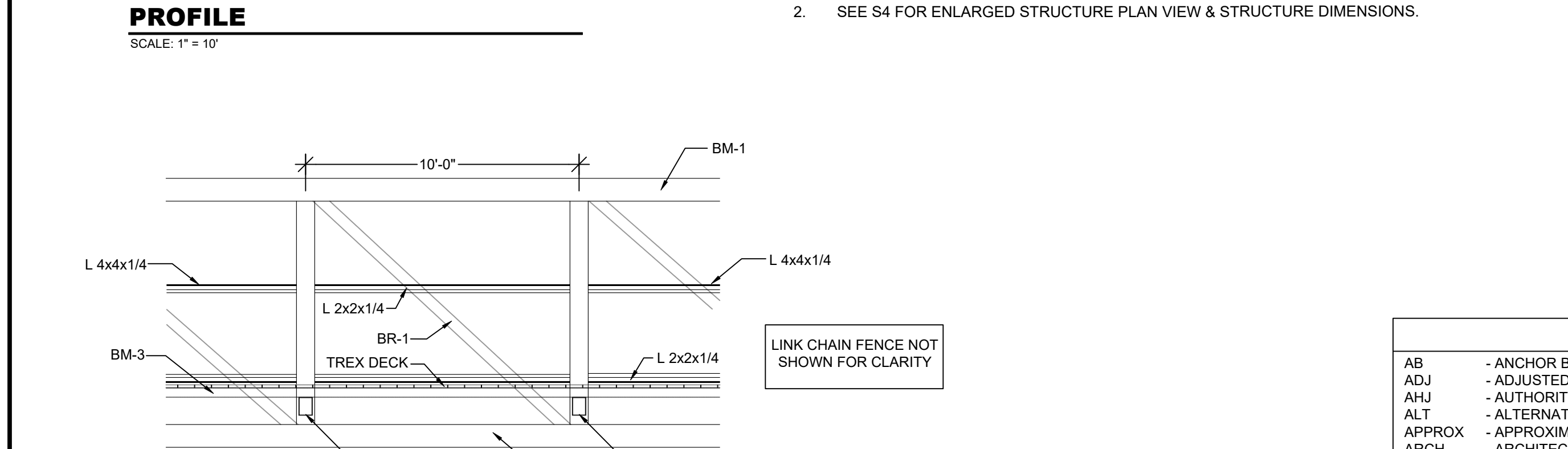
**PLAN VIEW**

SCALE: 1" = 10"

**NOTES:**

- SEE S3 FOR FOUNDATION STRUCTURE INFORMATION.
- SEE S4 FOR ENLARGED STRUCTURE PLAN VIEW & STRUCTURE DIMENSIONS.

**PROFILE**

SCALE: 1" = 10"

**NOTES:**

- SEE S3 FOR FOUNDATION STRUCTURE INFORMATION.
- SEE S4 FOR ENLARGED STRUCTURE PLAN VIEW & STRUCTURE DIMENSIONS.

**DETAIL A-A**

SCALE: 1/4" = 1'-0"

**MEMBER SCHEDULE**

MARK	TYPE	REINFORCEMENT / CONNECTION	NOTES
<b>BEAMS</b>			
BM-1	HSS 10"x10"x $\frac{5}{16}$ "	SEE DETAILS 1-7	BRIDGE TOP & BOTTOM BEAMS
BM-2	HSS 8"x6"x $\frac{5}{16}$ "	SEE DETAILS 5, 6, & 7	BRIDGE FLOOR BEAM
BM-3	HSS 4"x2"x $\frac{1}{2}$ "	SEE DETAILS 5 & 6	BRIDGE FLOOR STRINGER
<b>COLUMNS</b>			
C-1	HSS 8"x8"x $\frac{1}{2}$ "	SEE DETAILS 1-6	BRIDGE COLUMN
C-2	20"x20" CONC. COLUMN	(8) #7 VERT. BARS; #3 STIRRUPS	REINF. CONC. COLUMN BELOW BRIDGE; SEE DETAILS ON S3
<b>BRACES</b>			
BR-1	HSS 5"x3"x1/4"	SEE DETAILS 2&4	BRIDGE DIAGONAL BRACE
BR-2	HSS 4"x4"x1/4"	SEE DETAIL 7	BRIDGE FLOOR DIAGONAL BRACE
<b>CONCRETE FOOTING</b>			
F-1	4'-6"W x 1'-4"D x CONT.	#5 BARS @ 12" O.C. T&B E.W.	SEE FOUNDATION PLAN & DETAILS ON S3
F-2	6'-0"W x 1'-4"D x CONT.	#5 BARS @ 12" O.C. T&B E.W.	SEE FOUNDATION PLAN & DETAILS ON S3
<b>CONCRETE RETAINING WALL</b>			
R-1	12" THICK REINF. CONC.	(2) #5 BARS @ 12" O.C. VERT. (2) #5 BARS @ 18" O.C. HORIZ.	UNDER 6" THICK CONCRETE SLAB; SEE DETAILS ON S3

ABBREVIATIONS			
AB	- ANCHOR BOLT	KLF	- KIPS PER LINEAR FOOT
ADJ	- ADJUSTED/ADJACENT	KJ	- CONSTRUCTION JOINT
AHJ	- AUTHORITY HAVING JURISDICTION	L	- ANGLE
ALT	- ALTERNATE	LA	- LONG
APPROX	- APPROXIMATELY	MAS	- MASONRY
ARCH	- ARCHITECT	MAX	- MAXIMUM
BC	- BOTTOM CHORD	MFR	- MANUFACTURER
BLDG	- BUILDING	MIN	- MINIMUM
BM	- BEAM	MISC	- MISCELLANEOUS
BOIT	- BOTTOM	MO	- MASONRY OPENING
BRG	- BEARING	MPH	- MILES PER HOUR
CCOL	- COASTAL CONST. CONTROL LINE	MTL	- METAL
CFS	- COLD FORMED STEEL	NGVD	- NATIONAL GEODETIC VERTICAL DATUM
CIP	- CAST IN PLACE	NIC	- NOT IN CONTRACT
CJ	- CONTRACTION JOINT	NTS	- NOT TO SCALE
CL	- CENTERLINE	OC	- ON CENTER
CLR	- CLEAR	OPNG	- OPENING
CMU	- CONCRETE MASONRY UNIT	PAF	- POWDER ACTUATED FASTENERS
COL	- COLUMN	PART	- PARTITION
CONC	- CONCRETE	PCF	- POUNDS PER CUBIC FOOT
CONST	- CONSTRUCTION	PCI	- POUNDS PER CUBIC INCH
CONT	- CONTINUOUS	PL	- PLATE
CTR	- CENTER	PLF	- POUNDS PER LINEAR FOOT
D&E	- DRILLED AND EPOXIED	PSF	- POUNDS PER SQUARE FOOT
DBL	- DOUBLE	PSI	- POUNDS PER SQUARE INCH
DET	- DETAIL	PT	- POST TENSIONED/PRESSURE TREATED
DIA	- DIAMETER	R	- RISER/RADIUS
DIM	- DIMENSION	REG	- REGULAR
DN	- DOWN	REINF	- REINFORCING
DR	- DOOR/DRAIN	REM	- REMAINDER
DWG	- DRAWING	REQ'D	- REQUIRED
EA	- EACH	REV	- REVISION/REVISED
EE	- EACH END	RM	- ROOM
EF	- EACH FACE	RO	- ROUGH OPENING
EJ	- EXPANSION JOINT	RQMTS	- REQUIREMENTS
EL	- ELEVATION	SCHED	- SCHEDULE
ELEV	- ELEVATION/ELEVATOR	SECT	- SECTION
ENGR	- ENGINEER	SER	- STRUCTURAL ENGINEER OF RECORD
EOR	- ENGINEER OF RECORD	SIM	- SIMILAR
EOS	- EDGE OF SLAB	SL	- SLOPE
EQ	- EQUAL	SOG	- SLAB-ON-GRADE
EW	- EACH WAY	SP	- SPIRAL
EXIST	- EXISTING	SPECS	- SPECIFICATIONS
EXP	- EXPANSION	SQ	- SQUARE
EXT	- EXTERIOR	SS	- STAINLESS STEEL
FBC	- FLORIDA BUILDING CODE	SSE	- SPECIALTY STRUCTURAL ENGINEER
FIN	- FINISH	STD	- STANDARD
FLR	- FLOOR	STL	- STEEL
FND	- FOUNDATION	SW	- SHEAR WALL/ SHORT WAY
FT	- FEET/FOOT	T/	- TOP OF
FTG	- FOOTING	TB	- TIE BEAM
GA	- GAUGE	TC	- TIE COLUMN/TOP CHORD
GALV	- GALVANIZED	TEMP	- TEMPERATURE
GC	- GENERAL CONTRACTOR	TJ	- TIE JOIST
GT	- GIRDER TRUSS	TR	- TREAD/TRUSS
HC	- HOLLOW CORE	TYP	- TYPICAL
HDG	- HOT DIPPED GALVANIZED	UNO	- UNLESS NOTED OTHERWISE
HG	- HIP GIRDER	VERT	- VERTICAL
HK	- HOOK	VF	- VERIFY IN THE FIELD
HORIZ	- HORIZONTAL	WI	- WITH
HS	- HIGH STRENGTH	W/O	- WITHOUT
IJ	- ISOLATION JOINT	WD	- WOOD
IN	- INCH/INCHES	WWF	- WELDED WIRE FABRIC
INFO	- INFORMATION	WWM	- WELDED WIRE MESH
INT	- INTERIOR	#4	- STEEL REINFORCING BAR (REBAR) #4 (1/2")
JT	- JOINT	#5	- STEEL REINFORCING BAR (REBAR) #5 (5/8")
K	- KIP(s) - 1000 POUNDS	#6	- STEEL REINFORCING BAR (REBAR) #6 (3/4")

**STRUCTURAL-GENERAL NOTES****DESIGN CRITERIA AND LOADS:**

- WIND DESIGN:  
WIND SPEED (MPH) [PER SDG TABLE 2.4.1-1] V(ADJ)=132 V(ULT)=170  
RISK CATEGORY II  
EXPOSURE CATEGORY C  
ENCLOSURE CLASSIFICATION OPEN  
INTERNAL PRESSURE COEFFICIENT CPI = +/- 0.00  
TOPOGRAPHIC FACTOR KZT = 1.0
- DESIGN LIVE LOADS:  
a. LIVE LOADS: 90 PSF  
b. RAILING & GUARDRAILS: 50 LBS/FT OR 200 LBS TO BE APPLIED AT ANY DIRECTION TO THE TOP RAIL.
- DESIGN DEAD LOADS :  
a. COMPOSITE DECK DEAD LOAD: 5 PSF
- THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE STRUCTURAL ENGINEER OF RECORD (SER) OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL/ STRUCTURAL/ MECHANICAL/ ELECTRICAL OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.
- LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

**GENERAL REQUIREMENTS**

- PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THE STRUCTURAL GENERAL NOTES.
- VEHICULAR AND PEDESTRIAN BRIDGES STEEL SPECIFICATION ASTM A709  
ALL WORK SHALL CONFORM TO CURRENT VERSIONS OF THE FOLLOWING DOCUMENTS:  
- FDOT REFERENCES:  
STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (SPECIFICATIONS)  
STRUCTURES MANUAL (SDG)  
PLANS PREPARATION MANUAL (PPM)  
  
- AASHTO SPECIFICATIONS:  
LRFD GUIDE SPECIFICATION FOR THE DESIGN OF PEDESTRIAN BRIDGES  
AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS (LRFD)  
AASHTO/AWS D1.5M/D1.5: BRIDGE WELDING CODE  
AASHTO STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS AASHTO GUIDE DESIGN SPECIFICATIONS FOR BRIDGE TEMPORARY WORKS  
  
- STRUCTURAL ELEMENTS SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING REFERENCES:  
  
FLORIDA BUILDING CODE, FBC, SIXTH EDITION  
BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE ACI 318  
SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS AISC 360.  
AASHTO M270 (ASTM A709) STANDARD SPECIFICATION FOR STRUCTURAL STEEL FOR BRIDGES.  
MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7
- WHERE OTHER STANDARDS ARE NOTED IN THE DRAWINGS, USE THE LATEST EDITION OF THE STANDARD UNLESS A SPECIFIC DATE IS INDICATED. REFERENCE TO A SPECIFIC SECTION IN A CODE DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE ENTIRE STANDARD.
- REFER TO THE CIVIL DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING BUT NOT LIMITED TO: DIMENSIONS, ELEVATIONS, SLOPES, AND OTHER NONSTRUCTURAL ITEMS.
- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK WITH ARCHITECT, ENGINEER(S) AND OTHER TRADES; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS; FOR SELECTING FABRICATION PROCESSES; FOR TECHNIQUES OF ASSEMBLY; AND FOR PERFORMING WORK IN A SAFE AND SECURE MANNER.
- IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS PLANDTAILS, REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT/ENGINEER AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.
- THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH).
- THE STRUCTURAL DRAWINGS ARE INTENDED TO SHOW THE GENERAL CHARACTER AND EXTENT OF THE PROJECT AND ARE NOT INTENDED TO SHOW ALL DETAILS OF THE WORK.
- ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION THAT SIGNIFICANTLY DEViate FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.
- ALL BUILDING SITES SHALL BE GRADED TO PROVIDE DRAINAGE UNDER ALL PORTIONS OF THE BUILDING / STRUCTURE AND AROUND THE BUILDING/ STRUCTURE PERIMETER TO ALLOW DRAINAGE AWAY FROM THE STRUCTURE.
- SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, DIMENSIONS, ELEVATIONS,ETC.
- SHOP DRAWINGS SHALL BE REVIEWED BY CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.
- CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY CLOUDED AND NOTED. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.
- DISCREPANCIES, OMISSIONS, OR INCONSISTENCIES WITH APPLICABLE CODE REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE SUBMITTING A BID OR PROCEEDING WITH THE WORK.
- THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATIONS, SHORING, AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY COMPLETE.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE PROCEEDING WITH THE WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. DO NOT LOAD STRUCTURES, NEW OR EXISTING, WITH WEIGHT THAT WILL ENDANGER STRUCTURE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE

FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

**FOUNDATION AND SLABS ON GRADE**


- FOUNDATION IS DESIGNED BASED ON PRESUMPTIVE SAFE ALLOWABLE BEARING PRESSURE OF 2,000 PSF. CONTRACTOR SHALL VERIFY THAT THE MINIMUM BEARING PRESSURE IS OBTAINED PRIOR TO FOOTING PLACEMENT.
- THE ARCHITECT /ENGINEER ASSUMES NO RESPONSIBILITY FOR ANY INTERPRETATION THAT THE SUBSURFACE CONDITIONS DESCRIBED IN THE TEST BORING LOGS OCCUR CONSISTENTLY THROUGHOUT THE JOB SITE. TEST BORINGS ARE INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT SOIL CONDITIONS ONLY AT THE SPECIFIC LOCATIONS AND AT THE PARTICULAR TIMES THEY WERE TAKEN.
- REINFORCED FOUNDATION REQUIREMENTS USED IN THE DESIGN:  
a. MINIMUM DEPTH BELOW FINISHED GRADE.....1'-0"  
b. MAXIMUM ALLOWABLE BEARING CAPACITY .....2,000 PSF  
c. MODULUS OF SUBGRADE REACTION.....200 PCI  
d. PASSIVE LATERAL PRESSURE.....250 PSF  
e. ACTIVE LATERAL PRESSURE (UNRESTRAINED) .....55 PSF  
f. ACTIVE LATERAL PRESSURE (RESTRAINED).....35 PSF  
g. COEFFICIENT OF SLIDING FRICTION.....0.4
- ALL FOUNDATION CONCRETE SHALL BE CAST IN THE DRY. DEWATERING OPERATION SHALL BE DONE IN SUCH A WAY THAT GROUND WATER LEVELS OUTSIDE THE SITE WILL BE MAINTAINED TO AVOID SETTLEMENT AND DAMAGE TO NEARBY BUILDINGS AND STRUCTURES.
- THE CONCRETE MIXTURE SHALL COMPLY WITH FDOT CLASS II MIXTURE REQUIREMENTS. THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE THE MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS AND THE WATER/CEMENT RATIO IS THE MAXIMUM. THE SPECIFIED SLUMP IS THE MAXIMUM PRIOR TO THE ADDITION OF ADMIXTURES. CONCRETE SHALL BE STANDARD WEIGHT CONCRETE (145 PCF)

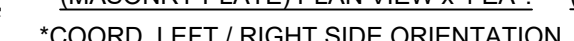
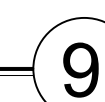
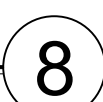
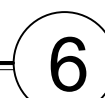
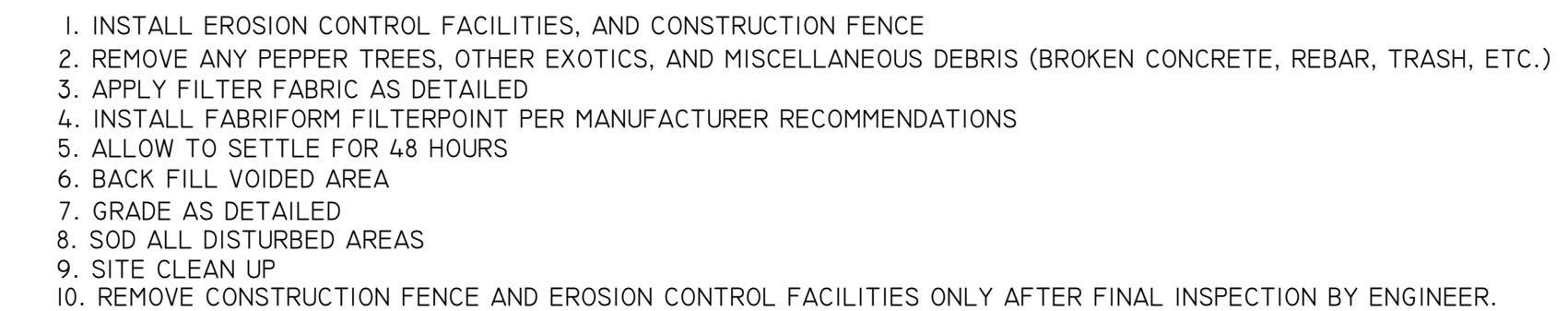
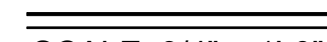
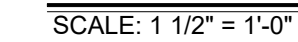
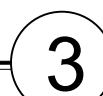
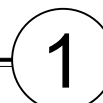
ITEM OF CONSTRUCTION	STRENGTH (PSI)	AGG (IN)	SLUMP (IN)	WATER/CEMENT (LB/LB)
FOUNDATION PADS	4,000	$\frac{3}{4}$ - $1\frac{1}{2}$	3-6	0.53
SLABS ON GRADE	4,000	$\frac{3}{4}$ - $1\frac{1}{2}$	3-6	0.53
COLUMNS	4,000	$\frac{3}{4}$ - $1\frac{1}{2}$	3-6	0.53
RETAINING WALL	4,000	$\frac{3}{4}$ - $1\frac{1}{2}$	3-6	0.53
CURB	4,000	$\frac{3}{4}$ - $1\frac{1}{2}$	3-6	0.53

- IN ADDITION, SLABS-ON-GRADE SHALL HAVE A COMPRESSIVE STRENGTH OF AT LEAST 2400 PSI AT THREE DAYS IF SUBJECT TO CONSTRUCTION TRAFFIC.
- REMOVE AND REPLACE MINIMUM 1 FEET OF EXISTING SOIL BELOW FOUNDATION WITH COMPACTED, MOISTURE-TREATED, NON-EXPANSIVE FILL MATERIAL. FILL AREA SHALL EXTEND 1 FOOT BEYOND FOUNDATION FOOTPRINT.
- FOR SITE PREPARATION, REMOVE DELETERIOUS MATERIAL SUCH AS VEGETATION, ORGANIC SOILS AND ROOT ZONES, EXISTING FILL, OR LOOSE, SOFT FROZEN, OR OTHERWISE UNSUITABLE MATERIALS FROM BELOW THE PROPOSED FOUNDATION AREAS.
- SOIL BENEATH SLABS AND FOOTINGS SHALL BE EXCAVATED AS REQUIRED TO REMOVE ALL ORGANIC AND DELETERIOUS MATERIALS. PLACE CLEAN SAND FILL IN MAXIMUM OF 12 INCH LIFTS. SUBGRADE AND EACH LIFT SHALL BE COMPACTED TO A MINIMUM OF 98 PERCENT OF ITS MODIFIED PROCTOR VALUE IN ACCORDANCE WITH ASTM D 1557.
- SUBGRADE SHALL BE UNIFORM OVER THE ENTIRE FOUNDATION AREA.
- FOUNDATIONS SHALL BEAR ON EITHER COMPETENT NATIVE SOIL OR COMPACTED STRUCTURAL FILL AS PER THE GEOTECHNICAL REPORT. EXTERIOR PERIMETER FOOTINGS SHALL BEAR NOT LESS THAN 24 INCHES BELOW FINISH GRADE, UNLESS OTHERWISE SPECIFIED BY THE GEOTECHNICAL ENGINEER AND/OR THE BUILDING OFFICIAL.
- PROVIDE 6 MIL 'VISQUEEN' VAPOR BARRIER UNDER ALL SLABS ON FILL (UNLESS OTHERWISE NOTED ON PLANS).
- COLUMNS, BEAMS, WALLS OR ANY OTHER STRUCTURAL MEMBER PENETRATING SLABS ON FILL SHALL BE ISOLATED BY PRE-MOLDED JOINT FILLER (1/2" THICK) COMPLYING WITH ASTM D1752, TYPE 1.
- TOPS OF FOOTINGS AND SLABS ON GRADE SHALL BE AS SHOWN ON PLANS WITH VERTICAL CHANGES AS INDICATED WITH STEPS IN THE FOOTINGS; LOCATIONS OF STEPS SHOWN AS APPROXIMATE AND SHALL BE COORDINATED WITH THE CIVIL GRADING PLANS TO ENSURE THAT THE EXTERIOR PERIMETER FOOTINGS BEAR NO LESS THAN 24 INCHES BELOW FINISH GRADE, OR AS OTHERWISE INDICATED BY THE GEOTECHNICAL ENGINEER OR BUILDING OFFICIAL.
- CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:  
a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"  
b. EXPOSED TO EARTH OR WEATHER:  
NO. 5 AND SMALLER BARS 1 1/2"  
NO. 6 AND LARGER BARS 2"
- NON-EXPANSIVE BACKFILL SHALL BE PLACED IN CONTROLLED LIFTS NOT TO EXCEED 12 INCHES AND SHALL BE COMPACTED TO AT LEAST 98% OF LABORATORY MAXIMUM DENSITY (ASTM D 1557).
- AREA DRAINAGE SHALL BE DIRECTED AWAY FROM THE FOUNDATION.
- GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SHORING, SHEETING AND BRACING OF EXCAVATIONS.
- GENERAL CONTRACTOR SHALL INSTALL ALL PIPE SLEEVES, BOXED OPENINGS, ANCHOR BOLTS, ETC., AS REQUIRED FOR THE VARIOUS TRADES. WALL POCKETS TO RECEIVE BEAMS AND SLABS SHALL BE PROVIDED AS REQUIRED FOR THE SUPER-STRUCTURE. SHOP DRAWINGS SHOWING THE POSITION OF OPENINGS SHALL BE SUBMITTED TO THE CONTRACTING OFFICER PRIOR TO PLACEMENT OF CONCRETE.
- LOCATE SAWCUTS PER PLAN OR IF NOT SHOWN ON PLANS COMPLY WITH ACI 224.3.
- IN NO CASE SHALL TRUCKS, BULLDOZERS OR OTHER HEAVY EQUIPMENT BE PERMITTED CLOSER THAN 8'-0" FROM ANY FOUNDATION WALL UNLESS APPROVED BY ENGINEER.

**STRUCTURAL STEEL**

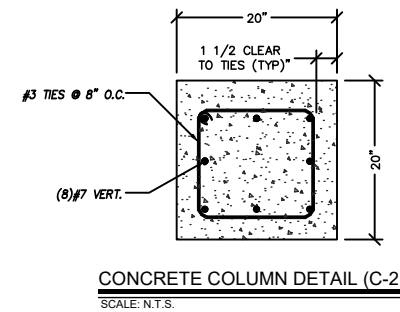
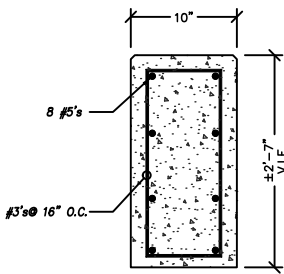
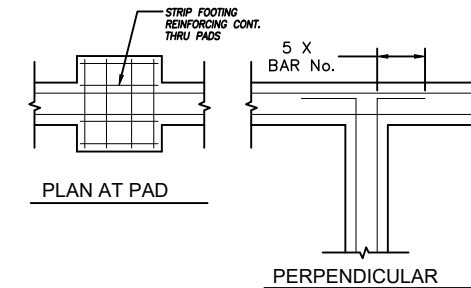
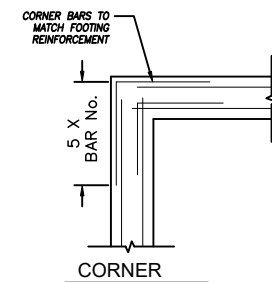
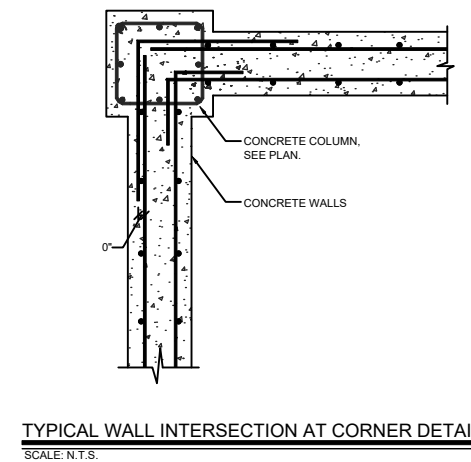
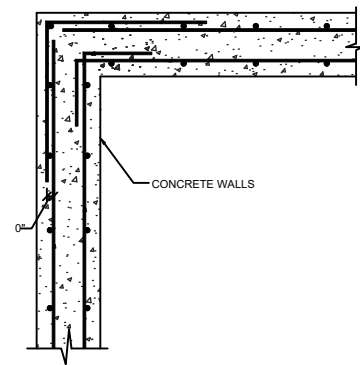
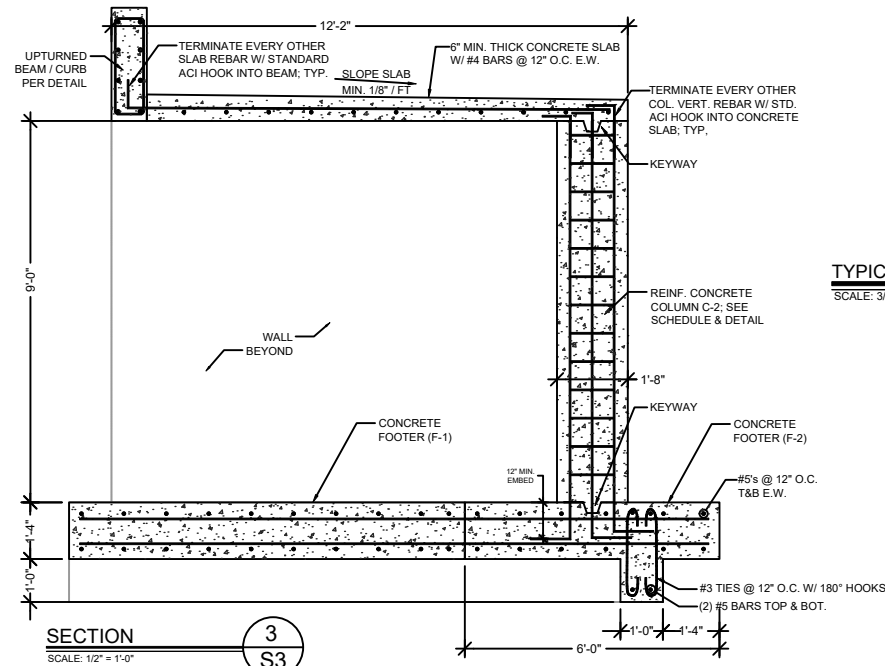
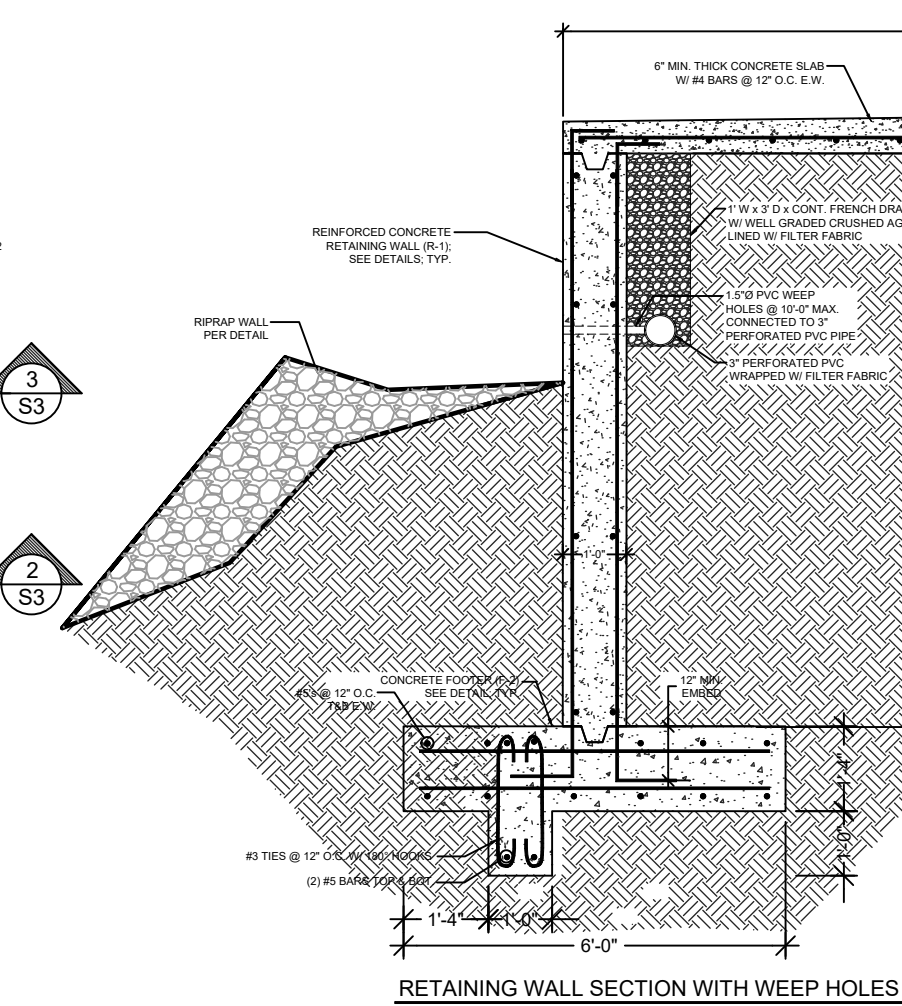
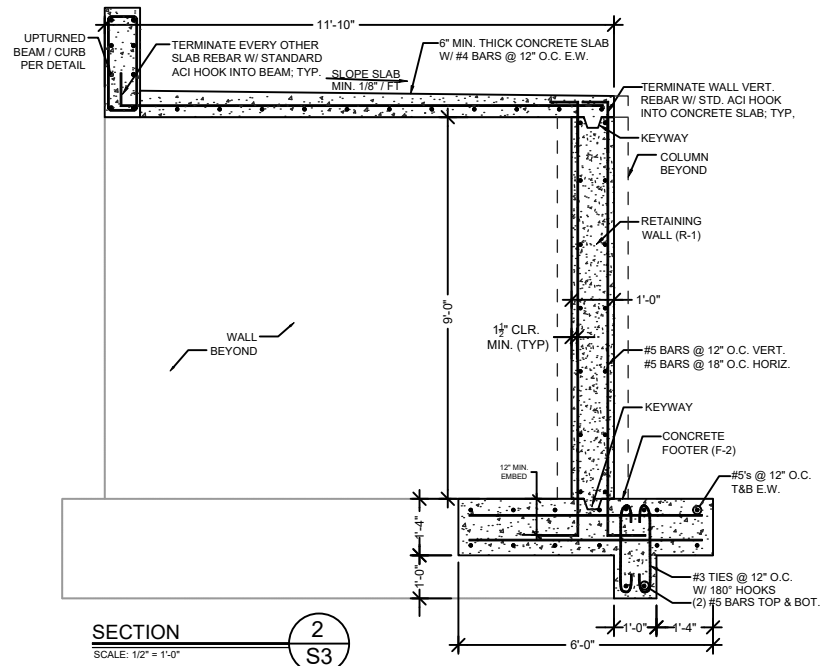
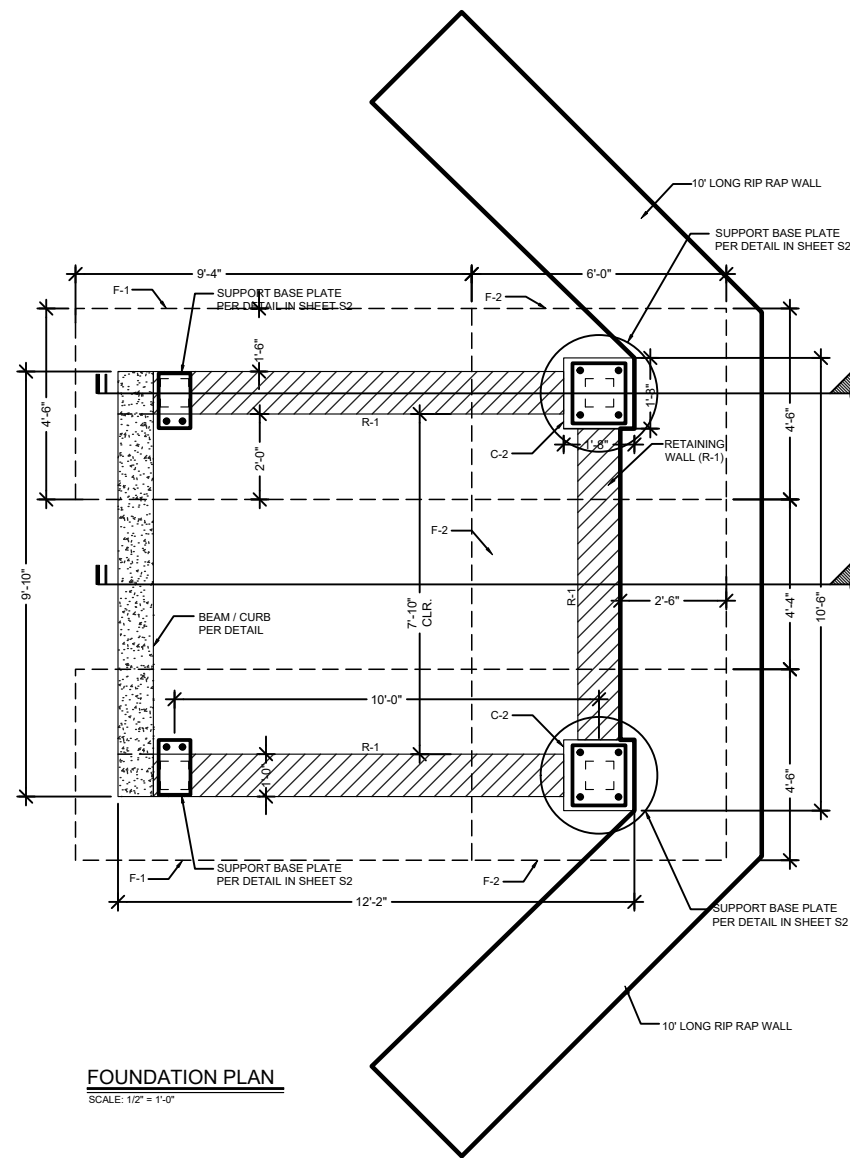
- STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE", LATEST EDITION. PROVIDE STRUCTURAL STEEL IN ACCORDANCE WITH STRUCTURAL STEEL FOR BRIDGES, ASTM A709.
- STRUCTURAL STEEL GRADES AND MIN. STRESS SHALL BE AS FOLLOWS:  
a. M, S, C MC, L SHAPES ASTM A36, FY = 36 KSI, FU = 58 KSI  
b. HSS RECT. ASTM A500", FY = 50 KSI, FU = 62 KSI (\*TESTING REQ'D.)  
\*CHARPY V-NOTCH TESTING  
ASTM A1085, FY = 50 KSI, FU = 65 KSI  
c. PLATES AND BARS (CARBON) ASTM A36, FY = 36 KSI, FU = 58 KSI  
d. ANCHOR BOLTS (COMMON) ASTM A307, FY = 105 KSI  
e. HIGH STRENGTH BOLTS (STRUCTURAL) ASTM A325, FY = 105 KSI  
f. ANCHOR RODS ASTM F 1554, FY = 36 KSI, FU = 58 KSI  
g. HEAVY HEX NUTS ASTM A563  
h. HARDENED STEEL WASHERS ASTM F436
- HIGH STRENGTH BOLTS SHALL BE TIGHTENED USING "SNUG TIGHT" METHOD.
- WELDING OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH A.W.S. D1.1 AND AASHTO/AWS D1.5M/D1.5 WITH E70XX ELECTRODES. FILLET WELDS SHALL BE MIN. 3/16" UNLESS NOTED OTHERWISE. AASHTO/AWS D1.5M/D1.5/2002 BRIDGE WELDING CODE.
- GROUT UNDER BEARING PLATES MUST BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 5,000 PSI IN 28 DAYS.
- THE CAMBER OF STEEL MEMBERS MUST BE VERIFIED IN THE SHOP AND THE FIELD; WHEN NO CAMBER IS INDICATED, TURN THE MEMBER NATURAL CAMBER UP.
- ALL STRUCTURAL STEEL MEMBERS EXPOSED TO EXTERIOR CONDITIONS MUST BE HOT DIPPED GALVANIZED PER ASTM A123 AND ALL FASTENERS AND HARDWARE MUST BE HOT DIPPED GALVANIZED PER ASTM A 153.
- ALL ANCHOR BOLTS AND RODS, NUTS, WASHERS AND OTHER TIE-DOWN HARDWARE ARE TO BE HOT-DIP GALVANIZED. BOLTS SHALL BE HOT-DIP GALVANIZED ASTM F3125 GRADE A325 BOLTS.
- WELDING COMPONENTS TOGETHER AFTER GALVANIZING IS NOT ACCEPTABLE.

19-0077		SHEET <b>S1</b> OF 4		RODOLFO VILLANIZAR FL. P.E.#51000		JACKIE ROBINSON TRAINING COMPLEX WALKING TRAIL		FLORIDA		CITY OF VERO BEACH		PLAN VIEW, PROFILE, ELEVATIONS, AND STRUCTURAL NOTES		 <b>MBV</b> ENGINEERING, INC. MOA BOWLES VILLANIZAR & ASSOCIATES CONSULTING ENGINEERING CA #3728		1835 - 20TH STREET VERO BEACH, FL 32960 PH. (772) 569-0035 FX. (772) 778-3617  MELBOURNE, FL - PH (321) 253-1510 FT. PIERCE, FL - PH (772) 488-9055 SCALE AS NOTED		△		REVISIONS		DATE															
																		JOB NO.		19-0077		DESIGNED		AC		1		2		3		4		5		6	
																		DRAWN		AER & JT		DATE		OCT 2019		CHECKED		RV		DATE ISSUED		2023-1510		SCALE		AS NOTED	



SHEET  
**S2**  
OF 4  
19-0077

19-0077



REVISIONS		DATE
1		
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JOB NO.	DESIGNED	AC	19-0077
DRAWN	AER & JT		
DATE	OCT 2019		
CHECKED	RV		
DATE ISSUED			
SCALE	AS NOTED		

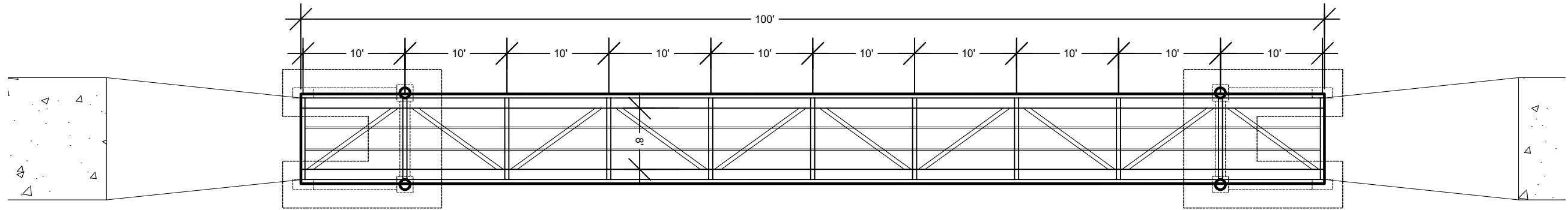
1835 - 20TH STREET  
VERO BEACH, FL 32960  
PH. (772) 569-0035  
FAX. (772) 778-3617  
MELBOURNE, FL - PH (321) 258-1510  
FT. PIERCE, FL - PH (772) 468-9055

**MBV ENGINEERING, INC.**  
MOJIA BOWLES VILLAMIZAR & ASSOCIATES  
CONSULTING ENGINEERING CA #3728

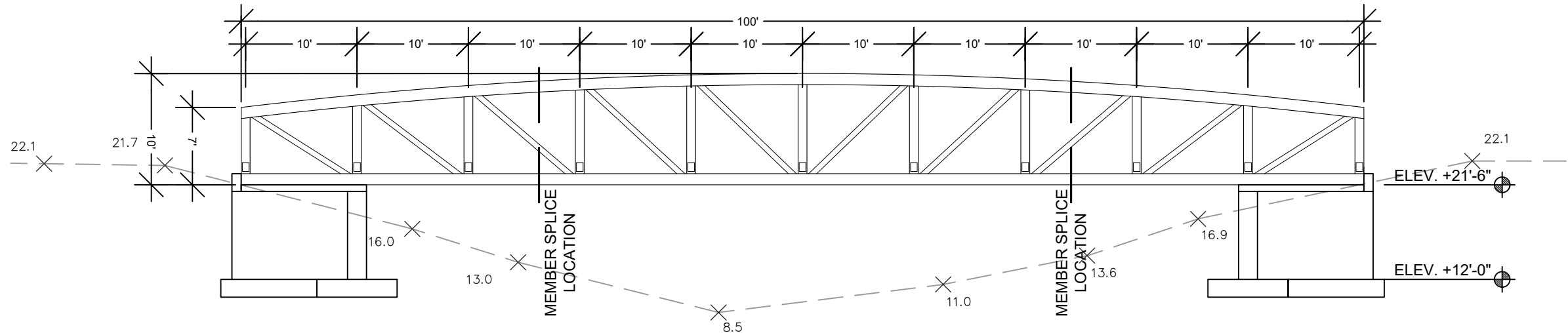
FOUNDATION PLAN, SECTIONS,  
AND STRUCTURAL DETAILS

JACKIE ROBINSON TRAINING  
COMPLEX WALKING TRAIL  
CITY OF VERO BEACH  
FLORIDA

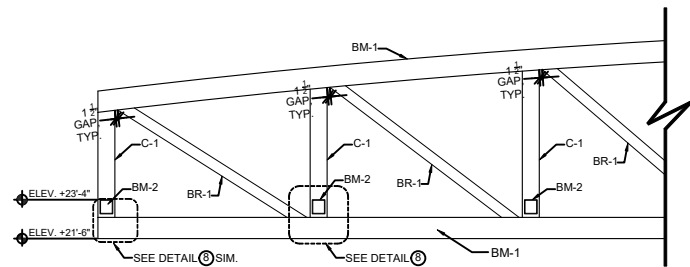
RODOLFO VILLAMIZAR  
FL P.E. #61000  
DATE:  
SHEET  
**S3**  
OF 4  
19-0077



PLAN VIEW  
SCALE: 1" = 5'

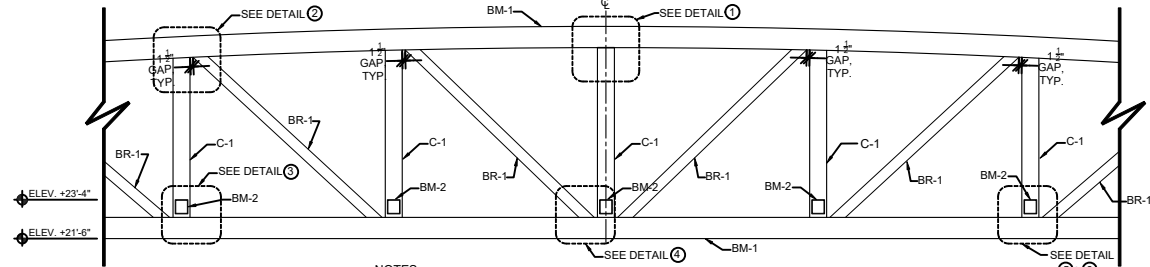


PROFILE  
SCALE: 1" = 5'



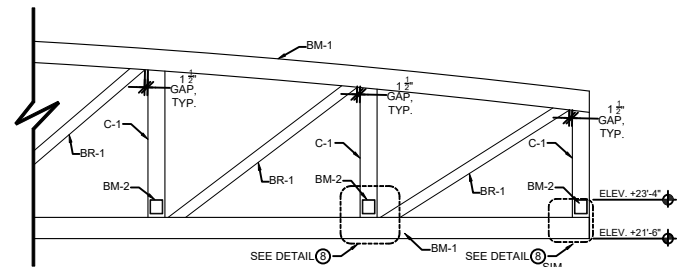
WEST END SECTION  
SCALE: 1/4" = 1'-0"

NOTE: SEE NOTES THIS SHEET.



MIDDLE SECTION  
SCALE: 1/4" = 1'-0"

- NOTES:
1. SEE SHEET S2 FOR CLOSE-UP CONNECTION DETAILS.
  2. PROVIDE WEEPHOLES TO ALLOW WATER DRAINAGE AT UNDERSIDE OF ALL MEMBERS, TYP.
  3. FOR BRIDGE MEMBER SPLICE CONNECTIONS, SEE SPLICE CONNECTION DETAIL ① IN SHEET S2.
  4. FOR BRIDGE BEARING CONNECTIONS TO CONCRETE STRUCTURE, SEE COLUMN AND WALL BASE PLATE DETAILS IN S2.



EAST END SECTION  
SCALE: 1/4" = 1'-0"

NOTE: SEE NOTES THIS SHEET.

<b>MBV</b> ENGINEERING, INC. MOJIBOWLES VILLAMIZAR & ASSOCIATES CONSULTING ENGINEERING CA #3728	1835 - 20TH STREET VERO BEACH, FL 32960 PH: (772) 569-0035 FX: (772) 778-3617	JACKIE ROBINSON TRAINING COMPLEX WALKING TRAIL CITY OF VERO BEACH FLORIDA	RODOLFO VILLAMIZAR FL P.E. #1000	
	DATE: _____	DATE: _____	DATE: _____	
	DESIGNED: AC	19-0077	DATE: _____	DATE: _____
	DRAWN: AER & JT	DATE: OCT 2019	DATE: _____	DATE: _____
	CHECKED: RV	DATE ISSUED: _____	DATE: _____	DATE: _____
	SCALE: AS NOTED	DATE: _____	DATE: _____	DATE: _____
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