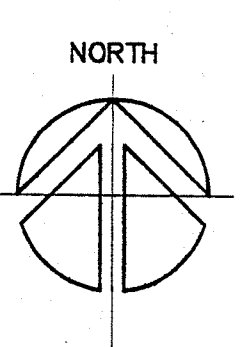
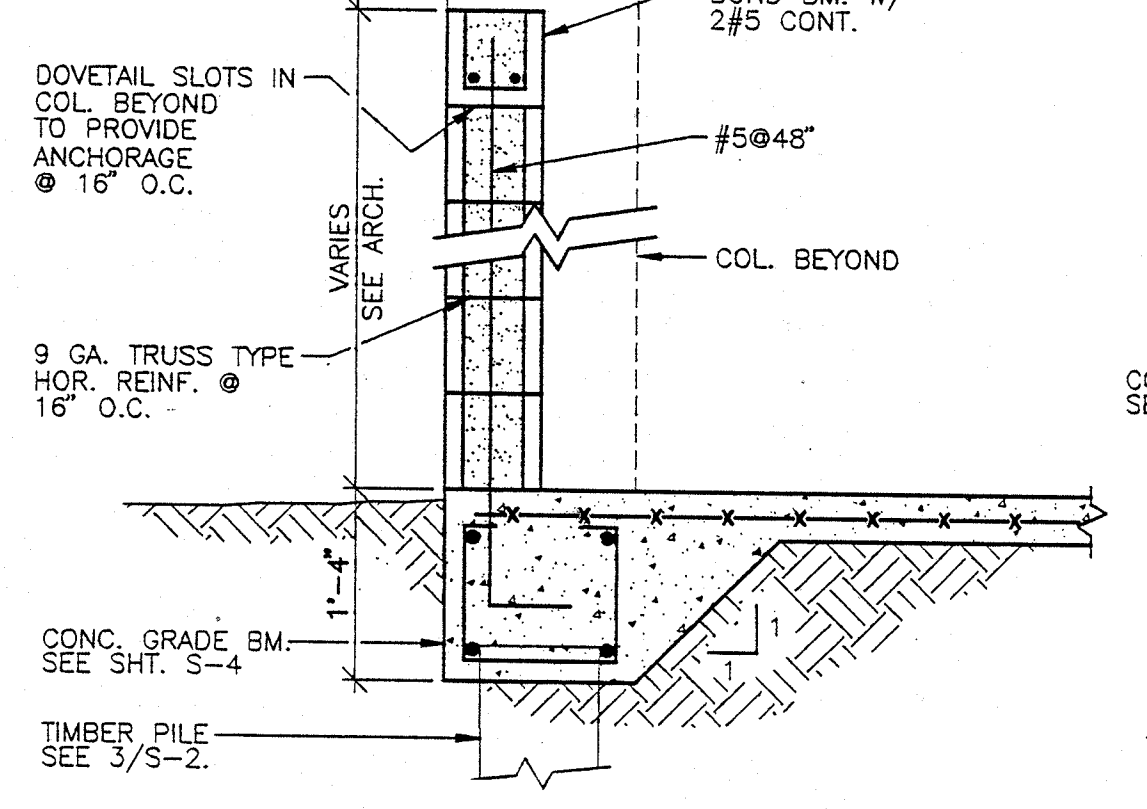


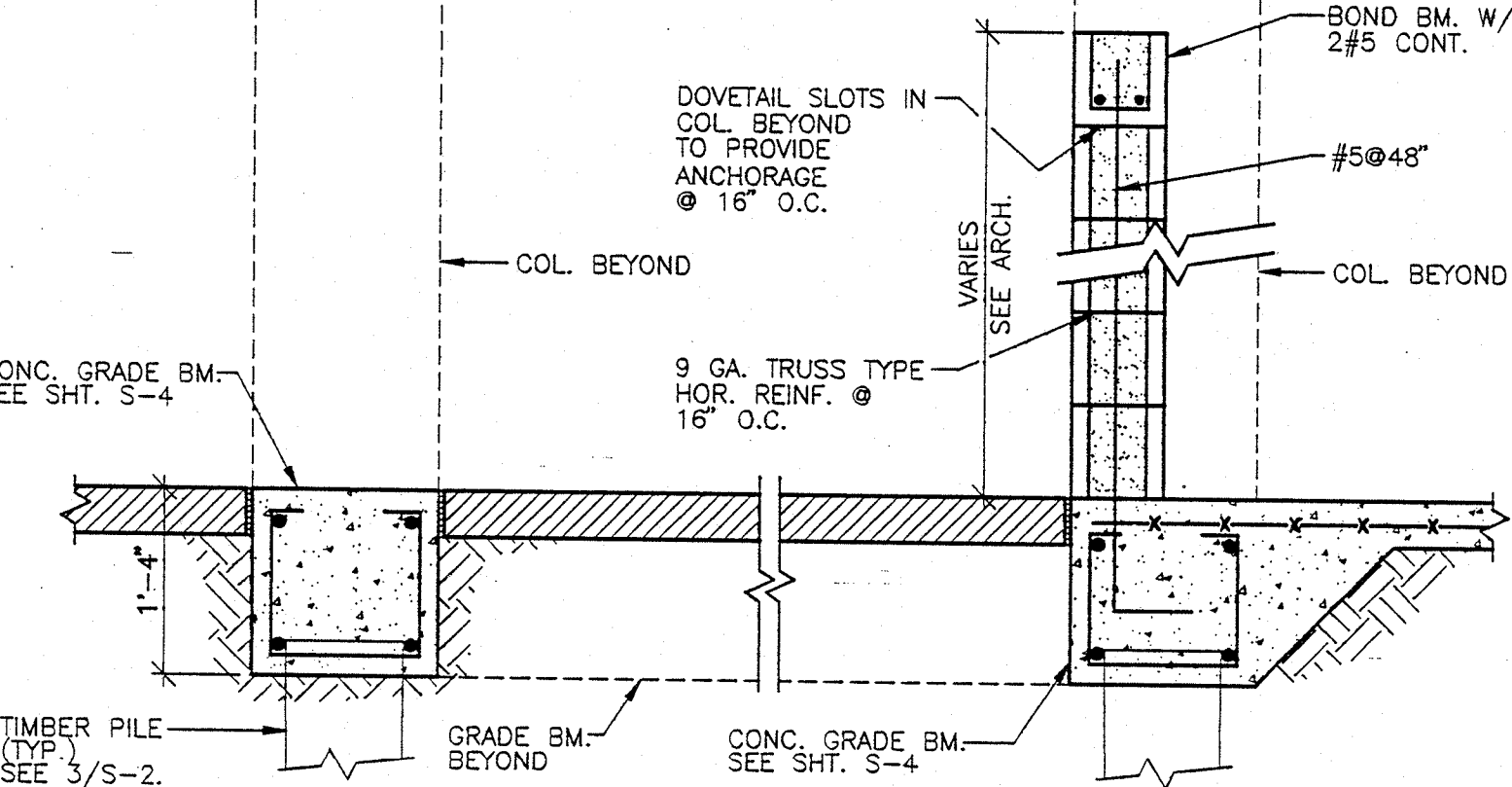
1 RETAIL FOUNDATION PLANS
SCALE: 1/8" = 1'-0"



- LEGEND:
- = WALL NUMBER REFER TO SHEET S-5
 - C.J. = CONSTRUCTION OR CONTROL JOINT
 - O.H. = OPPOSITE HAND



2 SECTION
S-1 SCALE: 3/4" = 1'-0"



3 SECTION
S-1 SCALE: 3/4" = 1'-0"

STRUCTURAL GENERAL NOTES

- 1.00 GENERAL**
- 1.01 ALL CONSTRUCTION SHALL CONFORM TO THE STANDARD BUILDING CODE, 1993 EDITION. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE LATEST STANDARD OR CODE ADOPTED AND PUBLISHED.
- 1.02 DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. FOR DETAILS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN. FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.
- 1.03 VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS BEFORE STARTING WORK. NOTIFY CONTRACTING OFFICER OF ANY DISCREPANCY.
- 1.04 THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 1.05 COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL. NOTIFY ENGINEER OF RECORD OF ANY CONFLICT AND/OR OMISSION. CONTRACTOR SHALL MAKE NO DEVIATION FROM DESIGN DRAWINGS WITHOUT WRITTEN APPROVAL OF THE ENGINEER OF RECORD.
- 1.06 REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- 1.07 SUPERIMPOSED DESIGN LIVE LOADS (OR DEAD LOAD AS NOTED):
- | | |
|----------------|----------|
| ROOF DEAD LOAD | 15 PSF |
| ROOF LIVE LOAD | 20 PSF** |
- ** live load reduction for tributary area is taken as per code.
- WIND LOAD IS BASED ON A WIND SPEED OF 120 MILES PER HOUR.

- 3.02 CONCRETE COVER OVER REINFORCING SHALL BE 2" MINIMUM, 3" FOR CONCRETE EXPOSED TO EARTH.
- 3.03
- | | STRENGTH | MAX W/C RATIO | % SILICA FUME BY WT. OF CEMENT |
|---|----------|---------------|--------------------------------|
| RETAIL BLDG. GRADE BMS. & SLAB-ON-GRADE | 4000 | .45 | 0 |
| PAVILION GRADE BMS. & SLAB-ON-GRADE | 4000 | .45 | 8 |
| RETAIL & PAVILION COLUMNS & BEAMS | 5000 | .40 | 8 |
- 3.04 THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE STRUCTURAL ENGINEER. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S.
- 3.05 USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.
- 3.06 DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 315-80, DETAILING MANUAL. SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
- 3.07 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- 3.08 WELDED WIRE FABRIC (MESH) SHALL CONFORM TO ASTM A185 AND SHALL BE PROVIDED IN FLAT SHEETS (ROLLS NOT PERMITTED). LAP EDGES 3 CROSS WIRES MINIMUM.
- 3.09 PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE FULL TENSION SPLICES (CLASS "B") UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT AND SHALL BE LAPPED WITH FULL TENSION SPLICES (CLASS "B") UNLESS NOTED OTHERWISE. TERMINATE BARS WITH STANDARD HOOKS.
- 3.10 DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
- 3.11 ALL REINFORCING STEEL PLACEMENT SHALL BE REVIEWED BY THE ENGINEER OF RECORD, OR BY A REPRESENTATIVE RESPONSIBLE TO HIM. (RE: ACI 318-89, 1.3.1)
- 3.12 FOR CONCRETE PADS SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.
- 3.13 PROVIDE 3/4" CHAMFER AT ALL CORNERS OF CAST-IN-PLACE CONCRETE BEAMS, COLUMNS, AND WALLS EXPOSED TO VIEW.
- 3.14 ALL CONCRETE WORK EXPOSED TO VIEW (COLUMNS, BEAMS, AND WALLS) SHALL RECEIVE A SMOOTH FORMED FINISH AND A SMOOTH RUBBED FINISH AS DEFINED IN THE PROJECT SPECIFICATIONS.

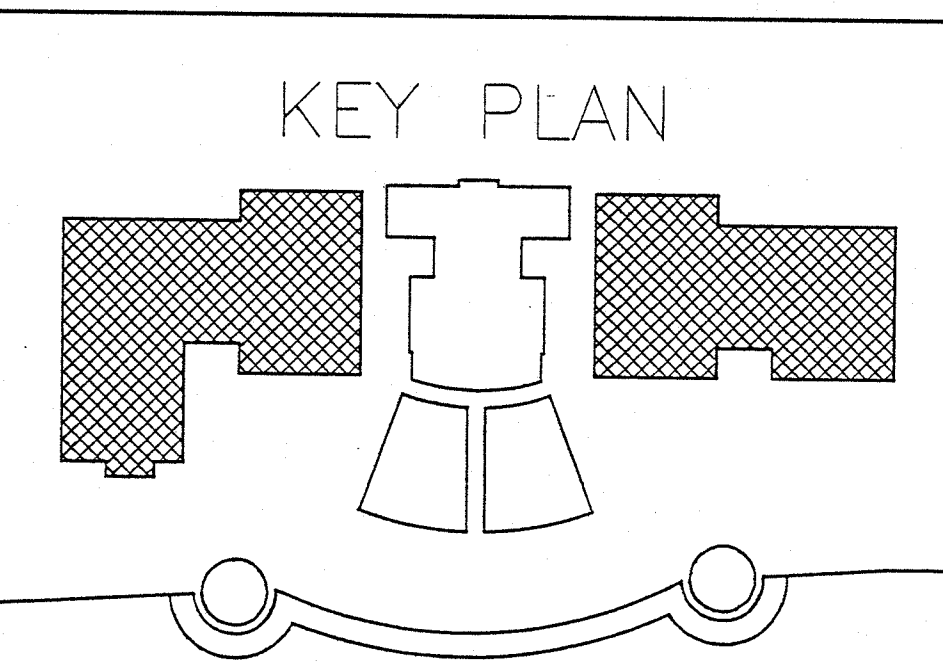
- 4.05 PROVIDE GROUT FOR REINFORCED MASONRY IN ACCORDANCE WITH ASTM C476 WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI UNLESS NOTED OTHERWISE.
- 4.06 PROVIDE HORIZONTAL JOINT REINFORCEMENT COMPLYING WITH ASTM A82, NO. 9 GAUGE OR HEAVIER, ZINC COATED, PLACED 16 INCHES ON CENTER UNLESS NOTED OTHERWISE.
- 4.07 PROVIDE MINIMUM 2 #5 VERTICAL, GROUDED FULL HEIGHT, AT EACH SIDE OF OPENINGS AND WALL ENDS. PROVIDE #5 @ 48 VERTICAL IN CELLS GROUDED SOLID IN ALL LOADBEARING WALLS.
- 4.08 PROVIDE RUNNING BONDS WITH VERTICAL JOINTS LOCATED AT CENTER OF MASONRY UNITS IN THE ALTERNATE COURSE BELOW, UNLESS NOTED OTHERWISE.
- 4.09 TOPS OF ALL MASONRY WALLS SHALL BE ANCHORED TO STRUCTURE BY DOVETAIL ANCHORS, METALS STRAPS, OR EQUIVALENT. CONTRACTOR SHALL SUBMIT DETAILS OF PREFERRED ANCHORING METHOD FOR REVIEW AND ACCEPTANCE.
- 4.10 USE DEVICES TO KEEP VERTICAL BARS CENTERED IN CELLS.
- 4.11 AT THE TIME OF LAYING, ALL MASONRY UNITS SHALL BE FREE OF EXCESSIVE DUST AND DIRT.
- 4.12 CLEANOUT CELLS SHALL BE PROVIDED AT THE BOTTOM OF ALL CELLS TO BE FILLED AT EACH POUR OF GROUT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. THE CLEANOUTS SHALL BE SEALED BEFORE GROUTING, AND AFTER INSPECTION.
- 4.13 VERTICAL STEEL REINFORCING BARS SHALL EXTEND A MINIMUM OF 40 BAR DIAMETERS ABOVE THE TOP COURSE TO BE POURED. REINFORCING BARS IN THE NEXT MASONRY LIFT SHALL LAP THE DOWELS FOR THE FULL 40 BAR DIAMETERS. TERMINATE BARS WITH STANDARD HOOKS.
- 4.14 ALL MASONRY BEARING WALLS SHALL BE SHORED TO BRACE THE WALLS AGAINST EARTH PRESSURE, WIND FORCES AND CONSTRUCTION LOADINGS. THESE SHORES SHALL REMAIN IN PLACE UNTIL SLABS AND ROOFS ARE IN PLACE. CONTRACTOR TO SUBMIT SHORING AND BRACING DRAWINGS.
- 4.15 FILL ALL CELLS BELOW GRADE WITH GROUT.
- 4.16 DEFORMED BAR ANCHORS (DBA) FOR ANCHORING STRUCTURAL STEEL AND PLATES TO MASONRY SHALL BE MADE FROM COLD DRAWN WIRE CONFORMING TO ASTM A-496 WITH fy = 70 ksi. FILL ALL MASONRY CELLS WITH GROUT AT ANCHOR LOCATIONS.
- 4.17 PROVIDE BOND BEAMS WITH 2 #5 CONTINUOUS IN 8" CMU WALLS AT TOP AND MID HEIGHT OF WALL AND AT EQUAL SPACES BETWEEN NOT TO EXCEED 12'-0" CENTER TO CENTER, U.N.O. PROVIDE A MINIMUM OF 16" BEARING ON EACH SIDE OF OPENING FOR ALL THE BOND BEAMS.

- 6.00 TIMBER DECKING**
- 6.01 RETAIL BUILDINGS**
- TONGUE AND GROOVE DECKING SHALL BE 2X6 NOMINAL (AS NOTED ON ROOF FRAMING PLANS) SOUTHERN PINE SELECT DECKING SURFACED AT 15% MOISTURE CONTENT, K.D. JOINTS IN DECKING MAY BE RANDOMLY SPACED, PROVIDED THE SYSTEM IS APPLIED TO NOT LESS THAN THREE CONTINUOUS SPANS, PLANKS ARE CENTER MATCHED AND END MATCHED OR SPLINED, EACH PLANK BEARS ON AT LEAST ONE SUPPORT, AND JOINTS ARE SEPARATED BY AT LEAST 24" IN ADJACENT PLANKS. JOINTS WITHIN 8' OF BEING IN LINE EACH WAY MUST BE SEPARATED BY AT LEAST TWO INTERVENING COURSES. DECKING IS TO BE INSTALLED WITH TONGUES UP ON SLOPED OR PITCHED ROOFS. IT IS TO BE LAID WITH THE PATTERN FACES DOWN AND EXPOSED ON THE UNDERSIDE. DECKING SHOULD BE TOENAILED AT EACH SUPPORT WITH ONE 16d NAIL AND FACE NAILED WITH ONE 16d NAIL.
- 6.02 PAVILION BUILDING**
- ROOF DECKING SHALL BE LAMINATED 3X6 NOMINAL (2 3/16" ACTUAL) INLAND RED CEDAR SELECT DECKING AS MANUFACTURED BY POTLATCH (OR EQUIV.). ATTACHMENT OF DECKING SHALL BE IN STRICT CONFORMANCE TO MANUFACTURER'S RECOMMENDATIONS.
- 7.00 TIMBER BEAMS AND TRUSSES**
- 7.01 RETAIL BUILDINGS**
- EXPOSED TIMBER BEAMS AND TRUSSES SHALL BE NO.1 SOUTHERN PINE SURFACED AT 15% MOISTURE CONTENT, K.D., UNLESS NOTED OTHERWISE.
- 7.02 PAVILION BUILDING**
- TIMBER BEAMS AND TRUSSES SHALL BE NO. 2 WESTERN RED CEDAR AT 19% MOISTURE CONTENT, MAXIMUM. EACH CEDAR MEMBER SHALL BE FREE OF KNOTS IN LOWER THIRD AND UPPER THIRD OF THE MEMBER'S WIDTH (I.e. KNOTS WILL ONLY BE ACCEPTABLE WHEN LOCATED WITHIN CENTER ONE-THIRD OF MEMBER WIDTH). CEDAR TIMBERS ARE SUBJECT TO INSPECTION BY PROJECT STRUCTURAL ENGINEER.

THESE DESIGN PLANS AND SPECIFICATIONS ARE IN COMPLIANCE WITH THE STANDARDS ESTABLISHED IN SECTION 16B-33.007, FLORIDA ADMINISTRATIVE CODE.

THE MAIN WIND-FORCE RESISTING SYSTEM HAS BEEN DESIGNED IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990) "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.

THE COMPONENTS AND CLADDING HAVE BEEN SELECTED AND THEIR USE INCORPORATED INTO THE DESIGN AND SPECIFICATIONS IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990) "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.



- 2.00 FOUNDATIONS AND SLAB-ON-GROUND**
- 2.01 THE DESIGN OF FOUNDATIONS, RETAINING WALLS AND SLAB ON GRADE IS BASED ON THE GEOTECHNICAL REPORT #94-193 DATED MAY 31, 1994, BY LARRY M. JACOBS AND ASSOCIATES, INC. THE RECOMMENDATIONS IN THAT REPORT SHALL BE CONSIDERED AN INTEGRAL PART OF THE CONTRACT DOCUMENTS.
- 2.02 A QUALIFIED GEOTECHNICAL ENGINEER SHALL VERIFY CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACKFILLS, ETC. ALL FOOTINGS SHALL REST EITHER ON UNDISTURBED SOIL OR A MANUALLY OPERATED VIBRATORY SLED OR TAMPER SHOULD BE USED TO DENSIFY ANY SOILS IN THE BOTTOM OF THE FOOTING TRENCHES LOOSENED DURING THE EXCAVATION OPERATION.
- 2.03 SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS POURED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY GEOTECHNICAL ENGINEER AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT.
- 3.00 REINFORCED CONCRETE**
- 3.01 ALL CONCRETE WORK SHALL CONFORM TO ACI 301-89, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. DESIGN IS BASED ON ACI 318-89, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
- 4.00 MASONRY
- 4.01 CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO: ACI 530-88, BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES AND ACI 530.1-88, SPECIFICATIONS FOR CONCRETE MASONRY CONSTRUCTION.
- 4.02 PROVIDE LIGHTWEIGHT, HOLLOW, LOAD-BEARING CONCRETE MASONRY UNITS (CMU) CONFORMING TO ASTM C90, GRADE N, TYPE 1, fc' = 1900 PSI (NET), UNLESS NOTED OTHERWISE.
- 4.03 PROVIDE MASONRY WITH MINIMUM COMPRESSIVE STRENGTH, F'm = 1,500 PSI.
- 4.04 PROVIDE TYPE "S" OR TYPE "M" MORTAR IN ACCORDANCE WITH ASTM C270, UNLESS NOTED OTHERWISE.

BASKERVILLE-DONOVAN, INC.

DESIGNED BY: LJD/KLH
DRAWN BY: KLH
CHECKED BY: KLH/LJD
DATE: 7/15/94

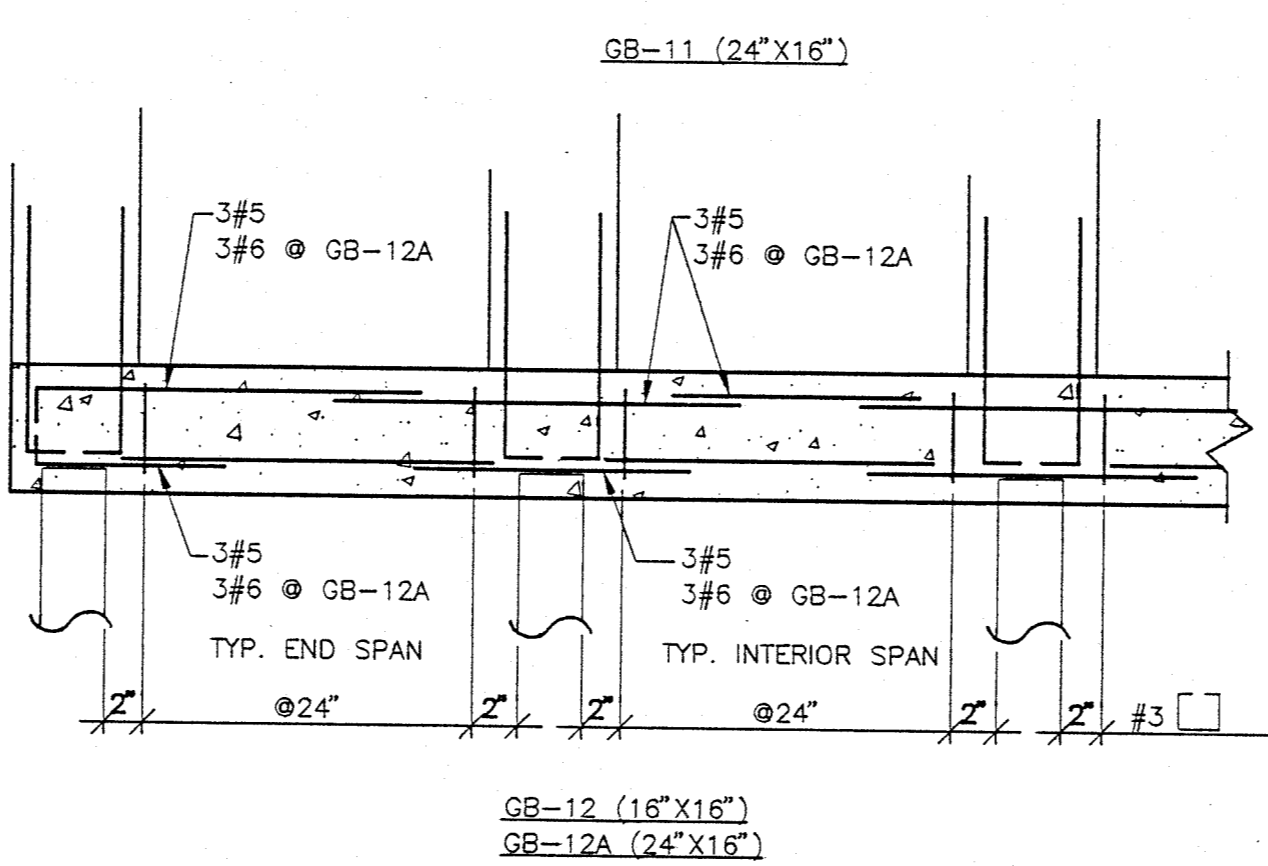
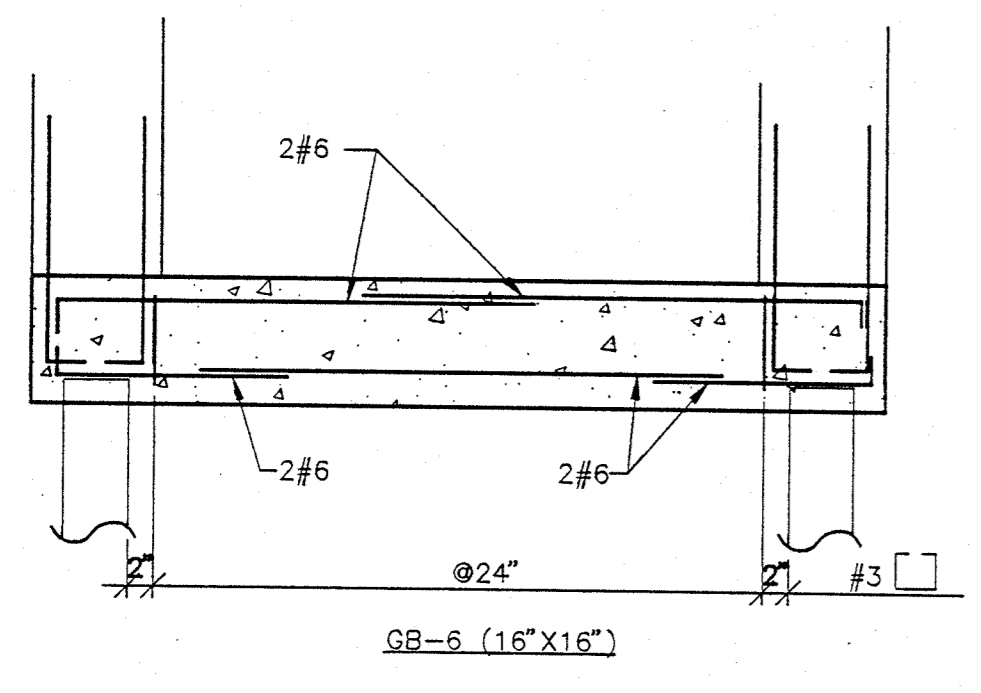
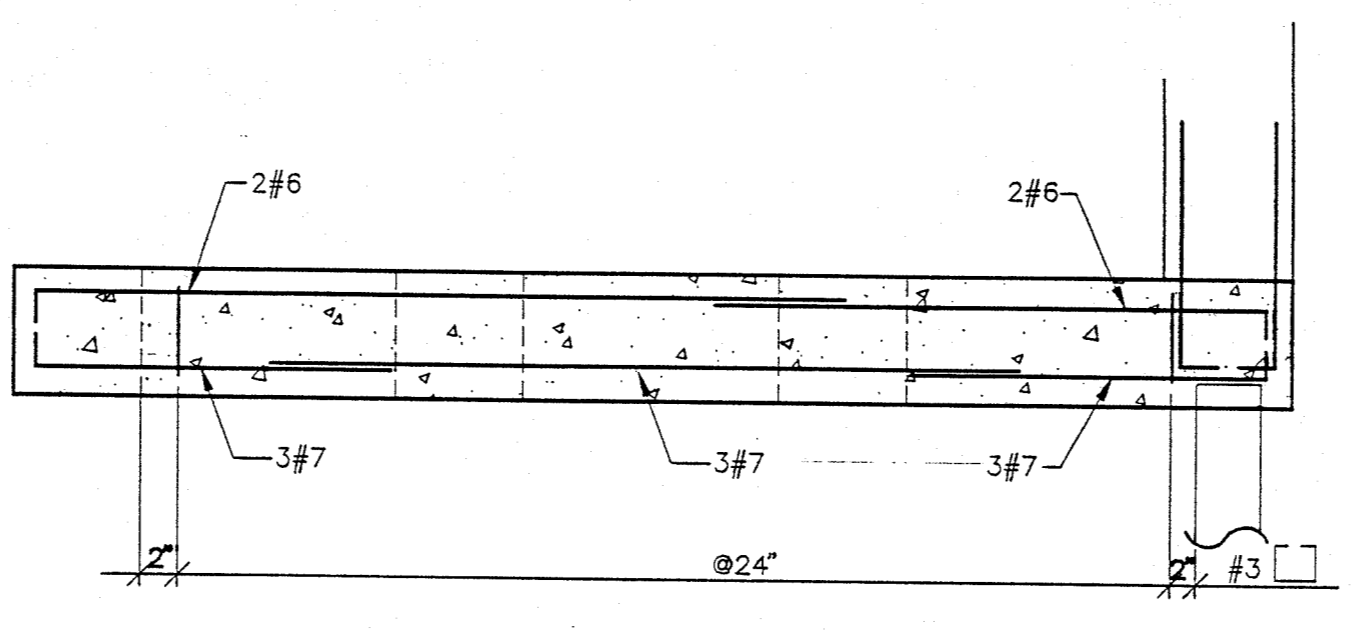
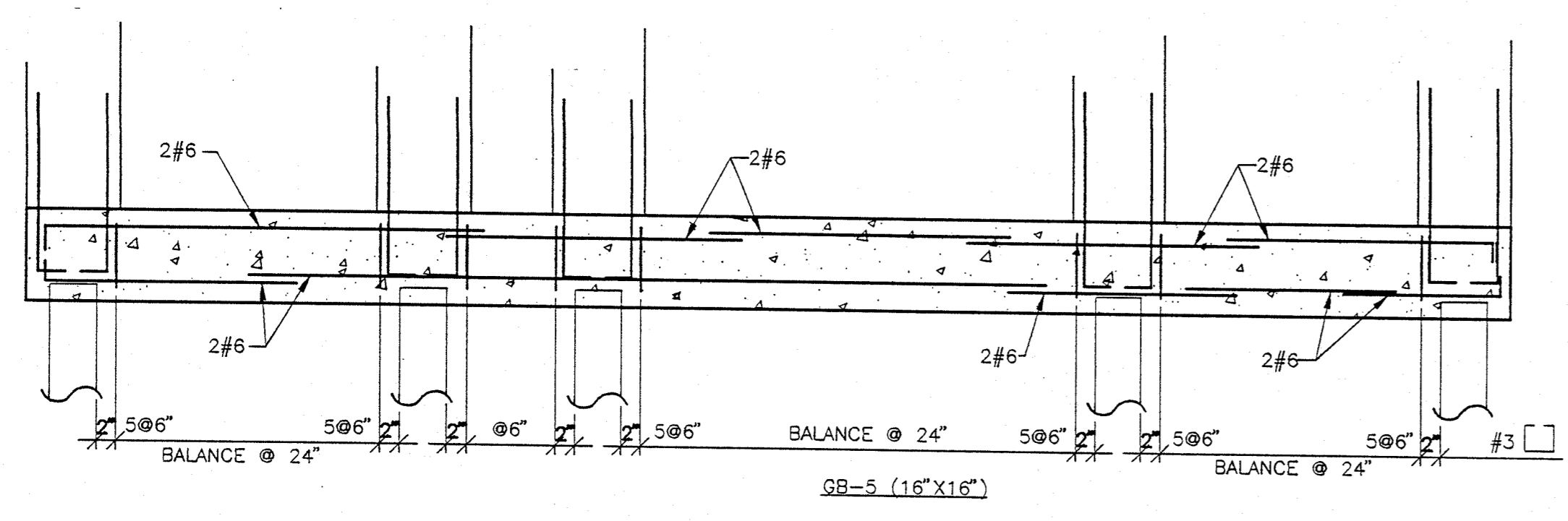
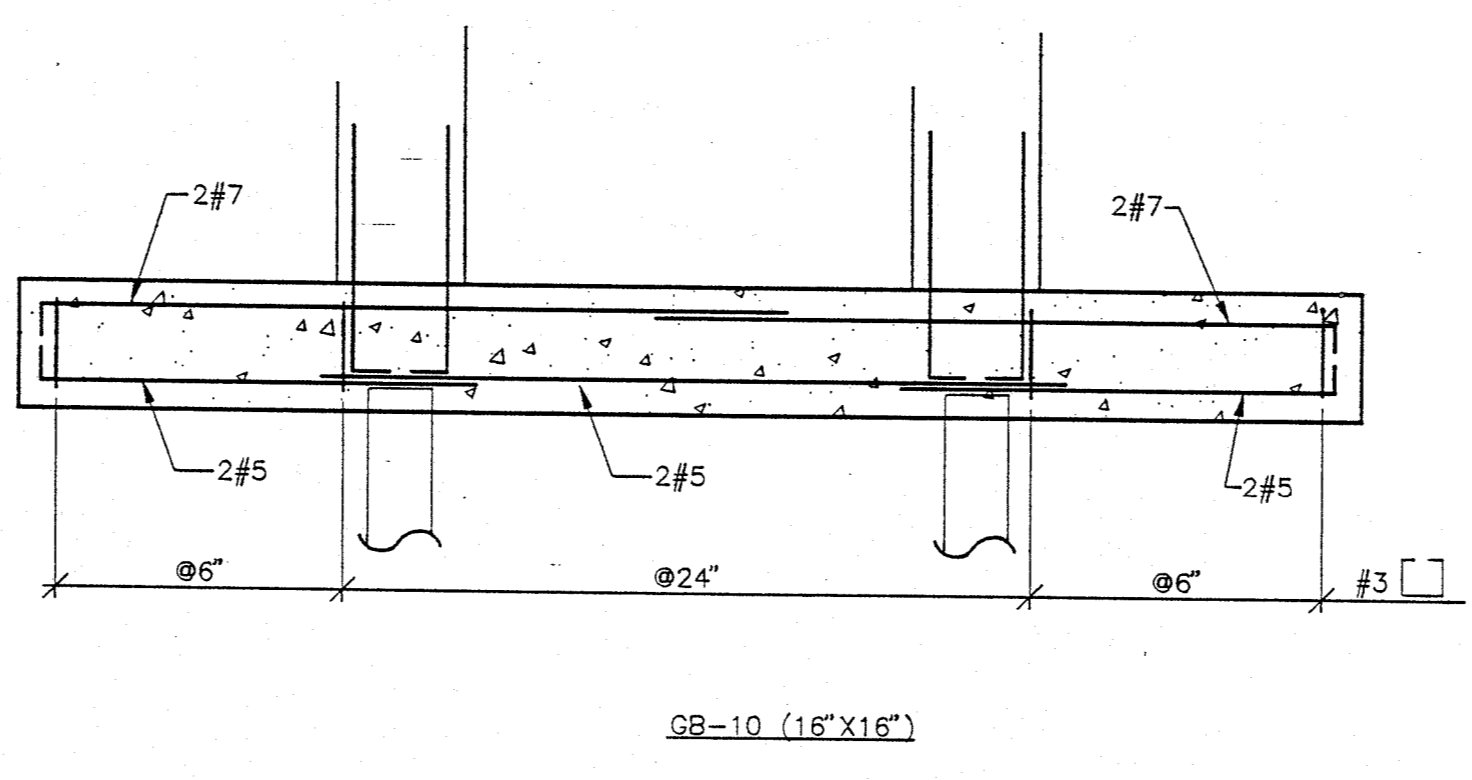
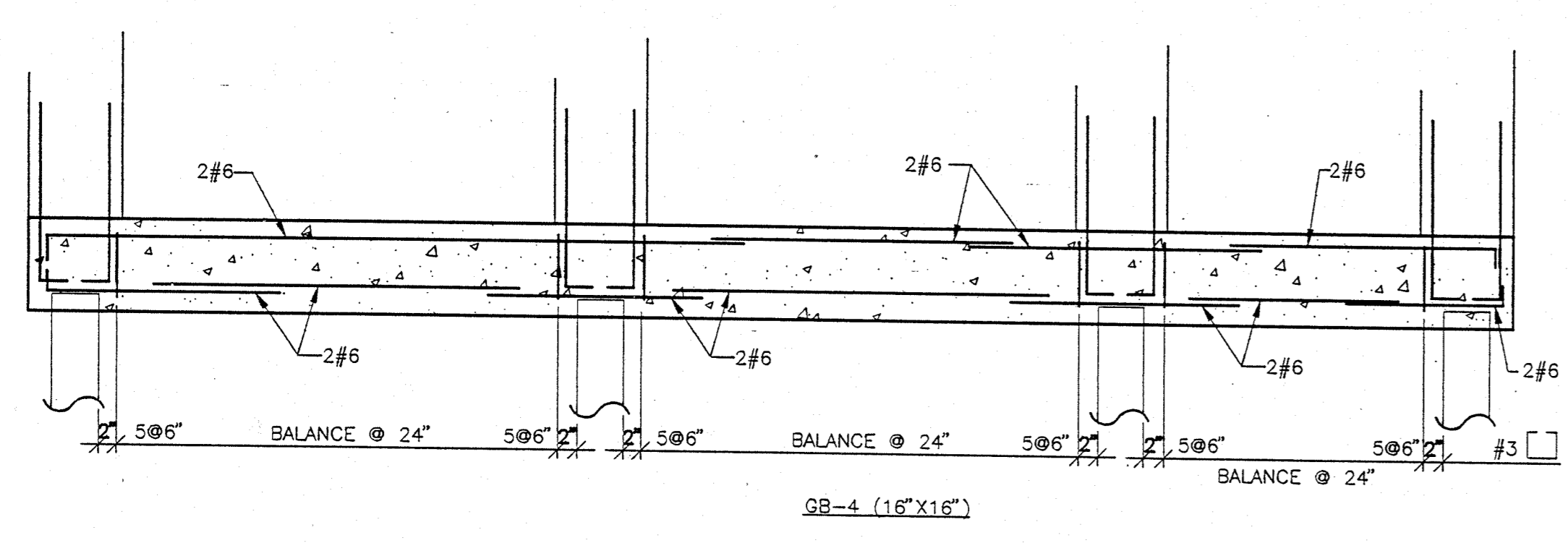
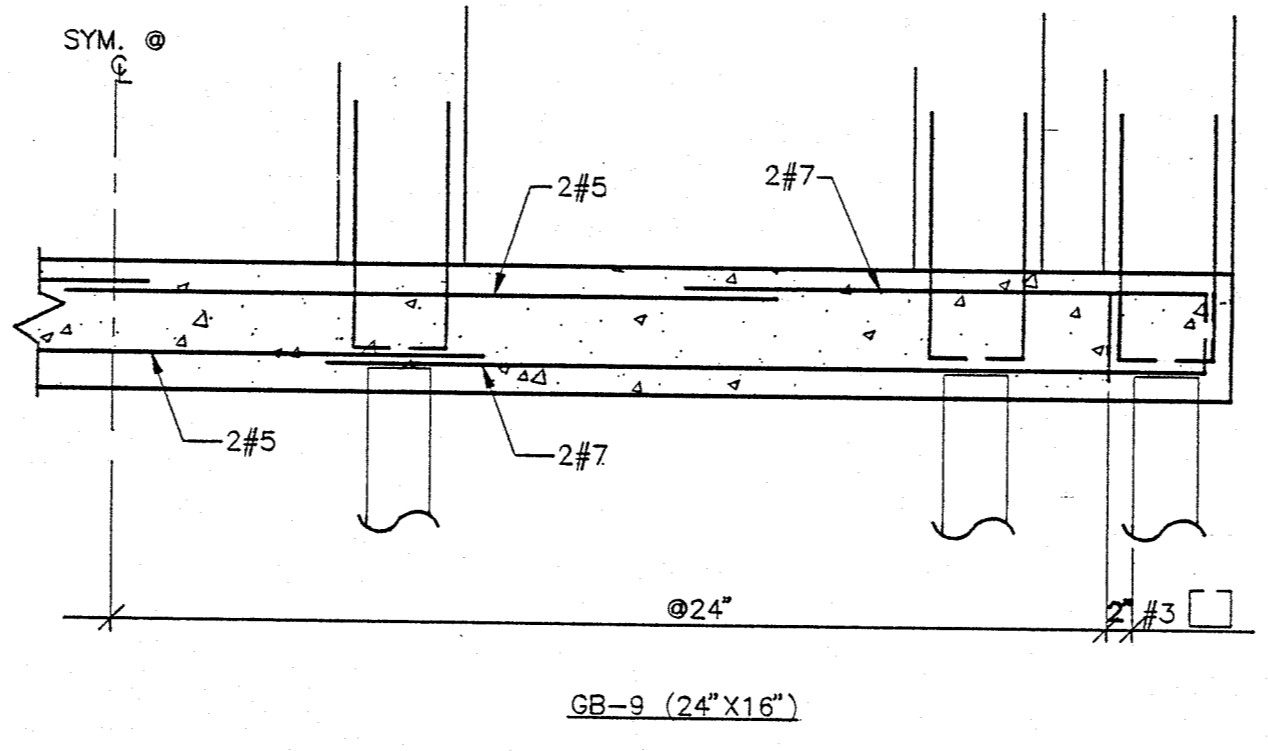
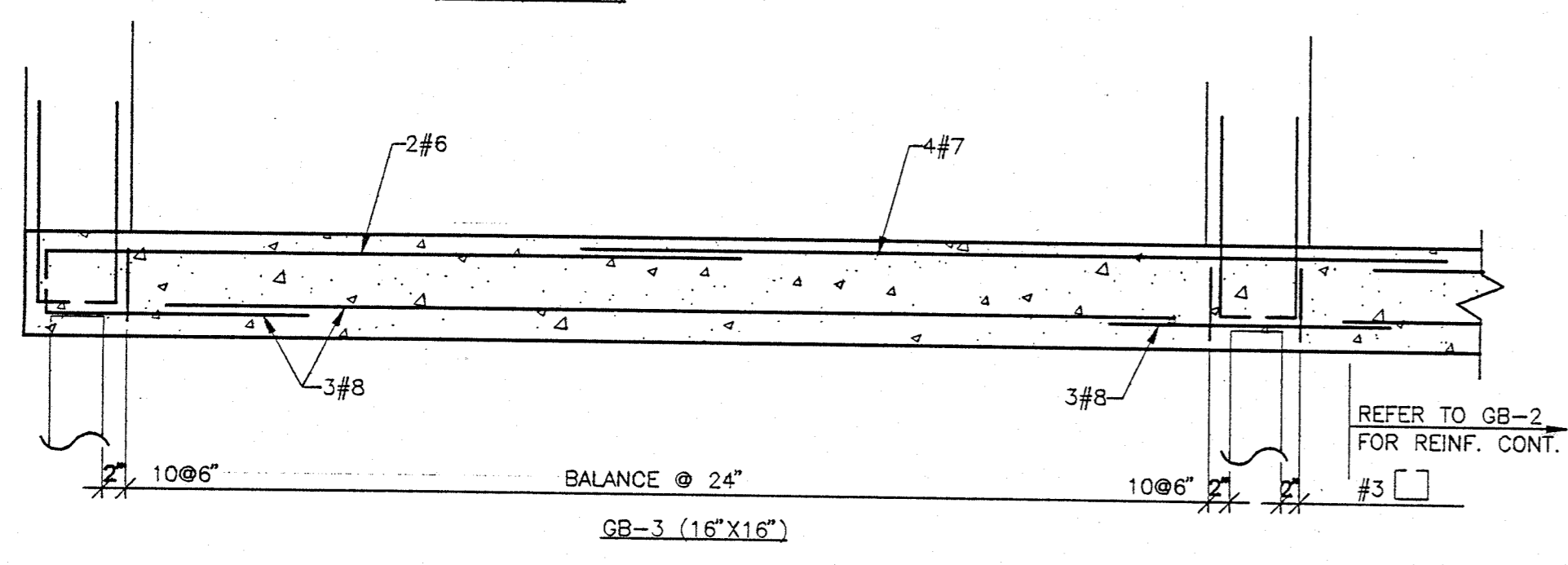
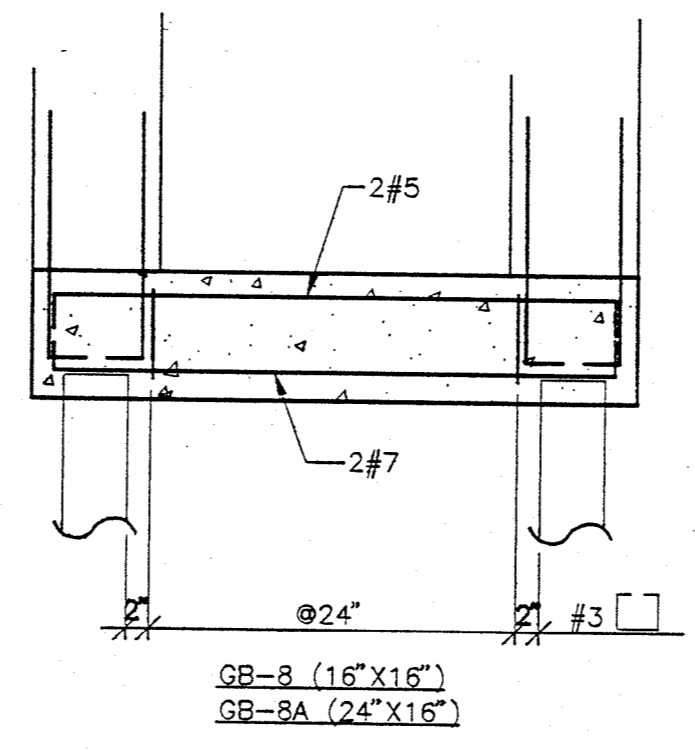
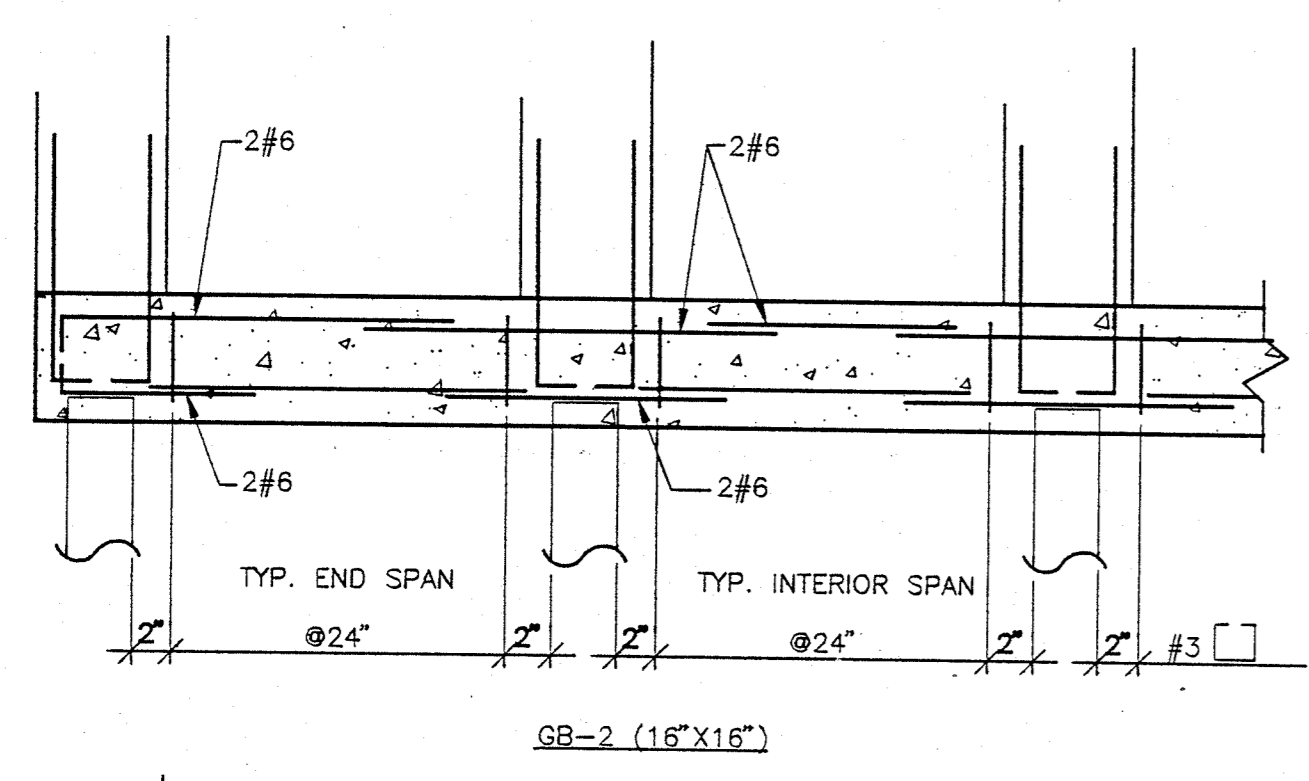
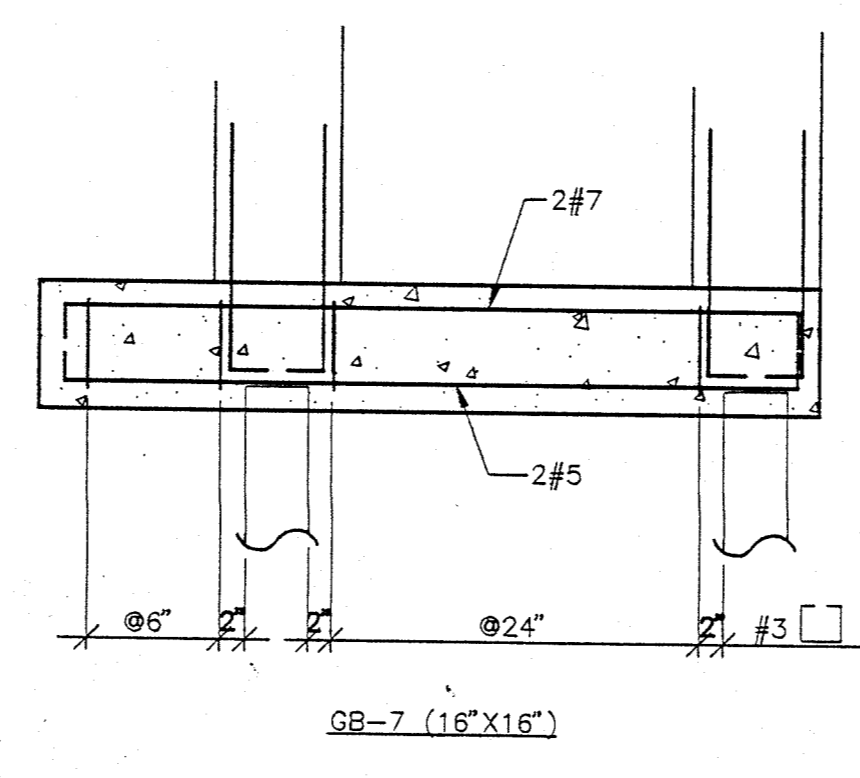
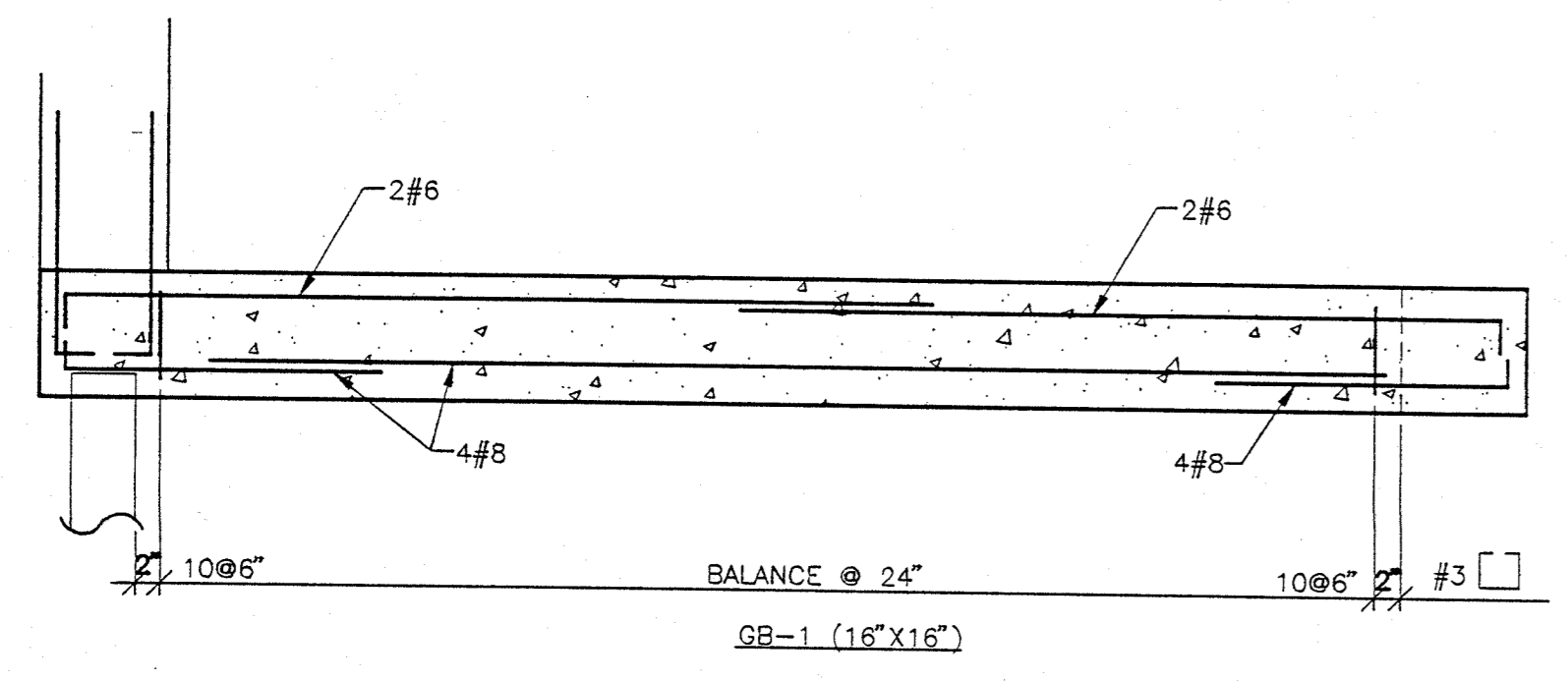
FOUNDATION PLANS & NOTES

CASINO BEACH AREA RENOVATION
PENSACOLA BEACH, FLORIDA
FOR THE SANTA ROSA ISLAND AUTHORITY

PROJECT NO. 21213.00
INDEX

REVISION

NO.	DATE	APPR.	REVISION



1 GRADE BEAM DIAGRAMS
SCALE: N.T.S.

THESE DESIGN PLANS AND SPECIFICATIONS ARE IN COMPLIANCE WITH THE STANDARDS ESTABLISHED IN SECTION 16B-33.007, FLORIDA ADMINISTRATIVE CODE.

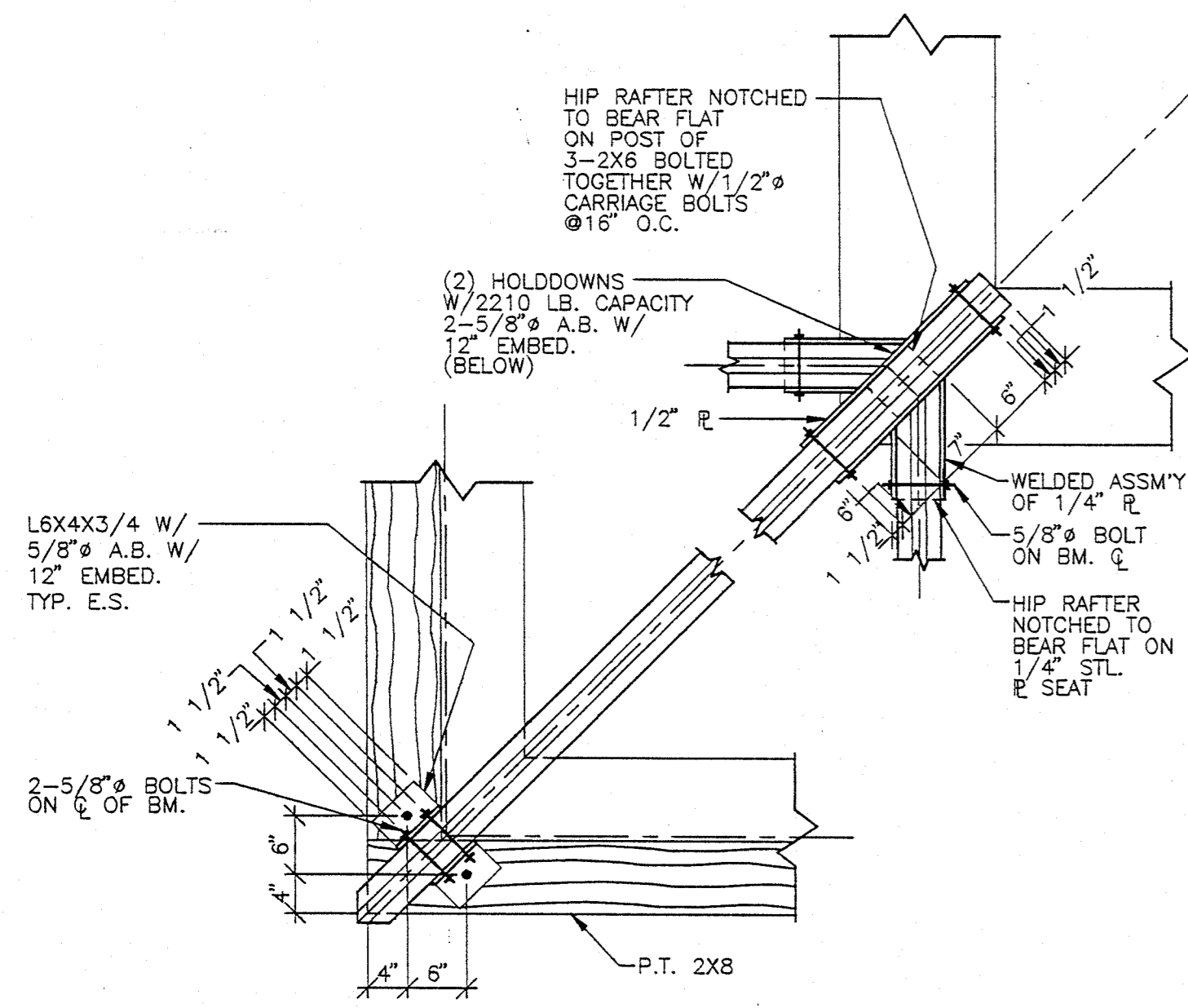
THE MAIN WIND-FORCE RESISTING SYSTEM HAS BEEN DESIGNED IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990)

MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.

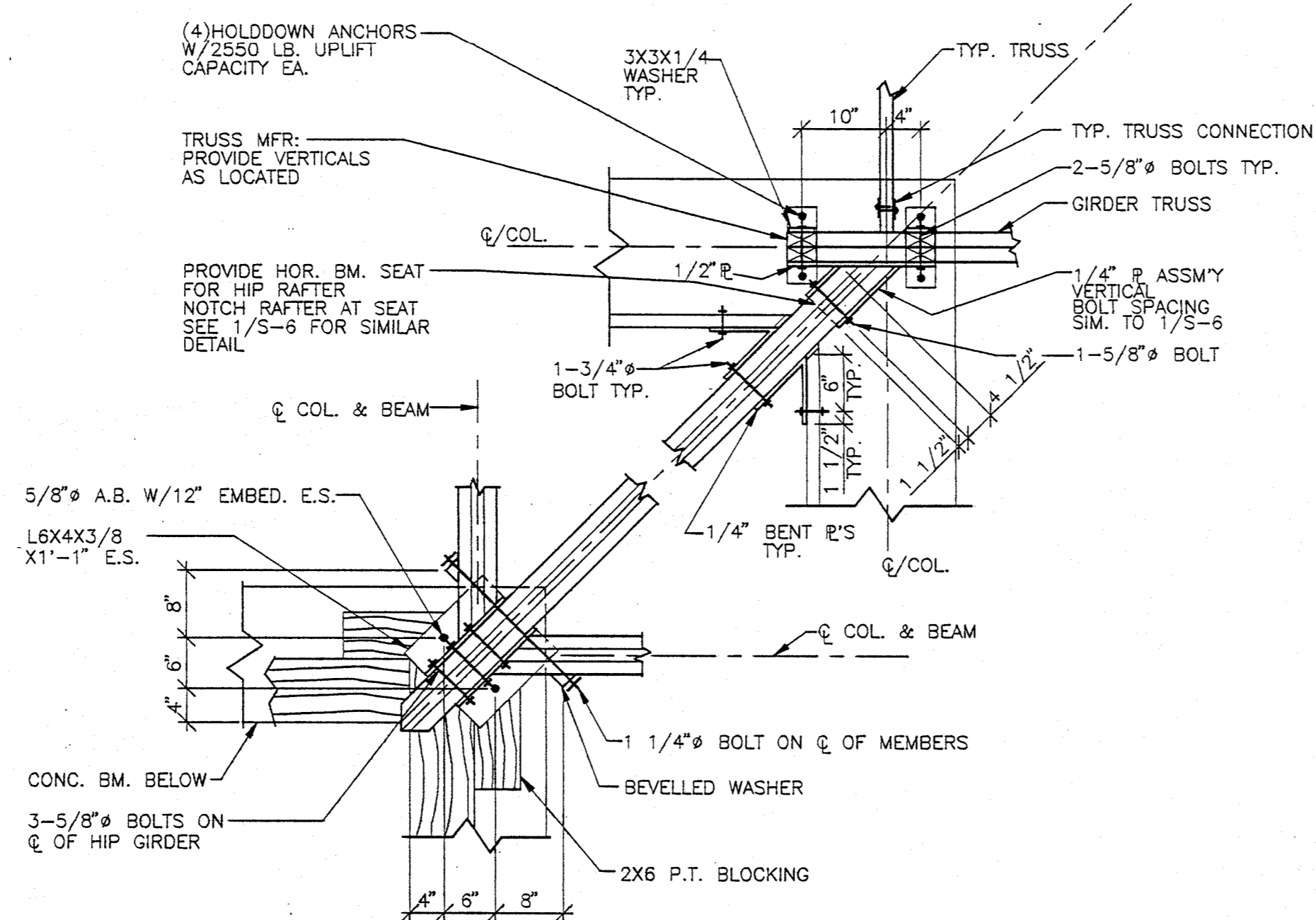
THE COMPONENTS AND CLADDING HAVE BEEN SELECTED AND THEIR USE INCORPORATED INTO THE DESIGN AND SPECIFICATIONS IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990)

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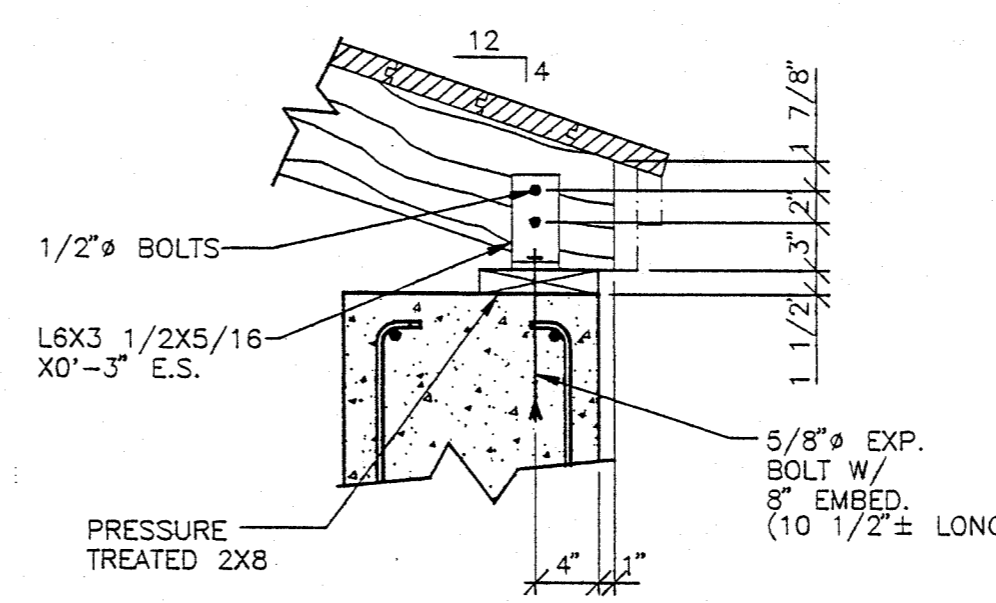
PROJECT NO. 21213.00		DATE 7/15/94	
INDEX		SHEET NO. 9-4	
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DESIGNED	DRAWN	CHECKED	DATE
LJD/KLH	KCH	KLH/LJD	7/15/94
NO.		REVISION	
DATE		APPR.	
NO.		REVISION	
DATE		APPR.	
NO.		REVISION	
DATE		APPR.	



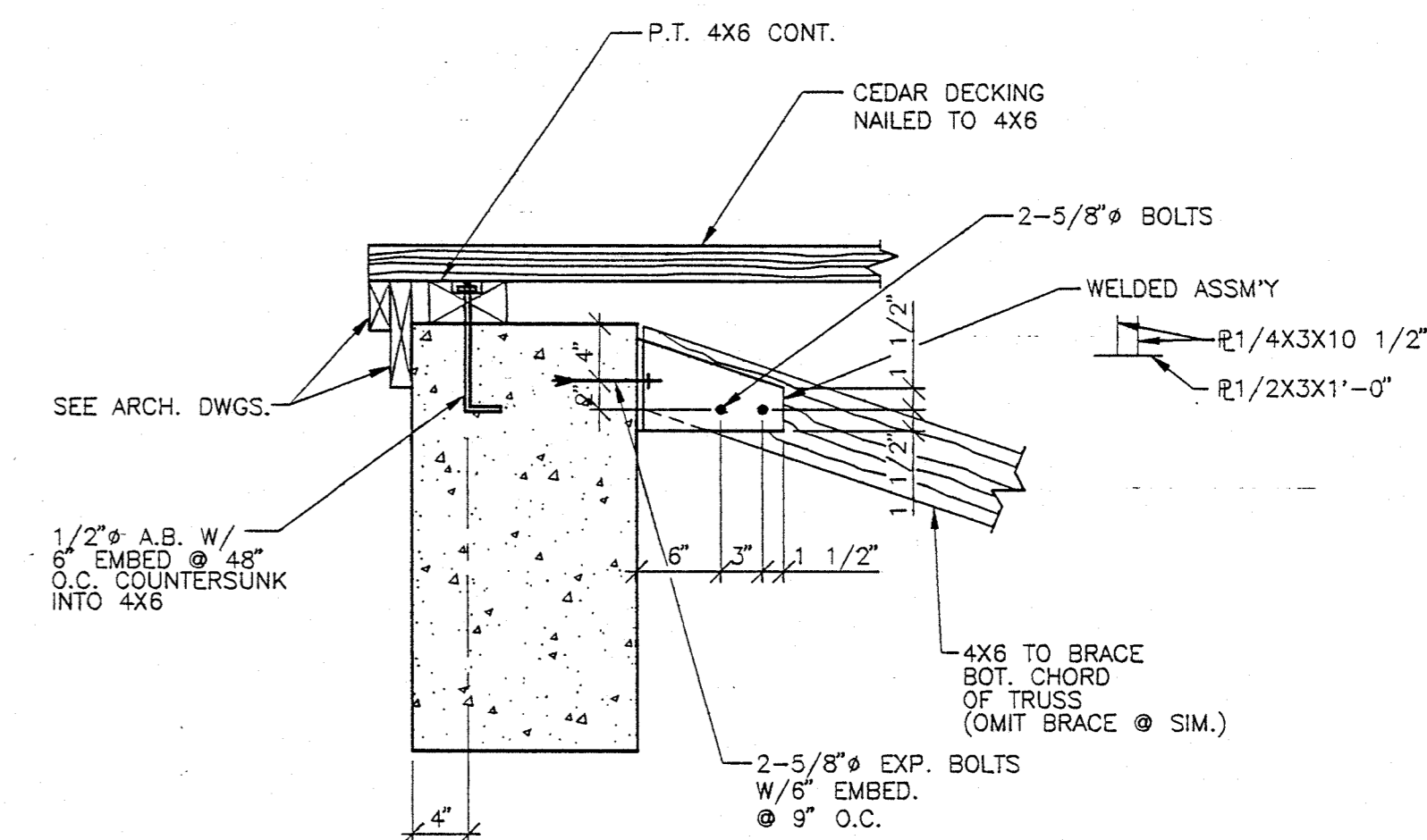
1 SECTION
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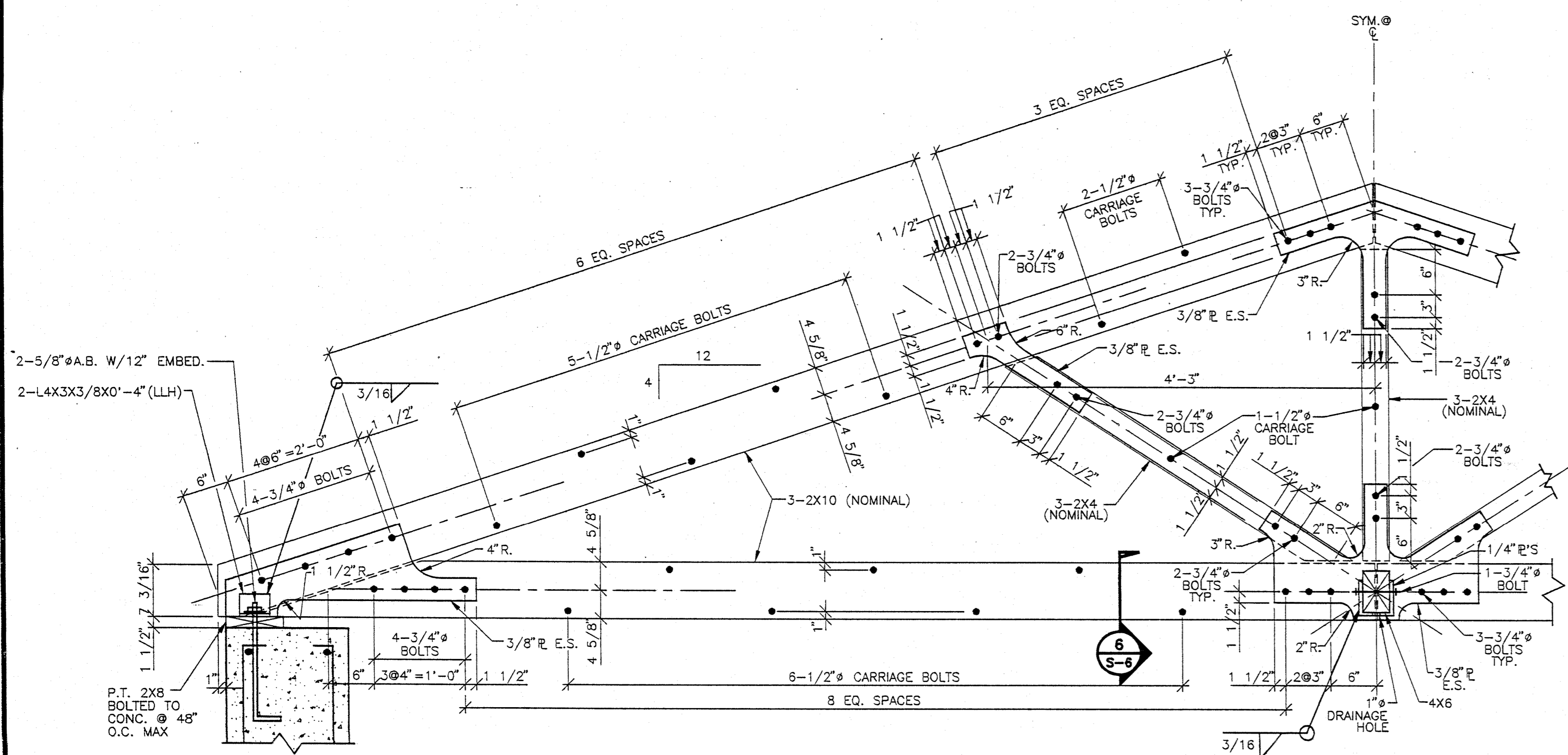
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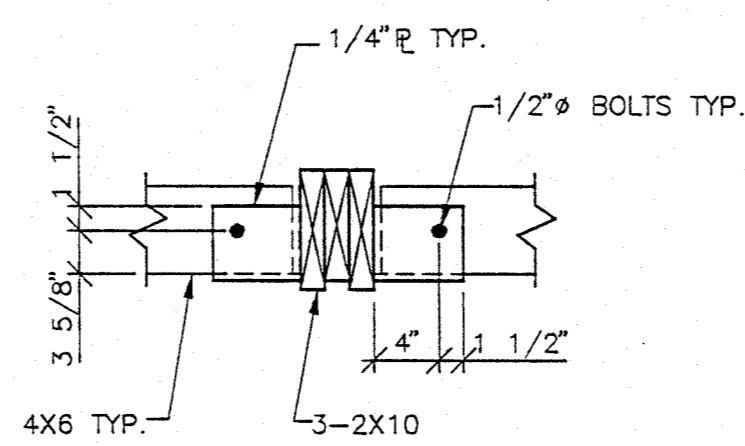
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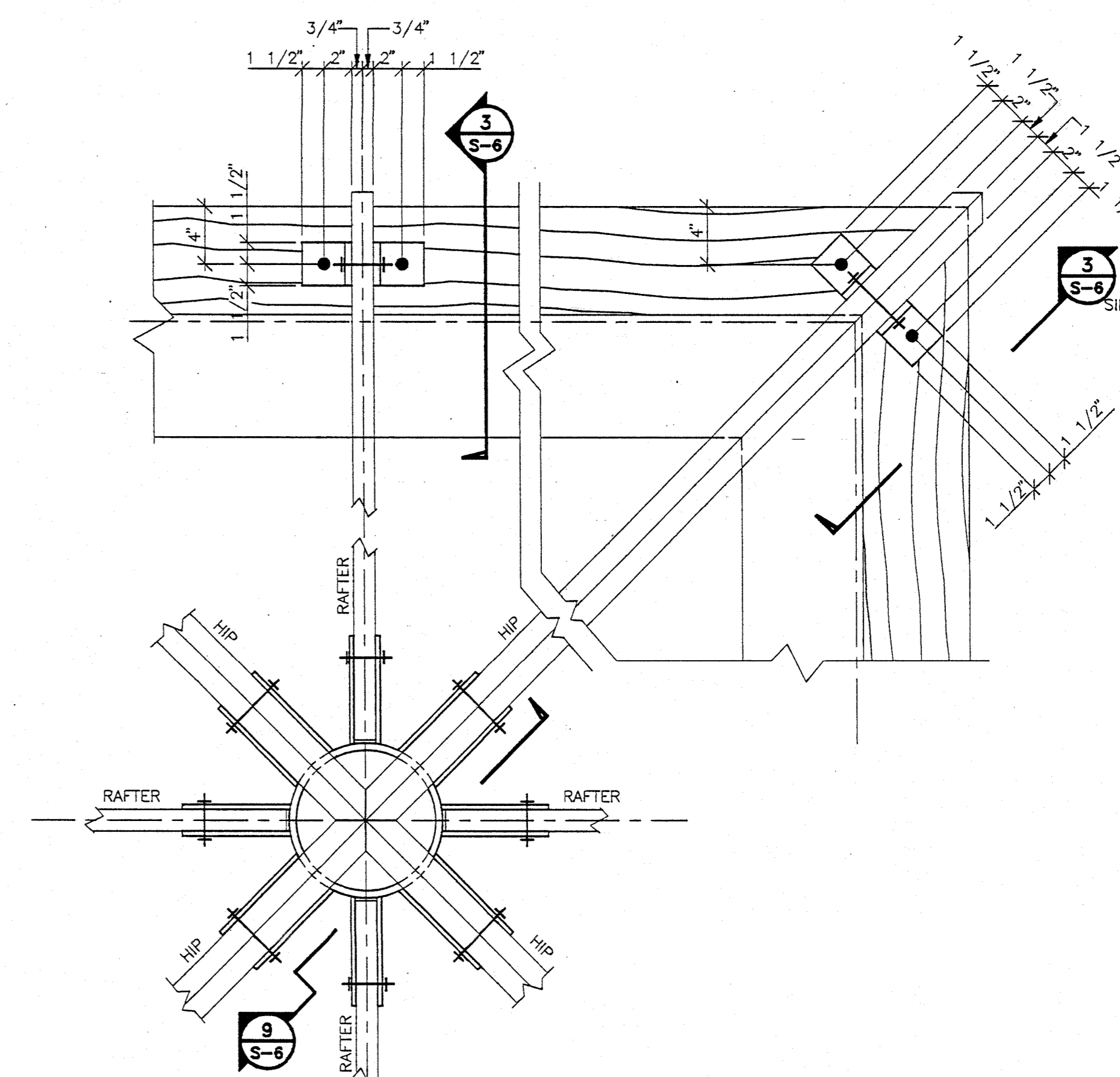
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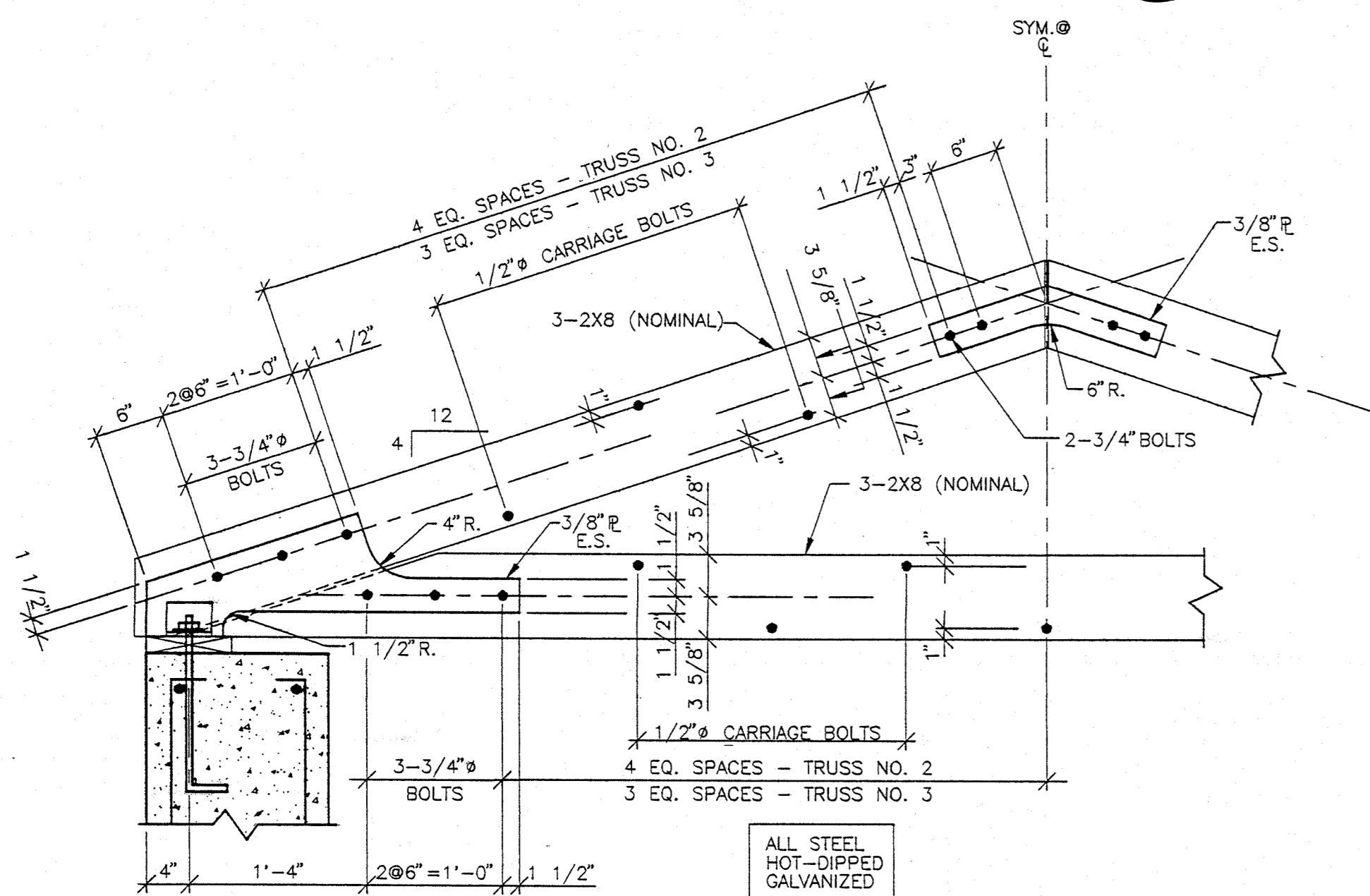
5 TRUSS NO. 1
S-6 SCALE: 1" = 1'-0"



6 SECTION
S-6 SCALE: 1" = 1'-0"



8 DETAIL
S-6 SCALE: 1 1/2" = 1'-0"

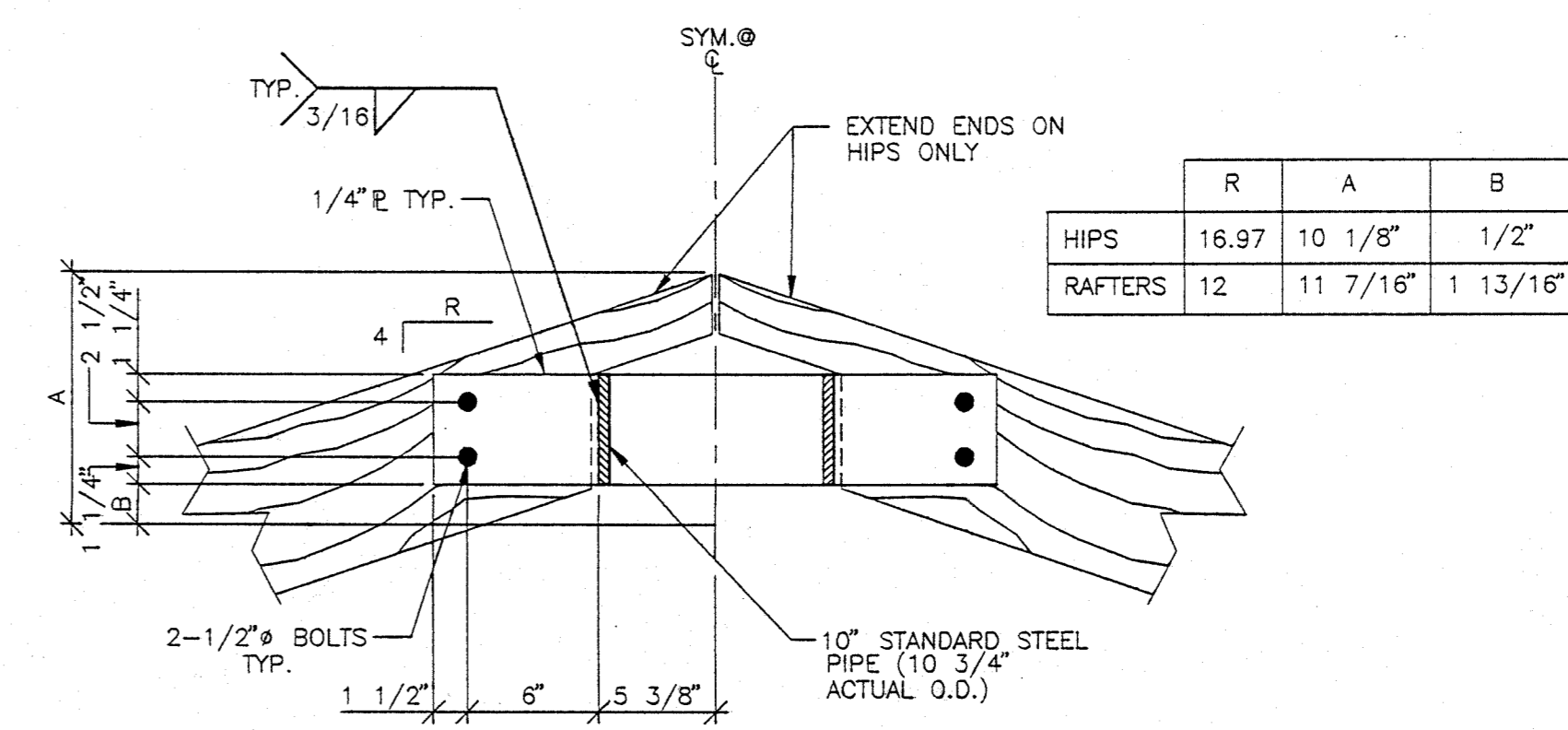


7 TRUSS NOS. 2 & 3 SIMILAR TO TRUSS NO. 1 EXCEPT AS NOTED
S-6 SCALE: 1" = 1'-0"

THESE DESIGN PLANS AND SPECIFICATIONS ARE IN COMPLIANCE WITH THE STANDARDS ESTABLISHED IN SECTION 168-33.007, FLORIDA ADMINISTRATIVE CODE.

THE MAIN WIND-FORCE RESISTING SYSTEM HAS BEEN DESIGNED IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990) "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.

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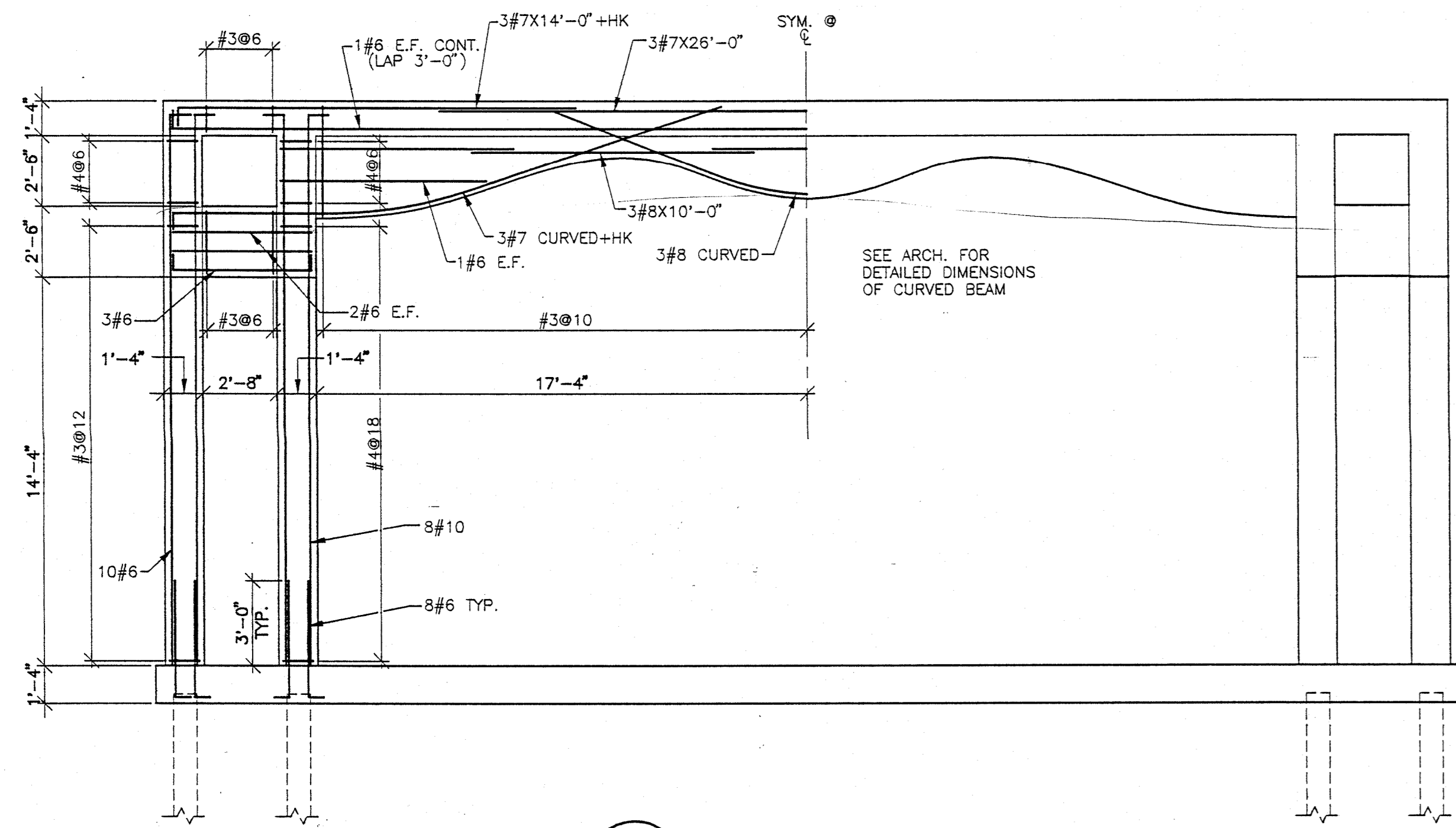
9 DETAIL
S-6 SCALE: 1 1/2" = 1'-0"

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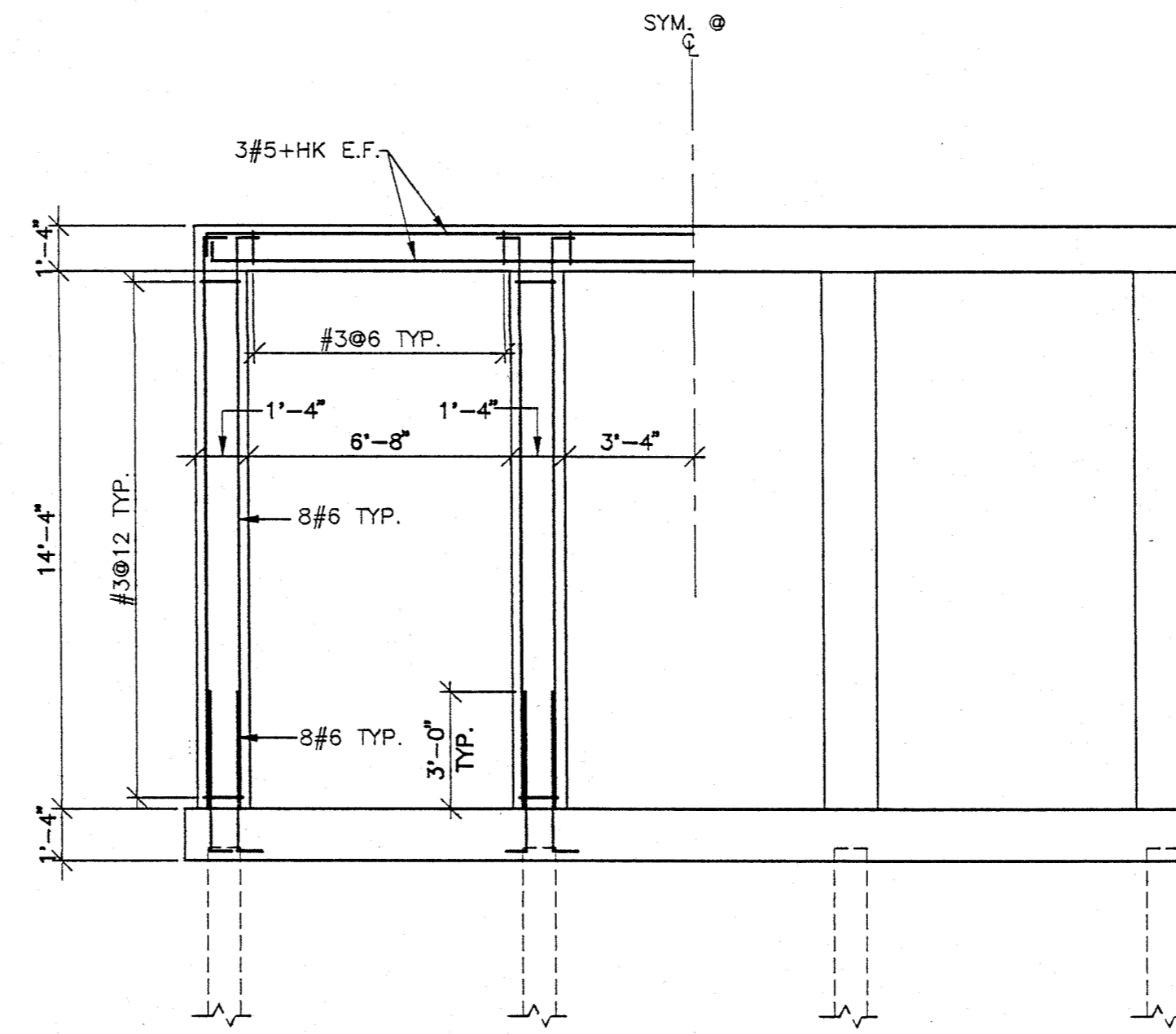
REVISION		DATE		APPR.		NO.	

BASKERVILLE-DONOVAN, INC.	DESIGNED	DRAWN	CHECKED	DATE
	LJD/KLH	KLH	KLH/LJD	7/15/94
	SCALE	1/8" = 1'-0"		

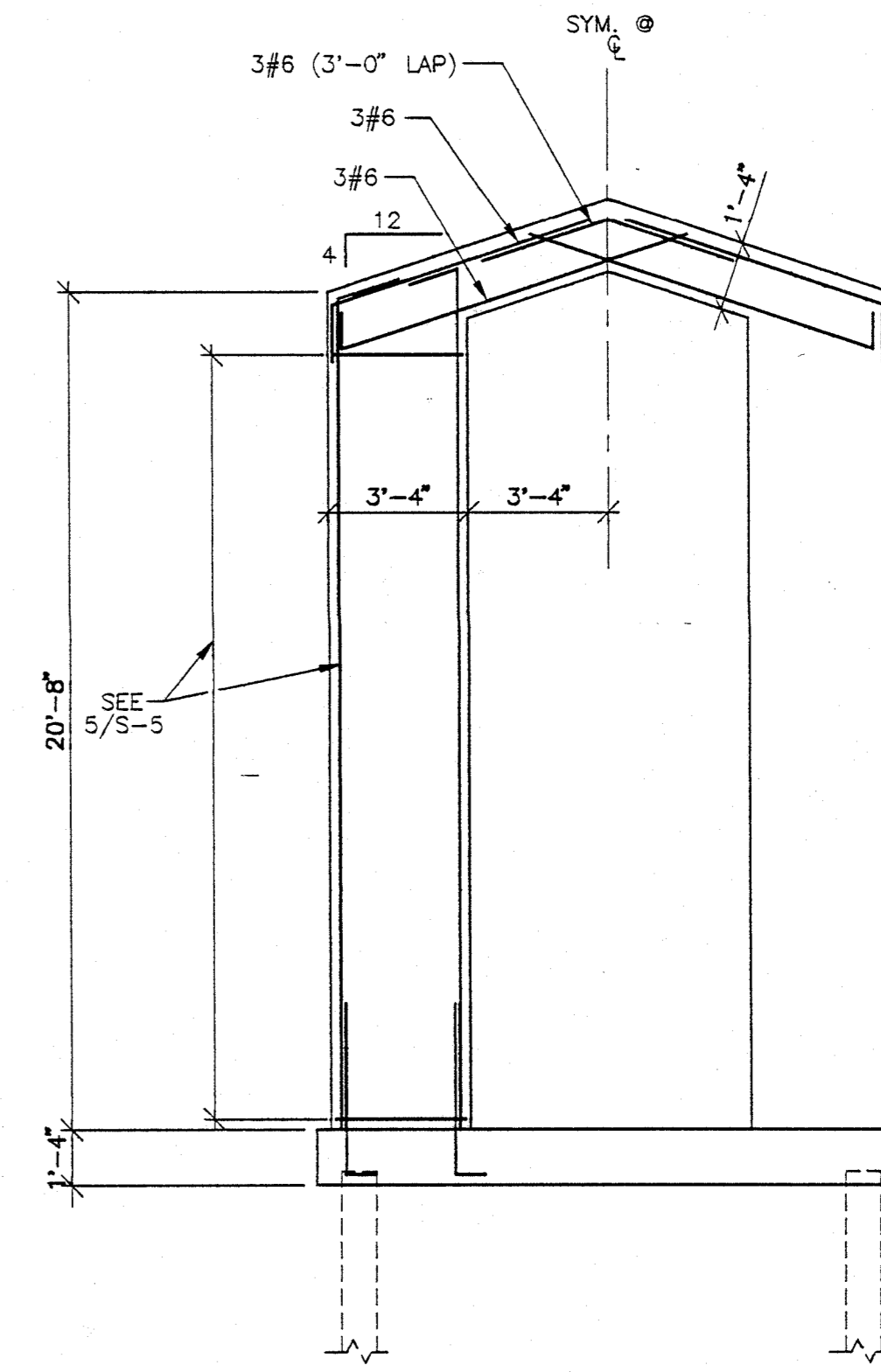
SECTIONS AND DETAILS	
CASINO BEACH AREA RENOVATION	
PENSACOLA BEACH, FLORIDA	
FOR THE SANTA ROSA ISLAND AUTHORITY	
PROJECT NO.	S-6
21213.00	9
INDEX	SHT.



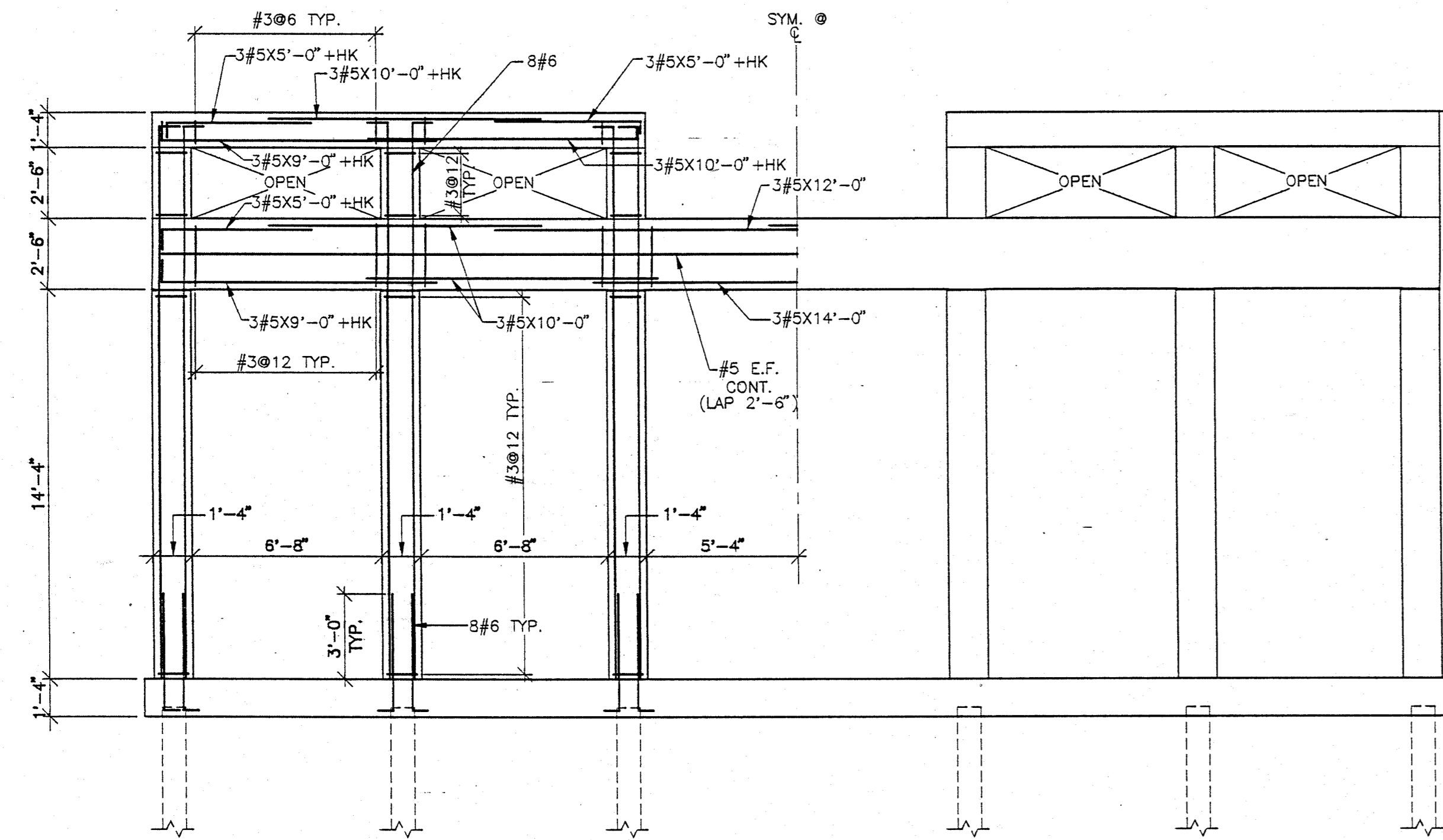
1 WALL NO. 4
S-7 SCALE: 1/4" = 1'-0"



3 WALL NO. 6
S-7 SCALE: 1/4" = 1'-0"

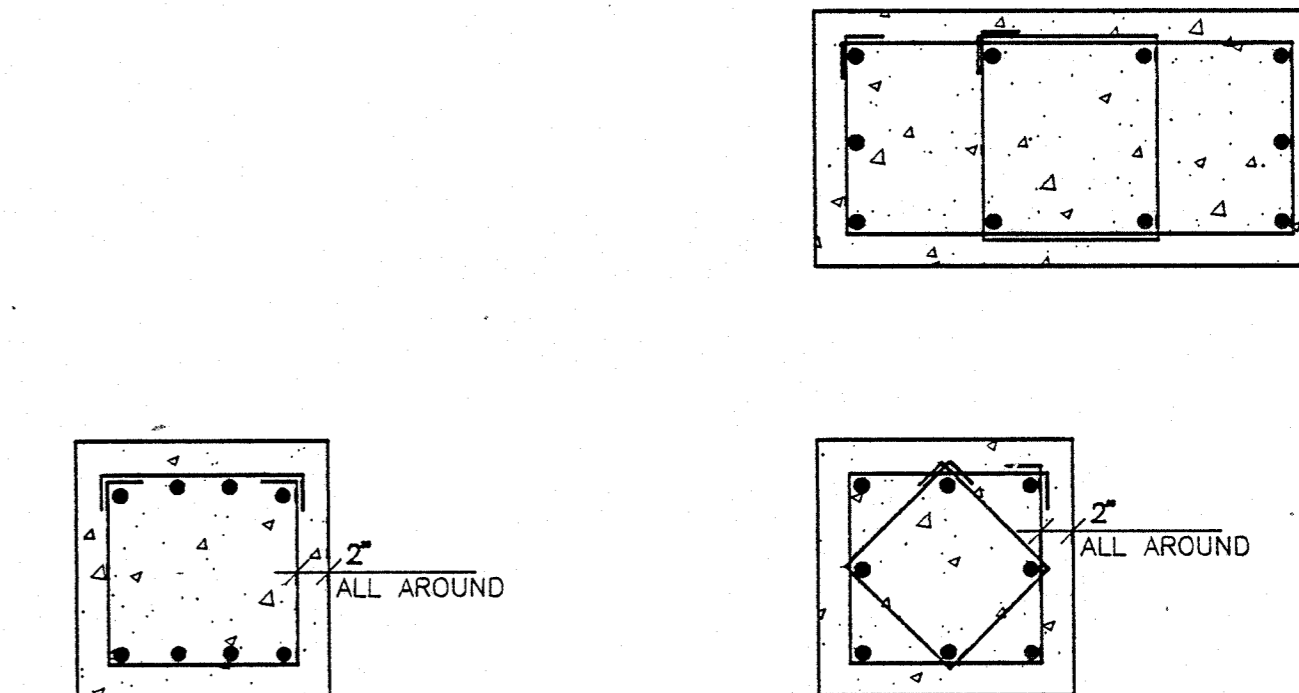


4 WALL NO. 7
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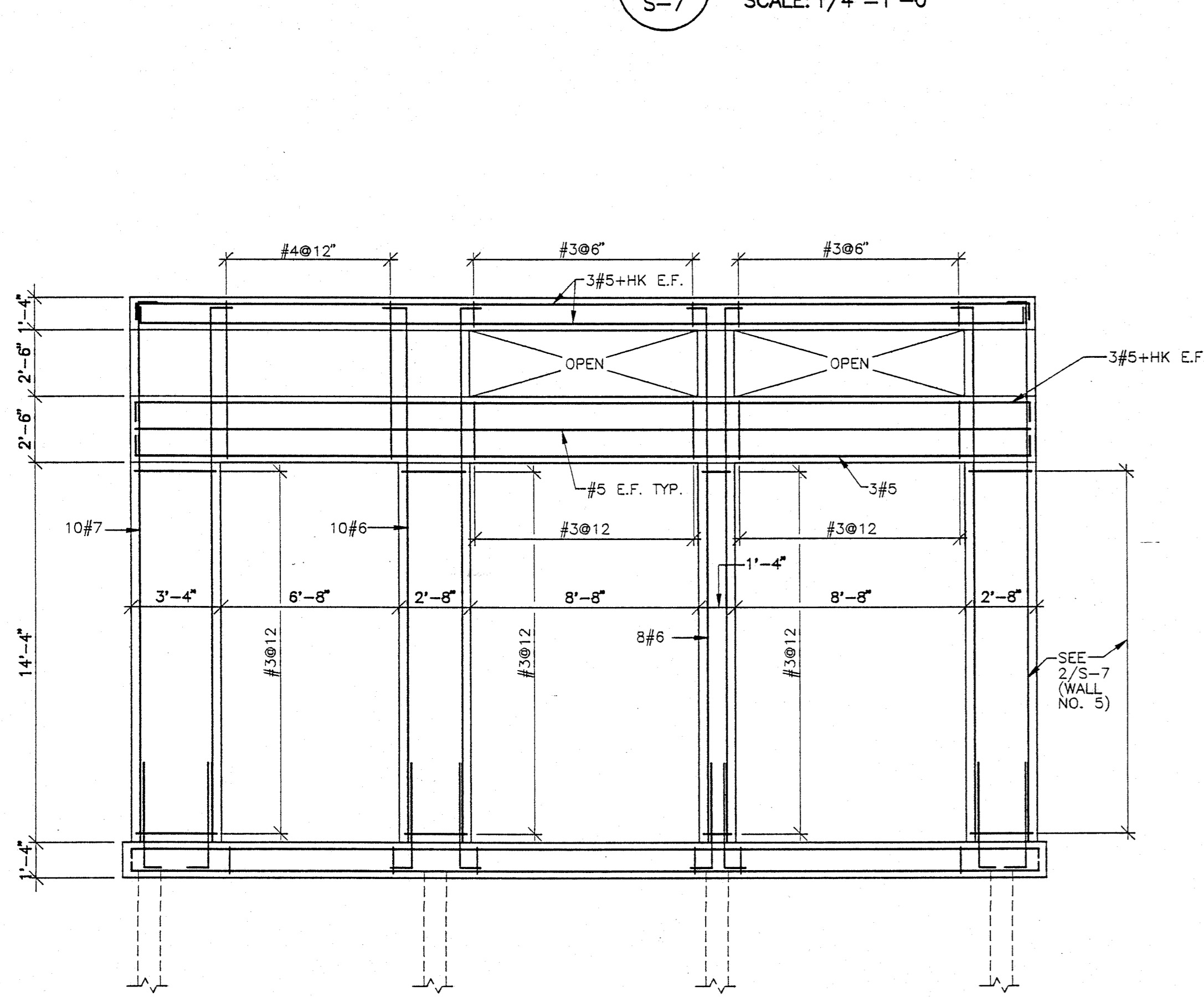


2 WALL NO. 5
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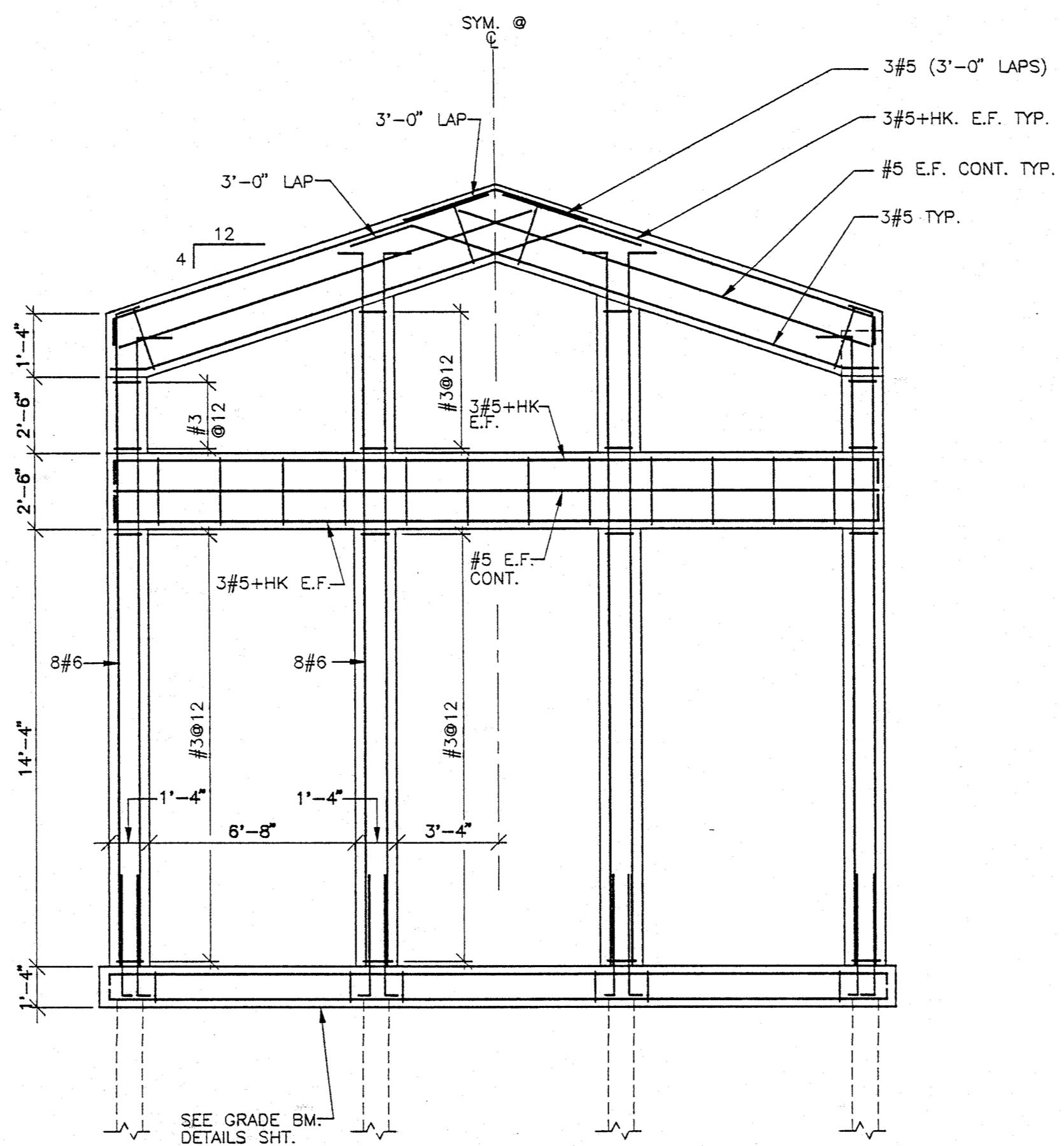
NOTES:
1. ALL WALL ELEVATIONS ARE VIEWED FROM THE OUTSIDE LOOKING IN.
2. REFER TO SHT. S-2 FOR PAVILION FOUNDATION PLAN AND TO SHT. S-4 FOR GRADE BEAM DIAGRAMS.



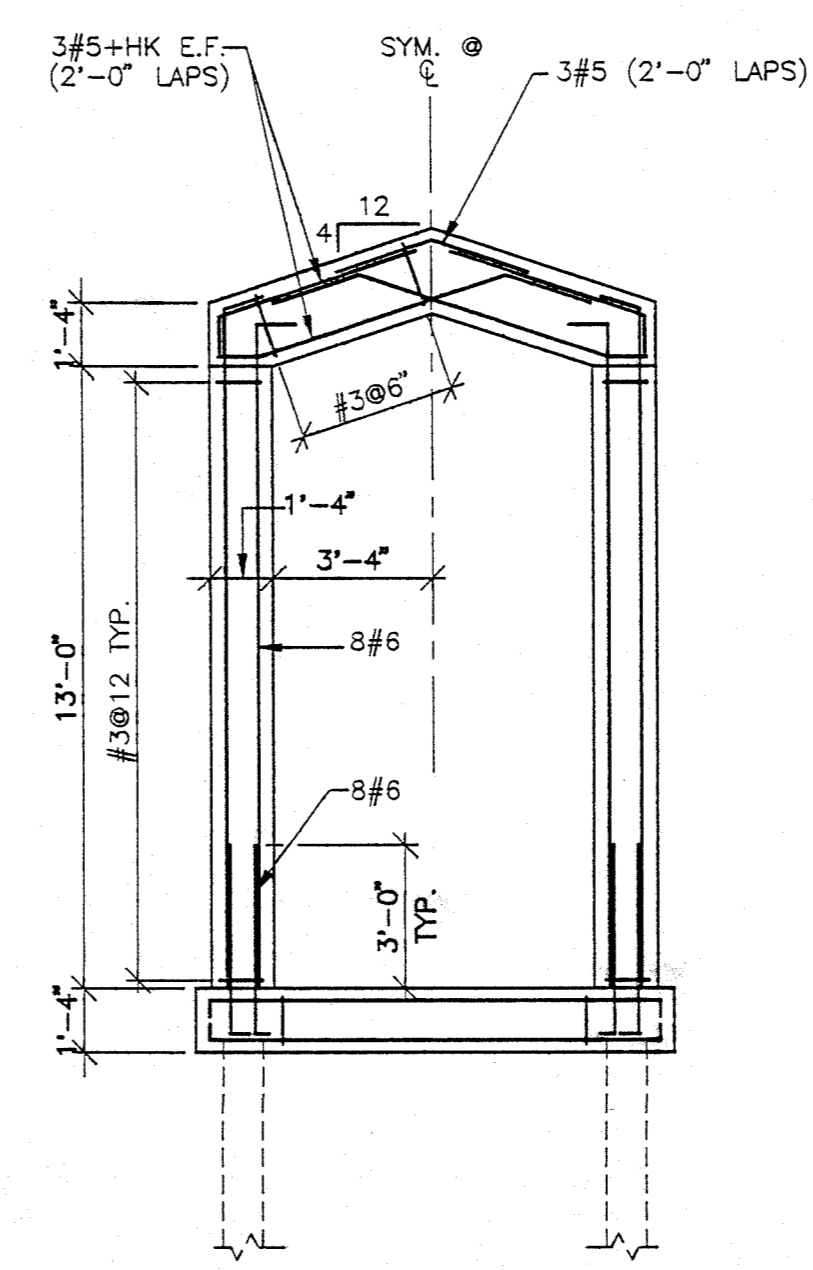
8 DETAILS
S-7 SCALE: N.T.S.



5 WALL NO. 8
S-7 SCALE: 1/4" = 1'-0"



6 WALL NO. 9
S-7 SCALE: 1/4" = 1'-0"



7 WALL NO. 10
S-7 SCALE: 1/4" = 1'-0"

THESE DESIGN PLANS AND SPECIFICATIONS ARE IN COMPLIANCE WITH THE STANDARDS ESTABLISHED IN SECTION 168-33.007, FLORIDA ADMINISTRATIVE CODE.
THE MAIN WIND-FORCE RESISTING SYSTEM HAS BEEN DESIGNED IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990) "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.
THE COMPONENTS AND CLADDING HAVE BEEN SELECTED AND THEIR USE INCORPORATED INTO THE DESIGN AND SPECIFICATIONS IN ACCORDANCE WITH SECTION 6, AMERICAN NATIONAL STANDARDS/AMERICAN SOCIETY OF CIVIL ENGINEERING 7-88 (JULY 1990) "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" TO WITHSTAND THE WIND LOADS ASSOCIATED WITH A MINIMUM BASIC WIND SPEED OF 120 MILES PER HOUR.

REVISION		REVISION	
NO.	DATE	NO.	DATE

PROJECT NO.	21213.00
DATE	7/15/94
DESIGNED	LJD/KLH
DRAWN	KLH
CHECKED	KLH/LJD
DATE	7/15/94

INDEX	S-7

BASKERVILLE-DONOVAN, INC.
 CASINO BEACH AREA RENOVATION
 PENSACOLA BEACH, FLORIDA
 FOR THE SANTA ROSA ISLAND AUTHORITY

DETAILS AND ELEVATIONS