LOWER EAST FORK LATERALS OF THE TRINITY WATERSHED FLOODWATER RETARDING STRUCTURE NO. 3 UPGRADE KAUFMAN COUNTY, TEXAS

SPONSORED BY KAUFMAN-VAN ZANDT SOIL AND WATER CONSERVATION DISTRICT #505 KAUFMAN COUNTY

SEE SHEET G-1 FOR INDEX OF DRAWINGS



DRAINAGE AREA TOTAL STORAGE EFFECTIVE HEIGHT OF DAM 1342 ACRES 1824 AC. FT. 25.5 FEET

COOPERATING WITH

NATURAL RESOURCES CONSERVATION SERVICE OF THE U.S. DEPARTMENT OF AGRICULTURE AND TEXAS STATE SOIL AND WATER CONSERVATION BOARD

MARCH 2021 100% DELIVERABLE





SYMBOLS LEGEND



-INDEX.dwg 3:34:33 PM ıwings\LEF3-GN-larch 25, 2021 δŽ

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ABBREVIATIONS

IG TREES
IG TREE TO BE REMOVED
LINE
_OPE
LOPE
ATION WHEN LOOKING DOWNSTREAM
SECTION NUMBER

SHEET ON WHICH SECTION IS SHOWN

- DETAIL NUMBER

SHEET ON WHICH DETAIL IS TAKEN

FT	FEET
.DD'L	ADDITIONAL
PPROX.	APPROXIMATLEY
L,B	BASELINE
MP	BEST MANAGEMENT PRACTICE
М	BENCH MARK
:/C	CENTER TO CENTER
FS	CUBIC FEET PER SECOND
L,Q	CENTERLINE
.J.	CONSTRUCTION JOINT
T. J.	CONTRACTION JOINT
CLR	CLEAR
VC	CONVENTIONAL CONCRETE
EG.	DEGREES
	DIAMETER
/S	DOWNSTREAM
WLS	DOWELS
A	EACH
/F	EACH FACE
/W	EACH WAY
	EAST, EASTING
LEV., EL.	ELEVATION
Q	EQUALS
Q SPA	EQUALLY SPACED
XP JT	EXPANSION JOINT
XIST.	EXISTING
L	FLOWLINE
E-RCC	GROUI-ENRICHED RCC
,IN.	INCHES
D. -	
11N	MINIMUM
101	NOTICE OF INTENT
l	NORTH, NORTHING
I.T.S	NOT TO SCALE
).C.	ON CENTER
)F	OUTSIDE FACE
RM	PERMANENT REFERENCE MARKER
SI	POUNDS PER SQUARE INCH
	RADIUS
CC	ROLLER COMPACTED CONCRETE
EINF.	REINFORCED, REINFORCING
СH	SCHEDULE
F	SILT FENCE
HT.	SHEET
IM.	SIMILAR TO
TD	STANDARD
TA	STATION
.S.	STAINLESS STEEL
WPPP	STORM WATER POLLUTION PREVENTION PLAN
.D.	STUB DIVERSION
&В	TOP AND BOTTOM
YP.	TYPICAL
I.N.O	UNLESS NOTED OTHERWISE
/S	UPSTREAM

WATER POLLUTION ABATEMENT PLAN

YEAR

WPAP

YR

Sł Ni	neet Jmbei
GE	
	G-2
	G-3
CI	VIL
	C-1
	C-2
	C-3
	C-4
	C-5
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	S_11 S_12
	S=12 S=17
	S-13

SHEET INDEX Sheet Title COVER INDEX, LEGEND, & ABBREVIATIONS	Freese and Nichols, Inc. kds Registered Engineering Firm F-2144 SAPATE OF 7544 EMILY ANN SESTAK EMILY ANN SESTAK EMILY ANN SESTAK
GENERAL NOTES GENERAL PLAN	EAS NBB DGM 2021
EXISTING SITE PLAN PROPOSED SITE PLAN EMBANKMENT – PLAN AND PROFILE EMBANKMENT – PLAN AND PROFILE EMBANKMENT – TYPICAL SECTIONS EMBANKMENT – SECTIONS EMBANKMENT – SECTIONS PRINCIPAL SPILLWAY – EXISTING PLAN AND PROFILE EXISTING PRINCIPAL SPILLWAY–PROPOSED FILTER DIAPHRAGM	DESIGNED BY: DESIGNED BY: DRAWN BY: DRAWN BY: DRAWN BY: DRAWN BY: DRAWN BY: DRAWN BY: DRECKED BY: DATE CHECKED: DATE CHECKED: DA
PROPOSED PRINCIPAL SPILLWAY - PLAN AND PROFILE PROPOSED PRINCIPAL SPILLWAY SECTIONS AND DETAILS PROPOSED FOUNDATION DRAIN PLAN AND PROFILE DRAINS-SECTIONS AND DETAILS AUXILIARY SPILLWAY - PLAN AND PROFILE AUXILIARY SPILLWAY - TYPICAL SECTIONS AUXILIARY SPILLWAY - SECTIONS STORMWATER POLLUTION PREVENTION PLAN SWPPP DETAILS FIELD FENCE DETAILS GEOLOGICAL INVESTIGATION - BORING PLAN GEOLOGICAL INVESTIGATIONS PROFILES GEOLOGICAL INVESTIGATIONS PROFILES GEOLOGICAL INVESTIGATION PROFILES GEOLOGICAL INVESTIGATION PROFILES GEOLOGICAL INVESTIGATION - BORROW PLAN XL STRUCTURAL GENERAL NOTES RISER - PLAN, ELEVATION, AND SECTIONS RISER - REINFORCEMENT DETAILS 1 RISER - REINFORCEMENT DETAILS 2 RISER - REINFORCEMENT DETAILS 3 RISER - SLIDE GATE DETAILS IMPACT BASIN - PLAN, ELEVATION, AND SECTIONS IMPACT BASIN - REINFORCEMENT DETAILS 1 IMPACT BASIN - REINFORCEMENT DETAILS 2 IMPACT BASIN - REINFORCEMENT DETAILS 2	INDEX, LEGEND, & ABBREVIATIONS FLOODWATER RETARDING STRUCTURE SITE No. 3 LOWER EAST FORK LATERALS IN TRINITY RIVER WATERSHED, TEXAS
	BULT CHERY Street, Suite 2800 Fort Worth Texas 76102 Phone - (817) 735-7330 Web - www.freese.com
REVISIO	SHEET NO.

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SEQUENCE NO.					
1 OF43					

GENERAL NOTES

- 1. PLANS AND SPECIFICATIONS WILL NOT BE SUBSTANTIALLY CHANGED WITHOUT EITHER WRITTEN AUTHORIZATION OF THE TCEQ EXECUTIVE DIRECTOR BEFORE THE WORK IS STARTED, OR NOTIFICATION OF THE CHANGES AS DEFINED IN 30 TAC 299.26, "CONSTRUCTION ORDERS".
- 2. THE GENERAL NOTES AND TYPICAL DETAILS ARE GENERAL AND APPLY TO THE ENTIRE PROJECT EXCEPT WHERE THERE ARE SPECIFIC INDICATIONS TO THE CONTRARY.
- CONTOURS WERE OBTAINED FROM SURVEY PERFORMED IN JANUARY OF 2020. CONTRACTOR SHALL MAKE SITE SURVEYS AS NECESSARY FOR CONSTRUCTION AND IN ACCORDANCE WITH CONSTRUCTION SPECIFICATION 7, CONSTRUCTION SURVEYS. HORIZONTAL DATUM IS TEXAS STATE PLANE, NAD83, NORTH CENTRAL ZONE, 4204, US SURVEY FEET WITH A LOCALIZED SCALE FACTOR OF 1.00016. VERTICAL DATUM IS NAVD88
- 4. CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFYING ALL EXISTING CONDITIONS, INCLUDING LOCATION AND DIMENSIONS OF ALL EXISTING UTILITIES. CONTRACTOR SHALL NOTIFY CONTRACTING OFFICER'S REPRESENTATIVE IF THERE IS A CONFLICT BETWEEN THE CONTRACT DOCUMENTS AND EXISTING CONDITIONS BEFORE PROCEEDING WITH WORK.
- 5. THE CONTRACTOR SHALL BE LIABLE FOR DAMAGE TO IMPROVEMENTS AND UTILITIES ALONG THE ACCESS ROUTE AND AT OR NEAR THE WORKSITE. UTILITIES MAY EXIST AND NOT BE SHOWN ON THE CONSTRUCTION DRAWINGS. THE SITE SHALL BE CAREFULLY SCRUTINIZED FOR EVIDENCE OF UTILITIES. AT A MINIMUM. PRIOR TO ANY GROUND DISTURBANCE, THE TELEPHONE NUMBER 811 SHALL BE CALLED TO ASCERTAIN IF UNDERGROUND UTILITIES EXIST IN THE GENERAL WORK AREA. CALLING THIS TELEPHONE NUMBER WILL ONLY ASCERTAIN THE EXISTENCE OF UNDERGROUND UTILITIES OWNED BY COMPANIES THAT SUBSCRIBE TO THIS ORGANIZATION. THERE MAY BE OTHER UNDERGROUND UTILITIES IN THE WORK AREA.
- 6. THE CONTRACTOR SHALL NOTIFY THE CONTRACTING OFFICER'S REPRESENTATIVE OF ALL UTILITIES A MINIMUM OF TEN (10) DAYS IN ADVANCE OF INTENT TO PERFORM WORK IN THE VICINITY OF THE AFFECTED UTILITY. THE NOTICE SHALL BE IN WRITING AND A COPY SHALL BE FURNISHED TO THE CONTRACTING OFFICER.
- 7. COMPLY AND CONDUCT WORK IN ACCORDANCE WITH OWNER'S SECURITY REGULATIONS AND REQUIREMENTS. PROVIDE SITE SECURITY AS NECESSARY TO PROTECT AGAINST VANDALISM AND LOSS BY THEFT.
- 8. CONTRACTOR SHALL MANAGE AND PROTECT THE WORK FROM FLOOD FLOWS, STREAM FLOWS, SURFACE WATER RUNOFF, GROUNDWATER OR ANY OTHER WATER ENCOUNTERED DURING THE PROGRESS OF THE WORK IN ACCORDANCE WITH CONSTRUCTION SPECIFICATION II. REMOVAL OF WATER.
- 9. THE APPROXIMATE LOCATIONS OF THE ACCESS ROAD(S), CONSTRUCTION CAMPSITE, SOIL STOCKPILE, BORROW AND WASTE AREAS ARE SHOWN. THE FINAL LOCATIONS OF THESE AREAS SHALL BE STAKED BY THE CONTRACTOR AND APPROVED BY THE CONTRACTING OFFICER'S REPRESENTATIVE PRIOR TO BEGINNING WORK.
- 10. THE AREA AROUND OVERHEAD ELECTRICAL TOWERS SHALL BE PROTECTED. PROTECTION SHALL BE PROVIDED TO ANY TOWER, POLE OR GUY STRUCTURE WHEN TRAFFIC OR CONSTRUCTION ACTIVITY IS WITHIN 50 FEET OF THE STRUCTURE.
- 11. WORK UNDER THIS CONTRACT IS AUTHORIZED UNDER THE TERMS AND CONDITIONS OF THE U.S. ARMY CORPS OF ENGINEERS NATIONWIDE PERMIT 3, MAINTENANCE. SEE THE SWPPP FOR DETAILS.
- 12. THE CONTRACTING OFFICER HAS PREPARED THE MINIMUM REQUIREMENTS FOR THE SWPPP AS SHOWN ON SHEET C-18. THE CONSTRUCTOR SHALL REVIEW THE MINIMUM REQUIREMENTS OF THE SWPPP AND SHALL PREPARE A PLAN WITH A DETAILED WORK SEQUENCE OUTLINE THAT DEFINES AND DELINEATES THE PROPOSED CONSTRUCTION OPERATION. REFER TO CONSTRUCTION SPECIFICATIONS 5.
- 13. CONSTRUCT THE STAGING AREAS AND VEHICLE MAINTENANCE AREAS IN A MANNER TO MINIMIZE THE RUNOFF OF POLLUTANTS.
- 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING, INSTALLING, AND MAINTAINING ALL BARRICADES, WARNING SIGNS, TRAFFIC CONTROL DEVICES, ETC. NECESSARY TO CONTROL TRAFFIC AND PROVIDE FOR PUBLIC SAFETY AT THE ENTRANCE TO THE SITE (REFER TO CONSTRUCTION SPECIFICATION 9).
- 15. CONSTRUCTION ACTIVITIES SHALL NOT OCCUR OUTSIDE THE DESIGNATED WORK LIMITS, UNLESS OTHERWISE AUTHORIZED.
- 16. PROVIDE PROTECTED STORAGE FOR PAINTS, CHEMICALS, SOLVENTS, AND OTHER POTENTIALLY HAZARDOUS MATERIALS.
- 17. HANDLING, STORAGE, AND DISPOSAL OF ALL WASTE MATERIAL SHALL CONFORM TO THE SWPPP.
- 18. PREVENT POLLUTION OF SURFACE WATER AND GROUNDWATER WITH PETROLEUM PRODUCTS OR OTHER HAZARDOUS OR REGULATED SUBSTANCES. TAKE SPECIAL MEASURES TO PREVENT CHEMICALS, FUELS, OILS, GREASES, HERBICIDES, AND INSECTICIDES FROM ENTERING DRAINAGE WAYS. DO NOT ALLOW WATER USED IN ON-SITE MATERIAL PROCESSING AND CLEANUP, AND OTHER WASTEWATERS TO ENTER A DRAINAGE WAY, STREAM, OR RIVER.
- 19. PROMPTLY REPAIR EQUIPMENT LEAKING OIL/HYDRAULIC FLUID/ETC. IMMEDIATELY REMOVE AND REPLACE, AS NECESSARY, ALL SOILS ON WHICH SUCH LEAKAGE OCCURRED. PREVENT THE SPREAD OF LEAKED FLUIDS OR FLUID CONTAMINATED MATERIALS FROM THE ORIGINAL LEAK AREA. BE RESPONSIBLE FOR THE PROPER HANDLING AND DISPOSAL OF ALL SUCH CONTAMINATED MATERIALS.
- 20. PROVIDE SECONDARY CONTAINMENT AROUND ANY FUEL AND CHEMICAL STORAGE AREAS TO ENSURE THAT SPILLS FROM ANY SUCH AREAS DO NOT DISCHARGE FROM THE SECONDARY CONTAINMENT AREA. THE SECONDARY CONTAINMENT CAPACITY SHALL BE ADEQUATE TO CONTAIN THE CAPACITY OF THE LARGEST TANK/CONTAINER PLUS SUFFICIENT FREEBOARD TO CONTAIN PRECIPITATION.
- 21. PRECAUTIONS SHALL BE TAKEN DURING EQUIPMENT FUELING AND CHEMICAL TRANSFER OPERATIONS IN ORDER TO PREVENT SPILLS FROM OCCURRING AND TO MINIMIZE THE IMPACT OF ANY SPILL THAT DOES OCCUR. ALL FUEL AND CHEMICAL TRANSFERS SHALL BE CONTINUOUSLY MONITORED. MAINTAIN APPROPRIATE EQUIPMENT ON-SITE FOR RESPONDING TO ANY OIL OR HAZARDOUS SUBSTANCE SPILL. ADDITIONALLY THERE SHALL BE AN ON-SITE PROHIBITION AGAINST THE TOPPING OFF OF TANKS AND EQUIPMENT.
- 22. REMOVE ALL FORM WORK FOLLOWING CONSTRUCTION.
- 23. NO CONSTRUCTION FILL OR MATERIALS SHALL BE PLACED OR STORED IN AREAS NOT SPECIFICALLY DESIGNATED FOR THAT PURPOSE.
- 24. EXISTING ROADS, ACCESS DRIVES, UTILITIES AND PROPERTY WITHIN THE LIMITS OF CONSTRUCTION DAMAGED BY CONTRACTOR AND ALL DISTURBED AREAS SHALL BE REPAIRED BY CONTRACTOR TO SAME OR BETTER CONDITION PRIOR TO END OF CONSTRUCTION AT THE CONTRACTOR'S EXPENSE. UNLESS PROVIDED FOR IN THE CONSTRUCTION SPECIFICATIONS.

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CALL BEFO	RE YOU DIG!					
Know what's below. Call before you dis.	participants request 48 hours notice before you dig, drill, or blast – stop and call 811					
THE LONE STAR NOTIFICATION COMPANY AT 1–800–669–8344						

EARTHFILL MATERIAL REQUIREMENTS

	MATERIAL PLACEMENT DATA												
ZONE	MATERIAL CLASSIFICATION	MATERIAL SOURCE	PLACEMENT LOCATION	LIQUID LIMIT	PLASTICITY INDEX	PERCENT PASSING NO 200	MAXIMUM PARTICLE SIZE	MAXIMUM LOOSE LIFT	MOIS DENSI	STURE IY TEST	MINIMUM PERCENT COMPACTION	MOIS LIM	TURE IITS
1	СН	ONSITE BORROW	EMBANKMENT	≤105	≤75	≥60	2"	8"	ASTM D698	А	95%	-1%	+4 %
2	CL	IMPORTED FILL	BACKFILL MATERIAL	<50	≤ 35	> 50	2"	8"	ASTM D698	А	95%	-1%	+3 %
3	CRUSHED STONE (FLEXIBLE BASE)	IMPORTED FILL	VARIES	SEE NOTE 5			ASTM D1557	С	100%	-3%	+1%		
4	FREE-DRAINING FILL	IMPORTED FILL	BELOW	N/A	N/A	<5	2"		2	SEE NOTE	8		

MATERIAL DATA TABLE NOTES:

1. MATERIALS FOR ALL TYPES OF EARTHFILLS AND BACKFILLS SHALL BE OBTAINED FROM REQUIRED SITE EXCAVATIONS. ADDITIONAL MATERIAL, IF REQUIRED. WILL COME FROM THE APPROVED BORROW AREAS DESIGNATED ON THE DRAWINGS. IMPORTED SOILS SHALL COME FROM AN APPROVED SOURCE. APPROVAL SHALL BE OBTAINED PRIOR TO DELIVERY OF MATERIALS TO THE PROJECT SITE.

- 2. THE SOIL MATERIALS EXCAVATED FROM THE EMBANKMENT (FROM ANY GIVEN LOCATION) SHALL BE REPLACED WITH LIKE MATERIALS, UNLESS OTHERWISE INDICATED.
- 3. CONDUCT SUCH FIELD TESTS DEEMED NECESSARY AS CONSTRUCTION PROGRESSES ON MATERIALS BEING EXCAVATED TO IDENTIFY DISPERSIVE SOILS. MATERIALS THAT TEST TO BE HIGHLY DISPERSIVE SHALL NOT BE USED TO CONSTRUCT ANY EMBANKMENT ZONE. (SEE CONSTRUCTION SPECIFICATION 23.)
- 4. ZONE 3 CRUSHED STONE SHALL CONFORM TO TXDOT 2014 STANDARD SPECIFICATION ITEM 247 FLEXIBLE BASE, TYPE A/D, GRADE 1/2. TXDOT TEX-113E MAY BE USED AS AN ALTERNATIVE METHOD TO ASTM D1557 FOR DETERMINING THE MOISTURE DENSITY CHARACTERISTICS OF CRUSHED STONE (FLEXIBLE BASE).
- 5. COMPACTION REQUIREMENTS FOR ZONES 3 VARIES BASED ON PLACEMENT LOCATION.
- 6. ZONE 4 FREE-DRAINING FILL SHALL BE ASTM C-33, NO. 4, NO. 467, NO. 5, NO. 56 MATERIALS OR APPROVED EQUIVALENT.
- 7. FREE-DRAINING FILL (ZONE 4) SHALL BE COMPACTED USING ROLLING OR PLATE VIBRATORY COMPACTORS. COMPACTION EFFORT SHALL BE BASED ON A TEST PASS THAT ESTABLISHES THE NUMBER OF PASSES REQUIRED TO ACHIEVE COMPACTION OF THE GRAVEL, AFTER REVIEWED AND APPROVED BY CONTRACTING OFFICER'S REPRESENTATIVE. MAXIMUM LIFT THICKNESS IS 12 INCHES, BUT SHALL BE REDUCED DEPENDING ON THE RESULTS OF THE TEST PASS. MAXIMUM LIFT THICKNESS WHEN USING WALK-BEHIND VIBRATORY COMPACTORS IS 4 INCHES.

DRAINFILL/FILTER GRADATIONS

THE DRAINFILL/FILTER MATERIALS FOR THE FOUNDATION TRENCH DRAIN SHALL MEET THE FOLLOWING CRITERIA UNLESS OTHERWISE SPECIFIED IN CONSTRUCTION SPECIFICATION 24:

> COARSE – ASTM C33 #89 COARSE AGGREGATE FINE – ASTM C33 FINE AGGREGATE WITH A MODIFIED

GRADATION MEETING THE FOLLOWING:

SIEVE SIZE	PERCENT PASSING
3/8"	100
NO. 4	100
NO. 8	75 TO 100
NO. 16	65 TO 90
NO. 30	50 TO 75
NO. 50	30 TO 60
NO. 100	5 TO 40
NO. 200	0 TO 5

ROCK RIPRAP GRADATIONS

ROCK RIPRAP FOR THE ROCK LINED OUTLET SHALL MEET THE GRADATION REQUIREMENTS OF ASTM D6092 FOR R-60 ROCK RIPRAP. IT IS ESTIMATED THAT 510 TONS OF R-60 ROCK RIPRAP WILL BE REQUIRED TO COMPLETE THE ROCK LINED OUTLET.

ROCK RIPRAP FOR THE ROCK LINED WAVE BERM SHALL MEET THE GRADATION REQUIREMENTS OF ASTM D6092 FOR R-60 ROCK RIPRAP. IT IS ESTIMATED THAT 810 TONS OF R-60 ROCK RIPRAP WILL BE REQUIRED TO COMPLETE THE WAVE BERM.

ALL ROCK RIPRAP SHALL COMPLY WITH CONSTRUCTION SPECIFICATION 61, AND MATERIAL SPECIFICATION 523.

SPALLS AND ROCK DUST THAT PASS A 3" SIEVE SHALL CONSIST OF LESS THAN 5 PERCENT BY WEIGHT.

ASTM R-60						
	GRADATIO	N				
MASS	PERCENT	PASSING				
LBS	MIN MAX					
150	100 -					
60 50 100						
30	15 50					
10	0	15				

GEOTECHNICAL BORING NOTES:

PARTIAL GEOTECHNICAL BORING LOGS AND EXISTING SOIL DESCRIPTIONS ARE INCLUDED IN THESE DRAWINGS FOR INFORMATION ONLY. ANY REPRESENTATION OF THE SOILS AND FOUNDATION CONDITIONS SHOWN IN THESE DRAWINGS SHOULD BE CONSIDERED PARTIAL, INCOMPLETE, AND NOT PART OF THE CONTRACT DOCUMENTS. ADDITIONAL GEOTECHNICAL MAY BE FOUND IN THE GEOLOGIC INVESTIGATION REPORT AND SOIL MECHANIC REPORT. THIS SEPARATE DOCUMENT IS ALSO NOT PART OF THE CONTRACT DOCUMENTS. THE GEOTECHNICAL DATA REPORT REPRESENTS DATA THAT WAS CONSIDERED SUFFICIENT FOR DESIGN, BUT MAY NOT BE SUFFICIENT FOR CONSTRUCTION. THE CONTRACTOR IS FULLY RESPONSIBLE FOR DETERMINING SUBSURFACE CONDITIONS TO A DEGREE NECESSARY TO SUBMIT THE ASSOCIATED BID ITEMS AND MAY USE THE PROVIDED INFORMATION AT THEIR OWN RISK. REFER TO THE GEOLOGIC INVESTIGATION REPORT FOR DETERMINATIONS OF SOIL AND ROCK PROFILE ELEVATIONS AND BORING LOGS. THE REPORT IS AND ITS FINDING ARE A REPRESENTATION OF THE SUBSURFACE BASED ON CURRENT PRACTICE FOR GEOTECHNICAL INVESTIGATION AND TESTING. ACTUAL FIELD CONDITIONS MAY VARY.

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			801 Cherry Street, Suite 2800 Fort Worth Texas 76102	Phone – (817) 735–7330 Web – www.freese.com
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200' SCALE IN FEET

400'

POST CONSTRUCTION DAM INFORMATION

	SURFACE AREA		RAGE
ELEVATION	(ACRES)	(ACRE-FT)	(INCHES)
398.0	11.4	171.9	1.5
400.0	19.6	244.3	2.2
402.0	28.3	291.1	2.6
404.0	38.3	357.4	3.2
406.0	51.8	446.0	4.0
408.0	68.4	567.2	5.1
410.0	82.7	718.4	6.4
411.5	94.2	851.9	7.6
412.0	98.3	898.9	8.0
414.0	114.1	1111.4	9.9
416.0	136.9	1361.3	12.2
418.0	159.7	1658.0	14.8
419.0	173.6	1824.3	16.3
420.0	188.2	2005.0	17.9
422.0	220.6	2413.5	21.6
TOP OF DAM (EFF	ECTIVE), EL.		419.0
EARTHEN AUXILIA	ARY SPILLWAY CRES	ST, EL.	411.5
PRINCIPAL SPILLW	398.0		
SEDIMENT POOL,	398.0		
DRAINAGE AREA,	1342		
SEDIMENT STORA	171.9		
MAX EARTHEN AU	6301.3		



PLAN

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	419.0	
	420.0	
	422.0	
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1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	EARTHEN AUXILIA	RY
	PRINCIPAL SPILLW	/A)
	SEDIMENT POOL	FI
121820	DRAINAGE AREA	
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PROFILE VIEW

		AV 100' FEET NTAL 20' FEET CAL	DESIGNED BY: EAS DESIGNED BY: EA DRAWN BY: NBB DRAWN BY: NBB CHECKED BY: BEK & DGM CHECKED BY: BEK & DGM FILE NAME: LEF3-PP-EMB2.dwg DATE CHECKED: 3/9/2021
			EMBANKMENT – PLAN AND PROFILE FLoodwater retarding structure site no. 3 LOWER EAST FORK LATERALS IN TRINITY RIVER WATERSHED, TEXAS
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AUXILIARY SPILLWAY CENTERLINE

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	EMBANKMENT – PLAN AND PROFILE FLOODWATER RETARDING STRUCTURE SITE No. 3 LOWER EAST FORK LATERALS IN TRINITY RIVER WATERSHED, TEXAS
REVISIONS DATE APPROVED TITLE	DRAWING NO. SHEET NO. C-4 SEQUENCE NO. 7 OF 4 3

UPSTREAM EXISTING AUXILIARY SPILLWAY CREST EL. 410.0 PRINCIPAL SPILLWAY CREST EL. 398.0 _____ UPSTREAM ELEVATION VARIES REFER TO PROFILE -RE: C–3 & C–4 AUXILIARY SPILLWAY CREST -EL. 411.5 _____ _ 3.5', | 1 TOP OF ROCK PRINCIPAL SPILLWAY CREST -EL. 398.0 EL. 400.0 _____ 15" THICK ROCK RIPRAP. RE: G-2 BOTTOM OF ROCK AT UPSTREAM TOE OR EL. 396.0, -WHICHEVER IS HIGHER GEOTEXTILE FILTER – FABRIC UNDER ALL ROCK RIPRAP

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DOWNSTREAM	Freese and Nichols, Inc. Texas Registered Engineering Firm F-2144 EMLY ANN SESTAK EMLY ANN SESTAK EMLY ANN SESTAK ANN SESTAK CENSE ANN ALLEN ANN SESTAK ANN ANN SESTAK ANN ANN SESTAK ANN ANN SESTAK ANN ANN ANN SESTAK ANN ANN ANN ANN ANN ANN ANN ANN ANN ANN
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	AL SECTIONS JRE SITE No. 3 NTERALS , TEXAS
DOWNSTREAM	EMBANKMENT – TYPIC FLOODWATER RETARDING STRUCTU LOWER EAST FORK LA IN TRINITY RIVER WATERSHED
NOTES: 1. STRIP AND REMOVE TOPSOIL AND REGRADE UPSTREAM EMBANKMENT AT 3:1 SLOPE. 2. FOR MATERIAL AND PLACEMENT DETAILS REFER TO SHEET G-2.	Rent Worth Texas 76102 Phone - (817) 735-7330 Web - www.freese.com
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460 DOWNSTREAM 450 440 430 420 410 400 390 -160 -160 -100 *—120 —140* -60 -80

NOTES:

1. STRIP AND REMOVE TOPSOIL AND REG **UPSTREAM EMBANKMENT AT 3:1 SLOPE**

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EMBANKMENT - SECTIONS		FLOODWATER RETARDING STRUCTURE SITE No. 3 I OWED EACT FODIX I ATEDALS	LOWEN EAST FORM LATENALS	TRINITY RIVER WATERSHED, TEXAS
			801 Cherry Street, Suite 2800 Fort Worth Texas 76102	Phone - (817) 735-7330 Web - www.freese.com
SHEE	T NO.	D.	6	

60' 0 10'20'30' SCALE IN FEET

-2144

2021









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0 10'20'30' 60' SCALE IN FEET

					450
	EMBANK	MENT	DOWNS	STREAM	750
					440
					430
רי רי					420
EL.	405.0_				410
	777				400
		<u>></u>			390
					380
	∕—FOU	NDATION	DRAIN		370
-60 -8	80 —1	00 -1	20 –1	40 -1	60

ROPOSED EMBANKMENT 43	0
42	0
EL. 405.0	0
40	0
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-60 -80 -100 -120 -140 -160	0

NOTES:

1. STRIP AND REMOVE TOPSOIL AND REGRADE UPSTREAM EMBANKMENT AT 3:1 SLOPE.

		SHEET NO.
RE	VISIONS	C-7
DATE APPROVED	TITLE	
		SEQUENCE NO.
		8 OF 43

Freese and Nichols, Inc. Texas Registered Engineering Firm F-2144	LE OF TEAL		EX 0 0 0 0 0 0 0 0 0 0 0 0 0 0	LANCH STORES	CC CENSE Sucrate CS ONAL EN 3112021
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		FLOODWATER RETARDING STRUCTURE SITE No. 3	LOWER EAST FORK LATERALS	2	TRINITY RIVER WATERSHED, TEXAS
			801 Cherry Street. Suite 2800	Fort Worth Texas 76102	Phone — (817) 735-7330 Web — www.freese.com
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EXISTING PRINCIPAL SPILLWAY PROFILE VIEW

SCALE IN FEET **HORIZONTAL**

SCALE IN FEET VERTICAL





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							10' SCALE IN FEET HORIZONTAL 10' SCALE IN FEET VERTICAL	AS Freese and Nichols, Inc. Freese and Nichols, Inc. Texos Registered Engineering Firm F-2144 EMILY ANN SESTAK EMILY ANN SESTAK CENSC CENSC ACTION 21 21 21 21 21 21 21 21 21 21
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CL 16'		CXISTING GROUND EL. 38 GEOTEXTILE FILT SEE DETAIL 5/C 20	39.00 GEOTEXTIL SEE DETA ER FABRIC 2–14 40	GRADE TO DRAIN E ANCHOR L 5/C-14	FILL EXIST CHANNEL	ING DISCHARGE WITH ZONE 1 EA	400 390 ARTHFILL 380	PROPOSED PRINCIPAL SPILLWAY – FLOODWATER RETARDING STRUCTURE SIT LOWER EAST FORK LATERAI IN TRINITY RIVER WATERSHED, TEXA
CL 16' —		-EXISTING GROUND) 89.0 ///// ///	GRADE TO DRAIN	FILL EXISTING CHANNEL WITH	DISCHARGE ZONE 1 EARTHF	400 390 	Rond Charly Street, Suite 2800 Fort Worth Texas 76102 Phone – (817) 735–7330 Web – www.freese.com
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VE # DELTA	(Δ) D	R L	P.C.	P.I. STA. 3+08.9	P.T. STA. 3+57.6	T		O N
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+00	-EXISTING GROUND -EL. 38 3 GEOTEXTILE FILTH SEE DETAIL 5/C 20	39.00 GEOTEXTILE SEE DETAIL ER FABRIC -14 40	GRADE TO DRAII	N FILL EXISTI CHANNEL V	ING DISCHARGE WITH ZONE 1 E 80	ZZZZ ARTHFILL	400 390 380	PROPOSED PRINCIPAL SPILLWAY – PLAN FLOODWATER RETARDING STRUCTURE SITE No. 3 LOWER EAST FORK LATERALS IN TRINITY RIVER WATERSHED, TEXAS
	-EXISTING GROUND GEOTEXTILE FILTI SEE DETAIL 5/C 20	39.00 GEOTEXTILE SEE DETAIL ER FABRIC 14 40	GRADE TO DRAII	N FILL EXISTI CHANNEL V	ING DISCHARGE WITH ZONE 1 E 80	ZZZZ EARTHFILL	400 390 380 370	FINCIPAL SPILLWAY PLAN OLS FLOODATER RETARDING STRUCTURE SITE No. 3 300 LOWER EAST FORK LATERALS 300 IN IN IN
	EXISTING GROUND	39.00 GEOTEXTILE SEE DETAIL ER FABRIC -14 40	GRADE TO DRAIN	N FILL EXISTI CHANNEL V	ING DISCHARGE WITH ZONE 1 E	EARTHFILL	400 390 380 370	FREESE PROPOSED PRINCIPAL SPILLWAY PLAN NICHOLS FLOODWATER RETARDING STRUCTURE SITE No. 3 3 , Suite 2800 TOWER EAST FORK LATERALS 10 , Suite 2800 JOUER EAST FORK LATERALS 10 , Suite 2800 JOUER EAST FORK LATERALS 10 , Source INITY RIVER WATERSHED, TEXAS 10
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). 4	16	801 Cherry Street, Suite 28(Fort Worth Texas 76102	10 LOWER EAST FORM LATERALS	FILE NAME: LEF3-ASP-XSEC01.dwg	EMILY ANN SESTAK
3		Phone — (817) 735–7330 Web — www.freese.com	TRINITY RIVER WATERSHED, TEXAS	DATE CHECKED: 3/9/2021	CENSE AND AN 121 JULY

REVISIONS

TITLE

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- THESE AREAS SHALL INCLUDE BUT ARE NOT LIMITED TO: EQUIPMENT STORAGE, FUEL STORAGE, CONCRETE
- AS REQUIRED.
- SURFACE MATERIAL DURING ANY PHASE OF CONSTRUCTION ACTIVITY.
- CONSTRUCTION ACTIVITY FOR MORE THAN 14 DAYS.
- SUFFICIENT TO CAUSE FAILURE TO THE STRUCTURE.
- SPLICED TOGETHER ONLY AT SUPPORT POST, WITH A MINIMUM OF 6-INCH LAP.
- GROUND (MINIMUM OF 12 INCHES).
- AT 4 FEET. IN THIS CASE, THE FABRIC IS STAPLED OR WIRED DIRECTLY TO THE POST.
- RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE IMMEDIATELY.
- INDICATE THAT THE REPLACEMENT IS UNNECESSARY.

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	DESIGNED BY: EAS EAS Freese and Nichols, Inc. DRAWN BY: NBB DRAWN BY: NBB CHECKED BY: BEK & DGM CHECKED BY: BEK & DGM FILE NAME: LEF3-PL-SSPP.dwg DATE CHECKED: 3/9/2021
STA. 13+84.73 PROPOSED DAM CENTERLINE = STA. 31-09.00 PROPOSED AUXILIARY SPILLWAY CENTERLINE = STA. 15+50 STA. 15+50 N: 6923183.39 E: 2604315.63	STORMWATER POLLUTION PREVENTION PLAN FLOODWATER RETARDING STRUCTURE SITE No. 3 LOWER EAST FORK LATERALS IN TRINITY RIVER WATERSHED, TEXAS
	DRAMING NO. Web – www.freese.com
REVISIONS DATE APPROVED TITLE	SHEET NO. C-18 SEQUENCE NO. 21 OF 43

NOTES:

- 1. STONE SIZE: 3"-5" OPEN GRADED ROCK.
- 2. LENGTH: AS EFFECTIVE BUT NOT LESS THAN 50' 3. THICKNESS: NOT LESS THAN 8"
- 4. WIDTH: NOT LESS THAN FULL WIDTH OF ALL POINTS OF INGRESS/EGRESS.
- 5. WASHING: WHEN NECESSARY, VEHICLE WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC ROADWAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE AND DRAINS INTO AN APPROVED TRAP OR SEDIMENT BASIN. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING ANY STORM DRAIN, DITCH OR WATERCOURSE USING APPROVED METHODS.
- 6. MAINTENANCE: THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRAKING OR FLOWING OF SEDIMENT ONTO PUBLIC ROADWAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND, AS WELL AS REPAIR AND CLEAN OUT OF ANY MEASURE DEVICES USED TO TRAP SEDIMENT. ALL SEDIMENT THAT IS SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC ROADWAY MUST BE REMOVED IMMEDIATELY.
- 7. DRAINAGE: ENTRANCE MUST BE PROPERLY GRADED OR INCORPORATE A DRAINAGE SWALE TO PREVENT RUNOFF FROM LEAVING THE CONSTRUCTION SITE.
- 8. PROVIDE GEOTEXTILE FABRIC BETWEEN NATURAL GRADE AND 3"-5" ROCK.

ROCK BERM NOTES:

- 1. USE ONLY OPEN GRADE ROCK 4-8 INCH DIAMETER FOR STREAM FLOW CONDITION; USE OPEN GRADED ROCK 3-5 INCHES DIAMETER FOR OTHER CONDITIONS.
- 2. THE ROCK BERM SHALL BE SECURED WITH A WOVEN WIRE SHEATHING HAVING MAXIMUM ONE INCH OPENING AND MINIMUM WIRE DIAMETER OF 20 GAUGE.
- 3. THE ROCK BERM SHALL BE INSPECTED WEEKLY OR AFTER EACH RAIN, AND THE STONE AND/OR FABRIC CORE-WOVEN WIRE SHEATHING SHALL BE REPLACED WHEN THE STRUCTURE CEASES TO FUNCTION AS INTENDED DUE TO SILT ACCUMULATION AMONG THE ROCKS, WASHOUT, CONSTRUCTION TRAFFIC DAMAGE, ETC.
- 4. WHEN SILT REACHES A DEPTH EQUAL TO ONE-THIRD THE HEIGHT OF THE BERM OR ONE FOOT, WHICHEVER IS LESS, THE SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED SITE AND IN A MANNER AS TO NOT CREATE A SILTATION PROBLEM.
- 5. DAILY INSPECTION SHALL BE MADE ON SEVERE SERVICE ROCK BERMS; SILT SHALL BE REMOVED WHEN ACCUMULATION REACHES ONE FOOT.
- 6. WHEN THE SITE IS COMPLETELY STABILIZED, THE BERM AND ACCUMULATED SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED MANNER.

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	HIGH-SERVICE	ROCK	BERM	DETA
ノ	NOT TO SCALE			

NOTES:

- 1. STEEL POSTS WHICH SUPPORT THE SILT FENCE SHALL BE INSTALLED ON A SLIGHT ANGLE TOWARD THE ANTICIPATED RUNOFF SOURCE. POST MUST BE EMBEDDED A MIN. OF ONE (1') FOOT.
- 2. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW. WHERE FENCE CANNOT BE TRENCHED IN (E.G. PAVEMENT) WEIGHT FABRIC FLAP WITH WASHED GRAVEL ON UPHILL SIDE TO PREVENT FLOW UNDER FENCE.
- 3. THE TRENCH MUST BE A MINIMUM OF 6 INCHES DEEP AND 6 INCHES WIDE TO ALLOW FOR THE SILT FENCE FABRIC TO BE LAID IN THE GROUND AND BACKFILLED WITH COMPACTED MATERIAL. 4. SILT FENCE SHOULD BE SECURELY FASTENED TO EACH STEEL SUPPORT POST OR TO WOVEN WIRE, WHICH IN TURN IS
- ATTACHED TO THE STEEL FENCE POST. 5. SILT FENCE SHALL BE REMOVED WHEN THE SITE IS COMPLETELY STABILIZED SO AS NOT TO BLOCK OR IMPEDE STORM
- FLOW OR DRAINAGE. 6. ACCUMULATED SILT SHALL BE REMOVED AFTER EACH STORM EVENT OR WHEN IT REACHES A DEPTH OF 6 INCHES. THE SILT SHALL BE DISPOSED OF IN AN APPROVED SITE AND IN SUCH A MANNER AS TO NOT CONTRIBUTE TO ADDITIONAL SILTATION.

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DATE APPROVED

Freese and Nichols, Inc. Texas Registered Engineering Firm F-2144		EMILY ANN SESTAK	No 101210 EV CENSES ENT Actak SS ONAL EN 3112021
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		801 Cherry Street, Suite 2800 Fort Worth Texas 76102	Phone – (817) 735–7330 Web – www.freese.com
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0 20' 40' 60' 120' SCALE IN FEET HORIZONTAL 0 10' 20' SCALE IN FEET VERTICAL	DESIGNED BY: EAS DESIGNED BY: EAS DRAWN BY: NBB DRAWN BY: NBB CHECKED BY: BEK & DGM FILE NAME: LEF3-PL-GEO.dwg DATE CHECKED: 3/9/2021
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BORROW PLAN

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<u>GEI</u>	NERAL NOTES	16.	NON-(REQUI
۱.	VERIFY ALL DIMENSIONS, ELEVATIONS, AND OPENING SIZES PRIOR TO STARTING WORK.	17	REINE
2.	REMOVE ALL ABANDONED FOUNDATIONS, UTILITIES, PIPELINES, ETC. THAT INTERFERE WITH NEW CONSTRUCTION.	17.	CONCF
3.	FIELD VERIFY ALL EXISTING CONDITIONS, INCLUDING LOCATION AND DIMENSIONS OF ALL EXISTING CONSTRUCTION AND UTILITIES. NOTIFY ENGINEER IF THERE IS A CONFLICT BETWEEN THE CONTRACT DOCUMENTS AND EXISTING CONDITIONS BEFORE PROCEEDING WITH WORK.	18.	IN CAS LIMITEI POSSIE
4.	STRUCTURES ARE DESIGNED FOR OPERATIONAL LOADS ON COMPLETED STRUCTURES ONLY. DURING CONSTRUCTION, STRUCTURES SHALL BE PROTECTED BY TEMPORARY BRACING AND SHORING AS REQUIRED FOR STABILITY DURING CONSTRUCTION. THE DESIGN OF BRACING AND SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.	19. 20.	HOOKS UNLES REINFO 1" BE
5.	SEE OTHER DISCIPLINE DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS, DEPRESSIONS, OFFSETS, SLEEVES, CURBS, PADS, INSERTS, ETC. NOT SHOWN ON STRUCTURAL DRAWINGS.		CONDU EMBED
6.	PLANS, SECTIONS, AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.	21.	THE C BRACIN
7.	THE GENERAL NOTES AND TYPICAL DETAILS ARE GENERAL AND APPLY TO THE ENTIRE PROJECT	<u>F0l</u>	JNDATIC
	EXCEPT WHERE THERE ARE SPECIFIC INDICATIONS TO THE CONTRARY.	1.	REF T
<u>C0</u>	NCRETE AND REINFORCEMENT	2.	ΜΙΝΙΜΙ
1.	CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITIONS OF ACI 301, AND ACI 350.		A. AC
2.	ALL DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS, UNLESS NOTED OTHERWISE, SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT."	LOA	<u>DS:</u>
3.	CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4500 PSI, UNLESS NOTED OTHERWISE.	1.	DESIGI LOCAL
4.	REINFORCING SHALL BE IN ACCORDANCE WITH ASTM A615, GRADE 60, DEFORMED.	2.	LIVE L
5.	CONCRETE CLEAR COVER OVER REINFORCING SHALL BE AS NOTED BELOW, UNLESS NOTED		A. RI
	OTHERWISE. A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"	3.	GROUI
	B. ALL OTHER: 2"C. SEE DRAWINGS FOR EXCEPTIONS	4.	LATER
6.	ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" INSIDE FORMS OR TOOLED TO 3/4" RADIUS ON SLABS, UNLESS NOTED OTHERWISE.		A. RI B. RI
7.	ADDITIONAL CONSTRUCTION JOINTS SHALL HAVE PRIOR APPROVAL OF THE ENGINEER.		C. SE
8.	PENETRATIONS OTHER THAN SHOWN SHALL NOT BE ALLOWED WITHOUT PRIOR APPROVAL FROM		i.
٦ ٦	ALL DEINFORCING SUALL DE CONTINUOUS CONTINUOUS DARS SUALL LAR IN ACCORDANCE WITH		ii.
м.	ALL REINFORGING SHALL BE CONTINUOUS, CONTINUOUS BARS SHALL LAP IN ACCORDANCE WITH		:::
0.	THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. ALL REBAR EMBEDMENT LENGTHS SHALL BE		

REINFORCEMENT PLACED SU THAT MORE THAN 12 OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE BARS (INCLUDING WALLS). "OTHER BARS" ARE ALL BARS OTHER THAN "TOP BARS".

REBAR LAP SPLICE LENGTHS (INCHES)							
BAD	4,500) PSI					
SIZE	TOP BARS	OTHER BARS					
#4	19	14					
#5	23	18					
#6	28	21					
#7	40	31					
#8	46	35					
# 9	56	44					
#10	70	54					

- 10. THE SYMBOL _____ INDICATES A LAP SPLICE, NOT A BEND IN THE BAR.
- 11. LAP SPLICES IN BEAMS AND WALLS SHALL BE STAGGERED, UNLESS NOTED OTHERWISE.
- 12. LAP SPLICES SHALL BE LOCATED WHERE INDICATED ON THE DRAWINGS OR AS APPROVED BY THE ENGINEER.
- 13. WHEN REINFORCING BARS OF DIFFERENT SIZES ARE TO BE SPLICED, THE LENGTH OF LAP SHALL BE GOVERNED BY THE SMALLER BAR.
- 14. IF BARS ARE SPACED CLOSER THAN 6" OR 6 BAR DIAMETERS, CENTER-TO-CENTER SPACING, LAP SPLICES SHALL BE STAGGERED TO PROVIDE 12" CLEAR BETWEEN ENDS OF ADJACENT SPLICES.
- 15. LAP SPLICES IN REINFORCING BARS AT VERTICAL CONSTRUCTION JOINTS MAY BE SHIFTED TO AGREE WITH THE SEQUENCE OF CONSTRUCTION, UNLESS NOTED OTHERWISE.

CONTACT LAP SPLICES SHALL NOT BE SPACED FARTHER APART THAN ONE-FIFTH THE RED LENGTH OF LAP SPLICE OR 6", WHICHEVER IS LESS.

ORCING BARS PARALLELING CONSTRUCTION JOINTS SHALL HAVE A MINIMUM OF 2" CLEAR RETE COVER FROM JOINT.

SES WHERE REINFORCING BARS CANNOT BE EXTENDED AS FAR AS REQUIRED DUE TO THE ED EXTENT OF THE ADJACENT CONCRETE STRUCTURE, THE BARS SHALL EXTEND AS FAR AS 2. INSTRUCTIONS BELOW ARE NOT INTENDED TO CONFLICT WITH APPLICABLE SAFETY OR OSHA BLE AND END IN STANDARD HOOKS.

S SHOWN ON DRAWINGS SHALL BE ASSUMED TO BE STANDARD HOOKS PER ACI 350, SS NOTED OTHERWISE.

ORCING BARS MAY BE ADJUSTED LATERALLY TO MAINTAIN A CLEAR DISTANCE OF AT LEAST TWEEN THE REINFORCING BARS AND KEYS. WATERSTOPS. ANCHOR RODS. FORM TIES. UITS, AND OTHER EMBEDDED MATERIAL. IN HEAVILY REINFORCED AREAS, RELOCATION OF DDED MATERIAL MUST BE CONSIDERED.

CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL FORMING, TEMPORARY NG, AND SHORING.

<u> 0N</u>

O SHEETS C-21 TO C-26 FOR GEOTECHNICAL INFORMATION AND LOG OF BORINGS.

UM ALLOWABLE NET BEARING PRESSURES USED FOR FOUNDATION DESIGN ARE AS FOLLOWS: 7

GGREGATE FILL AT INLET RISER: 3.000 PSF _EXIBLE BASE AT IMPACT BASIN: 3,500 PSF

N IS IN ACCORDANCE WITH NRCS NEM, USACE, AND 2018 INTERNATIONAL BUILDING CODE, AMENDMENTS, AND APPLICABLE CODE REFERENCE STANDARDS.

_OADS:

SER TOP SLAB = 100 PSF.

ND SNOW LOADS: Pg = 5 PSF

RAL LOADS:

ISK CATEGORY III

SER WIND LOAD = 50 PSF

EISMIC LOAD:

SEISMIC IMPORTANCE FACTOR: I = 1.25

MAPPED SPECTRAL ACCELERATIONS: SS = 0.094, S1 = 0.055

SITE CLASS: C

SPECTRAL RESPONSE COEFFICIENT: SDS = 0.082, SD1 = 0.055

v. SEISMIC DESIGN CATEGORY: A

vi. INTAKE RISER BASIC SEISMIC FORCE-RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALL.

vii. DESIGN CATEGORY: A

viii. RESPONSE MODIFICATION FACTOR:

A. INTAKE RISER: R=4B. MPACT BASIN: R=2

ix. ANALYSIS PROCEDURE:

A. INTAKE RISER: MODAL ANALYSIS

B. IMPACT BASIN: EQUIVALENT LATERAL FORCE PROCEDURE

POST-INSTALLED ANCHORS (EXPANSION OR ADHESIVE):

- NOT LESS THAN INDICATED BELOW.

3. ADHESIVE ANCHORS SHALL ONLY BE INSTALLED BY CONSTRUCTION PERSONNEL CERTIFIED UNDER ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM OR APPROVED EQUAL. SUBMIT CERTIFICATIONS AS RECORD DATA.

4. ANCHOR DIAMETER AND EMBEDMENT SHALL BE AS INDICATED.

5. HOLES SHALL BE DRILLED USING ROTARY HAMMER DRILL WITH ANSI MATCHED TOLERANCE CARBIDE-TIPPED DRILLED BITS. DRILL BIT DIAMETER SHALL MATCH DIAMETER RECOMMENDED BY MANUFACTURER, DRILL HOLES USING HILTI SAFESET TECHNOLOGY OR APPROVED EQUAL.

6. USE CARE AND CAUTION WHEN INSTALLING TO AVOID CUTTING OR DAMAGING EXISTING REINFORCING STEEL. FIELD VERIFY EXISTING REINFORCING LOCATIONS PRIOR TO FABRICATION OR CONSTRUCTION, AND THEN COORDINATE REBAR LOCATIONS WITH SHOP DRAWINGS.

ADHESIVE ANCHORS SHALL BE DEFORMED REINFORCING BARS (ASTM A615, GR 60 OR GALVANIZED THREADED ROD (ASTM F1554 GRADE 55), UNLESS OTHERWISE NOTED, AND AS NOTED BELOW:

A. ADHESIVE SHALL BE HILTI HIT-RE 500 V3 10 APPROVED EQUAL. USE HILTI HIT-HY 270 FOR HALLOW OR GROUTED MASONRY OR APPROVED EQUAL. SUBMIT PUBLISHED COMPARISONS BETWEEN EACH SPECIFIED AND EACH ALTERNATE ANCHOR.

B. PRIOR TO INSTALLATION: ALL DEFORMED BARS AND THREADED ROD SHALL BE CLEANS, FREE OF OIL, GREASE, OR OTHER RESIDUE, IN ACCORDANCE WITH MPIL

C. VERIFY HOLE IS CLEAR OF DUST AND DEBRIS.

D. INSTALL ADHESIVE STARTING AT BACK OF HOLE. AS REQUIRED BY MPII, USE MANUFACTURER SUPPLIED PISTON PLUG INJECTION SYSTEM FOR ALL HORIZONTAL AND VERTICALLY INCLINED HOLES.

E. INSTALL ANCHOR BY SIMULTANEOUSLY TWISTING AND INSERTING INTO HOLE.

F. ALLOW ANCHOR TO SET REQUIRED TIME, DO NOT DISTURB.

G. TIGHTEN NUT, DO NOT OVER-TORQUE.

H. MINIMUM CONCRETE AGE AT TIME OF INSTALLATION 28 DAYS.

J. CONCRETE MOISTURE CONDITION AT TIME FOR INSTALLATION DRY.

1. INSTALL IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII), BUT

REGULATIONS OR TO RELIEVE CONTRACTOR OF COMPLIANCE WITH ALL APPLICABLE SAFETY AND OSHA REGULATIONS. IN CASE OF CONFLICT WITH SAFETY OR OSHA REGULATIONS, CONTACT THE ENGINEER FOR GUIDANCE BEFORE PROCEEDING WITH FABRICATION OR CONSTRUCTION.

I. CONCRETE TEMPERATURE RANGE AT TIME OF INSTALLATION SHALL BE 41°F TO 104°F.

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CTDIICTIIDAI CENEDAI NOTEC	SINCOLORAL GENERAL NOILS	FLOODWATER RETARDING STRUCTURE SITE No. 3	LOWER EAST FORK LATERALS	Ζ	TRINITY RIVER WATERSHED, TEXAS
				Fort Worth Texas 76102	Phone — (817) 735–7330 Web — www.freese.com
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SCHEDULE OF	QUANTITIES	
ITEM	SIZE	QUANTITY
CHANNEL	C 15X33.9	8
BRACKET	SEE DETAIL	32
BOLT A*	1/2"X2-1/2"	64
BOLT B*	5/8"X12"	24
PIPE SLEEVE	3/4"X10"	24

5 58	31 AND 5	82.		
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			FLOODWATER RETARDING STRUCTURE SITE No. 3	LOWER EAST FORK LATERALS	Z	TRINITY RIVER WATERSHED, TEXAS	
6				. 801 Cherry Street. Suite 2800	Fort Worth Texas 76102	Phone - (817) 735-7330 Web - www.freese.com	
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						ST	EEL S	SCHE	EDUL	E
MARK	SIZE (bar #)	QUANTITY	TYPE	Length (ft) (in)	B (ft) (in)	C (ft) (in)	TOTAL LENGTH (ft) (in)		MARK	size (bar #
B 1	5	52	1	10-0			520-0		Τ1	5
B 2	5	20	1	25-7			511-8		Т2	5
В3	5	4 4	21	11-5	0-10	10-7	502-4		Т3	5
B 4	5	20	1	25-7			511-8	-	T4	5
B 5	5	52	1	10-0	_		520-0	4	T5	5
<u> </u>	5	12	21	11-5	0 - 10	10 - /	137 - 0	4	16	5
<u> </u>	5	9	21	11-5	0 - 10	10 - 7	102 - 9	-		5
<u> </u>	5	42	2	-5	0 - 10	10 - 7	4/9-6			5
B 9	5 5	10	1	167			90-0	-	T10	5 5
	5	20	1	16 - 7			431 - 2	-	T10 T11	5
B10a	5	20	21	15 - 7	4 — 1	11-6	405 - 2	-	T12	5
B11a	5	26	1	11 - 6			299 - 0	-	T 1 3	5
B12	5	26	21	8-1	0 - 10	7-3	210-2	-	T14	5
B13	5	26	21	8-1	0 - 10	7-3	210-2	-	T15	5
B14	5	2	21	9-7	0-10	8-9	19-2	-	T16	5
B15	5	2	21	9-4	0-10	8-6	18-8	-	T17	5
B16	5	2	21	9-0	0-10	8-2	18-0	-	T18	5
B17	5	2	21	8-9	0-10	7-11	17-6	1	T19	5
B18	5	2	21	8-6	0-10	7-8	17-0		T20	5
B18a	5	2	21	8-2	0-10	7-4	16-4			
B19	5	2	21	9-7	0-10	8-9	19-2	_	T21	5
B 2 0	5	2	21	9-4	0 - 10	8-6	18-8	-	T22	5
B 2 1	5	2	21	9-0	0 - 10	8-2	18 - 0	_	123	5
B 2 2	5	2	21	8-9	0 - 10		17-6	-	124	5
BZ3	<u>Э</u> Б	2		0-0	0 - 10	7 1	17-0	-	CZI .	C
BZJU	5	2	21	18 - 1	1 - 1	7 - 4	10-4	-	T26	5
B25	5	2	21	18-9	4 – 1	15 - 8	37 - 6	-	T27	5
B26	5	2	21	21-2	4 - 1	17 - 1	$\frac{37}{42-4}$	_	T28	5
B 2 7	5	2	1	14-4	0	14-4	28-8	-	T29	5
B28	5	2	1	15-8	0	15-8	31-4	1	Т30	5
B29	5	2	1	17-1	0	17-1	34-2		T31	5
B30	5	10	26	13-5	1'-9	7-8	134-2	1	T32	5
B 3 1	5	6	25	17-1	2-7	7-8	102-6		Т33	5
B32	5	19	17	7-0	0-10	6-2	133-0		T34	5
B33	5	19	1	6-2	*****		117-2	_	T35	5
								-	T36	5
								4	T37	5
								4	138	5
								-	139	5 E
								4	T_{140}	5 5
								-	T41	5
								1	T43	5
								-	T44	5
S1	5	8	1	2-8			21-4	1	L	L
S2	5	8	1	2-0			$\frac{-16-0}{16-0}$	1		
S3	5	16	1	5-6			88-0	1	WEIGI	HT PFR
S4	5	8	1	2-8	Manyana.	unantari	21-4	1	LINEA	R FOOT
								1	1	.043
L	a				-					

- NOTES: 1. BAR DIMENSIONS ARE OUT TO OUT OF BAR.
- 2. RADIUS OF BENDS EQUAL 3 BAR DIAMETERS FOR SIZES < #7 AND 4 BAR DIAMETERS FOR #8.
- 3. THE 2", 3", AND 4" DISTANCES FROM SPECIFIED CONCRETE SURFACES ARE CLEAR DISTANCE.
- 4. STEEL NOT SHOWN IS IDENTICAL TO INSIDE ENDWALL STEEL.

	_		_		
			_	~	TOTA
		LENGTH	B	C	IOTAL
an i l'Y	ΙΥΡΕ	(ft) (in)	(†t)	(†t)	LENGTH
			(in)	(in)	(†t) (in)
12	1	9-1			109-0
24	1	4-3	0	0	102-0
8	1	5-0	******		40-0
16	1	13-0			208-0
2	21	9-11	0-10	9-1	19-10
10	21	10-10	1-9	9-1	108-4
28	1	4 – 3			119-0
32	21	12-2	4 – 1	8-1	389-4
4	1	12-0	VIIIIVI		48-0
4	1	10-8			42-8
4	1	14-8			58-8
4	1	18-8			74-8
4	1	22-2			88-8
2	1	13-6			27-0
26	1	5-6			143-0
<u> </u>	1	5-1			40 - 8
<u> </u>		4-/			36-8
Ö		4-1			32-8 20 1
0 	1	$3-\delta$			29-4
0		J-2			20-4
8	1	2-8			21-4
8	21	12-2	4 – 1	8 – 1	97-4
2	1	12-0		-	24-0
2	1	10-8			21-4
2	1	14-8			29-4
2	1	18-8	*****		37-4
2	1	22-2			44-4
8	21	7-3	1 - 9	5-6	58-0
4	21	6-10	1-9	5 – 1	27 - 4
4	21	6-4	1-9	4-7	25-4
4	21	5-10	1-9	4 - 1	23-4
4	21	5-5	1-9	3-8	21-8
4	21	4-11	1-9	3-2	19-8
4	21	4-5	1-9	2-8	1/-8
4	1	6-/		******	$\frac{26-4}{10}$
2	1	10-5			20 - 10
<u> </u>	1	14-2			340-0
	1	$22-\delta$			2/2-0
	1	9 - 10 10 - 5			39 - 4 20 - 10
20	1	14 - 2			20-10
<u>20</u>	1	14-2			56-8
12	1	22-8			272-0
4	1	9 - 10			39-4
	, ,				
STEFI	OUANTITI	ES			
VILLE	~~~				

JILLE	QOANTIES	
	LENGTH IN	WEIGHT IN
R SIZE	FEET	POUNDS
5	10,010	10,440

DATE APPROVED

9" C E 26	B01 Cherry Street, Suite 2800 Fort Worth Texas 76102 Phone - (817) 735-7330 Web - www.freese.com
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	SHEET NO.
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	STREN	IGTH REQUIREMENTS				JOINT REQU	JIREMENTS		
	INTERNAL LOAD		AL LOAD		LENGTH OF PIPE SECTION	MINIMUM JOINT LENGTH	MAXIMUM DEF PER ALIGNM	LECTION ANGLE	
INSIDE	PRESSURE	IN POUNDS PER LI	NEAL FOOT OF PIPE		FEET	INCHES	RADIANS	DEGREES	
DIAMETER OF PIPE INCHES		APPLICABLE STANDARD SPECIFICATION			10	3-1/4	0.005	0.29	
	HEAD OF WATER	AWWA C-301	AWWA C-300		20	3-1/4	0.008	0.43	
INCHES	LOAD TO PRODUCE LOAD TO PRODUCE 0.001 IN. CRACK 0.01 INCH CRACK S FEET ONE FOOT LONG ONE FOOT LONG			FOR PIPE CONFIGURATIONS OTHER THAN SHOWN, JOINT					
48	50	2880 LB/FT	-						
THE OUTSID WHERE THE ASSUMED IN FURNISHED	E DIAMETER OF PIF PIPE FURNISHED H N DESIGN, THE THR MUST NOT BE LESS	PE ASSUMED IN DESIGN AS AN OUTSIDE DIAME EE-EDGE BEARING STRE THAN THE SPECIFIED T	IS 57.5 INCHES. TER GREATER THAN NGTH OF THE PIPE HREE-EDGE BEARING		WHERE PIPES OF DIFFERENT LENGTH ARE CONNECTED, ADJOINTING PIPES SHALL MEET THE REQUIREMENTS OF THE LONGER PIPE.				
STRENGTH I	MULTIPLIED BY THE HED TO THE OUTSI	RATIO OF THE OUTSIDE	E DIAMETER OF THE D IN DESIGN.		PRIOR TO DELIVE	ERY OF PIPE. THE F	PIPE JOINT DETAI	 L	
REFER TO MATERIAL SPECIFICATIONS 541 FOR FURTHER DETAILS ON PIPE STRENGTH REQUIREMENTS.					PROPOSED FORUSE SHALL BE SUBMITTED TO THE CONTRACTING OFFICER FOR APPROVAL.				
						JOINT LENGTH EQUALS WATERTIGHT JOINT EXTENSIBILITY PLUS JOINT GAP.			

1. PROVIDE TWO LAYERS OF REINFORCING. REINFORCING BARS SHALL BE PLACED 3" CLEAR BELOW THE TOP OF CONCRETE AND 3" CLEAR ABOVE BOTTOM OF CONCRETE.

NOTE:

CRADLE CONCRETE QUANTITES							
INSIDE DIAMETER OF PIPE INCHES	OUTSIDE DIAMETER OF PIPE INCHES	CONCRETE PER LINEAL FOOT OF CRADLE CU. YDS.	LENGTH OF CRADLE FEET	TOTAL CONCRETE QUANTITY CU. YDS.			
48	56.5	0.49	70	34.3			
TOTAL CONCRETE QUANTITY = 34.3							

QUANTITY PER STRUCTURE	LENGTH	TYPE	В	С	TOTAL LENGTH
16	3'-6"	17	2'-6"	0'-6"	56'-0"
8	3'-6"	7	1'-6"	1'-0"	28'-0"
					84'-0"
	QUANTITY PER STRUCTURE 16 8	QUANTITY PER STRUCTURELENGTH163'-6"83'-6"	QUANTITY PER STRUCTURELENGTHTYPE163'-6"1783'-6"7	QUANTITY PER STRUCTURELENGTHTYPEB163'-6"172'-6"83'-6"71'-6"	QUANTITY PER STRUCTURE LENGTH TYPE B C 16 3'-6" 17 2'-6" 0'-6" 8 3'-6" 7 1'-6" 1'-0"

	DESIGNED BY:	DRAWN BY: Checker DV.	CHECKEU DI.		DATE CHECKED:
	PRINCIPAL SPILLWAY PIPE DETAILS	FLOODWATER RETARDING STRUCTURE SITE No. 3	LOWER EAST FORK LATERALS	R	TRINITY RIVER WATERSHED, TEXAS
	DRAWIN		801 Cherry Street, Suite 2800	Fort Worth Texas 76102	Phone — (81/) /35—/350 Weh — www freese com
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3080 Sq Ft 77 Cu Yds

EEL	QUANT	ITIES
BAR SIZE	TOTAL LENGTH IN FEET	TOTAL WEIGHT IN PONDS
4	5258-2	3512
5	4801	5007
6	3215-2	4829
7	502-0	1026
8	_	_
EIGHT A	ALL BARS	14375

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			801 Cherry Street, Suite 2800	Fort Worth Texas 76102	Phone - (817) 735-7330 Web - www.freese.com	
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LOCATION	MARK	SIZE	QUA	NLEN	GTH T	TYPE	В	С	LENGTH	LOCATI	ON MA	ARK	SIZE	QUAN	LENG	TYP	×ε B	C	LENGT	HLO	CATION	MAR	SIZE	QUA	NLENO	STH T	YPE E	В	CL	ENGT	LOCATIO	DN MAI	RK SIZ	EQUA	NLENG	THTY	PE B	С	LENGT	HLOCATI	ON MAR	rk si	IZE QUA	NLEN	NGTH	YPE	В	С	LENGTH
Headwall	1	5	2	18-	-10 5	St			37-8	Apron	. 4	3	6	2	7-11	21	2-11	5-0	15-10	Wi	<u>ngwall</u>	85	6	2	6-9		21 1-	-0	5-9	13-6	Sidewal	1 127	7 4	2	10-	8 St			21-4	Sidewa	11 169	4	1 2	7	7-9	St			15-6
	2	4	4	18-	-10 3	St			75-4	"	4	4	6	2	8-5	21	3-5	5 5 - 0	16-1	0		86	6	2	7-0		$\frac{21}{1}$	-0	6-0	14-0	"	128	3 4	2	11-	3 St			22-6	"	170	4	1 2	8	8-2	St	· .		16-4
11	3	5	5	6-	-0 3	St			30-0	····	4	5	6	2	9-9	21	4-9	$\frac{1}{5-0}$	13-1	0		8/	6	2	1-2		$\frac{21}{1}$	-0	6-2	14-4	<u>k "</u>	129	$\frac{2}{4}$	14	13-	6 21		$\mathbf{y} = 1 - \mathbf{q}$	189-0) "	171	4	1 2	8	8-6	<u>St</u>			17-0
"	4	5	$\frac{2}{2}$	6-	-11	St _			$\frac{13-0}{107-0}$		4	6	6	10	8-8	21	3-8	5-0	86-8		<u>-11</u>	88	6	$\frac{2}{2}$	7-5		$\frac{21}{1}$	-0	6-5	14-10		130	2 4	12	5-0	$\frac{1}{1}$			50-0		172	4	1 2	8	8-10	<u>St</u>			17-8.
	5	5		7-	-Z 15	St			197-2	Wingwai	11 4		4	18	<u>15-11</u>	St		+	1.27-4			89	6	2	/-9		$\frac{21}{1}$	-0	6-9	1.5-6		131	4	16	5-3		0-6	1-9	84-0	<u>"</u>	173	4	1 2	9	9-2	St			18-4
	6	4	5	6-					$\frac{30-0}{12}$		4	8	7	8	27-0	St			216-0			90	6	$\frac{2}{2}$	1 7-0					14-11		132			10	7 10	3-11	1.4-5	$\frac{520-5}{0}$		174	4	$\frac{1}{2}$		9-/	<u>St</u>			19-2
	+	4 4	2	6-	-5				$\frac{12-10}{13-10}$	1	4	<u>9</u>	- <u>-</u>	4	$\frac{25-8}{24-2}$	St St			06.9			91	6	12	6-3		SL			11 2		. 133		18	19-	$\frac{5}{19}$	1-3	10-	0 340-0	5 "		4		9	9-11	St			19-10
		4-4-N/		6-	-11	50			13-10			1	4	4	21 0	<u> </u>			90-5			-92		1 5	<u>J-/</u>		S6 C+			0.0		1.34			1/	1.06					177	4	$\frac{1}{2}$	-10	$\frac{0-3}{2}$	St			20-6
	<u> </u>							(1 //		2	4	4	15 0	- <u> 01</u> C+		+	62 0		и	93	4.	1-5-	4-1		SL			9-8		130	$\frac{2}{2}$		15-				30-6		170	4		10					21-2
	10	4	22	5-	-1 5	St			111 - 10			2	4	4	10 6	<u> </u>		+	103-0			94	4	15	$\frac{4-1}{3-5}$		St 56			6-10		12	$\frac{1}{7}$		15-0				20.0	п	179	4	$\frac{1}{1}$			<u>St</u>			22-0
	10	5		21	2	7 1	0 0	12	160 /	1			4	- 4-	26			+	5_0	Ha	aduall	95	4		7-4		21 5-	-5 1	-11	7-4	1 "	120		2	12	$\frac{1}{5}$ $\frac{5}{5}$			26.10		100			11	1 0	C+			22-0
	12	5	22	6	-6	S't	0-0		109-4 143-0	ii -	F	55	4	2	$\frac{2-0}{2-10}$	St		+	5-8	116	II	07	4	2	7-7		21 5-	-8 1	-11	15-2	i "	120	$\frac{1}{2}$	2	11.	10 136		· 	20-10	П	181	4	$\frac{1}{2}$	12	2_0	<u>St</u>			23-4
11	11	5		18-	-1	St			54-3	П	5	6	4	'2	3-2	St		1	6-4		П	98	NC	ŇF							//	110	$\frac{9}{1}$	$\frac{2}{2}$	10-	$\frac{1}{2}$ $\frac{1}{5}$			20-6	11	182		$\frac{1}{2}$	12	2-11	St St			-74-0
11	15	5	22	5-	-3	St I			115-6	.11	F	57	4	2	3-7	St		1	7-2		П	99	5	12	14-6		21 12-	-7	1-11	29-0		140	$\frac{1}{1}$	$\frac{1}{2}$	8-1	2 St			17_1	"	183		$\frac{r}{1}$ $2/$	12	2_7	St St			202 0
Wingwall	16	6	2	16-	-2	19	1-3	14-11	32-4	11	, F	58	4	2	á-7	St		1	8-0		11	100	4	12	12-1	1	21 11-	-0 1	1-11	155-0	۳ ا	11/2	2 5	1	12-1	2 21	1-3	10-1	11 18-8	Wingwa	11 184		1 8	16	5-5	19 1	1-0 1	5-5	121-4
11	17	4	4	16-	-6	19	1-7	14-11	66-0	"		59	4	2	4-4	St	-	1	8-8		П	101	1	18	5-7		T1 0-	-6	1_11	100-6	"	11	3 5	1	17_	2 22	1_2	15 (1 60 0	11	185		1 2	12	2-0	St			210
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11	24	6	6	6-	-10	St			41-0	11	6	66	4	2	6-11	St			13-10		Н	1.08	4	8	6-10		St			54-8	"	150	0 6	2	5-2	2 19	1-3	3-1	11 10-4	11	192	5	2	7	-2	St			14-4
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REVISED BY	DATE	REFERENCES
FREESE AND NICHOLS, INC.	AUGUST 19, 2020	NRCS RECOMMENDATION

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DESIGNED BY:	DRAWN BY:	CHECKED BY:			DATE CHECKED:	
INDACT BASIN _ STEEL SCHEDLILE		FLOODWATER RETARDING STRUCTURE SITE No. 3	LOWER EAST FORK LATERALS	Z	TRINITY RIVER WATERSHED, TEXAS	
			801 Cherry Street, Suite 2800	Fort Worth Texas 76102	Phone - (817) 735-7330 Web - www.freese.com	
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