

4049 Reid Street • P.O. Box 1429 • Palatka, FL 32178-1429 • (386) 329-4500 On the Internet at www.sjrwmd.com.

- DATE: August 17, 2017
- TO: Prospective Respondents
- FROM: Pam Paulk, Sr. Contracts Administrator
- SUBJECT: Addendum #2 to Invitation for Bids #31729 2nd Call, 10-Pack Structure "A" Rehab & Modification and 10-Pack Structure "B" Rehab and Demolition

As a result of the Mandatory Pre-Bid meeting the following information is provided for your consideration. Please make all appropriate changes to your bid / proposal documents. Note: changes are reflected with original language shown with strike-through and new language is underlined.

MODIFICATIONS:

1. The plans Attachment D – Revised, cost schedule and statement of work have been modified. The modifications in the plans are denoted with a cloud around the text. The Revised Cost Schedule and Revised Statement of Work, Attachment A-1 were changed to match the plan modifications and are all attached to this addendum.

ADDITIONAL INFORMATION PROVIDED PURSUANT TO THE PRE-BID MEETING ON 8/16/17:

- 1. Attached as Exhibit A to this addendum is a copy of the Mandatory Pre-Bid Sign in Sheets for all three pre-bid meetings.
- 2. The canal at structure A is an estimated 10 14 feet deep.
- 3. The District has dive logs for the culverts as the water is too murky for visibility.
- 4. Structure A does not require a slope modification and the District will not be mowing the levee slopes, therefore vegetative matter may be removed from the slopes for this work by the Contractor, but the slopes shall be returned to a vegetative state to prevent slope erosion.
- 5. Contractor is required to maintain a walkway for cyclists and pedestrians solely at Structure A.
- 6. Turbidity barriers are required at both construction sites as denoted on the plans.
- 7. Removal of Structure B piles see Q&A #2 in Statement of Work under Site B.
- 8. All debris shall be removed from the existing culverts refer to 1st bullet in Section III Scope in the Revised Statement of Work attached.
- 9. Slope modification at Structure B is required and only on the old gate side refer to plans for detailed information.
- 10. It is recommended to walk the heavy equipment (such as the excavator) to Structure B due to the current condition of the levee.

NOTE: The Bid Opening remains the same, September 11, 2017, 2017 at 2:00 PM.

Please acknowledge receipt of this Addendum on the BID FORM provided in the bid package.

If you have any questions, please call me at (386) 329-4469 or e-mail ppaulk@sjrwmd.com.

COST SCHEDULE <u>REVISED</u>

Include this form in the response

Bid to be opened at 2:00 p.m., a September 11, 2017

To: ST. JOHNS RIVER WATER MANAGEMENT DISTRICT

In accordance with the advertisement requesting bids for the

APOPKA MARSH FLOW-WAY 10-PACK STRUCTURE "A" REHAB & MODIFICATION AND 10-PACK STRUCTURE "B" REHAB & DEMOLITION, subject to the terms and conditions of the Agreement, the undersigned proposes to perform the Work for the price contained in the following schedule (fill in all blanks). If said bid exceeds the estimated amount previously provided, the District expressly reserves the right to increase, decrease, or delete any class, item, or part of the Work, as may be determined by the District. Respondents are reminded to refer to "PREPARATION AND ORGANIZATION OF BID DOCUMENTS" for information to be included with the bid package.

The costs for each item below shall include all materials and labor for the installation and/or through completion.

Item #	Description - Structure "A"	<u>Units</u>	Quantity	Unit Cost	Total Value
1	Mobilization	LS	1	\$	\$
2	HDPE DR 32.5 Pipe Liner	LF	600	\$	\$
3	HP 8x36 WF8 x 13.0 x 25' Aluminum Piles	EA	17	\$	\$
4	WF8x13.0x4.5' Aluminum Beams	EA	6	\$	\$
5	48" Aluminum Slide Gates (144- inch frame)	EA	10	\$	\$
6	Structural SteelAluminum Walkway	LS	1	\$	\$
7	Cleanup & Demobilize	LS	1	\$	\$
8	As-Built Drawings	LS	1	\$	\$
			STRUCT	URE "A" TOTALS	\$
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Item #	Description - Structure "B"	<u>Units</u>	Quantity	_	Total Value
9	Mobilization	LS	1		\$
10	Demolition Wooden Walkways, Gates, etc.	LS	1		\$
11	HDPE DR 32.5 Pipe Liner	LF	700		\$
12	Cleanup & Demobilize	LS	1		\$
13	As-Built Drawings	LS	1		\$
			STRUCT	JRE "B" TOTALS	\$
			PROJECT	GRAND TOTAL	\$

PROJECT GRAND TOTAL (in words):

ATTACHMENT A -1 — REVISED STATEMENT OF WORK

APOPKA MARSH FLOW-WAY 10-PACK STRUCTURE "A" REHAB & MODIFICATION AND 10-PACK STRUCTURE "B" REHAB & DEMOLITION

I. INTRODUCTION BACKGROUND

The Lake Apopka Inlet Structures were constructed as part of the Lake Apopka Flow-way Project in the late 1990s to provide wetland treatment for Lake Apopka. Continuous flow through the wetland cells has occurred over the years since then using infrastructure constructed prior to the Flow-way to bring water in from the lake. This infrastructure included two sets of ten 54-inch corrugated metal culverts with a set of gates at one of the locations. These culverts and associated gates are decayed, leak badly and no longer function properly. The culverts can no longer be closed to flow as a result, and periodic maintenance on the wetland cells is hampered as a result of the failed culverts and gates. Currently, the 10-culverts incorporated in the "A" Structure do not have gates to control flows and the 10 culverts incorporated in the "B" Structure have gates for controlling flows through the system. It is anticipated that since flows to the wetland flowway can not be suspended, all work will be done in the "wet" with divers and no cofferdams.

II. OBJECTIVE

The objective of this project is to rehabilitate twenty (20) deteriorated 54-inch corrugated steel culverts (approximately 65 linear feet per culvert or 1300 linear feet total) by slip-lining with 48-inch O.D. DR 32.5 HDPE pipe at both Structures "A" & "B", install 10-new screw gates and gate supports and structural Aluminum walkway at Structure "A", and demolition of existing gates and wooden walkways at Structure "B" location, as shown on the attached drawings. Each culvert shall be reinforced with a high-density polyethylene (HDPE) pipe culvert sleeve inserted, together with a low-density cellular grouting application to correct current culvert leakage and deterioration due to rusting. The construction shall be conducted in a manner that minimizes damage to the existing road crossings and accommodates full uninterrupted flows through the system. No work at Structure "B" shall commence until all gates have been installed, secured and approved at Structure "A".

III. SCOPE

Prior to the pre-construction meeting, Contractor shall provide a dive plan for underwater activities, plus the methodology that will be employed to install continuous lengths of 48-inch O.D. diameter HDPE pipelining sleeves into existing 54-inch corrugated metal culverts at each location as shown on the attached drawings. The methodology shall include timelines for all materials deliveries, mobilization/demobilization, labor, equipment, transportation, jointing and installation of the HDPE pipe lining sleeve material including the grouting operation. Contractor shall install a new gate onto each newly slip-lined pipe at Structure "A" and secure with seventeen (17), each 25 linear feet minimum, new aluminum wide flange (WF) 8x13.0 steel HP 8x36 piles driven to top elevations as shown on the drawings. Contractor shall fabricate and install structural Aluminum access walkway in accordance with the plan details and dimensions at Structure "A". Contractor shall demolish and properly dispose of all wooden walkway structures together with existing gates, sand-cement bag headwall and associated components at Structure "B".

At a minimum, the Contractor shall provide details on how it will accomplish the following work:

- Cleaning method that will be used to remove the debris from the existing culvert, including disposition/disposal of debris
- Installation methodology for the HDPE pipe liner, including, but not limited to the following:
 - How line pipe joints will be formed to prevent separation.
 - How liner pipe will be centered in the existing pipe
 - How the pipe grade will be maintained parallel to grade of the existing culvert.
 - How flotation of the liner pipe will be controlled

- How the liner insertion will be controlled at Structure "A" only such that the ends result in near perfect alignment across the channel for uniform alignment of the gate frames and subsequent walkway access to be installed by others.
- Methodology that will be utilized to patch existing culverts and form ends to contain grout during injection. Grout must not interfere with installation or operation of the new screw gates where applicable at Structure "A" only.
- Grouting methodology for filling of all annular void spaces between the existing culvert and the new HDPE liner, including number and length of grout injection pipes and vent pipes. The grout shall be foamed with a density of not more than 50 lbs. per cubic foot and have a minimum strength of 300 psi. The price of the grout shall be included in the linear cost of the slip-lining operation.
- Installation methodology of the new <u>HPWF</u> piles to be used for securing the gates at Structure "A".
- Connection methodology of the new screw gates at Structure "A" to the new pipe liner together with how the new gate frames will be secured for stabilization. All dissimilar metals shall be isolated to prevent direct contact. All hardware connectors shall be stainless steel, with a generous coating of anti-seize compound applied prior to assembly.
- Fabrication methodology for the new walkway structure in accordance with the plans and specifications for access to the gate operators.
- Erosion and sediment control plan to protect waters from siltation and grout (i.e., installation of floating turbidity barriers/silt screens around the construction site, etc.)
- Maintaining a clean and safe work environment

The Contractor shall provide details on the following materials to be provided for the project:

- Specifications and source of the solid wall HDPE pipe lining material with minimum DR 32.5 rating and 1.477 inch wall thickness, including the hydraulic flow capacity.
- Specifications and source for Low-density grout materials (type, density, etc.)
- Specifications and source of ten (10) Aluminum Slide Gates to fit 48" O.D. HDPE Liner, with leakage not to exceed 0.05 gpm/ft of the seal perimeter, 144-inch tall frame as measured from gate invert to top of frame, minimum 24" diameter hand wheel, and fabricated in accordance with the latest edition of the American Water Works Association (ASSA) C562 Standard for Fabricated Aluminum Slide Gates, formerly manufactured by Mechanical Associated of Fresno, CA, or approved equal.
- The vendors listed below may be able to provide gates comparable to the ones previously available from Mechanical Associates. However, gates may be obtained from other sources provided the specifications of the pipe/gate connection type can be maintained.

Whipps Slide Gates

James R Wahl The Mack Company 17088 Gulf Pine Circle Wellington, FL 33414 Off 561-798-3131 Cell 954-646-3465 jwahl@mackcompany-fl.com

Hydro Gates

Karen Wolfe EnviroSales of Florida, Inc. Representative for Hydro Gate (863) 314-0616 karen@envirosalesofflorida.com

Golden Harvest, Inc.-Golden Gates

Tom Cluin Regional Sales Manager (901) 489-6769 Tom.cluin@gmail.com

- Specification and source of seventeen (17) walkway support piles, 25 feet each in length.
- Specification and source of material to be used for isolation of dissimilar material, including concrete, on gate frames and walkway supports where required to be in contact for mounting.
- Specification and source of materials, fabrication and installation of the structural steel gate controls access walkway.

Upon District approval to proceed, Contractor shall perform each item of work identified above in accordance with the approved methodologies and procedures.

NOTES:

- 1. The Contractor shall provide portable toilets for its work crews and subcontractors as there are no facilities in the job site vicinity.
- 2. No hunting, fishing, or firearms are allowed on the job site. The Contractor is responsible for contacting the District and the local Florida Fish and Wildlife Conservation Commission should any problem arise with nuisance animals.
- 3. The Contractor shall be responsible for retaining a trapper to provide diver protection for this project
- 4. The Contractor shall provide the District with written notice 48-hours prior to when the HDPE liners will be installed in order to provide sufficient time for the District to schedule appropriate staff to observe the installation.
- 5. The Contractor shall provide traffic control and signage to discourage entry of the work site by pedestrians, bicyclists and other vehicles that access the property on a daily basis.
- 6. Contractor shall adhere to OSHA Standards 29 CFR 1910 Subpart T Commercial Diving Operations, US Navy Diving Standards, And all other governing bodies for minimum standards.
- 7. Contractor shall submit Daily Logs, Dive Logs, videos and other documentation as deemed appropriate at the end of each week or when requested by the District.
- 8. All dive helmets must be fitted with underwater hard wired HD video with monitor unit at all times for engineers/inspectors to monitor the work performed.
- 9. All hydraulics shall use bio-degradable oil. Contractor shall have a spill response kit on site at all times.
- 10. District will provide Contractor with access to the structures.
- 11. One of the gator trappers approved by the Florida Fish and Wildlife Conservation Commission (FWC) is George Walrath who may be contacted via e-mail <u>g.w.walrath@gmail.com</u> or phone: 407-466-4544. Although, the following website: <u>http://myfwc.com/wildlifehabitats/managed/alligator/</u> additional information and a list of other trappers may be obtained by contacting FWC

IV. TIMEFRAMES AND DELVERABLES

Work is anticipated to begin upon execution of the contract documents, and be completed by March 2, 20187. The Contractor shall be required to provide daily reports of work accomplished, together with signed and sealed as-built drawings and AutoCAD compatible DWG file.

V. BUDGET

The Contractor shall be paid based on the completion of each item listed in the Cost Schedule which includes all labor and materials through completion. The estimated cost for this project is \$750,000.

VI. Numerous questions were received during the first soliciting of this project therefore the below questions and answers are provided for your consideration.

SITE A

- Q.1. How much material is in the pipe?
- A.1. Based on recent inspections, the District understands the pipes are reasonable clean and will not require excessive sediment removal but they do have to be cleaned prior to liner installation.
- Q.2. How much rip rap is to be placed on the bank?
- A.2. There is not need for additional stabilization. Contractor may have to trim the existing pipes back but they extend far enough out from the slope that this is not anticipated to be problem.
- Q.3. Will the slip line pipe be here?
- A.3. Yes
- Q.4. Can aluminum I Beams be used as opposed to steel for piles?
- A.4. Aluminum WF 8x13 will be acceptable, provided adequate additional length is available to be able to trim any damage resulting from driving the piles to the required depth. <u>All steel structural shapes have been eliminated</u> and are now to be aluminum. Refer to revised set of plans included in Addendum #1.
- Q.5. Is there any recent soil boring information?
- A.5. No, only soil boring data the District has available is from several years ago. The old data is not in a form suitable for distribution. The District Geotechnical Engineer has reviewed the data and is confident that the pile lengths identified should generally be adequate, however there does exist a possibility that splicing one or more piles may be necessary to achieve adequate bearing.
- Q.6. Will the contractor be allowed to pump sediments in the pipes into deeper water?
- A.6. No, the material should be removed and placed on nearby uplands as opposed to deposited in the canal. Depending on the actual volume of material in the pipes the District can evaluate this further in the field. However, for bidding purposes, the Respondent should plan to completely remove the sediments from the system.
- Q.7. Is the total metal (in the existing pipe) lost?
- A.7. All pipes have been inspected and determined to be suitable for lining with minimal distortion and/or loss of metal within the embankment. Some loss has occurred in the crowns of the pipes from periodic exposure but it is typically minimal. Some repair may be required to seal these for grouting the annular space.

<u>SITE B</u>

- Q.1. Will the telemetry equipment be on site during construction?
- A.1. No, it will be removed by the District prior to construction.
- Q.2. How far down are the existing piles to be removed?
- A.2. The piles may be cut off at the mudline or extracted by pulling.
- Q.3. Will the gates be reused?
- A.3. No, they are to deteriorated and shall be removed and disposed of by the Contractor.
- Q.4. Is there any embankment work to be completed?

- A.4. The District has not planned on the need for armoring (i.e. rip-rap). Shaping of the surface will be required with sod on the slopes. If it appears that more is needed a change order will be issued.
- Q.5. What is the weight limit of the levee, will the levee hold for a 150 Ton or a 70 Ton crane?
- A.5. The levees were built by former farmers and they are predominantly muck levees and may not support this type of loads. Contractor may proceed at their own risk. Contract must take caution to avoid causing excessive damage to the levees and restore any damaged areas to preconstruction conditions.
- Q.6. Any gates to be installed at Site B?
- A.6. No gates will be installed on this site. This site will result in open end, unrestricted flow culverts.
- Q.7. What are the work hours?
- A.7. Refer to Section 50, WORK SCHEDULE, in the agreement. Any modifications must be agreed to in writing.

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EXHIBIT A

ST. JOHNS RIVER WATER MANAGEMENT DISTRICT SIGN-IN SHEET FOR ATTENDEES

IFB # 31729 MANDATORY PRE-BID MEETING

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Attendee (Print Legibly): Ryan Harrington	Phone: 502 - 93 9-9909	Business Card Provided Or fill in contact information		
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IFB # 34040 MANDATORY PRE-BID MEETING

10-PACK STRUCTURE A REHAB & MOD. AND 10-PACK STR B REHAB & DEMO

Attendee (Print Legibly):	Phone:	Business Card Provided 🔀
Greg Sale	772 286 5123	Or fill in contact information
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IFB # 34040 MANDATORY PRE-BID MEETING

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	Sheet List Table
Sheet Number	Sheet Title
C1	COVER SHEET & VICINITY MAP
C2	OVERALL SITE PLAN
C3	SITE PLAN - STRUCTURE A
C4	PLAN & SECTION - STRUCTURE A
C5	SITE PLAN - STRUCTURE B
S1	WALKWAY PLAN AND ELEVATION - STRUCTURE A
S2	WALKWAY DETAILS - STRUCTURE A

1

FOR BID PURPOSES ONLY **NOT FOR CONSTRUCTION**

CERTIFICATION:	DRAWING FILENAME:	
	10-PACKS REHAB.dwg	
ROYCE CLIFTON GANDY	SHEET:	
P.E. NUMBER:23017		
DATE: AUGUST 9, 2017	<u> </u>	

BM SJRWMD 00-12-022-0 ELEVATION = 66.607' NAVD (88) FT.

RANCH ROAD

STRUCTURE A

REVISION

NOTES: 1. ALL ELEVATIONS SHOWN HERE ON ARE REFERENCED TO NAVD(88) FT. 2. ALL COORDINATES SHOWN HERE ON ARE REFERENCED TO HPGN AND PROJECTED TO FLORIDA ZONE (901) EAST.

BY DATE APPROVED DATE

APOPKA MARSH FLOW-WAY 10-PACK STRUCTURES A & B REHAB LAKE COUNTY, FLORIDA

TOPOGRAPHIC SURVEY A PARCEL OF LAND LOCATED IN SECTION 15, TOWNSHIP 21 SOUTH, RANGE 26 EAST, LAKE COUNTY, FLORIDA.

OVERVIEW

STRUCTURE B

	WATER	ST. JOHNS RIVE MANAGEMENT	
DRAWN:		DATE:MARCH 8, 2017	REVIEWER:
SCALE:	AS NOTED	DESIGNER: RCG	SECTION CHIEF:



TOPOGRAPHIC SURVEY A PARCEL OF LAND LOCATED IN SECTION 15, TOWNSHIP 21 SOUTH, RANGE 26 EAST, LAKE COUNTY, FLORIDA.

NOTE: ALL PIPES IN STRUCTURE "A" ARE 54" C.M.P. AS PER ENGINEER, AND NOT FIELD MEASURED (PIPES UNDERWATER)

PIPE ELEVATIONS AND LENGTHS NAVD (88) FT					
PIPE #	LENGTH	TOP OF PIPE (SOUTH)	TOP OF PIPE (NORTH)		
1	59.26	63.31	63.44		
2	59.53	63.37	63.43		
3	59.18	63.36	63.36		
4	58.95	63.36	63.33		
5	58.95	63.24	63.42		
6	58.59	63.38	63.32		
7	59.36	63.31	63.25		
8	59.30	63.46	63.51		
9	58.52	63.26	63.37		
10	59.81	63.31	63.59		

BY DATE APPROVED DATE



APOPKA MARSH FLOW-WAY 10-PACK STRUCTURES A & B REHAB LAKE COUNTY, FLORIDA

REVISION



NOTE: SEE C4 FOR MORE DETAIL

WATER	ST. JOHNS RIVE MANAGEMENT P.O. BOX 1429 PALATKA, FLOR	R DISTRICT
DRAWN:	DATE: MARCH 8, 2017	REVIEWER:
SCALE:1" = 10' - 0"	DESIGNER: RCG	SECTION CHIEF:

FOR BID PURPOSES ONLY **NOT FOR CONSTRUCTION**

SITE PLAN - STRUCTURE A

CERTIFICATION: ROYCE CLIFTON GANDY

P.E. NUMBER: _ 23017 MARCH 8, 2017 DATE:

FILE NAME:	
10-PA	CKS REHAB.dv

SHEET:

PROJECT NO .:

C3



REVISION

BY DATE APPROVED DATE

					(SJR)
	P.	O. BOX 1429 PA	LATKA, FLOR	IDA	
		DATE: MARC	H 8. 2017	REVIEWER	
DRAWN:					
DRAWN:					

C4

AUGUST 9, 2017

DATE:

TOPOGRAPHIC SURVEY A PARCEL OF LAND LOCATED IN SECTION 15, TOWNSHIP 21 SOUTH, RANGE 26 EAST, LAKE COUNTY, FLORIDA.

NOTE: ALL PIPES IN STRUCTURE "B" ARE 54" C.M.P.

36² 31

56³.56

PIPE ELEVATIONS AND LENGTHS NAVD (88) FT

36⁰.2⁰

PIPE #	LENGTH	TOP OF PIPE (EAST)	PIPE INVERT (EAST)	TOP OF PIPE (WEST)	PIPE INVERT (WEST)
1	70.19	63.13	58.63	63.62	59.12
2	70.28	63.31	58.81	63.77	59.27
3	69.67	63.17	58.67	63.49	58.99
4	70.16	63.14	58.64	63.54	59.04
5	70.67	63.28	58.78	63.41	58.91
6	70.43	63.31	58.81	63.42	58.92
7	70.73	63.29	58.79	63.52	59.02
8	70.45	63.22	58.72	63.40	58.90
9	70.66	63.16	58.66	63.24	58.74
10	65.90	63.36	58.86	63.31	58.81

BY DATE APPROVED DATE

APOPKA MARSH FLOW-WAY 10-PACK STRUCTURES A & B REHAB LAKE COUNTY, FLORIDA

351 .9 59.5

35⁸ 3

55⁵1.53

REVISION





NOTE: WOOD DOCK SUPPORT PILES TO BE SXTRACTED OR CUT OFF AT OR **BELOW MUD LINE.**

/BM SJRWMD 1861-15

TOP OF BANK

6' FT DIA. RISER W/ GATE (TYPICAL) -TO BE DEMOLISHED AND REMOVED

SAND-CEMENT BAG HEADWALL -TO BE DEMOLISHED AND REMOVED

DEMOLISHED AND REMOVED

FOR BID PURPOSES ONLY **NOT FOR CONSTRUCTION**

CERTIFICATION: ROYCE CLIFTON GANDY P.E. NUMBER: 23017 MARCH 8, 2017 DATE:

ILE NAME: 10-PACKS REHAB.dwg

SITE PLAN - STRUCTURE B

PROJECT NO .: SHEET:

C5



R	WATER	ST. JOHNS RIVE MANAGEMENT P.O. BOX 1429 PALATKA, FLOR	
DRAWN:	PLC	DATE:MARCH 8, 2017	REVIEWER:
SCALE:	AS NOTED	DESIGNER:WRC	SECTION CHIEF:

WALKWAY PLAN AND ELEVATION - STRUCTURE A

NOTE SPECIFICATIONS:

CONCRETE:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION, SECTION 400 WITH SUPPLEMENTS AND ALL PERTINENT SPECIFICATIONS CONTAINED THEREIN.
- 2. ALL CONCRETE SHALL BE FDOT CLASS I WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3000 PSI. PORTLAND CEMENT SHALL BE TYPE II IN ACCORDANCE WITH ASTM C-150. THE AGGREGATES SHALL CONFORM TO ASTM C-33 AND SHALL HAVE A 3/4-INCH MAXIMUM SIZE.
- 3. REINFORCING STEEL SHALL BE GRADE 60 DEFORMED BILLET STEEL BARS CONFORMING TO ASTM A-615.
- 4. THE MINIMUM CLEAR CONCRETE COVER FOR REINFORCEMENT SHALL BE 3 INCHES FOR CONCRETE CAST AGAINST EARTH AND 2 INCHES ELSEWHERE, UNLESS OTHERWISE NOTED.
- 5. CONCRETE ANCHORS SHALL UTILIZE THE HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM, OR EQUAL. THREADED ANCHOR RODS, SHALL BE 3/4" DIA. X 8-1/2" LONG HAS-R 316 STAINLESS STEEL WITH A MINIMUM EMBEDMENT DEPTH OF 6-3/4". NUTS AND WASHERS SHALL ALSO BE SS-316.

DELETED STRUCTURAL STEEL AND PAINTS AND PROTECTIVE COATINGS NOTES STRUCTURAL ALUMINUM:

- 1. STRUCTURAL ALUMINUM DESIGN AND FABRICATION SHALL BE IN ACCORDANCE WITH THE ALUMINUM ASSOCIATION, INC. "SPECIFICATIONS FOR ALUMINUM STRUCTURES", LATEST EDITION.
- 2. WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) "STRUCTURAL WELDING CODE - ALUMINUM" AWS D1.2.
- 3. ALUMINUM STRUCTURAL SHAPES SHALL BE NEW AND CONSIST OF ALLOY 6061-T6 CONFORMING TO THE REQUIREMENTS OF THE AMERICAN SOCIETY OF TESTING AND MATERIALS (ASTM) STANDARD B308.
- 4. ALUMINUM BARS, RODS, AND WIRE SHALL BE NEW AND CONSIST OF ALLOY 6061-T6 CONFORMING TO THE REQUIREMENTS OF ASTM STANDARD B211.
- 5. ALUMINUM PLATE SHALL BE NEW AND CONSIST OF ALLOY 5052-H32 CONFORMING TO THE REQUIREMENTS OF ASTM STANDARD B209.

6. ALL BOLTS, NUTS, AND WASHERS SHALL CONSIST OF SS316 STAINLESS STEEL CONFORMING TO THE REQUIREMENTS OF ASTM STANDARDS F593 AND F594. MINIMUM BOLT SIZE SHALL BE 3/4-INCH UNLESS OTHERWISE NOTES.

- _____ 7. ALL WELDING SHALL UTILIZE ER4043 FILLER ALLOY AND SHALL BE SHOP WELDED TO THE GREATEST EXTENT POSSIBLE.
- 8. THE MINIMUM THICKNESS OF ALL CONNECTION ANGLES AND GUSSET PLATES SHALL BE ¼-INCH UNLESS NOTED OTHERWISE.
- 9. FIELD CORRECTING OF FABRICATED COMPONENTS SHALL NOT BE PERMITTED ON STRUCTURAL MEMBERS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- 10. ALUMINUM GRATING SHALL BE RECTANGULAR BAR TYPE (SERRATED), SWAGE-LOCKED, AND CONSIST OF ALUMINUM ALLOY 6063-T6. THE BEARING BARS SHALL BE 1-1/2" X 3/16" AT 1-3/16" ON CENTER. RECTANGULAR CROSS BARS SHALL BE 4" ON CENTER.
- 11. THE HANDRAIL POSTS AND RAILS SHALL BE 1-1/2 INCH DIAMETER SCHEDULE 40 PIPE FORMED FROM EXTRUDED 6063-T6 ALUMINUM EXCEPT THAT FORMED ELBOWS SHALL BE 6063-T4 ALUMINUM. THE MAXIMUM POST SPACING SHALL BE 6'-0" CENTER TO CENTER.
- 12. THE STRUCTURES ARE DESIGNED AS STABLE UNITS AFTER ALL COMPONENTS, INCLUDING BRACING, ARE IN PLACE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING TEMPORARY BRACING AS REQUIRED TO ENSURE THE VERTICAL AND LATERAL STABILITY OF THE ENTIRE STRUCTURE OR ANY PORTION THEREOF DURING CONSTRUCTION.

FOR BID PURPOSES ONLY **NOT FOR CONSTRUCTION**

WILLIAM R. COTE	
CERTIFICATION:	

.E. NUMBER:	53746
ATE:	AUGUST 9, 2017

FILE NAME:	
10-PACK WALKWAY.dwg	
PROJECT NO.:	
SHEET:	

S1



DETAILS -	- STRUCTURE /	4
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WILLIAM R. COTE	

WIL	LIAM R. COTE	
NUMBER:	53746	
Ē	AUGUST 9 2017	

FILE NAME:
10-PACK WALKWAY.dwg
PROJECT NO.:
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