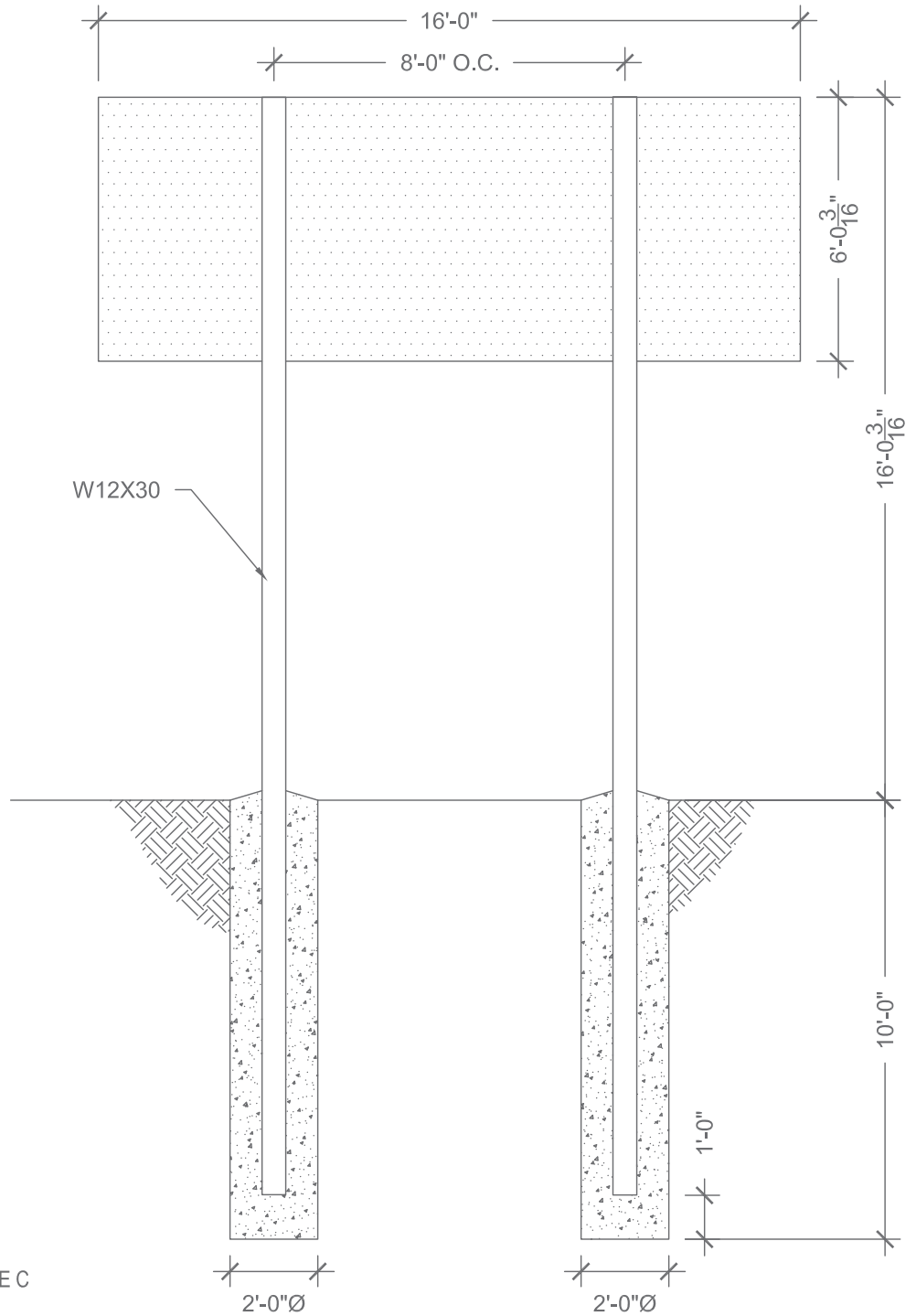




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PROJECT: MEL ROBERTS PARK - DOUBLE POLE SCOREBOARD - 901 NORTH CEDAR ST., FOLEY, AL
 PROJECT #: 44185
 CLIENT: NEVCO INC.

DATE: 2-5-2024
 ENGINEER: JD
 LAST REVISED:



2-5-24

GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY: 160 MPH EXPOSURE C
4. CONCRETE 2500 PSI MIN.
5. WIDE FLANGE STEEL ASTM A992, $F_y = 50$ KSI MIN.
6. PROVIDE PROTECTION AGAINST DISSIMILAR METALS USING ANTI-CORROSIVE PAINT OR NEOPRENE GASKETS.
7. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
8. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION.

1 FRONT ELEVATION



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v5.5

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

$$F = q_z * G * C_f * A_f \quad \text{with } q_z = 0.00256 K_z K_{zt} K_d V^2 \quad (29.3.2 \& 29.4)$$

$C_f = 1.684$ (Fig. 29.3-1) 2 pole C_f factor = 0.96 8 max. height = 16.02
 $K_{zt} = 1.0$ (26.8.2) (=1.0 unless unusual landscape) $s = 6.02$
 $K_z =$ from table 28.3-1 Exposure = c
 $K_d = 0.85$ for signs (table 26.6-1)
 $V = 160$ mph
 $G = 0.85$ (26.9) weight = 1.163 kips
 $s/h = 0.376$ $M_{DL} = 0.00$ k-ft
 $B/s = 2.66$

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure $q_z * G * C_f$	A_f	shear	Wind Moment M_w				
	1	5.0	0.850	47.3	67.79	20.0	1356	6779				
	2	12.5	0.850	47.3	67.79	80.0	5423	67787				
	3	15.5078125	0.856	47.7	68.27	16.3	1109	17203				
							sums:	116.3	7888	91.77	(M_w) k-ft	arm = 11.6
two pole distribution factor *b*s (asce fig. 29.4-1):					x 0.78		6162	71.68				
							$P_u = 1.40$	kip	$M = 71.68$	k-ft	$M = \sqrt{M_{DL}^2 + M_w^2}$	
							$M_u = \sqrt{1.2M_{DL}^2 + 1.0M_w^2} = 71.68$	k-ft				

Pole Design section; W flange

$M_u \leq \phi M_n$ with $M_n = f_y Z$	$f_y = 50$ ksi	$\phi = 0.9$				
H	M_u (k-ft)	Z req'd. (in)	Size (in)	t (in)	Z	USE
at grade	71.7	19.12				W12X30, $\phi M_n = 97.16$ k-ft

Unbraced length = 16'-3/16", no lateral bracing provided

Footing Design footprint: round

$\omega = 1.3$	IBC 1605.3.2	IBC Table 1806.2, sections 1806.3.4, 1807.3.2	$S = (1.3x2x)$
$P = 4.81$	kip	$S_1 = S \times d / 3$	$A = 2.34 \times P / (S_1 \times b)$ $S = 400$
$S_1 = 1310$		$d = 0.5 \times A (1 + (1 + 4.36 \times h/A)^{.5})$	IBC 1807.3.2.1
$A = 4.29$			

footing: 2' - 0" dia.

9' - 10" deep
 USE 10' DEEP

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units; pounds, feet unless noted otherwise

Check W12X30 for flexure(AISC 14 F3)

Yielding:	$M_u = (\text{see page \#2}) = M_u =$	860.2 k-in		$F_y =$	50 ksi
				$Z_x =$	43.1 in ³
	$M_n = M_p = F_y Z =$	2155 k-in	(eq'n. F2-1)	$L_b =$	192.2 in
LTB:				$L_p =$	64.43 in
	$M_n = F_{cr} S_x =$	1296 k-in	(eq'n. F2-3)	$L_r =$	187.1 in
	$F_{cr} = C_b \pi^2 E / (L_b / r_{ts})^2 (1 + 0.078 J_c / S_x h_o (L_b / r_{ts})^2)^{1/2} =$	33.56 ksi	(eq'n. F2-4)	$E =$	29000 ksi
FTB:				$r_y =$	1.52 in
	$M_n = M_p - (M_p - 0.7 F_y S_x) (l - l_{pf}) / (l_r - l_{pf}) =$	2249 k-in	(eq'n. F3-1)	$S_x =$	38.6 in ³
				$J =$	0.457 in ⁴
	$\phi M_n =$	1166 k-in	OK	$h_o =$	11.9 in
				$c =$	1
	$(M_u / M_c) =$	0.738	(eq'n. H1-1)	$r_{ts} =$	1.769
				$\lambda_{pf} =$	9.152
				$\lambda_{rf} =$	24.08
				$\lambda =$	7.41
				$\phi =$	0.9
				$k_c = 4 / (h / t_w)^{1/2} =$	0.62
				$C_b =$	1.00