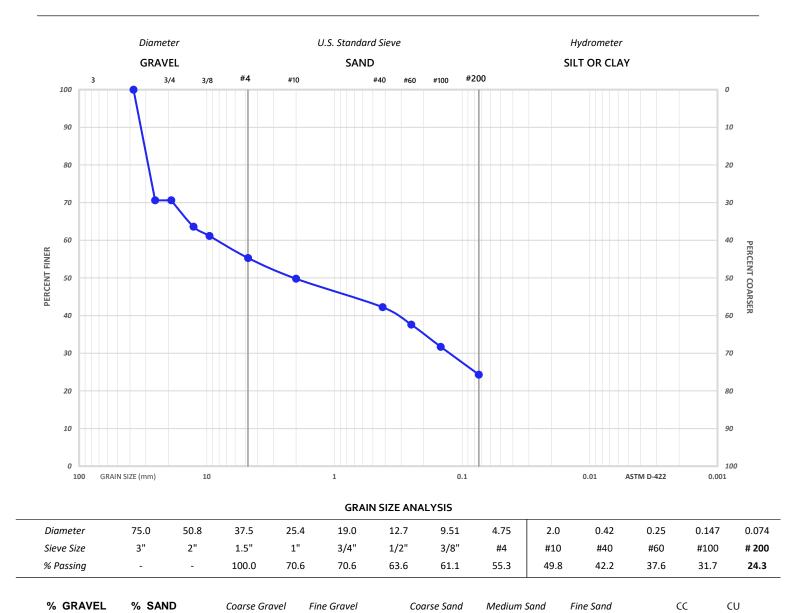
Location:

Arlington, VA

JAY KAY TESTING, INC.

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Phone: (814) 404-9283



44.7	31.0	29.4	15.3				
Moisture Content	11.6	Organic Content	-				
рН	-	Other	-				
ATTERBERG LIMITS	CLASSIFICATION						
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Reddish-brown silty clayey GRAVEL with sand							

						ASTM I	D-4318
90				/	u-line /		
80							a-line
70							
60		1	/	СН			
50							
40		//	_				
30					мн		
20		/ " /					
10	CL-ML	ML					
0	20	40	60	80	100	120	140

17.9

7.6

Boring: LB-11 Project No.:

Sample: **S-4**

% GRAVEL

% SAND

Depth:

7-9'

Sample Date:

Location:

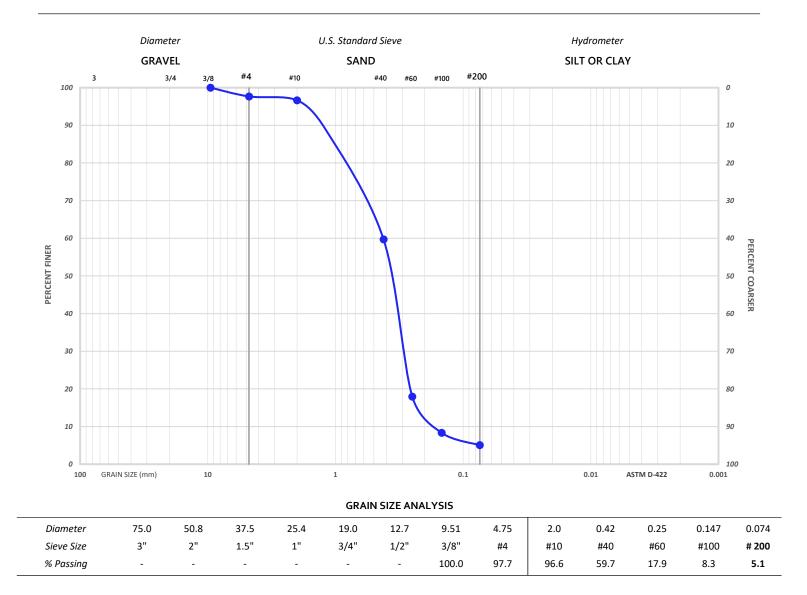
Arlington, VA

270060005

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Phone: (814) 404-9283



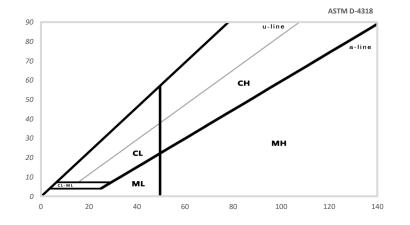
Coarse Sand

1.1

2.3	92.6	-	2.3				
Moisture Content	4.0	Organic Content Other	-				
ATTERBERG LIMITS	5	CLASSIFICATION					
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Light brown poorly graded SAND with silt							

Coarse Gravel

Fine Gravel



Fine Sand

54.6

CC

1.15

CU

2.44

Medium Sand

Boring: **LB-13** Project No.: 270060005

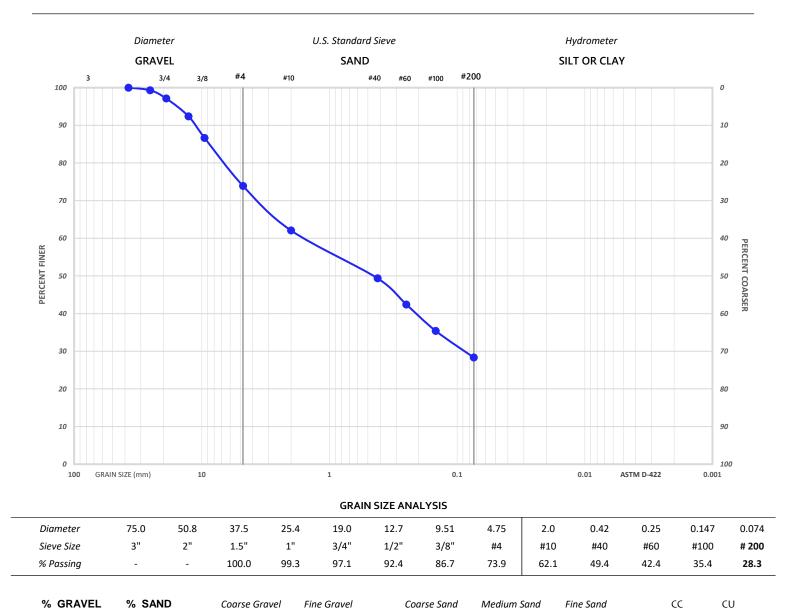
Sample: Bulk Sample Date:

Depth: 1-5' Location: Arlington, VA

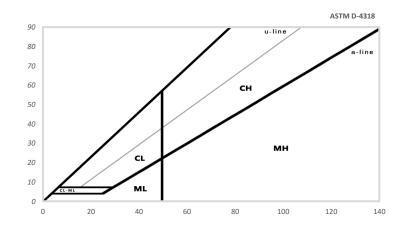
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26.1	45.6	2.9	23.2				
Moisture Content	5.7	Organic Content Other	-				
ATTERBERG LIMITS	5	CLASSIFICATION	-				
Liquid Limit	-	AASHTO	-				
Plastic Limit	-	USCS	-				
Plasticity Index	-						
VISUAL SOIL DESCRIPTION							
Light brown silty SAND with gravel							



21.1

11.8

Boring: LB-14 Project No.: 270060005

Sample: Bulk

Light brown silty SAND

Depth:

1-5'

Sample Date:

Location:

Arlington, VA

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 CC

100

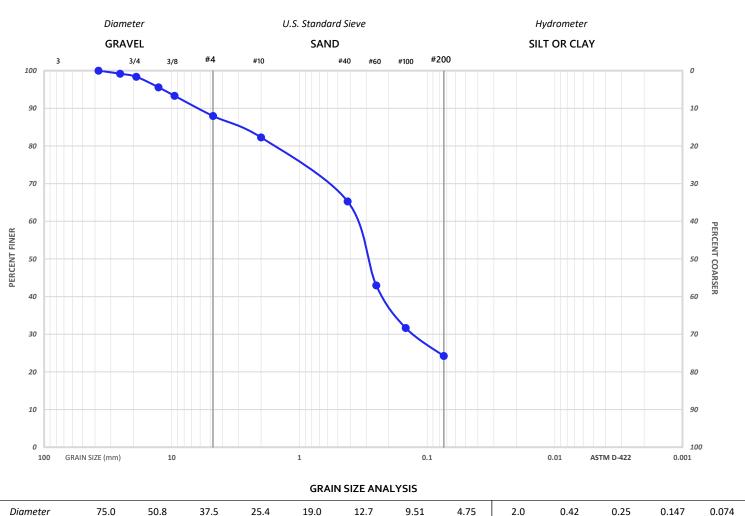
120

140

60

CU

ASTM D-4318



Diameter	75.0	50.8	37.5	25.4	19.0	12.7	9.51	4.75	2.0	0.42	0.25	0.147	0.074
Sieve Size	3"	2"	1.5"	1"	3/4"	1/2"	3/8"	#4	#10	#40	#60	#100	# 200
% Passing	-	-	100.0	99.2	98.4	95.6	93.3	88.0	82.3	65.3	42.9	31.7	24.2

% GRAVEL	% SAND	Coarse Gravel	Fine Gravel	Coarse Sand	Medium Sand	Fine Sand	(
12.0	63.8	1.6	10.4	5.7	17.0	41.1	
				90			u-line
Moisture Content	4.5	Organic Content	-	80			d-IIIe
рН	-	Other	-	70			
				60		CH	. /
ATTERBERG LIMI	ITS	CLASSIFICATION		50	1		
Liquid Limit	-	AASHTO	-			/ /	
Plastic Limit	-	USCS	-	40			
Plasticity Index	-			30	///		мн
				20	"		
VISUAL SOIL DES	SCRIPTION			10 CL-ML	ML		

REVIEWED BY: AG/KL 04/22/20 TESTED BY: ST/JT PAGE 17 OF 37

Boring: **LB-1** Project No.: 270060005

Sample: **Bulk** Sample Date:

Depth: 2-5' Location: Arlington, VA

JAY KAY TESTING, INC.

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CORRECTED *

STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

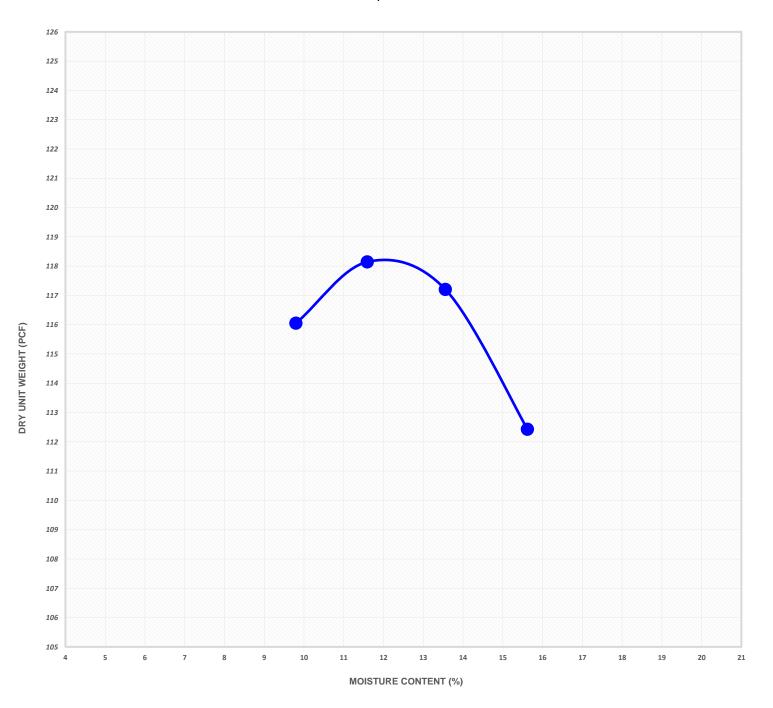
*Corrected for 21.8% retained on #4 sieve

Maximum Dry Unit Weight
Optimum Moisture Content

118.2 11.9

PCF 125.6

MC 9.7



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
13.5	-	-	-	-	-	26.6	Brown clayey SAND with gravel

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 18 OF 37

Boring: LB-2 Project No.: 270060005

Sample: Bulk Sample Date:

Depth: 2-5' Location: Arlington, VA JAY KAY TESTING, INC.

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CORRECTED *

STANDARD PROCTOR TEST RESULTS

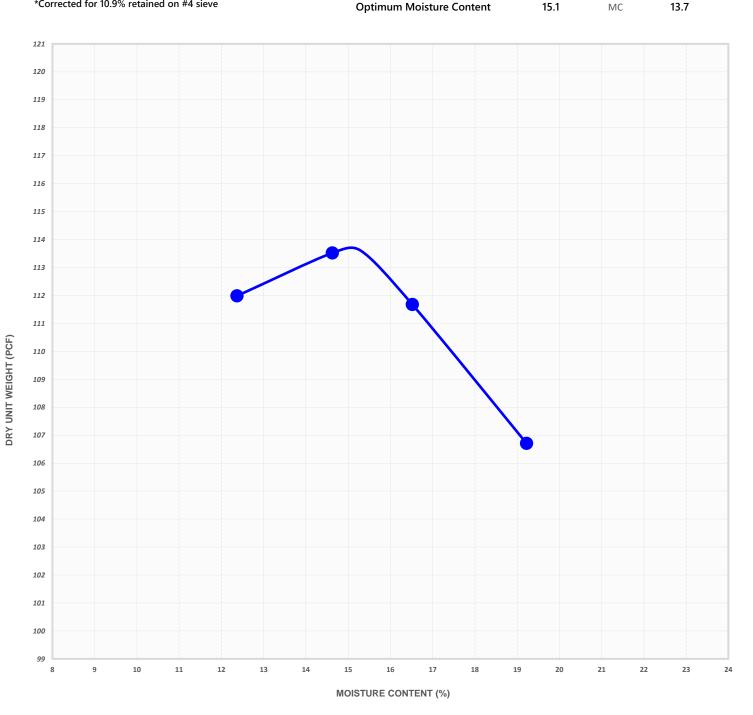
TEST METHOD: VTM-1

*Corrected for 10.9% retained on #4 sieve

Maximum Dry Unit Weight **Optimum Moisture Content** UNCORRECTED 113.8 15.1

PCF 117.6

13.7



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
15.0	-	-	-	-	-	-	Brown silty SAND

REVIEWED BY: AG/KL 04/22/20 TESTED BY: ST/JT PAGE 19 OF 37

Boring: **LB-4** Project No.: 270060005

Sample: **Bulk** Sample Date:

Depth: 2-5' Location: Arlington, VA

JAY KAY TESTING, INC.

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STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

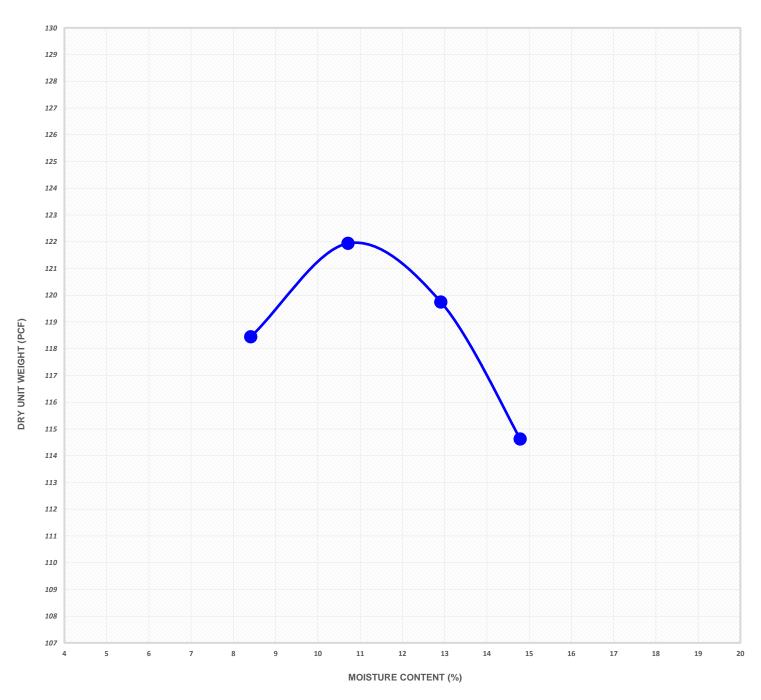
MAXIMUM DRY UNIT WEIGHT

OPTIMUM MOISTURE CONTENT

121.9

10.9

PCF



MC LL PL PI USCS AASHTO FINES VISUAL SOIL DESCRIPTION

9.4 - - - - 44.3 Brown clayey SAND

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 20 OF 37

Boring: **LB-5** Project No.: 270060005

Sample: Bulk Sample Date:

Depth: 2-5' Location: Arlington, VA

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Phone: (814) 404-9283

CORRECTED *

STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

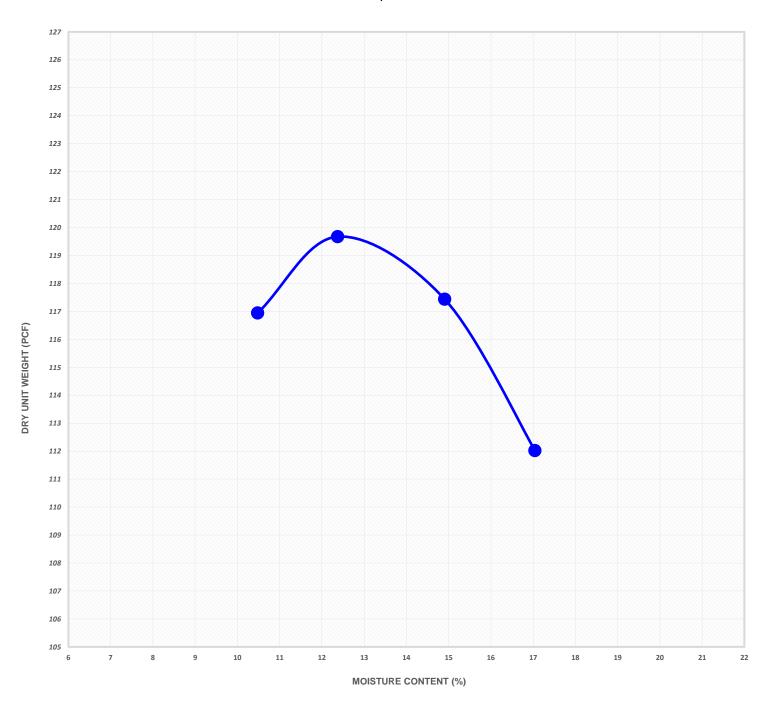
*Corrected for 19.0% retained on #4 sieve

Maximum Dry Unit Weight
Optimum Moisture Content

119.7 12.4

PCF 126.0

MC **10.4**



MC LL PL PI USCS AASHTO FINES VISUAL SOIL DESCRIPTION

9.4 - - - - - Brown silty SAND with gravel

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 21 OF 37

Boring: LB-7 Project No.: 270060005

Sample: Bulk Sample Date:

2-5' Location: Arlington, VA Depth:

JAY KAY TESTING, INC.

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STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

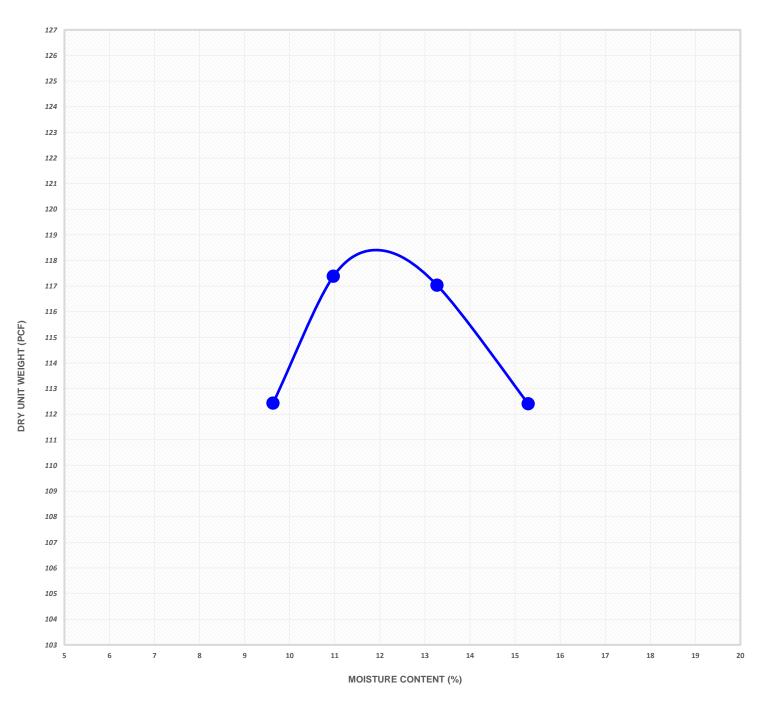
*Corrected for 20.9% retained on #4 sieve

Maximum Dry Unit Weight **Optimum Moisture Content** UNCORRECTED 118.4

CORRECTED * PCF

125.5

11.9 MC 9.8



VISUAL SOIL DESCRIPTION MC LL USCS **AASHTO FINES** 9.3 32.3 Dark brown silty SAND with gravel

Boring: LB-9 Project No.: 270060005

Sample: Bulk Sample Date:

2-5' Location: Arlington, VA Depth:

JAY KAY TESTING, INC.

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Phone: (814) 404-9283

STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

9.8

*Corrected for 26.8% retained on #4 sieve

Maximum Dry Unit Weight

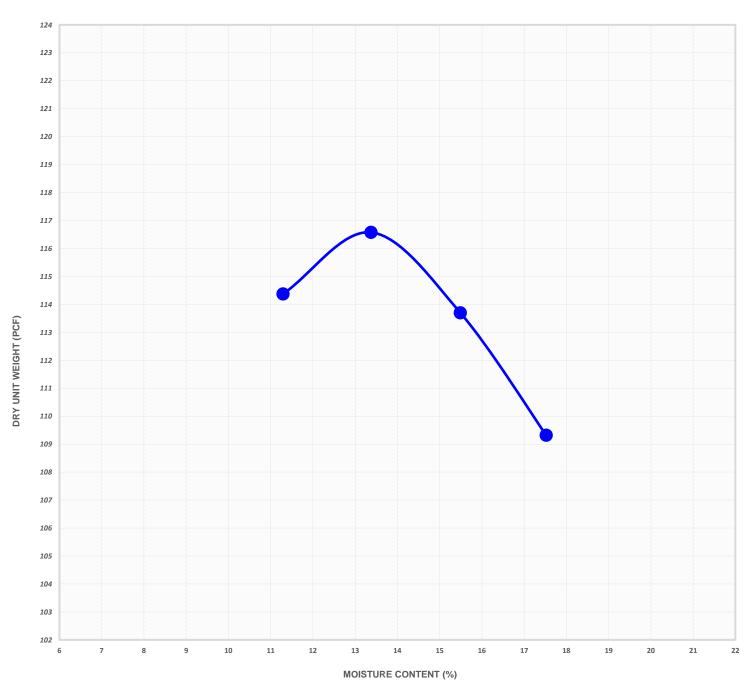
UNCORRECTED 116.6

PCF

126.1

CORRECTED *

Optimum Moisture Content 10.2 13.2 MC



VISUAL SOIL DESCRIPTION MC LL USCS **AASHTO FINES** Reddish-brown silty SAND with gravel

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 23 OF 37

Boring: **LB-11** Project No.: 270060005

Sample: Bulk Sample Date:

Depth: 1-3' Location: Arlington, VA

JAY KAY TESTING, INC.

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STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

MC

14.7

LL

MAXIMUM DRY UNIT WEIGHT

OPTIMUM MOISTURE CONTENT

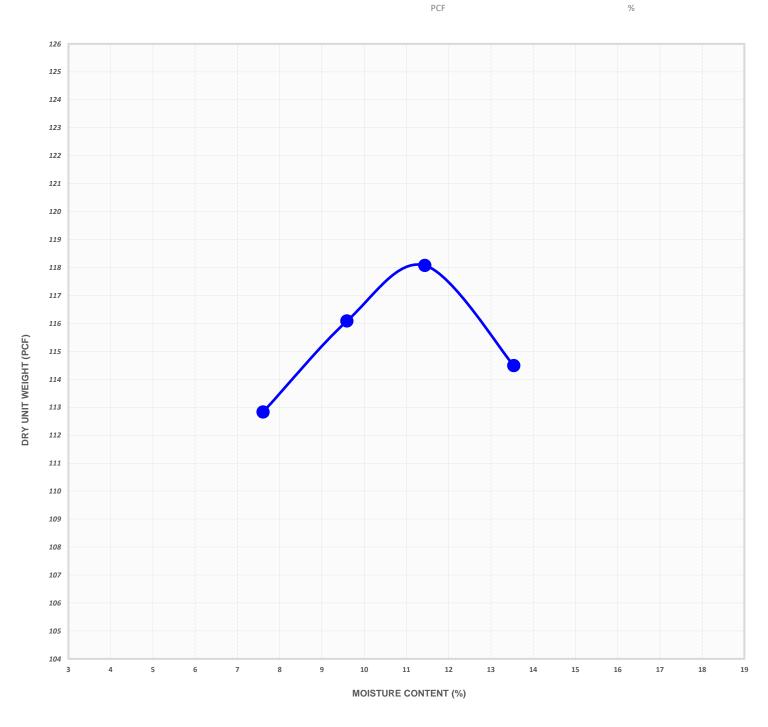
11.2

118.1

. .

VISUAL SOIL DESCRIPTION

Dark brown sandy clay



04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 24 OF 37

FINES 51.3

AASHTO

USCS

Boring: LB-12 Project No.:

Sample: **Bulk** Sample Date:

Depth: 1-5' Location: Arlington, VA

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CORRECTED *

17

18

STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

111

*Corrected for 26.5% retained on #4 sieve

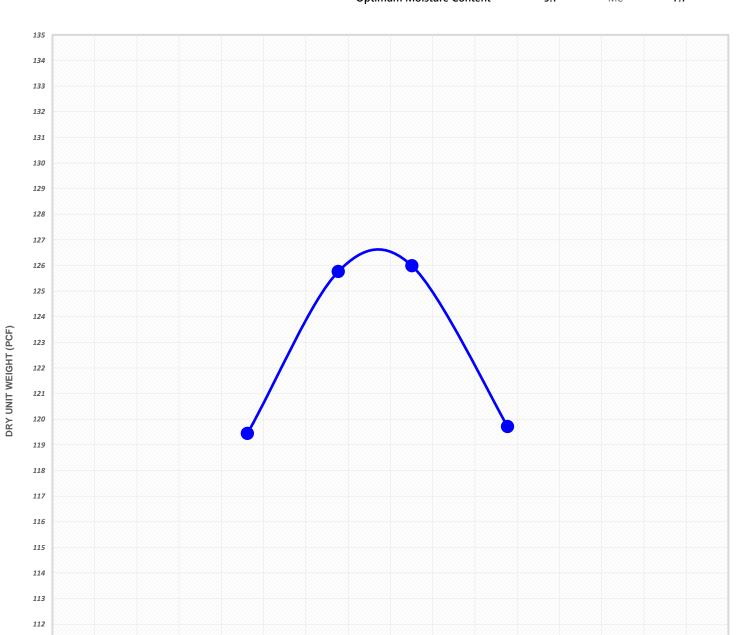
Maximum Dry Unit Weight
Optimum Moisture Content

270060005

126.7 9.7

PCF 134.5

MC 7.7



MC LL PL PI USCS AASHTO FINES VISUAL SOIL DESCRIPTION
4.5 - - - - - Light brown silty SAND with gravel

MOISTURE CONTENT (%)

Boring: LB-13 Project No.: 270060005

Sample: Bulk Sample Date:

1-5' Location: Arlington, VA Depth:

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CORRECTED *

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STANDARD PROCTOR TEST RESULTS

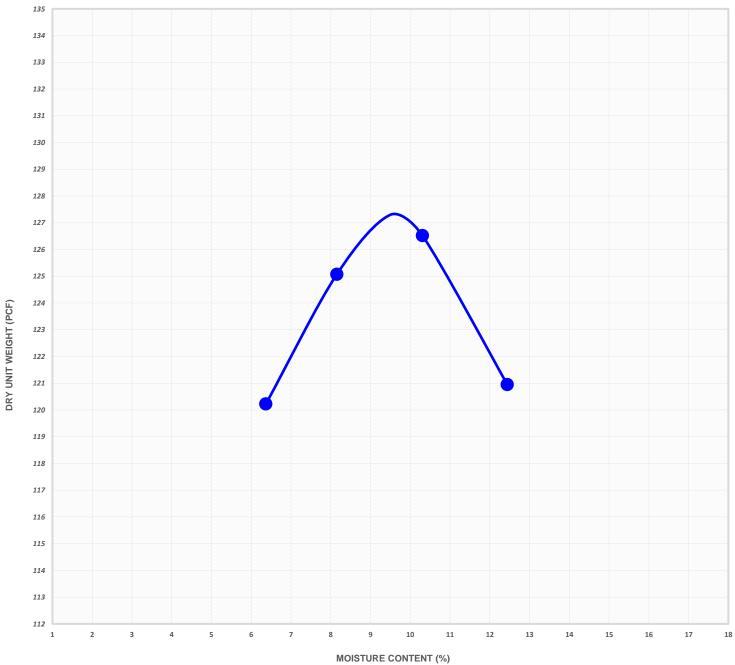
TEST METHOD: VTM-1

*Corrected for 26.1% retained on #4 sieve

Maximum Dry Unit Weight **Optimum Moisture Content** UNCORRECTED 127.3

PCF 134.9

9.5 7.5 MC



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
5.7	-	-	-	-	-	28.3	Light brown silty SAND with gravel

REVIEWED BY: AG/KL 04/22/20 TESTED BY: ST/JT PAGE 26 OF 37

Boring: LB-14 Project No.:

Sample: Bulk Sample Date:

1-5' Location: Arlington, VA Depth:

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STANDARD PROCTOR TEST RESULTS

TEST METHOD: VTM-1

*Corrected for 12.0% retained on #4 sieve

Maximum Dry Unit Weight **Optimum Moisture Content**

270060005

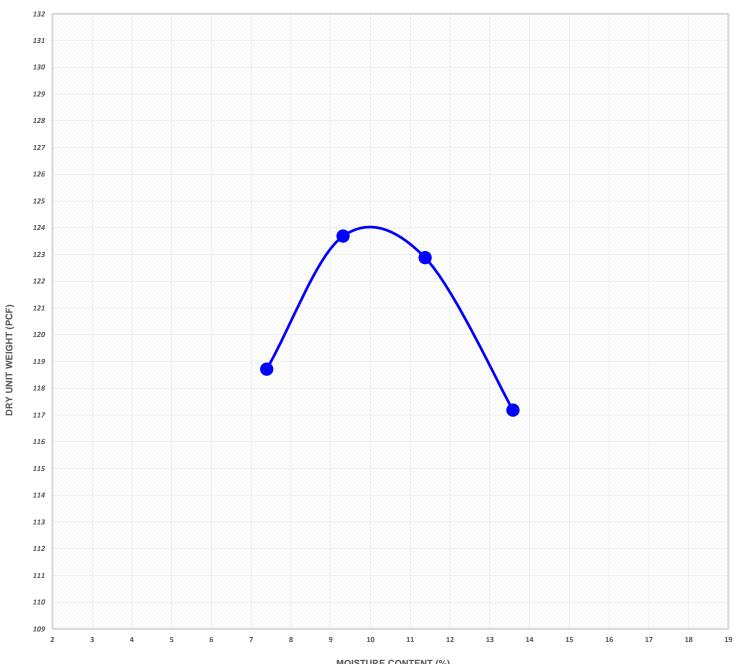
UNCORRECTED 124.0

PCF

127.6

CORRECTED *

9.9 9.0 MC



MOISTURE CONTENT (%)

MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
4.5	-	-	-	-	-	24.2	Light brown silty SAND

REVIEWED BY: AG/KL 04/22/20 TESTED BY: ST/JT PAGE 27 OF 37

Boring: LB-1 Project No.:

Sample: **Bulk**

Depth:

2-5' Location: Arlington, VA

Sample Date:

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CALIFORNIA BEARING RATIO TEST RESULTS

CBR AT 0.1"

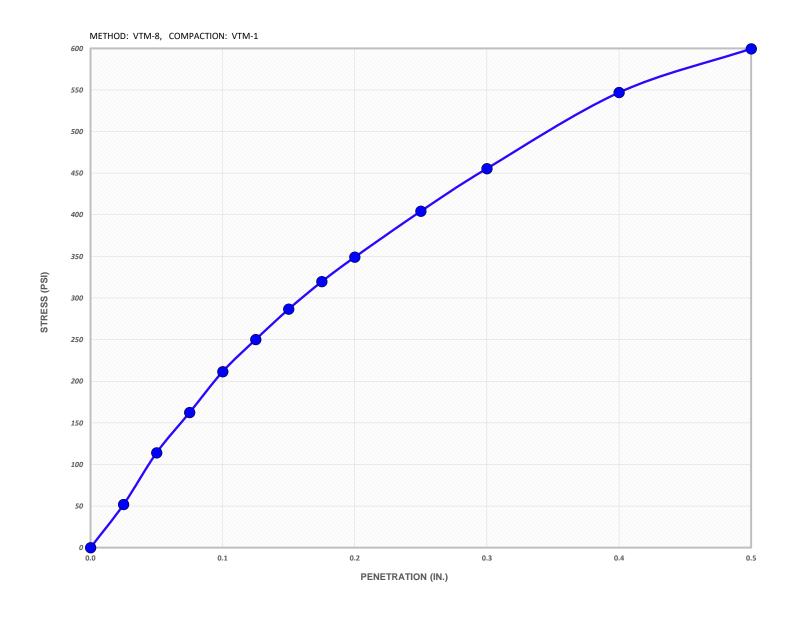
CBR AT 0.2"

21.1

23.3

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	125.3	10.1	99.8	-	50
After Soak	125.3	11.5	99.7	0.25	50
•	PCF	%	%	%	PSF

270060005



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
13.5	-	-	-	-	-	26.6	Brown clayey SAND with gravel

Boring: LB-2 Project No.:

Sample: **Bulk** Sample Date:

Depth: 2-5' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CBR AT 0.1"

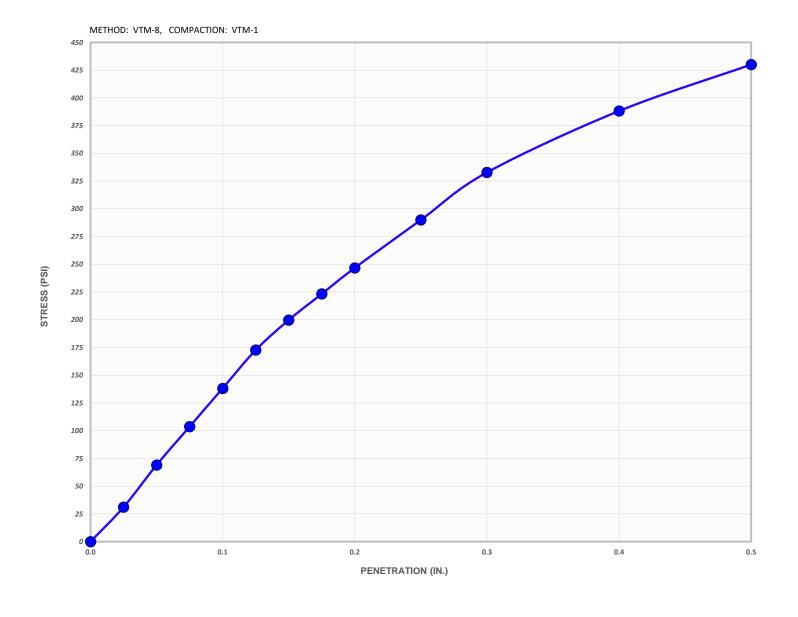
CBR AT 0.2"

13.8

16.4

		Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
-	As Molded	118.2	13.5	100.5	-	50
	After Soak	116.7	16.1	99.2	0.66	50
		PCF	%	%	%	PSF

270060005



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
15.0	-	-	-	-	-	-	Brown silty SAND

Boring: LB-4 Project No.: 2

Sample: **Bulk**

2-5'

Depth:

Sample Date:

Location:

270060005

Arlington, VA

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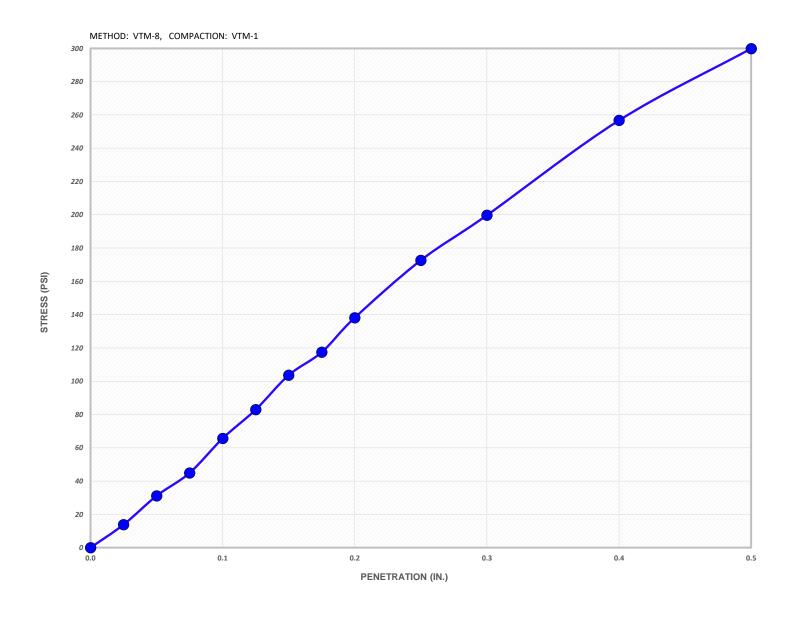
CALIFORNIA BEARING RATIO TEST RESULTS

CBR AT 0.1"

CBR AT 0.2"

6.6

		Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
-	As Molded	124.8	11.0	102.4	-	50
	After Soak	125.3	11.9	102.8	0.31	50
		PCF	%	%	%	PSF



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
9.4	-	-	-	-	-	44.3	Brown clayey SAND

Boring: LB-5 Project No.: 27000

Sample: **Bulk**

2-5'

Depth:

Sample Date:

Location:

270060005

Arlington, VA

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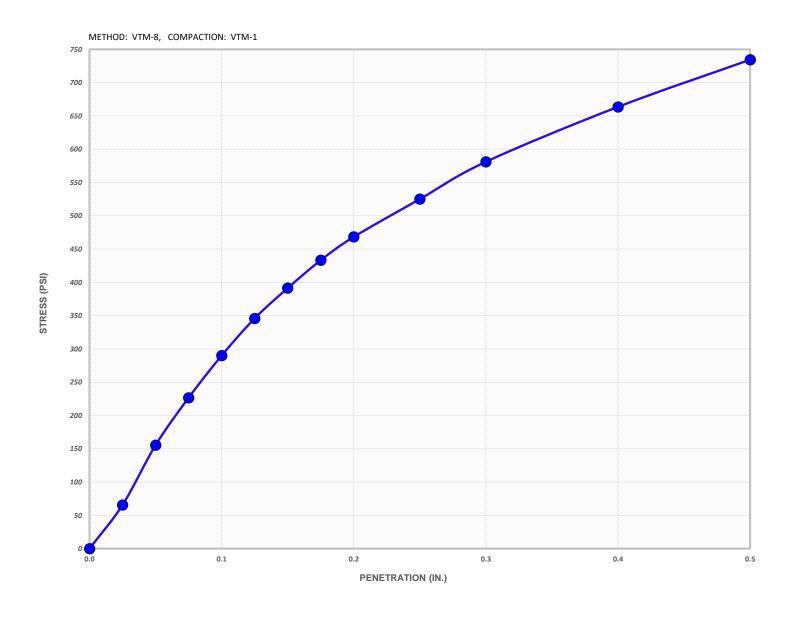
CALIFORNIA BEARING RATIO TEST RESULTS

CBR AT 0.1"

CBR AT 0.2"

29.0

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	125.3	10.2	99.5	-	50
After Soak	125.5	12.1	99.6	0.39	50
•	PCF	%	%	%	PSF



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
9.4	-	-	-	-	-	-	Brown silty SAND with gravel

Boring: **LB-7** Project No.: 270060005

Sample: **Bulk**

Depth: 2-5' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CBR AT 0.1"

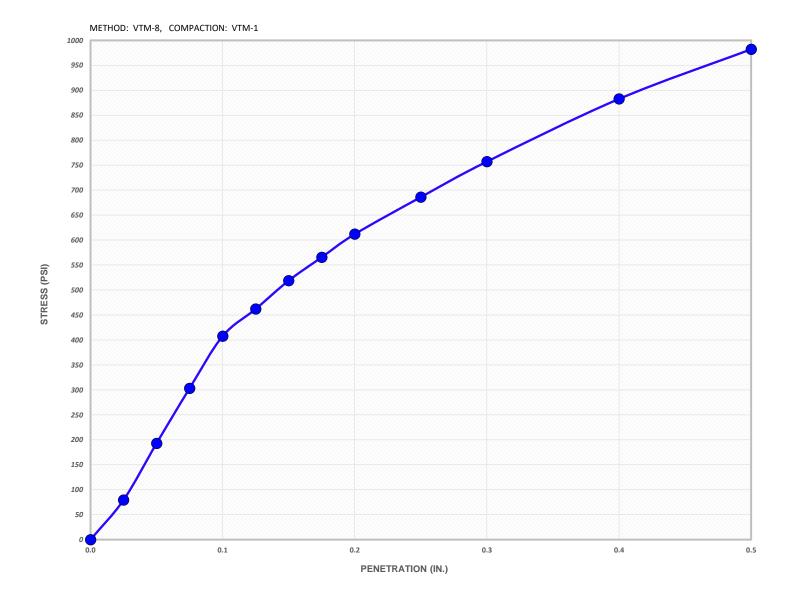
CBR AT 0.2"

40.7

40.8

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	123.1	9.5	98.1	-	50
After Soak	123.4	11.6	98.3	0.11	50
	PCF	%	%	%	PSF

Sample Date:



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
9.3	-	-	-	-	-	32.3	Dark brown silty SAND with gravel

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Boring: **LB-9** Project No.: 270060005

Sample: **Bulk**

Depth:

Bulk Sample Date:
2-5' Location:

Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

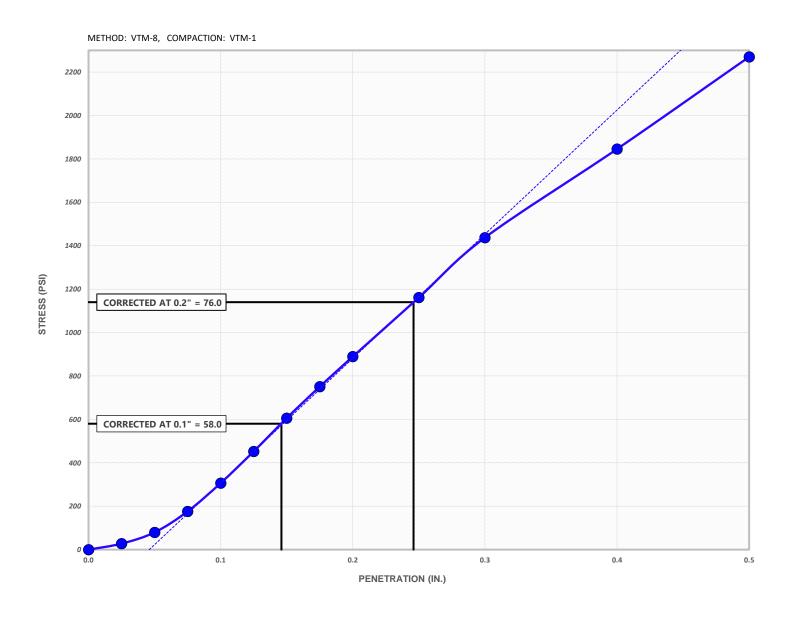
CORRECTED CBR AT 0.1"

CORRECTED CBR AT 0.2"

58.0

76.0

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	120.4	11.7	95.5	-	50
After Soak	120.8	12.8	95.8	-0.11	50
	PCF	%	%	%	PSF



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
9.8	-	-	-	-	-	-	Reddish-brown silty SAND with gravel

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Boring: LB-11 Project No.:

Sample: **Bulk**

Depth:

1-3' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CORRECTED CBR AT 0.1"

CORRECTED CBR AT 0.2"

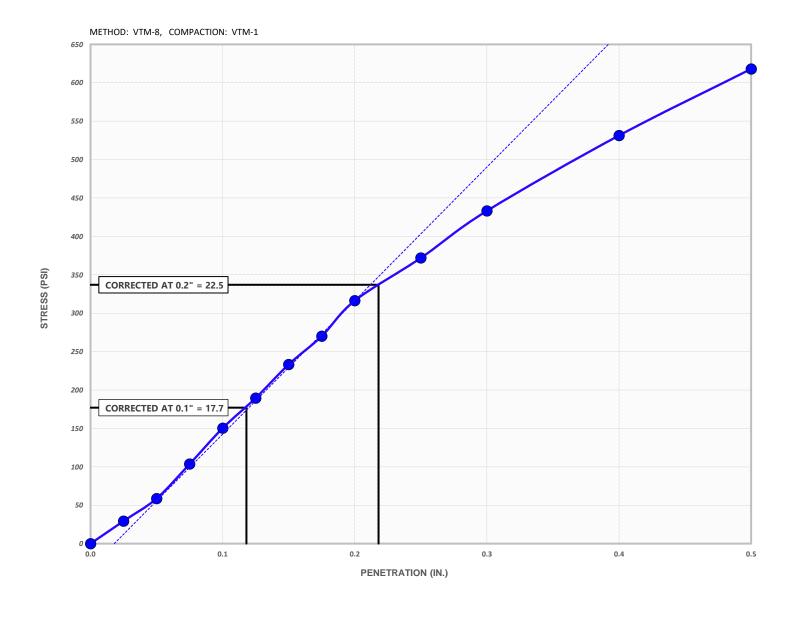
17.7

22.5

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	120.1	10.9	101.7	-	50
After Soak	119.8	12.6	101.5	0.46	50
•	PCF	%	%	%	PSF

270060005

Sample Date:



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
14.7	-	-	-	-	-	51.3	Dark brown sandy clay

Boring: LB-12 Project No.:

Sample: **Bulk** Sample Date:

Depth: 1-5' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CORRECTED CBR AT 0.1"

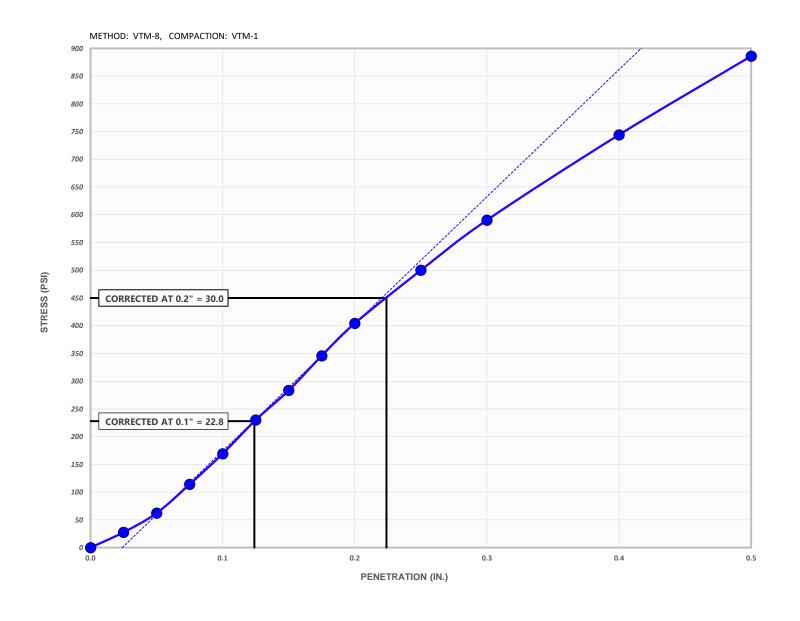
CORRECTED CBR AT 0.2"

22.8

30.0

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	135.2	7.6	100.5	-	50
After Soak	135.8	8.2	100.9	-0.03	50
•	PCF	%	%	%	PSF

270060005



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
4.5	-	-	-	-	-	-	Light brown silty SAND with gravel

04/22/20 TESTED BY: ST/JT REVIEWED BY: AG/KL PAGE 35 OF 37

ARMY NAVY DRIVE COMPLETE STREETS PROJECT

Boring: LB-13 Project No.:

Sample: **Bulk** Sample Date:

Depth: 1-5' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CORRECTED CBR AT 0.1"

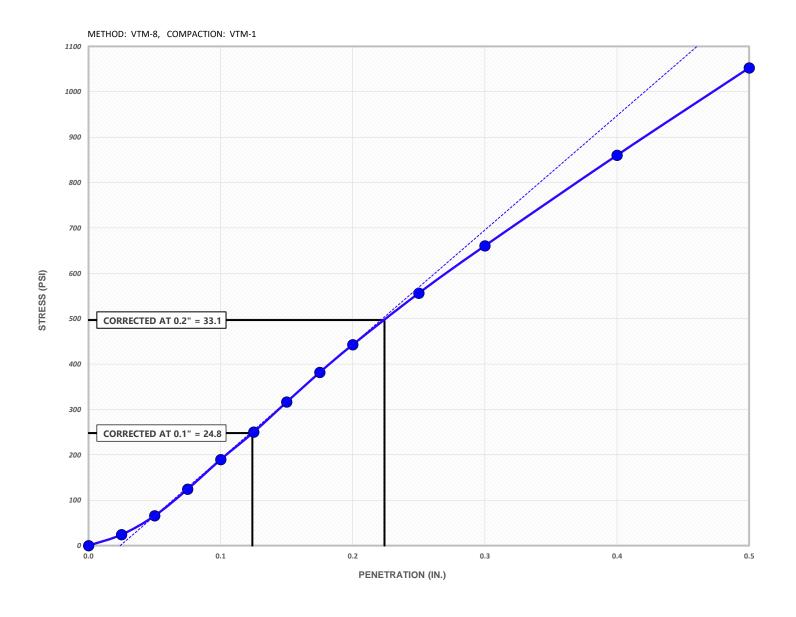
CORRECTED CBR AT 0.2"

24.8

33.1

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	134.9	7.3	100.0	-	50
After Soak	134.9	8.4	100.0	-0.02	50
	PCF	%	%	%	PSF

270060005



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
5.7	-	-	-	-	-	28.3	Light brown silty SAND with gravel

ARMY NAVY DRIVE COMPLETE STREETS PROJECT

Boring: LB-14 Project No.:

Sample: **Bulk** Sample Date:

Depth: 1-5' Location: Arlington, VA

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CALIFORNIA BEARING RATIO TEST RESULTS

CORRECTED CBR AT 0.1"

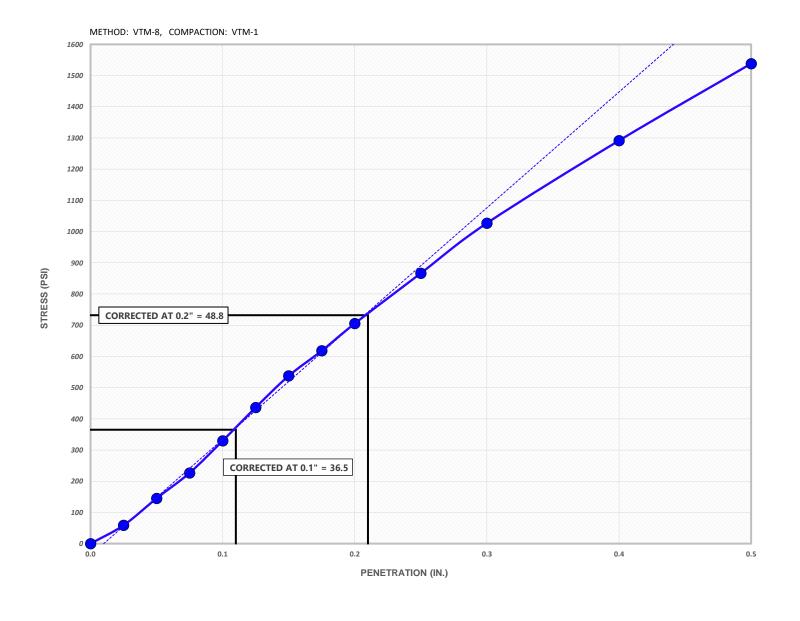
CORRECTED CBR AT 0.2"

36.5

48.8

	Dry Unit Weight	Moisture Content	Compaction	Swell	Surcharge
As Molded	127.0	8.9	99.5	-	50
After Soak	126.3	11.0	99.0	-0.11	50
•	PCF	%	%	%	PSF

270060005



MC	LL	PL	PI	USCS	AASHTO	FINES	VISUAL SOIL DESCRIPTION
4.5	-	-	-	-	-	24.2	Light brown silty SAND

Appendix E

Calculations

Pavement Design Requirements for Eads-12th (CBR Value = 10)

1. Design Requirements

For a pavement design life of 30 years, use the Anticipated Traffic Data for design:

Anticipated Traffic Data

Average Daily Traffic (2019)^{see note 1}: 8,158 Vehicles/Day (Average Daily Traffic, ADT) Annual Growth Rate (G)^{see note 2}: 0.38 9,141 Vehicles/Day (Design ADT) in year 2050 Anticipated Traffic Data for Design ADT at 30 years:

Anticipated Daily Traffic Breakdown by Vehicle

8,708	Cars/Passenger Vehicles	4	5 Axle Single Trailer Trucks
146	Buses	2	6+ Axle Single Trailer Trucks
201	2 Axle Single Unit Trucks	1	5- Axle Multi Trailer Trucks
36	3+ Axle Single Unit Trucks	0	6 Axle Multi Trialer Trucks
6	4- Axle Single Trailer Trucks	2	7+ Axle Multi Trailer Trucks

Notes:

2. Determine Number of ESALs:

- (1) Traffic information provided by Arlington County.
- (2) Annual Growth Rate estimated from VDOT AADT projections for 2020 and 2040.

VDOT Vehicle Factors

(3) Anticipated Daily Traffic Breakdown by Vehicle mirrors daily vehicle proportions given in traffic counts.

Vehicle Classification	ESAL Factor
Cars/Passenger Vehicles	0.0002
Buses	2.5
2 Axle Single Unit Trucks	0.46
3+ Axle Single Unit Trucks	0.46
4- Axle Single Trailer Trucks	1.05

Vehicle Classification	ESAL Factor
5 Axle Single Trailer Trucks	1.2
6+ Axle Single Trailer Trucks	1.05
5- Axle Multi Trailer Trucks	1.05
6 Axle Multi Trialer Trucks	1.06
7+ Axle Multi Trailer Trucks	1.39

ESAL = (Number Vehicles per day) x (ESAL Factor) x (DD) x (DL) x (Design Life in Days)

Design Life = 30 years x 364 days/year = $10,92$	20 days			
Design Life for Vehicle Traffic less than 7 days/week:	N/A	_days/week, Design Life =	N/A	days
Directional Distribution Factor, D _D = 1	·	_		
Lane Distribution Factor, D _L = 0.9				
Number of Passes per Vehicle = 1				
17,116 Cars/Passenger Vehicle ESALS	47,174	_5 Axle Single Trailer Truck	ESALs	
3,587,220 Bus ESALS	20,639	_6+ Axle Single Trailer Truc	k ESALs	;
908,697 2 Axle Single Unit Truck ESALS	10,319	_5- Axle Multi Trailer Truck I	ESALs	
162,752 3+ Axle Single Unit Truck ESALS	0	_6 Axle Multi Trialer Truck E	SALs	
61,916 4- Axle Single Trailer Truck ESALS	27,322	7+ Axle Multi Trailer Truck	ESALs	

Total Equivalent Single Axle Loads = 4,843,156 **ESALs**

alculation Title LANGAN 270060005 1300 Wilson Boulevard Suite 450 Arlington, Virginia 22209 P: +1.571.366.6800 F: +1.571.366.6801 www.langan.com **Army Navy Drive** 4/15/2020 **Complete Streets Project Asphalt Pavement Design** Calculations By 4/15/20 ANG VIRGINIA PENNSYLVANIA NEW YORK CONNECTICUT FLORIDA NEW JERSEY CALIFORNIA ABU DHABI DUBAI ATHENS DOHA ISTANBUL Checked By Page 1 of 2 4/21/20

$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left(\frac{\Delta PSI}{4.2-1.5}\right)}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32 \times \log_{10}(M_R) - 8.07$$

Equivalent Single Axle Loads, W₁₈ = 4,843,156

CBR = 10

Standard Deviation = 0.49

Subgrade Resilient Modulus, M_R = 1,500 x CBR =

15,000

Initial Serviceability Index =

4.2

Reliability Factor, R =

10,000

Terminal Serviceability Index =

2.8

Standard Normal Deviate, Z_R = -1.282

 ΔPSI = Initial Serviceability Index - Terminal Serviceability Index =

1.4

Therefore, Required Structural Number, $SN_{Required} = 3.67$

4. Determine Minimum Recommended Pavement Section:

Material Structural Number = Thickness of Material (inches) x AASHTO Material Coefficient Recommended SN = Surface Course SN + Base Course SN + Aggregate SN ≥ SN_{Required}

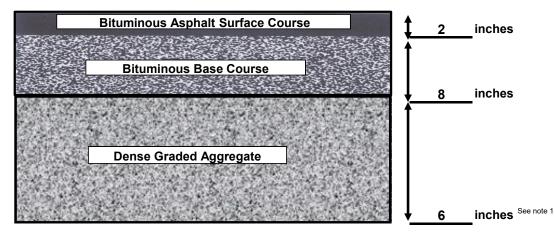
Material	AASHTO Material Coefficient
Bituminous Concrete Asphalt Surface Course	
·	0.44
Bituminous Base Course	0.44
Dense Graded Aggregate	0.12

Recommended Thicknesses:

- 2 inches of Bituminous Concrete Asphalt Surface Course
- 8 inches of Bituminous Base Course
- 6 inches of Dense Graded Aggregate

Therefore, Recommended SN = 5.12 ≥ 3.67 SECTION IS OK

Notes: (1) Dense Graded Aggregate can be increased to 8 inches for drainage considerations





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ABU DHABI DUBAI ATHENS DOHA ISTANBUL

Army Navy Drive Complete Streets Project Asphalt Pavement Design

Project No. 270060005

Date 4/15/2020

Calculations By ANG 4/15/20

Checked By Page 2 of 2

Pavement Design Requirements for Fern-Eads (CBR Value = 20)

1. Design Requirements

For a pavement design life of 30 years, use the Anticipated Traffic Data for design:

Anticipated Traffic Data

Average Daily Traffic (2019)^{see note 1}: 19,744 Vehicles/Day (Average Daily Traffic, ADT)

Annual Growth Rate (G)^{see note 2}: 0.00 %

Anticipated Traffic Data for Design ADT at 30 years: 19,744 Vehicles/Day (Design ADT) in year 2050

	Anticipated Dails	/ Traffic Breakdown b	y Vehicle
--	--------------------------	-----------------------	-----------

18,826	Cars/Passenger Vehicles	8	_5 Axle Single Trailer Trucks
316	Buses	5	6+ Axle Single Trailer Trucks
434	2 Axle Single Unit Trucks	2	5- Axle Multi Trailer Trucks
79	3+ Axle Single Unit Trucks	0	6 Axle Multi Trialer Trucks
12	4- Axle Single Trailer Trucks	4	7+ Axle Multi Trailer Trucks

Notes:

2. Determine Number of ESALs:

- (1) Traffic information provided by Arlington County.
- (2) Annual Growth Rate estimated from VDOT AADT projections for 2020 and 2040
- **VDOT Vehicle Factors**
- (3) Anticipated Daily Traffic Breakdown by Vehicle mirrors daily vehicle proportions given in traffic counts.

Vehicle Classification	ESAL Factor
Cars/Passenger Vehicles	0.0002
Buses	2.5
2 Axle Single Unit Trucks	0.46
3+ Axle Single Unit Trucks	0.46
4- Axle Single Trailer Trucks	1.05

Vehicle Classification	ESAL Factor
5 Axle Single Trailer Trucks	1.2
6+ Axle Single Trailer Trucks	1.05
5- Axle Multi Trailer Trucks	1.05
6 Axle Multi Trialer Trucks	1.06
7+ Axle Multi Trailer Trucks	1.39

ESAL = (Number Vehicles per day) x (ESAL Factor) x (D_D) x (D_L) x (Design Life in Days)

Design Life = 30_ years x 364 days/year = 10,920 days

Design Life for Vehicle Traffic less than 7 days/week: N/A days/week, Design Life = N/A days

Directional Distribution Factor, $D_D = 1$ Lane Distribution Factor, $D_L = 0.9$ Number of Passes per Vehicle = 1

37,004	_Cars/Passenger Vehicle ESALS	94,349	_5 Axle Single Trailer Truck ESALs
7,764,120	Bus ESALS	51,597	_6+ Axle Single Trailer Truck ESALs
1,962,062	2 Axle Single Unit Truck ESALS	20,639	_5- Axle Multi Trailer Truck ESALs
357,150	3+ Axle Single Unit Truck ESALS	0	6 Axle Multi Trialer Truck ESALs
123,833	4- Axle Single Trailer Truck ESALS	54,644	7+ Axle Multi Trailer Truck ESALs
	-	•	_

Total Equivalent Single Axle Loads = 10,465,397 ESALs

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$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left(\frac{\Delta PSI}{4.2-1.5}\right)}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32 \times \log_{10}(M_R) - 8.07$$

Equivalent Single Axle Loads, W₁₈ = 10,465,397 CBR =

Standard Deviation = 0.49 Subgrade Resilient Modulus, M_R = 1,500 x CBR = 30,000

Initial Serviceability Index = 4.2 Reliability Factor, R = 0.9

Terminal Serviceability Index = 2.8 Standard Normal Deviate, $Z_R = -1.282$

ΔPSI = Initial Serviceability Index - Terminal Serviceability Index = 1.4

Therefore, Required Structural Number, $SN_{Required} = 3.16$

4. Determine Minimum Recommended Pavement Section:

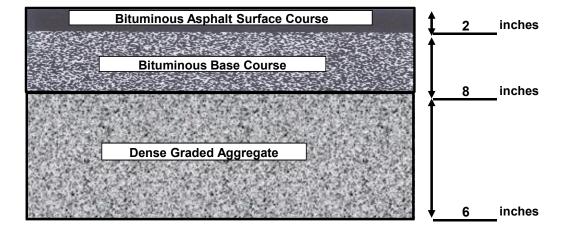
Material Structural Number = Thickness of Material (inches) x AASHTO Material Coefficient Recommended SN = Surface Course SN + Base Course SN + Aggregate SN \geq SN_{Required}

Material	AASHTO Material Coefficient
Bituminous Concrete Asphalt Surface Course	0.44
Bituminous Base Course	0.44
Dense Graded Aggregate	0.12

Recommended Thicknesses:

- 2 inches of Bituminous Concrete Asphalt Surface Course
- 8 inches of Bituminous Base Course
- 6 inches of Dense Graded Aggregate

Therefore, Recommended SN = 5.12 ≥ 3.16 SECTION IS OK



Notes:

(1) Dense Graded Aggregate can be increased to 8 inches for drainage considerations

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Pavement Design Requirements for Hayes-Fern (CBR Value = 20)

1. Design Requirements

For a pavement design life of 30 years, use the Anticipated Traffic Data for design:

Anticipated Traffic Data

Average Daily Traffic (2019)^{see note 1}: 27,334 Vehicles/Day (Average Daily Traffic, ADT)

Annual Growth Rate (G)^{see note 2}: 0.07 %

Anticipated Traffic Data for Design ADT at 30 years: 27,914 Vehicles/Day (Design ADT) in year 2050

Anticipated Daily Traffic Breakdown by Vehicle

26,068	Cars/Passenger Vehicles	50	5 Axle Single Trailer Trucks
670	Buses	33	6+ Axle Single Trailer Trucks
753	2 Axle Single Unit Trucks	20	5- Axle Multi Trailer Trucks
93	3+ Axle Single Unit Trucks	3	6 Axle Multi Trialer Trucks
194	4- Axle Single Trailer Trucks	17	7+ Axle Multi Trailer Trucks

Notes:

2. Determine Number of ESALs:

- (1) Traffic information provided by Arlington County.
- (2) Annual Growth Rate estimated from VDOT AADT projections for 2020 and 2040
- **VDOT Vehicle Factors**
- (3) Anticipated Daily Traffic Breakdown by Vehicle mirrors daily vehicle proportions given in traffic counts.

Vehicle Classification	ESAL Factor
Cars/Passenger Vehicles	0.0002
Buses	2.5
2 Axle Single Unit Trucks	0.46
3+ Axle Single Unit Trucks	0.46
4- Axle Single Trailer Trucks	1.05

Vehicle Classification	ESAL Factor
5 Axle Single Trailer Trucks	1.2
6+ Axle Single Trailer Trucks	1.05
5- Axle Multi Trailer Trucks	1.05
6 Axle Multi Trialer Trucks	1.06
7+ Axle Multi Trailer Trucks	1.39

ESAL = (Number Vehicles per day) x (ESAL Factor) x (D_D) x (D_L) x (Design Life in Days)

Design Life = 30 years x 364 days/year = 10,920 days

Design Life for Vehicle Traffic less than 7 days/week: N/A days/week, Design Life = N/A days

Directional Distribution Factor, $D_D = 1$ Lane Distribution Factor, $D_L = 0.9$ Number of Passes per Vehicle = 1

51,239 Cars/Passenger Vehicle ESALS	589,680	_5 Axle Single Trailer Truck ESALs
16,461,900 Bus ESALS	340,540	_6+ Axle Single Trailer Truck ESALs
3,404,223 2 Axle Single Unit Truck ESALS	206,388	_5- Axle Multi Trailer Truck ESALs
420,442 3+ Axle Single Unit Truck ESALS	31,253	6 Axle Multi Trialer Truck ESALs
2,001,964 4- Axle Single Trailer Truck ESALS	232,236	7+ Axle Multi Trailer Truck ESALs

Total Equivalent Single Axle Loads = 23,739,864 ESALs

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$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN + 1) - 0.20 + \frac{\log_{10}\left(\frac{\Delta PSI}{4.2 - 1.5}\right)}{0.40 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \times \log_{10}(M_R) - 8.07$$

Equivalent Single Axle Loads, W₁₈ = 23,739,864

Standard Deviation =

Initial Serviceability Index = 4.2

Terminal Serviceability Index = 2.8

Standard Normal Deviate, $Z_R = -1.282$

ΔPSI = Initial Serviceability Index - Terminal Serviceability Index =

1.4

Therefore, Required Structural Number, SN_{Required} = 3.66

4. Determine Minimum Recommended Pavement Section:

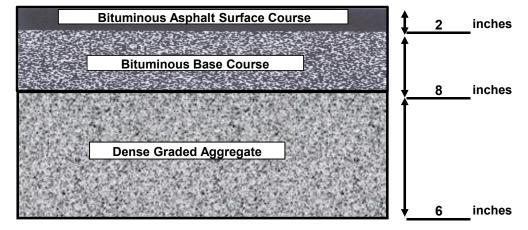
Material Structural Number = Thickness of Material (inches) x AASHTO Material Coefficient Recommended SN = Surface Course SN + Base Course SN + Aggregate SN \geq SN_{Required}

Material	AASHTO Material Coefficient
Bituminous Concrete Asphalt Surface Course	0.44
Bituminous Base Course	0.44
Dense Graded Aggregate	0.12

Recommended Thicknesses:

- 2 inches of Bituminous Concrete Asphalt Surface Course
- 8 inches of Bituminous Base Course
- 6 inches of Dense Graded Aggregate

Therefore, Recommended SN = 5.12 ≥ 3.66 SECTION IS OK



Calculation Title

Notes:

(1) Dense Graded Aggregate can be increased to 8 inches for drainage considerations

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Army Navy Drive Complete Streets Project

Asphalt Pavement Design

Project No.	
270060005	
Date	
4/15/2020	
Calculations By	7
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Checked By	

KJL 4/21/20

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Pavement Design Requirements for Joyce-Hayes (CBR Value = 7)

1. Design Requirements

For a pavement design life of 30 years, use the Anticipated Traffic Data for design:

Anticipated Traffic Data

Average Daily Traffic (2019)^{see note 1}: 21,234 Vehicles/Day (Average Daily Traffic, ADT)

Annual Growth Rate (G)^{see note 2}: 0.05 %

Anticipated Traffic Data for Design ADT at 30 years: 21,555 Vehicles/Day (Design ADT) in year 2050

Anticipated Daily Traffic Breakdown by Vehicle

19,737	Cars/Passenger Vehicles	40	5 Axle Single Trailer Trucks
0	Buses	22	6+ Axle Single Trailer Trucks
611	2 Axle Single Unit Trucks	16	5- Axle Multi Trailer Trucks
100	3+ Axle Single Unit Trucks	5	6 Axle Multi Trialer Trucks
130	4- Axle Single Trailer Trucks	6	7+ Axle Multi Trailer Trucks

Notes:

2. Determine Number of ESALs:

- (1) Traffic information provided by Arlington County.
- (2) Annual Growth Rate estimated from VDOT AADT projections for 2020 and 2040

VDOT Vehicle Factors

(3) Anticipated Daily Traffic Breakdown by Vehicle mirrors daily vehicle proportions given in traffic counts.

Vehicle Classification	ESAL Factor
Cars/Passenger Vehicles	0.0002
Buses	2.5
2 Axle Single Unit Trucks	0.46
3+ Axle Single Unit Trucks	0.46
4- Axle Single Trailer Trucks	1.05

Vehicle Classification	ESAL Factor
5 Axle Single Trailer Trucks	1.2
6+ Axle Single Trailer Trucks	1.05
5- Axle Multi Trailer Trucks	1.05
6 Axle Multi Trialer Trucks	1.06
7+ Axle Multi Trailer Trucks	1.39

ESAL = (Number Vehicles per day) x (ESAL Factor) x (D_D) x (D_L) x (Design Life in Days)

Design Life = <u>30</u> years x 364	days/year = 10,92	20 da	ys		
Design Life for Vehicle Traffic less	than 7 days/week:	N/A	_days/week, Design Life =	N/A	days
Directional Distribution Factor, D_D	= 1		_		
Lane Distribution Factor, D_L =	0.9				
Number of Passes per Vehicle =	1				

38,795 Cars/Passenger Vehicle ESALS	471,744	_5 Axle Single Trailer Truck ESALs
0 Bus ESALS	227,027	_6+ Axle Single Trailer Truck ESALs
2,762,258 2 Axle Single Unit Truck ESALS	165,110	_5- Axle Multi Trailer Truck ESALs
452,088 3+ Axle Single Unit Truck ESALS	52,088	_6 Axle Multi Trialer Truck ESALs
1,341,522 4- Axle Single Trailer Truck ESALS	81,966	_7+ Axle Multi Trailer Truck ESALs

Total Equivalent Single Axle Loads = 5,592,598 ESALs

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$$\log_{10}(W_{18}) = Z_{R} \times S_{o} + 9.36 \times \log_{10}(SN + 1) - 0.20 + \frac{\log_{10}(\frac{\Delta PSI}{4.2 - 1.5})}{0.40 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \times \log_{10}(M_{R}) - 8.07$$

Equivalent Single Axle Loads, $W_{18} = 5,592,598$

Standard Deviation =

Initial Serviceability Index =

Reliability Factor, R =

Terminal Serviceability Index = 2.8 Standard Normal Deviate, Z_R = -1.282

ΔPSI = Initial Serviceability Index - Terminal Serviceability Index =

1.4

Therefore, Required Structural Number, SN_{Required} = 4.33

4. Determine Minimum Recommended Pavement Section:

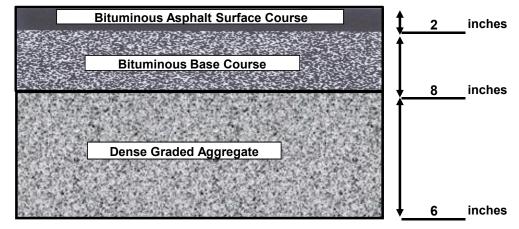
Material Structural Number = Thickness of Material (inches) x AASHTO Material Coefficient Recommended SN = Surface Course SN + Base Course SN + Aggregate SN ≥ SN_{Required}

Material	AASHTO Material Coefficient
Bituminous Concrete Asphalt Surface Course	0.44
Bituminous Base Course	0.44
Dense Graded Aggregate	0.12

Recommended Thicknesses:

- 2 inches of Bituminous Concrete Asphalt Surface Course
- 8 inches of Bituminous Base Course
- 6 inches of Dense Graded Aggregate

Therefore, Recommended SN = $\underline{5}.12 \geq 4.33$ **SECTION IS OK**



Calculation Title

Notes:

(1) Dense Graded Aggregate can be increased to 8 inches for drainage considerations

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Army Navy Drive Complete Streets Project

Asphalt Pavement Design

Project No.		
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Pavement Design Requirements for Joyce-Hayes Bus Lane (CBR Value = 7)

1. Design Requirements

For a pavement design life of 30 years, use the Anticipated Traffic Data for design:

Anticipated Traffic Data

Average Daily Traffic (2019)^{see note 1}:

Annual Growth Rate (G)^{see note 2}:

Anticipated Traffic Data for Design ADT at 30 years:

21,234 Vehicles/Day (Average Daily Traffic, ADT)

%

Vehicles/Day (Design ADT) in year 2050

Anticipated Daily Traffic Breakdown by Vehicle

0	Cars/Passenger Vehicles	0	5 Axle Single Trailer Trucks
901	Buses	0	6+ Axle Single Trailer Trucks
0	2 Axle Single Unit Trucks	0	5- Axle Multi Trailer Trucks
0	3+ Axle Single Unit Trucks	0	6 Axle Multi Trialer Trucks
0	4- Axle Single Trailer Trucks	0	7+ Axle Multi Trailer Trucks

Notes:

2. Determine Number of ESALs: (1) Traffic information provided by Arlington County.

(2) Annual Growth Rate estimated from VDOT AADT projections for 2020 and 2040

VDOT Vehicle Factors

(3) Anticipated Daily Traffic Breakdown by Vehicle mirrors daily vehicle proportions given in traffic counts.

Vehicle Classification	ESAL Factor	
Cars/Passenger Vehicles	0.0002	
Buses	2.5	
2 Axle Single Unit Trucks	0.26	
3+ Axle Single Unit Trucks	0.42	
4- Axle Single Trailer Trucks	0.3	

Vehicle Classification	ESAL Factor
5 Axle Single Trailer Trucks	1.2
6+ Axle Single Trailer Trucks	0.93
5- Axle Multi Trailer Trucks	0.82
6 Axle Multi Trialer Trucks	1.06
7+ Axle Multi Trailer Trucks	1.39

ESAL = (Number Vehicles per day) x (ESAL Factor) x (D_D) x (D_L) x (Design Life in Days)

Design Life =	30_ years x 364 days/year = 10,92	20 days	
Design Life for Vehicle Traffic less than 7 days/week: N/A days/week, Design Life = N/A days			
Directional Dis	stribution Factor, D _D = 1		
Non-Bus Lane Distribution Factor, $D_L = 0.9$		Bus Lane Distribution Factor, D _L = 1	
Number of Passes per Vehicle = 1			
0	Cars/Passenger Vehicle ESALS	0 5 Axle Single Trailer Truck ESALs	
24,597,300	Bus ESALS	0 6+ Axle Single Trailer Truck ESALs	
0	2 Axle Single Unit Truck ESALS	0 5- Axle Multi Trailer Truck ESALs	
0	_3+ Axle Single Unit Truck ESALS	0 6 Axle Multi Trialer Truck ESALs	
0	4- Axle Single Trailer Truck ESALS	0 7+ Axle Multi Trailer Truck ESALs	

Total Equivalent Single Axle Loads = 24,597,300 ESALs

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$$\log_{10}(W_{18}) = Z_R \times S_o + 9.36 \times \log_{10}(SN + 1) - 0.20 + \frac{\log_{10}\left(\frac{\Delta PSI}{4.2 - 1.5}\right)}{0.40 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \times \log_{10}(M_R) - 8.07$$

Equivalent Single Axle Loads, W₁₈ = 24,597,300 CBR = 7

Standard Deviation = 0.49 Subgrade Resilient Modulus, M_R = 1,500 x CBR = 10,500

Initial Serviceability Index = 4.2 Reliability Factor, R = 0.9

Terminal Serviceability Index = 2.8 Standard Normal Deviate, $Z_R = -1.282$

ΔPSI = Initial Serviceability Index - Terminal Serviceability Index = 1.4

Therefore, Required Structural Number, SN_{Required} = 5.43

4. Determine Minimum Recommended Pavement Section:

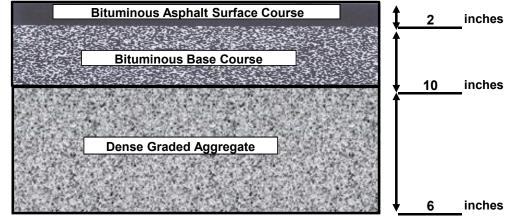
Material Structural Number = Thickness of Material (inches) x AASHTO Material Coefficient Recommended SN = Surface Course SN + Base Course SN + Aggregate SN \geq SN_{Required}

Material	AASHTO Material Coefficient
Bituminous Concrete Asphalt Surface Course	0.44
Bituminous Base Course	0.44
Dense Graded Aggregate	0.12

Recommended Thicknesses:

- 2 inches of Bituminous Concrete Asphalt Surface Course
- 10 inches of Bituminous Base Course
- 6 inches of Dense Graded Aggregate

Therefore, Recommended SN = 6 ≥ 5.43 SECTION IS OK



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