CITY OF WILSON TENNIS/PICKLEBALL FACILITY J. BURT GILLETTE ATHLETIC COMPLEX 3238 CORBETT AVE. WILSON, NC

CITY OF WILSON

MAYOR:

CARLTON L. STEVENS

COUNCIL MEMBERS:

GILLETTIA MORGAN

MICHAEL S. BELL

WILLIAM THOMAS FYLE

JAMES M. JOHNSON, III

DONALD I. EVANS

LOGAN T. LILES

DERRICK D. CREECH

CITY MANAGER:

GRANT GOINGS

DIRECTOR OF PUBLIC SERVICES:

WILLIAM T. BASS, IV

WILSON PARKS & RECREATION:

NO ADDITIONS, DELETIONS, CHANGES OR MODIFICATIONS WERE MADE TO

THIS PLAN OTHER THAN THOSE REQUESTED BY THE CITY OF WILSON

ENGINEER/SURVEYOR/ARCHITECT SIGNATURE & SEAL DATE

DIRECTOR:

TECHNICAL REVIEW COMMITTEE.

DAVID LEE

PARKS MANAGER:

DALE EDMONDS

3-12-2023

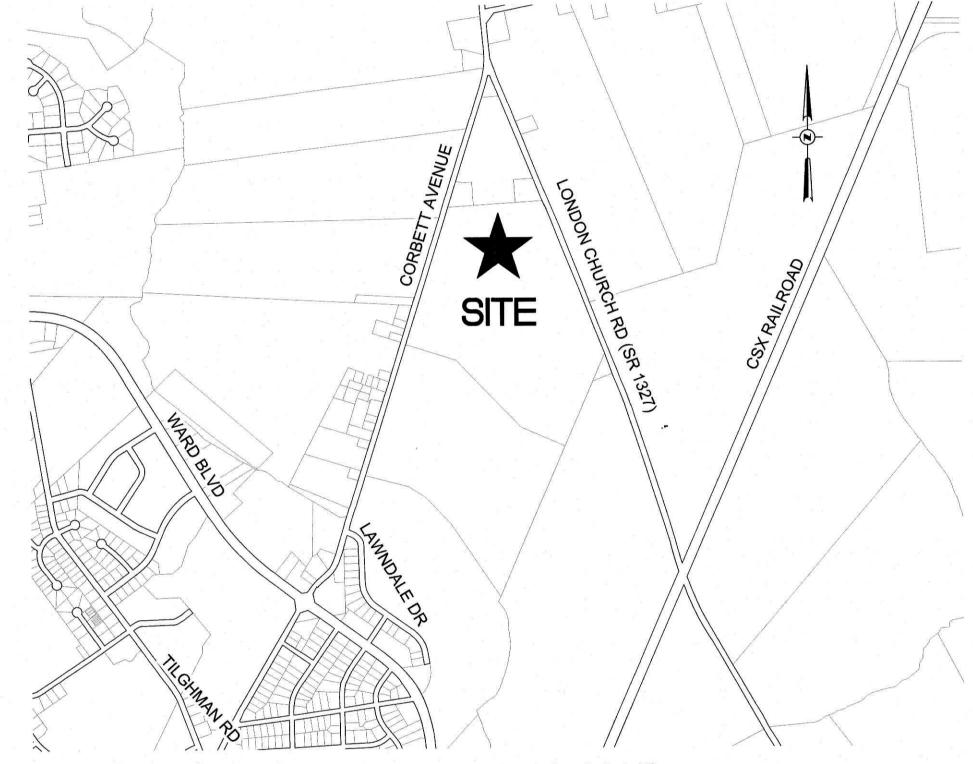
GREEN ENGINEERING

WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

NC FIRM LICENSE: P-0115

303 GOLDSBORO ST. E. P.O. BOX 609 WILSON, N.C. 27893

TEL (252) 237-5365 FAX (252) 243-7489 ØFFICE @ GREENENG.COM



LOCATION MAP SCALE 1" = 1000"

SITE NOTES

A PORTION OF THIS PROPERTY IS LOCATED WITHIN ZONE AE FLOOD

REGULATORY WETLANDS WERE DELINEATED ADJACENT TO THE PROJECT AREA. NO WETLAND IMPACTS ARE PROPOSED FOR THIS PROJECT.

ALL OUTDOOR LIGHTING SHALL BE SHIELDED IN SUCH A MANNER THAT NO DIRECT GLARE FROM THE LIGHT SOURCE CAN BE SEEN FROM A

STANDARD R7-80 RESERVED PARKING AND MAXIMUM PENALTY \$250 NCGS 20-37.6 SIGNS MUST BE INSTALLED IN FRONT OF THE HANDICAP PARKING SPACES. VAN ACCESSIBLE SIGN(S) MUST BE PROVIDED IN FRONT OF VAN ACCESSIBLE SPACES. (SEE DETAIL)

ALL CURB CUTS AND/OR RAMPS MUST MEET ALL REQUIREMENTS OF ICC/ANSI A117.1-2009 AMERICAN NATIONAL STANDARD (ACCESSIBLE AND

ALL REQUIRED EXITS WILL NEED TO BE TREATED AS AN ACCESSIBLE ENTRANCE AND THESE EXITS MUST MEET ALL REQUIREMENTS OF THE NORTH CAROLINA STATE BUILDING CODE AND ICC/ANSI A117.1-2009 AMERICAN NATIONAL STANDARD (ACCESSIBLE AND USABLE BUILDINGS AND

(PARKING AREA). MAXIMUM ACCESSIBLE RAMP SLOPE 1:12 AND RAMP RISE GREATER THAN 6 INCHES SHALL HAVE HANDRAILS. HANDICAP RAMP SHALL HAVE A SLOPE NO GREATER THAN 1 IN 12.

AN 8 FOOT WIDE ACCESS AISLE IS NEEDED FOR VAN ACCESSIBLE SPACE. ALL OTHERS CAN BE 5 FEET WIDE.

HANDICAP SYMBOL CAN NOT BE PAINTED ON PAVEMENT. WHEEL STOPS REQUIRED WHERE PARKING ENCROACHES ON TO SIDEWALK.

ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE SPECIFICATIONS OF THE LOCAL GOVERNING BODY. UTILITIES SHOWN ON PLANS ARE LOCATED APPROXIMATELY. CONTRACTOR

CONTRACTOR TO BE RESPONSIBLE FOR REMOVAL AND REPLACEMENT OF

ALL TRAFFIC CONTROL DEVICES (PAVEMENT MARKINGS, SIGNS AND SIGNALS) SHALL BE DESIGNED, INSTALLED AND MAINTAINED IN

ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE CITY OF WILSON STANDARD SPECIFICATIONS. USE MOST RESTRICTIVE REGULATIONS WHEN

CONTRACTOR TO CALL N.C.O.C.C. AT 1-800-632-4949 A MINIMUM OF 72

BE PLANTED SO AS TO HAVE AS LITTLE IMPACT ON THE FACILITIES AS POSSIBLE WHEN FULLY MATURE. THE CITY WILL RESERVE THE RIGHT T REMOVE TREES, WITHOUT ADDITIONAL TREES BEING REPLANTED, SHOULD FUTURE MAINTENANCE PROBLEMS EMERGE.

GARBAGE PICK-UP IS BY PRIVATE COLLECTION WITHOUT A DUMPSTER

ALL PARKING BUFFERS SHALL BE GRASS AND 5' IN WIDTH UNLESS

SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION OF ALL EXISTING UTILITIES AND SERVICES WHETHER SHOWN ON PLANS OR NOT.

THESE FACILITIES IF DAMAGED.

CONFORMANCE WITH THE STANDARDS SET FORTH IN THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD)

HOURS PRIOR TO EXCAVATION/GRADING.

ANY TREES TO BE LOCATED NEAR NATURAL GAS FACILITIES WILL NEED TO

J. BURT GILLETE ATHLETIC COMPLEX WATERSHED DATA

LOT AREA (SQ.FT.)	MAX. % IMPERVIOUS AREA	MAX. AREA IMPERVIOUS SURFACE ALLOWED (SQ.FT.)	EXISTING IMPERVIOUS AREA (SQ.FT.)	PROPOSED IMPERVIOUS AREA (SQ.FT.)	REMAINING IMPERVIOUS AREA ALLOWED (SQ.FT.)
 5,928,951	24.00%	1,422,948	706,233	975,736	447,212

PARCEL PAIRING-CURREN	IT	Annual residue and the second									A 154	TOTAL Nitragon	TOTAL	Evence
FEBRUARY 2023		Maximum	Undisturbed	Nitrogen	Managed	Nitrogen	4	Nitrogen	TOTAL N		Nitrogen	TOTAL Nitrogen		Excess
		Impervious	Open	Loading	Open	Loading	Impervious	Loading	Loading w/o		Reduction	and the second s	THE PROPERTY AND PROPERTY AND PROPERTY AND PARTY.	Nitrogen
Lot #	Acreage	Percentage	Space (ac)	(lbs)	Space (ac)	(lbs)	Area (ac)	(lbs)	BMP's (lbs)	BMP's	Factor	BMP's (lbs)	lbs/ac/yr	lbs/ac/yr
1 and 2 (to be recombined)	136.110	0.240	42.480	25.488	71.230	85.476	22.400	474.880	585.844	N/A		1 585.844	4.304	0.704
Parcel Pairing Conservation Areas	61.370	0.000	0.000	0.000	61.370	73.644	0.000	0.000	73.644	N/A		0.000		-3.600
Totals	197.480	property and the property of t							659.488	-		585.844	and the state of t	ļ
				en Description () enter a material representation (Aur	Augustin 100 - 100		1	Total Develo	opment Acreag	je	1	197.48		
			1	1	1			TOTAL NITI	ROGEN (LBS)			585.844		
And the second s		1		1		-	100000000000000000000000000000000000000	Company of the Compan	ROGEN (LBS/A	Areatestan	R)	2.967		
a signal and control are some termination socialistic are the more to model terminate and terminate terminate and the second sec			and the state of t	A CONTRACTOR OF THE PARTY OF TH		AN (1800) AND (1807) A	The second second second second second second	- Acceptance of the second sec	ITROGEN (LBS	CHITTOGETH AND AND STATEMENT OF THE PARTY OF	AND DESCRIPTION OF THE PARTY OF	-0.633		
and the second s	+			+	- Lamenta and Conserved				ITROGEN (LBS			-3752.52	-	
The state of the s					1	1			Control of the Contro		N OR BUY	DOWN REQUIRED)		
		A THE STREET AND ADDRESS OF THE STREET		and the state of t		The second second second	A CONTRACTOR OF CONTRACTOR CONTRACTOR	1					1	

SITE TABLE

NOTE: REGULATORY WETLANDS HAVE BEEN DELINEATED WITHIN THE PROJECT AREA ON THIS PROPERTY. IMPACTS TO WETLANDS (USACOE) AND BUFFER AUTHORIZATION NCDEQ) HAS BEEN APPLIED FOR AS PART OF THIS

PROJECT. 401/404 PERMITS TO BE ISSUED PRIOR TO

LOT AREA (2 LOTS): LOT AREA TOTAL RECOMBINED:

EXISTING IMPERVIOUS AREA:

IMPERVIOUS AREA ADDITION:

TOTAL IMPERVIOUS ADDITION:

TOTAL IMPERVIOUS AREA

AFTER CONSTRUCTION:

REQUIRED PARKING:

PROPOSED PARKING:

BUILDING HEIGHT:

EXISTING LAND USAGE

OWNER/DEVELOPER:

REFERENCE:

LOT 1-75.31 AND LOT 2-60.8 ACRES 136.11 ACRES

706,233 SQ. FT. (16.21 ACRES) (11.91 % IMPERVIOUS AREA)

169,609 SF CONCRETE/COURTS 72,691 SF DRIVE AND PARKING 21,123 SF WALKING TRACK 269,503 SF (6.19 ACRES)

975,736 SF (22.40 AC) 16.46 % IMP

DB 1362 PG 934; DB 1723 PG 701

NO REQUIREMENT

127 SPACES PLUS 6 ADA SPACES WITH ACCESSIBLE AISLES.

3238 CORBETT AVE.

PARKS & RECREATION

CITY OF WILSON P.O. BOX 10 WILSON, NC 27893

3722-59-7373; 3723-60-1835 PARCEL ID No .:

SHEET INDEX

SHEET INDEX:

- 1 COVER
- 2 GENERAL CONSTRUCTION NOTES AND CALCULATIONS
- 3 EXISTING CONDITIONS
- 4 SITE PLAN
- 5 ENTRANCE DRIVE PLAN AND
- WATERLINE PLAN AND PROFILE 6 - SANITARY SEWER PLAN AND PROFILE
- 7 SITE GRADING PLAN
- 8 DETAILED GRADING PLAN
- 9 PHASE 1 SEDIMENTATION AND **EROSION CONTROL PLAN**
- 10 PHASE 2 SEDIMENTAION AND **EROSION CONTROL PLAN**
- 11 17 DETAILS

PEAK RUN-OFF (RATIONAL METHOD)

Q=CIA	1998	2023	
	PREDEVELOPMENT	PROPOSED	
	CONDITIONS	CONDITIONS	"C" VALUE
TOTAL ACRES	136.11	136.11	
CROPLAND	75.26	0.00	0.5
WOODED PERVIOUS	59.23	42.15	0.25
MANAGED PERVIOUS	0.00	71.56	0.35
IMPERVIOUS AREA	1.62	22.4	0.9
COMPOSITE "C" VALUE	0.40	0.41	
INTENSITY (I)	7.2	7.2	
RUN-OFF (CFS)	388.05	401.35	

COMPLIANCE STATEMENT

5. AND 6. CERTIFICATE OF APPROVAL UNDER NEUSE RIVER BASIN STORMWATER PROTECTION PROGRAM FOR NITROGEN REMOVAL REGULATIONS

I CERTIFY THAT THE (PLAT/DEVELOPMENT PLAN) SHOWN HEREON IS EXEMPT FROM THE PEAK FLOW REQUIREMENTS SINCE THE INCREASE IN PEAK FLOW BETWEEN PRE-

CERTIFY THAT THE PLAT/DEVELOPMENT PLAN SHOWN HEREON COMPLIES WITH THE NEUSE RIVER BASIN STORMWATER PROGRAM FOR NITROGEN REMOVAL REGULATIONS FOR THE CITY OF WILSON

PUBLIC SERVICES / STORMWATER DATE

3.43% INCREASE (NO FLOW CONTROL REQUIRED)

THIS PLAT/DEVELOPMENT PLAN IS IN THE WS3-P WATERSHED DISTRICT AND THE DEVELOPER IS USING THE LOW DENSITY OPTION OF 24% IMPERVIOUS.

CERTIFICATE OF APPROVAL UNDER WATERSHED PROTECTION REGULATIONS. CERTIFY THAT THE PLAT/DEVELOPMENT PLAN SHOWN HEREON COMPLIES WITH THE WATERSHED PROTECTION REGULATIONS FOR THE CITY OF WILSON

PUBLIC SERVICES / WATERSHED ADMINISTRATOR

DATE

GENERAL CONSTRUCTION NOTES

- 1. ALL CONSTRUCTION MATERIALS AND METHODS SHALL BE IN FULL CONFORMANCE WITH THE CITY OF WILSON'S MANUAL OF SPECIFICATIONS, STANDARDS & DESIGN (MSSD). CONTRACTOR SHALL PROVIDE MATERIAL / PRODUCT SUBMITTALS FOR
- REVIEW, COMMENT AND APPROVAL PRIOR TO PURCHASE. THIS PROJECT IS LOCATED ENTIRELY ON PROPERTY OWNED BY THE CITY OF
- THE CONTRACTOR SHALL CALL NC811 TO LOCATE / HAVE LOCATED ALL EXISTING UTILITIES IN AND ADJACENT TO THE PROJECT AREA PRIOR TO BEGINNING ANY
- CONSTRUCTION ACTIVITIES. THERE WILL BE THREE MEANS OF ACCESS TO THE PROJECT SITE AS FOLLOWS:
- a. CORBETT AVENUE ON THE WEST SIDE OF THE SITE. b. LONDON CHURCH ROAD ON THE EAST SIDE OF THE SITE.
- c. GILLETTE ATHLETIC COMPLEX EXISTING INTERNAL DRIVES ON THE SOUTH SIDE d. CONTRACTORS ARE ENCOURAGED TO UTILIZE THE CORBETT AVENUE AND

LONDON CHURCH ROAD CONSTRUCTION ENTRANCES IN AN EFFORT TO

MINIMIZE THE CONSTRUCTION TRAFFIC / ACTIVITIES WITHIN AND AROUND THE

- GILLETTE ATHLETIC COMPLEX'S EXISTING AMENITIES. e. CONTRACTOR RESPONSIBLE FOR ALL TRAFFIC CONTROL MEASURES AT CONSTRUCTION ENTRANCE LOCATIONS, THE COST OF WHICH WILL BE INCIDENTAL TO THE CONTRACT.
- SEDIMENTATION / EROSION CONTROL MEASURES ARE TO BE INSTALLED AT LOCATIONS SHOWN ON THE PLANS OR DEEMED NECESSARY BY THE ENGINEER OR
- 6. ALL CLEARED AND GRUBBED MATERIAL SHALL BE REMOVED FROM THE SITE. NO
- ONSITE DISPOSAL WILL BE ALLOWED. ALL EXCAVATED TOPSOIL SHALL BE STOCKPILED ON SITE AND REDISTRIBUTED, AT
- A MINIMUM OF 4 INCHES, TO ALL OPEN SPACE, TRAFFIC ISLANDS AND PLANTER BEDS WITHIN THE PLAZA AREA PRIOR TO THE COMPLETION OF THE PROJECT. GREEN ENGINEERING WILL PROVIDE CONSTRUCTION STAKING FOR ALL SITE
- GREEN ENGINEERING SHALL PROVIDE ALL THIRD-PARTY MATERIALS TESTING SERVICES WHERE REQUIRED
- 10. A DIGITAL FILE WITH THE EXISTING AND PROPOSED SURFACE WILL BE AVAILABLE TO THE CONTRACTOR IF REQUESTED.
- 11. ALL SEDIMENTATION / EROSION CONTROL MEASURES SHALL BE REMOVED FROM THE SITE ONCE VEGETATION HAS BEEN ESTABLISHED

ENTRANCE ROAD

- 1. THE ENTRANCE ROAD WILL BE 25' B-B WITH A 24-INCH CONCRETE CURB AND
- THE EXISTING CURB AND GUTTER AND / OR ASPHALT TO BE REMOVED FOR
- ENTRANCE ROAD CONSTRUCTION SHALL BE SAW CUT. A VALLEY GUTTER SHALL BE INSTALLED ACROSS THE ENTRANCE ROAD AT THE
- LOCATION OF THE EXISTING CURB AND GUTTER. ALL EFFORTS SHALL BE MADE TO SALVAGE THE EXISTING TREE AT THE BEGINNING
- OF ENTRANCE ROAD, EAST SIDE. THERE WILL BE INSTALLED ON THE EAST SIDE OF THE ENTRANCE ROAD A
- CONCRETE SIDEWALK 5 FEET IN WIDTH AND 4 INCHES THICK. THIS SIDEWALK WILL NOT HAVE AN ABC OR CRUSHED CONCRETE BASE AND WILL TERMINATE AT THE PROPOSED 8' ASPHALT WALKING TRAIL (STATION ±7+10.00) HANDICAP RAMPS WILL BE INSTALLED ON EACH SIDE OF THE ENTRANCE ROAD EVEN THOUGH THERE WILL BE A SIDEWALK ON JUST ONE SIDE. THERE WILL ALSO
- BE A PAINTED CROSSWALK BETWEEN THE TWO RAMPS. ADDITIONAL SIDEWALKS MAY BE INSTALLED ON THE WEST SIDE OF THE ENTRANCE ROAD IN THE FUTURE. HANDICAP RAMPS WILL ALSO BE INSTALLED AT STATION ±7+10.00 AT THE LOCATION WHERE THE 8' ASPHALT WALKING TRAIL WILL CROSS THE ENTRANCE ROAD. THIS LOCATION WILL ALSO REQUIRE A PAINTED CROSSWALK.
- ALL ENTRANCE ROAD LIGHTING WILL BE PROVIDED / INSTALLED BY THE CITY. THIS INSTALLATION WILL TAKE PLACE AT SOME POINT DURING THE PROJECTS CONSTRUCTION AND COORDINATION BETWEEN THE CONTRACTOR, ENGINEER AND OWNER WILL BE REQUIRED.
- THERE WILL BE A 6 INCH DIAMETER WATER MAIN INSTALLED ON THE EAST SIDE OF THE ENTRANCE ROAD BEHIND THE SIDEWALK. CONSIDERATION FOR LIGHT POLE FOUNDATIONS SHALL BE MADE DURING THE WATER MAINS INSTALLATION IN ORDER THAT A CONFLICT IS NOT CREATED
- THERE WILL BE A 3 INCH DIAMETER SANITARY SEWER FORCE MAIN LOCATED ON THE WEST SIDE OF THE ENTRANCE ROAD. 11. THERE SHALL BE 2 - 4 INCH CONDUITS INSTALLED UNDER THE ENTRANCE ROAD
- (CAPPED ON EACH END) TO SERVE AS SLEEVES FOR FUTURE IRRIGATION MAINS AND ELECTRICAL. . THERE IS CURRENTLY EXISTING A RECLAIMED WATER IRRIGATION SYSTEM THAT IS TO BE RELOCATED. CONTRACTOR SHALL SPOT LOCATE EACH OF THE 4 INCH
- PARTIALLY CONTAINED WITHIN THE ENTRANCE ROAD CORRIDOR THAT WILL HAVE DIAMETER RECLAIMED IRRIGATION MAINS THAT CROSS THE PROPOSED ENTRANCE ROAD. THE ENGINEER / CITY WILL THEN DETERMINE WHETHER THE EXISTING MAINS HAVE ADEQUATE COVER OR IF THEY NEED TO BE LOWERED. THE CITY OF WILSON WILL LOWER THE IRRIGATION MAINS IF REQUIRED. 13. THE ENTRANCE ROAD DOES CROSS AN EXISTING STREAM THAT IS SUBJECT TO THE NEUSE RIVER RIPARIAN BUFFER REGULATIONS. THE CONTRACTOR SHALL
- EXERCISE CARE WHILE OPERATING IN THIS AREA. A COPY OF THIS PERMIT IS INCLUDED IN THE CONTRACT DOCUMENTS AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING ALL APPLICABLE PROVISIONS WITH ASSOCIATED COSTS BEING INCIDENTAL TO THE CONTRACT. 14. THE CONTRACTOR SHALL NOTE THAT A PORTION OF THE CURB ON THE EAST SIDE
- OF THE ENTRANCE ROAD WILL BE TURNED DOWN (VALLEY GUTTER) FOR ACCESS TO GRASS PARKING BETWEEN THE ENTRANCE ROAD AND BASEBALL FIELD IV. 15. THERE IS EARTHEN MATERIAL LOCATED BEHIND THE MIRACLE FIELD CONSTRUCTION SITE THAT WILL BE UTILIZED FOR THE CONSTRUCTION OF THE
- ENTRANCE ROAD. ALL MATERIAL DETERMINED TO BE SUITABLE WILL BE USED IN THE STRUCTURAL SECTION OF THE ROADWAY WHILE ALL REMAINING MATERIAL (UNSUITABLE OR TOPSOIL) WILL BE UTILIZED IN SHOULDER CONSTRUCTION. 16. ALL STORM DRAINAGE PIPE ALONG AND UNDER THE ENTRANCE ROAD SHALL BE

RCP UNLESS OTHERWISE DIRECTED BY THE ENGINEER / CITY.

- PARKING LOT CONSTRUCTION WILL CONSIST OF 8 INCHES OF CRUSHED CONCRETE ABC, 2 INCHES OF ASPHALT SURFACE COURSE S9.5B, 24 INCH CONCRETE CURB AND GUTTER AND ASSOCIATED PARKING ISLANDS.
- THERE WILL BE 6 HANDICAP PARKING SPOTS DESIGNATED AT THE HEAD OF THE PARKING LOT CLOSEST TO THE PROPOSED BUILDINGS. THE ENTIRE LENGTH OF THE CURB AND GUTTER AT THIS LOCATION WILL BE TURNED DOWN FOR EASE OF
- HANDICAP SIGNS WILL NEED TO BE PLACED IN THE PLANTER BEDS IN FRONT OF THE HC SPACES AND WILL BE INCIDENTAL TO THE CONTRACT AND SHALL BE INCLUDED IN THE UNIT PRICE OF THE ASPHALT SURFACE COURSE.
- ALL PARKING LOT STRIPING WILL BE INCIDENTAL TO THE CONTRACT AND SHALI BE INCLUDED IN THE UNIT PRICE OF THE ASPHALT SURFACE COURSE. THE PROPOSED SIDEWALK ALONG THE EAST AND WEST SIDES OF THE PARKING LOT
- WILL BE 6 FEET WIDE AND WILL NOT HAVE AN ABC OR CRUSHED STONE BASE. THE 6' WIDE SIDEWALK WILL TERMINATE AT THE PLAZA AREA. ALL PARKING ISLANDS WILL BE IRRIGATED WITH RECLAIMED WATER AND WILL HAVE LIGHTING INSTALLED. THE CONTRACTOR SHALL MAKE PROVISIONS DURING THE CONSTRUCTION OF THE PARKING LOT TO EXTEND TWO (2) SETS OF 4 INCH
- ALL PARKING LOT LIGHTING WILL BE PROVIDED / INSTALLED BY THE CITY. THIS INSTALLATION WILL TAKE PLACE AT SOME POINT DURING THE PROJECTS CONSTRUCTION AND COORDINATION BETWEEN THE CONTRACTOR, ENGINEER AND

DIAMETER CONDUITS (CAPPED ON EACH END) TO ALL PARKING ISLANDS. THE

CONTRACTOR SHALL COORDINATE THE LOCATIONS OF THESE SLEEVES WITH THE

OWNER WILL BE REQUIRED.

- 1. THE WALKING TRAIL WILL CONSIST OF A 6" ABC / CRUSHED CONCRETE BASE 10' WIDE WITH 2 INCHES OF ASPHALT SURFACE COURSE S9.5B.
- ALL EXPOSED STONE BASE OUTSIDE ASPHALT WILL BE COVERED WITH TOPSOIL UPON COMPLETION OF THE WALKING TRAIL AND SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
- 3. LIGHTING ALONG THE WALKING TRAILS LENGTH IS NOT BEING CONSIDERED AT

POTABLE WATER SYSTEM

- 1. A 6" PVC WATER MAIN WILL BE INSTALLED BY CONNECTING TO THE EXISTING 8" WATER MAIN LOCATED ON THE RIGHT SIDE OF THE ENTRANCE ROAD. CONNECTION SHALL BE BY CUT-IN SLEEVE, TEE AND 6 INCH GATE VALVE.
- THERE WILL BE A SINGLE FIRE HYDRANT TO BE INSTALLED AT THE LOCATION SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER / CITY DURING CONSTRUCTION. HYDRANT PUMPER NOZZLE SHALL BE INSTALLED FACING THE
- THE WATER SERVICE EXTENDING TO THE CONVENIENCE BUILDING WILL BE A 2-INCH SERVICE LINE. A 2-INCH TAPPED PLUG SHALL BE INSTALLED ON THE 6-INCH HYDRANT TEE
- A 2-INCH METER ASSEMBLY SHALL BE INSTALLED AT THE LOCATION SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER / CITY DURING CONSTRUCTION.
- THE REQUIRED BACKFLOW SHALL BE INSTALLED IN THE CONVENIENCE BUILDING'S
- MECHANICAL ROOM AS SHOWN ON THE BUILDING DRAWNGS. THERE WILL BE NO YARD HYDRANTS OR WATER FOUNTAINS CONNECTED TO THE POTABLE WATER MAIN AS PART OF THIS PROJECT.

SANITARY SEWER SYSTEM

- 1. THE SITE WILL BE SERVED WITH A 6" SERVICE LINE FROM THE CONCESSIONS BUILDING TO A PREFABRICATED DUPLEX GRINDER PUMP STATION LOCATED BETWEEN THE PARKING LOT AND THE CROQUET COURT.
- THE CONTRACTOR SHALL VERIFY THE 6 INCH SEWER SERVICE LINE INVERT LEAVING THE CONCESSIONS BUILDING PRIOR TO INSTALLATION OF THE 6 INCH SEWER SERVICE LINE FROM THE CONVENIENCE BUILDING DOWN TO THE PROPOSED
- ALL CLEANOUTS LOCATED IN NATURAL AREAS BE ENCLOSED IN A METAL OR COMPOSITE BOX STAMPED "SEWER"
- 4. THE DUPLEX GRINDER PUMP STATION WILL BE 2HP, SINGLE PHASE BARNES SERIES
- THE CONTRACTOR SHALL INSTALL THE PREFABRICATED DUPLEX GRINDER PUMP STATION COMPLETE WITH CONTROL PANEL AND AUDIBLE AND VISIBLE ALARM. THE CITY WILL PROVIDE POWER AND A METER BASE TO THE PUMP STATION.
- THIS PUMP STATION INSTALLATION IS TO SERVE ONLY THE CONCESSIONS BUILDING AND IS NOT REQUIRED TO ADHERE TO THE CITY'S PUMP STATION SITE DESIGN
- 7. AREA LIGHTING AND WATER SERVICE WILL BE PROVIDED BY THE CITY IF DEEMED NECESSARY
- THE SEWER WILL BE DISCHARGED OUT OF THE PUMP STATION THROUGH A 3" FORCE MAIN BACK ALONG THE ENTRANCE ROAD AND WILL BE CORED INTO AN EXISTING MANHOLE ADJACENT TO THE PLAYGROUND. THE MANHOLE CORE SHALL BE ALIGNED SUCH THE FORCE MAIN DISCHARGE INSIDE THE MANHOLE WILL BE ALIGNED WITH THE EFFLUENT GRAVITY SEWER LINE (DOWNSTREAM DIRECTION). IF TRENCHING WITHIN EXISTING ASPHALT AREAS IS REQUIRED FOR FORCE MAIN INSTALLATION ALL ASPHALT SHALL BE SAW CUT.
- THERE SHOULD BE NO NEED FOR AN AIR RELEASE VALVE ON THIS INSTALLATION.

- 1. THE PLAZA AREA WILL CONSIST OF A 4" ABC / CRUSHED CONCRETE BASE UNDER 3000 PSI CONCRETE 4 INCHES THICK WITH A 6"X6" WOVEN WIRE MESH REINFORCEMENT
- THERE ARE SEVERAL PLANTER BEDS LOCATED IN THE PLAZA AREA AND ADJACENT TO THE PARKING LOT THAT WILL REQUIRE EITHER LIGHTING OR IRRIGATION. LIGHTING IS ALSO PROPOSED WITHIN THE PLAZA BETWEEN THE PARKING LOT AND THE CONVENIENCE BUILDING / SHELTER. THE CONTRACTOR SHALL INSTALL UNDER THE PLAZA AREA TO EACH OF THE PLANTER BEDS TWO (2) SETS OF 4 INCH DIAMETER CONDUITS (CAPPED ON EACH END). THE CONTRACTOR SHALL
- COORDINATE THE LOCATIONS OF THESE SLEEVES WITH THE ENGINEER / CITY. THE PLAZA LIGHT POLE FOUNDATIONS AND ELECTRICAL CONDUIT TO THESE FOUNDATIONS WILL BE INSTALLED BY THE CITY DURING CONSTRUCTION AND
- SHALL BE COORDINATED WITH THE ENGINEER / CITY THE CONTRACTOR SHALL PROVIDE A SCORING / COLD JOINT PLAN OF THE PLAZA AREA FOR REVIEW AND APPROVAL BY THE ENGINEER / CITY PRIOR TO ANY CONCRETE INSTALLATION
- THERE ARE 2 SETS OF TRENCH DRAINS TO BE INSTALLED IN THE PLAZA AREA BETWEEN THE SHELTER / CONVENIENCE BUILDING AND THE PICKLEBALL STRUCTURE. THESE TRENCH DRAINS SHOULD BE LOCATED DIRECTLY UNDER THE DRIP LINES OF THE SHELTER / CONVENIENCE BUILDING.

- THE CROOUET COURT WILL HAVE A TURF SURFACE THAT WILL BE INSTALLED BY
- THE CITY ONCE THE PROJECT IS COMPLETED. THE CONCRETE WALK TO AND AROUND THE CROQUET COURT WILL BE 6 FEET WIDE AND 4-INCHES THICK (3000 PSI). THERE WILL BE NO ABC / CRUSHED CONCRETE BASE UNDER THIS WALK
- THE CONCRETE WALK WILL HAVE A 12-INCH TURNDOWN ON THE INTERIOR SIDE OF THE COURT. THE CONTRACTOR SHALL EXCAVATE TO A DEPTH OF 1 FOOT BELOW THE CONCRETE
- WALK IN THE AREA OF THE COURT. THIS EXCAVATION SHALL BE SLOPED TO THE CENTER OF THE COURT WHERE THE CONTRACTOR SHALL INSTALL A 4-INCH PERFORATED UNDERDRAIN PIPE EMBEDDED IN A WASHED STONE TRENCH THE CONTRACTOR SHALL INSTALL 6-INCHES OF WASHED STONE (#57) AS THE BASE
- 6. THE CITY OF WILSON WILL PROVIDE THE REMAINING GRANULAR MATERIAL LEVELING COURSE, PERIMETER NAILER BOARD AND TURF SURFACE.

- THE TENNIS COURT AREAS WILL BE CONSTRUCTED WITH AN 8-INCH ABC / CRUSHED CONCRETE BASE WITH A 2-INCH ASPHALT S9.5B SURFACE COURSE. THE ASPHALT SURFACE COURSE WILL BE INSTALLED IN A CROSS-COURT
- DIRECTION. BOTH SETS OF COURTS ARE SLOPED AT 1% ALONG THE COURT'S LENGTH, NOT CROSS COURT.
- 4. THE AREA BETWEEN THE COURTS WILL BE CONSTRUCTED THE SAME AS THE TENNI COURTS (8-INCH ABC / CRUSHED CONCRETE AND 2-INCH ASPHALT SURFACE COURS)
- JOINT THAT IS AS SEAMLESS AS POSSIBLE. ALL TENNIS NET POST GROUND SLEEVE AND CENTER ANCHOR FOUNDATIONS SHALL BE POURED IN PLACE WITH A SLEEVE INSTALLED PRIOR TO THE COURTS
- BEING PAVED. THE CONTRACTOR SHALL COORDINATE NET POLE FOUNDATION LOCATIONS WITH THE ENGINEER / CITY PRIOR TO INSTALLATION EACH SET OF 6 TENNIS COURTS SHALL BE ENCLOSED WITH A VINAL COATED (COLOR BLACK) FENCE 8 FEET IN HEIGHT WITH TWO (2) ACCESS GATES FIVE FEET IN
- WIDTH. GATES SHALL BE INSTALLED AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER / CITY THE CITY WILL BE RESPONSIBLE FOR TENNIS COURT SURFACE COATING AND
- THE CITY WILL BE RESPONSIBLE FOR INSTALLING FENCE WIND SCREENS.

BOCCE BALL / SHUFFLEBOARD / HORSESHOE COURT AREA

- 1. THE BOCCE BALL COURT WILL HAVE A TURF SURFACE THAT WILL BE INSTALLED
- BY THE CITY ONCE THE PROJECT IS COMPLETED. THE CONCRETE AREAS AROUND THE BOCCE BALL / SHUFFLEBOARD / HORSESHOE
- AREA WILL BE AND 4-INCHES THICK (3000 PSI). THERE WILL BE NO ABC / CRUSHED CONCRETE BASE UNDER THIS CONCRETE.
- THE CONCRETE WILL HAVE A 12-INCH TURNDOWN ON THE INTERIOR SIDE OF THE BOCCE BALL COURT AND HORSESHOE PITS. THE CONTRACTOR SHALL EXCAVATE TO A DEPTH OF 1 FOOT BELOW THE CONCRETE
- IN THE AREA OF THE BOCCE BALL COURT AND HORSESHOE PITS. THIS EXCAVATION SHALL BE SLOPED TO THE CENTER OF THE COURT / PIT AREAS WHERE THE CONTRACTOR SHALL INSTALL A 4-INCH PERFORATED UNDERDRAIN PIPE EMBEDDED IN A WASHED STONE TRENCH.
- THE CONTRACTOR SHALL INSTALL 6 INCHES OF WASHED STONE (#57) AS THE BASE FOR THE TURF BOCCE BALL COURT SURFACE.
- 6. THE CITY OF WILSON WILL PROVIDE THE REMAINING GRANULAR MATERIAL LEVELING COURSE, PERIMETER NAILER BOARD AND TURF SURFACE. THE CITY OF WILSON WILL PROVIDE THE SAND TO BE INSTALLED IN THE
- THE CONCRETE SURFACE FOR THE SHUFFLEBOARD COURTS SHALL HAVE A
- BROOMED FINISH 9. THE CITY OF WILSON WILL PROVIDE SURFACE COATING FOR THE SHUFFLEBOARD

PICKLEBALL COURTS (EXTERIOR)

- 1. THERE WILL BE TWO (2) PICKLEBALL COURT AREAS WITH EACH AREA HAVING
- THREE (3) PICKLEBALL COURTS. EACH COURT AREA WILL HAVE A 6" WASHED STONE (#57) BASE.
- THERE WILL BE A CLASS B VAPOR BARRIER INSTALLED AT SUBGRADE FOR EACH PICKLEBALL COURT AREA.
- 4. THE CONCRETE SLAB FOR THE PICKLEBALL COURT AREAS SHALL BE 5 INCHES, 3000 PSI WITH A BROOMED FINISH 5. ALL NET POST FOUNDATIONS SHALL BE POURED IN PLACE WITH A SLEEVE
- INSTALLED PRIOR TO THE COURTS BEING POURED. THE CONTRACTOR SHALL COORDINATE NET POLE FOUNDATION LOCATIONS WITH THE ENGINEER / CITY PRIOR TO THE INSTALLATION. 6. EACH SET OF 3 PICKLEBALL COURTS SHALL BE ENCLOSED WITH A VINAL COATED
- (COLOR BLACK) FENCE 6 FEET IN HEIGHT WITH TWO (2) ACCESS GATES FIVE FEET IN WIDTH. GATES SHALL BE INSTALLED AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER / CITY.
- 7. THE CITY WILL BE RESPONSIBLE FOR PICKLEBALL COURT SURFACE COATING AND
- . THE CITY WILL BE RESPONSIBLE FOR INSTALLING FENCE WIND SCREENS

CITY OF WILSON

- ALL STORM DRAINAGE PIPE SERVING THE SITE SHALL BE EITHER HDPE OR RCP. THERE ARE SIX (6) STORM DRAINAGE DISCHARGE LOCATIONS ON SITE (NOT INCLUDING THE ONES LOCATED ALONG THE ENTRANCE ROAD). THE LAST LEG OF STORM DRAIN PIPE AT THESE LOCATIONS SHALL BE RCP.
- ALL STORM DRAINAGE FLARED END SECTIONS SHALL BE RCP. . THERE IS AN EXISTING DRAINAGE FEATURE THAT TRAVERSES THROUGH THE PROPOSED TENNIS COURT LOCATIONS. THE EXISTING STORMWATER RUNOFF WILL BE REROUTED AWAY FROM THIS FEATURE WITH THE INSTALLATION OF NEW STORM
- DRAINAGE PIPE TO BE INSTALLED ADJACENT TO LONDON CHURCH ROAD. THE EXISTING DRAINAGE FEATURE SHALL BE OVER EXCAVATED TO A DEPTH OF 1.0 FOOT BELOW THE EXISTING DITCH INVERT. A 6 INCH PERFORATED UNDERDRAIN
- PIPE SHALL BE INSTALLED ON A BED OF 4 INCHES WASHED STONE AND COVERED TO A MINIMUM 6 INCHES WITH WASHED STONE 6. AN ENGINEERING STABILIZATION FABRIC SHALL BE INSTALLED OVER THE
- UNDERDRAIN TRENCH UNDER THE TENNIS COURT AREAS AT A MINIMUM WIDTH OF 7. IT IS IMPORTANT THAT ALL DRAINAGE BOXES TOP ELEVATIONS BE INSTALLED IN ACCORDANCE WITH THE ELEVATIONS SHOWN ON THE PLANS. USE OF PREFABRICATED CONCRETE "KNOCKOUT" BOXES WILL BE ALLOWED AS LONG AS THE TOP OF GRATE ELEVATIONS CAN BE MAINTAINED. IF KNOCKOUT BOXES ARE
- USED THE INVERTS SHALL BE GROUTED. 8. BLOCK BUILT DRAINAGE STRUCTURES ARE PREFERRED BUT NYLOPLAST DRAINAGE STRUCTURES WILL BE ALLOWED.
- 9. DRAINAGE WILL BE PROVIDED TO THE CONDENSATE LINE ON THE HVAC UNIT LOCATED ADJACENT TO THE CONVENIENCE BUILDING. 10. THERE WILL BE NO ROOF DRAINS SERVING THE PICNIC SHELTER OR CONVENIENCE
- 11. THE COVERED PICKLEBALL STRUCTURE WILL HAVE A ROOF THAT WILL BE SLOPED TO THE REAR OF THE SITE. IT WILL NOT BE KNOWN UNTIL THE BUILDING DRAWINGS ARE SUBMITTED FOR APPROVAL WHERE OR HOW MANY ROOF DRAINS WILL BE PROVIDED. FOR BID PURPOSES IT WILL BE ASSUMED THAT THERE WILL BE
- A ROOF LEADER LOCATED AT EACH REAR COLUMN. DOWNSPOUT) SHALL BE 6 INCH DIAMETER HDPE. DOWNSPOUT ADAPTORS SHALL BE USED AT THE INTERFACE OF THE 6 INCH DRAINAGE LATERAL AND THE
- 13. ALL DISSIPATOR PADS SHALL BE UNDERLAIN WITH AN ENGINEERED FABRIC TO PREVENT EROSION / UNDERMINING OF THE STONE DISSIPATER PAD.

STORM DRA		n = 0.013		, · · ·	01 (11)			-	 			}		1					1		1
				and the first and the first state of the sta	antigration of the second of t	Annesses of the electric section of	AND THE STATE OF THE PARTY OF T	-		**************************************				-			pile a minima da mas mas r		Total contract of the American		-
No		0		ar mental of Community of the Advance Lives of	Α			tc	İ	Cc	Сс	Q10	Care to the colonial of		() () () () () () () () () ()		41	The latest and the Total Section (Co.)	-		The state of the s
FROM	ТО	PIPE	INLET	INLET	TOTAL	INLET	PIPE	TIME OF	INTENSITY	RUNOFF	RUNOFF	DSCHRG	SLOPE	Dtheo	SIZE	Vfull	Qfull	LENGTH	SEGMENT	UPPER	LOWER
	Company of the second of the second	RUN	AREA	AREA	AREA	TIME	TIME	CONC		COEFF	COEFF				The second of th			PIPE	TIME	INVERT	INVER
	gletta and control chaldren (Advisor STA) (Advis Star Program Stroman, e. s.	The state of the s	(SF)	(ACRES)	(ACRES)	(MIN)	(MIN)	(MIN)	(IN/HR)			(CFS)	(FT/FT)	(INCHES)	(INCHES)	(FT/SEC)	(CFS)	(FT)	(MIN)		
FES#14	DI#24	I-3	180844	4.15	4.15	10.0	0.0	10.0	6.14	0,40	0.40	10.2	0.0144	16.6	18	7.1	12.6	278	0.7	128.00	124.00
DI#25	DI#24	1-2	14003	0.32	0.32	5.0	0.0	5.0	7.54	0.40	0.40	1.0	0.0050	8.4	12	3.2	2.5	77	0.4	123.18	122.80
DI#24	FES#13	I-1	11933	0.27	4.75	5.0	0.0	5.0	7.54	0.40	0.40	14.3	0.0087	20.7	24	6.7	21.1	34	0.1	122.80	122.50
DI#23	DI#21	H-3	41488	0.95	0.95	5.0	0.0	5.0	7.54	0.80	0.80	5.7	0.0100	14.3	15	5.3	6.5	182	0.6	122.14	120.32
DI#22	DI#21	H-2	21392	0.49	0.49	5.0	0.0	5.0	7.54	0,60	0.60	2.2	0.0040	11.9	12	2.9	2.3	84	0.5	120.65	120.32
DI#21	FES#12	H-1	9301	0.21	1.66	5.0	0.0	5.0	7.54	0.80	0.74	9.3	0.0080	17.9	18	5.3	9.4	46	0.1	120.32	119.95
DI#20	DI#19	G-4	31180	0.72	0.72	5.0	0.0	5.0	7.54	0.62	0.62	3.3	0.0100	11.7	12	4.5	3.6	133	0.5	124.93	123.60
DI#19	DI#17	G-3	20719	0.48	1.19	5.0	0.0	5.0	7.54	0.73	0.66	6.0	0.0100	14.5	15	5,3	6.5	186	0.6	123.60	121.74
DI#18	DI#17	G-2	4467	0.10	0.10	5.0	0.0	5.0	7.54	0.40	0.40	0.3	0.0030	6.0	12	2.5	2.0	40	0.3	121.86	121.74
DI#17	FES#11	G-1	3466	0.08	1.37	5.0	0.0	5.0	7.54	0.42	0.63	6.5	0.0040	17.9	18	3.8	6.6	147	0.7	121.74	121.15
FES#10	FES#9	F-1	525760	12.07	13.44	15.0	0.0	15.0	5.22	0.35	0.38	26.6	0.0150	23.6	24	8.8	27.7	116	0.2	121.04	119.30
DI#16	FES#8	E-1	44305	1.02	1.02	5.0	0.0	5.0	7.54	0.57	0.57	4.4	0.0050	14.7	15	3.7	4.6	57	0.3	118.69	118.40
DI#15	DI#10	D-16	30591	0.70	0.70	5.0	0.0	5.0	7.54	0.60	0.60	3.2	0.0090	11.7	12	4.3	3.4	166	0.6	124.62	123.13
DI#14	DI#13	D-15	16863	0.39	0.39	5.0	0.0	5.0	7.54	0.40	0.40	1.2	0.0050	9.0	12	3.2	2.5	28	0.1	124.56	124.42
DI#13	DI#11	D-14	5989	0.14	0.52	5.0	0.0	5.0	7.54	0.85	0.52	2.0	0.0050	11.1	12	3.2	2.5	112	0.6	124.42	123.86
DI#12	DI#11	D-13	11213	0.26	0.26	5.0	0.0	5.0	7.54	0.40	0.40	0.8	0.0050	7.7	12	3.2	2.5	25	0.1	123.98	123.86
DI#11	DI#10	D-12	6438	0.15	0.93	5.0	0.0	5.0	7.54	0.40	0.47	3.3	0.0090	11.8	12	4.3	3.4	81	0.3	123.86	123.13
DI#10	DI#9	D-11	26693	0.61	2.24	5.0	0.0	5.0	7.54	0.85	0.61	10.4	0.0100	17.9	18	5.9	10.5	93	0.3	123.13	122.20
DI#9	DI#8	D-10	4328	0.10	2.34	5.0	0.0	5.0	7.54	0.85	0.85	15.0	0.0210	17.9	18	8.6	15.2	56	0.1	122.20	121.02
DI#8	DI#7	D-9	6784	0.16	2.50	5.0	0.0	5.0	7.54	0.80	0.85	16.0	0.0055	23.5	24	5.3	16.8	75	0.2	121.02	120.61
DI#7	SDMH#1	D-8	2594	0.06	2.56	5.0	0.0	5.0	7.54	0.80	0.85	16.3	0.0060	23.3	24	5.6	17.5	55	0.2	120.61	120.28
TREHCH DRAINS	DI#6	D-7	5489	0.13	0.13	5.0	0.0	5.0	7.54	0.90	0.90	0.9	0.0100	7.0	8	3.5	1.2	35	0.2	121.36	121.01
DI#6	SDMH#1	D-6	4538	0.10	0.23	5.0	0.0	5.0	7.54	0.90	0.90	1.6	0.0050	10.0	12	3.2	2.5	47	0.2	121.01	120.78
SDMH#1	DI#5	D-5	0	0.00	2.79	5.0	0.0	5.0	7.54	0.90	0.85	17.9	0.0065	23.8	24	5.8	18.2	58	0.2	120.28	119.90
DI#5	DI#2	D-4	2079	0.05	2.84	5.0	0.0	5.0	7.54	0.80	0.85	18.2	0.0065	23.9	24	5.8	18.2	85	0.2	119.90	119.35
DI#4	DI#3	D-3	1711	0.04	0.04	5.0	0.0	5.0	7.54	0.60	0.60	0.2	0.0050	4.4	12	3.2	2.5	77	0.4	121.09	120.70
DI#3	DI#2	D-2	4612	0.11	0.15	5.0	0.0	5.0	7.54	0.80	0.75	0.8	0.0070	7.4	12	3.8	3.0	50	0.2	120.70	120.35
DI#2	FES#7	D-1	3944	0.09	3.07	5.0	0.0	5.0	7.54	0.70	0.84	19.5	0.0075	23.9	24	6.2	19.6	100	0.3	119.35	118.60
CB#5	DI#1	C-2	18808	0.43	0.43	5.0	0.0	5.0	7.54	0.90	0.90	2.9	0.0065	12.1	12	3.7	2.9	26	0.1	118.57	118.40
DI#1	FES#6	C-1	16463	0.38	0.81	5.0	0.0	5.0	7.54	0.45	0.69	4.2	0.0050	14.5	15	3.7	4.6	40	0.2	118.40	118.20
FES#5	CB#4	B-6	47497	1.09	1.09	5.0	0.0	5.0	7.54	0.60	0.60	4.9	0.0100	13.5	15	5.3	6.5	22	0.1	115.58	115.36
CB#4	CB#3	B-5	40367	0.93	2.02	5.0	0.0	5.0	7.54	0.90	0.74	11.2	0.0300	15.0	15	9.1	11.2	25	0.0	115.36	114.61
FES#4	CB#3	B-4	87410	2.01	2.01	5.0	0.0	5.0	7.54	0.55	0.55	8.3	0.0334	13.1	15	9.6	11.8	25	0.0	115.45	114.61
CB#3	CB#2	B-3	8099	0.19	4.21	5.0	0.0	5.0	7.54	0.90	0.66	20.8	0.0100	23.2	24	7.2	22.6	144	0.3	114.61	113.17
CB#2	CB#1	B-2	8577	0.20	4.41	5.0	0.0	5.0	7.54	0.90	0.67	22.1	0.0100	23.8	24	7.2	22.6	25	0.1	113.17	112.92
CB#1	FES#3	B-1	8577	0.20	4.60	5.0	0.0	5.0	7.54	0.90	0.68	23.5	0.0120	23.5	24	7.9	24.8	56	0.1	112.92	112.25
FES#2	FES#1	A-1	2260145	51.89	51.89	5.0	0.0	20.0	4.56		0.40	94.6	0.0075	43.3	60	11.5	225.5	80	0.1	108.60	108.00

DISSIPA	ATOR	PAD/	DUTL	ET P	ROTEC	TION	SC	CHEDU	LE		=		
				(NYDOT	METHOD)								111
							1010	NUMBER	APRON	APRON	1	STONE	D50
LOCATION	Q(full)	Q(10)	V (full)	V(10)	PIPE DIA.1			OF PIPES	LENGTH ²	WIDTH ³	CLASS	DEPTH4	STONE
	CFS	CFS	FPS	FPS	INCHES	ZONE	M		(La) FEET	FEET	STONE	INCHES	SIZE
FES #13	21.07	14.32	6.71	4.56	24	1	4	1	8	8	A	18	6
FES#12	9.38	9.26	5.31	5.24	18	1	4	1	6	6	Α	18	6
FES#11	6.63	6.53	3.75	3.69	18	1	4	1	6	6	Α	18	6
FES#9	27.67	26.56	8.81	8.46	24	1	4	1	8	8	Α	18	6
FES#8	4.56	4.37	3.72	3.56	15	1	4	1	5	5	Α	18	6
FES#7	19.56	19.47	6.23	6.20	24	1	4	1	8	8	Α	18	6
FES#6	4.56	4.21	3.72	3.43	15	1	4	1	5	5	Α	18	6
FES#3	24.75	23.48	7.88	7.47	24	1	4	1	8	8	Α	18	6
FES#1	225.24	94.64	11.47	4.82	60	2	6	1	30	20	В	22	9
					Ĭ.								

NOTE: INSTALL GEOTEXTILE FABRIC BENEATH RIPRAP AREAS Pipes: Do = Pipe Diameter; Channels: Do = Square root of the cross sectional area of flow at channel outlet 2 La = MxDo; (IF N>1, La = 1.25 x MxDo)

Type A: W = 4 x Pipe Diameter; Type B: W = Channel Bottom + Sides up to top of bank Type A: d = use table ; Type B: d = 1.5 x MAX. STONE SIZE PIPE DIAMETER IS 1.25 OF THE EQUIVALENT DIAMETER FOR THE GIVEN FLOW

SKIMMER	TOTAL	DISTURBED	Tc	Cc			REQ	REQ				WER		PROP	PROP	AREA	PROP	BAF	FLE PLACE	MENT	SKIMMER	ORIFICE	DEWA TERIN
BASIN	AREA	AREA		RUNOFF	INTENSITY	Q-10	SURFACE	VOL	LENGTH	WIDTH	DEPTH	LENGTH	BAFFLES	SURFACE	SIDE SLOPE	воттом	VOLUME	(MEA SURE	D FROM IN	FLOW END)	SIZE	SIZE	TIME
SB#	ACRES	ACRES	MIN	COEFF	IN/HR	CFS	SQ FT	CF	FT	FT	FT	FT	REQ	SQ FT	FT/FT	SQ.FT.	CF	BAFFLE 1	BAFFLE 2	BAFFLE 3	IN	IN.	DAYS
1	5.67	5.2	5	0.6	7.22	24.56	7983	9360	180	45	2.0	23	3	8041	2:1	6308	14314	45	90	135	3.0	2.50	2.0
2	9.1	8.55	5	0.6	7.22	39.42	12812	15390	180	72	2.0	37	3	12886	2:1	10937	23796	45	90	135	3.0	2.75	2.7
*MINIMUM		A	3	0.6	1.22	39.42	12012	15590	100		2.0	31	3	12000	2.1	10937	23/90	43	90	133	3.0	2.73	- Canada

10-YEAR DESIGN STORM																	
	n=												1				
LOCATION	DRAINAGE	RUNOFF	RAINFALL	REQUIRED	DITCH	BOTTOM	FLOW	SIDE	FLOW	WETTED	HY DRAULIC	MANN.	ACTUAL	TOP	SHEAR	TEMP.	TEMPORA RY
7	AREA	COEFFICIENT	10-YEAR	FLOW	SLOPE	WIDTH	DEPTH	SLOPE	AREA	PERIMETER	RADIUS	VEL.	CAPACITY	WIDTH	STRESS	LINER	LINER TY PE
	ACRES	C	INHR	Q	s	d	у	М	Α	Р	R	٧	Q	W	(lbs/sq.ft.)	REQUIRED	
TEMPORARY DIV. #1	2.43	0.60	7.22	10.53	0.0050	0	1.097	2	2.41	4.91	0.49	4.37	10.53	4.39	0.34	YES	S-150
TEMPORARY DIV.#2	3.38	0.60	7.22	14.64	0.0070	0	1.166	2	2.72	5.21	0.52	5.38	14.64	4.66	0.51	YES	S-150
TEMPORARY DIV. #3 / SWALE#1	4.80	0.60	7.22	20.79	0.0120	0	1.017	3	3.11	6.44	0.48	6.70	20.79	6.10	0.76	YES	SC-150
SWALE#2	4.80	0.60	7.22	20.79	0.0050	0	1.199	3	4.31	7.58	0.57	4.82	20.79	7.19	0.37	YES	S-150
SWALE#3	4.15	0.40	7.22	11.99	0.0100	0	0.856	3	2.20	5.42	0.41	5.45	11.99	5.14	0.53	YES	S-150

4.8 0.60 20.79 0.0050 0 1.84 3 10.16 11.64 0.87 2.05 20.79 11.04 0.57 1.79 0.5 14.04

4.15 0.40 11.99 0.0100 0 1.38 3 5.67 8.70 0.65 2.11 11.99 8.25 0.86 1.38 0.5

PERMANENT SWALE CALCULATIONS 0-YEAR DESIGN STORM (n VALUES OBTAINED FROM CHART 8.05c OF THE NC EROSION AND SEDIMENT CONTROL PLANNING AND DESIGN MANUAL) DRAINAGE C REQUIRED DITCH BOTTOM FLOW SIDE FLOW WETTED HYDRAULIC MANN. ACTUAL TOP WIDTH SHEAR VR FREE TOP WIDTH AREA COEFF FLOW SLOPE WIDTH DEPTH SLOPE AREA PERIMETER RADIUS VEL. CAPACITY (FLOW) STRESS (RXV) BOARD (CHANNEL 0.72 3.05 20.79 9.05 1.13 2.18 0.5 12.05

FOR PLAN REVIEW ONLY

Chowley 022594

GREEN ENGINEERING

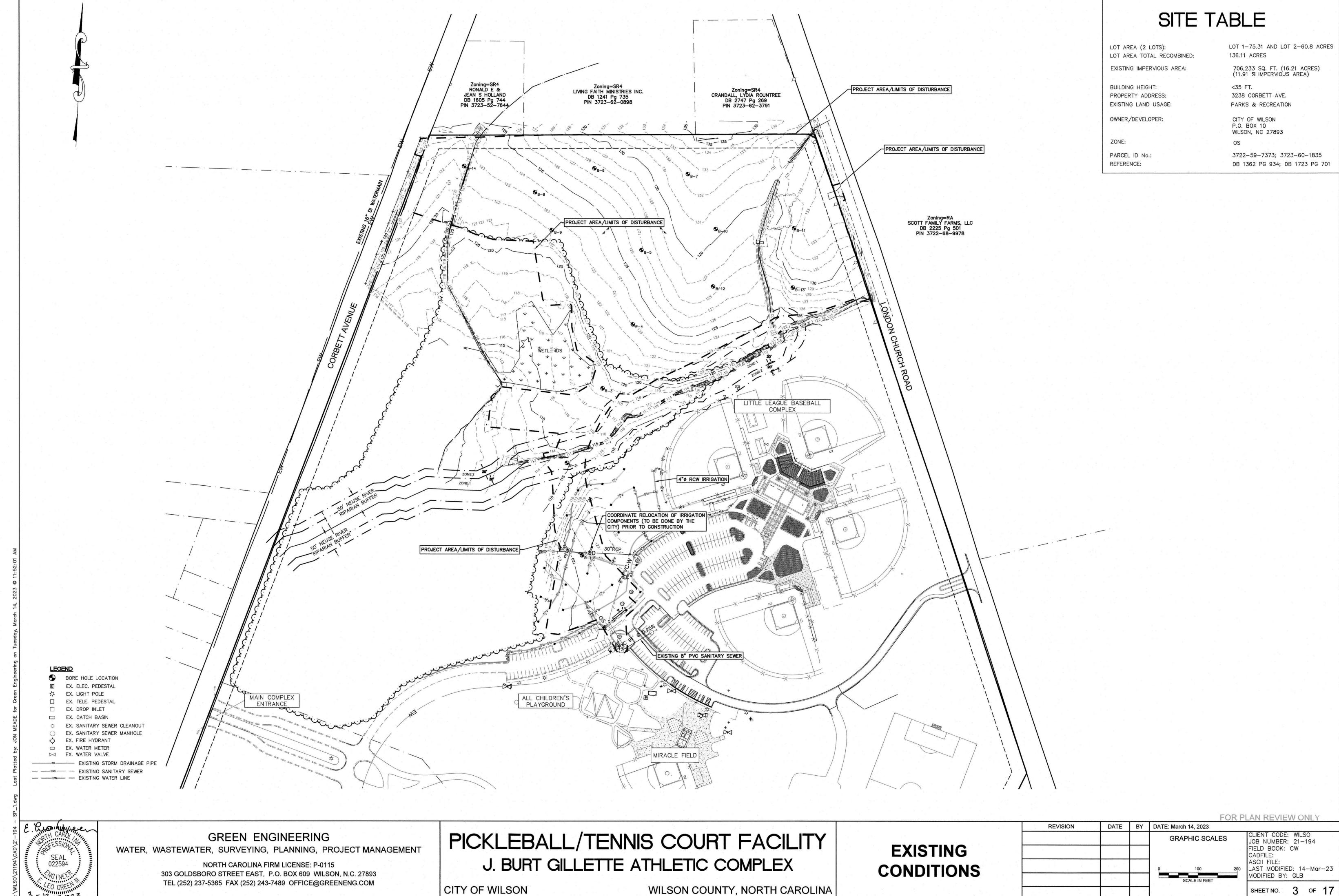
WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

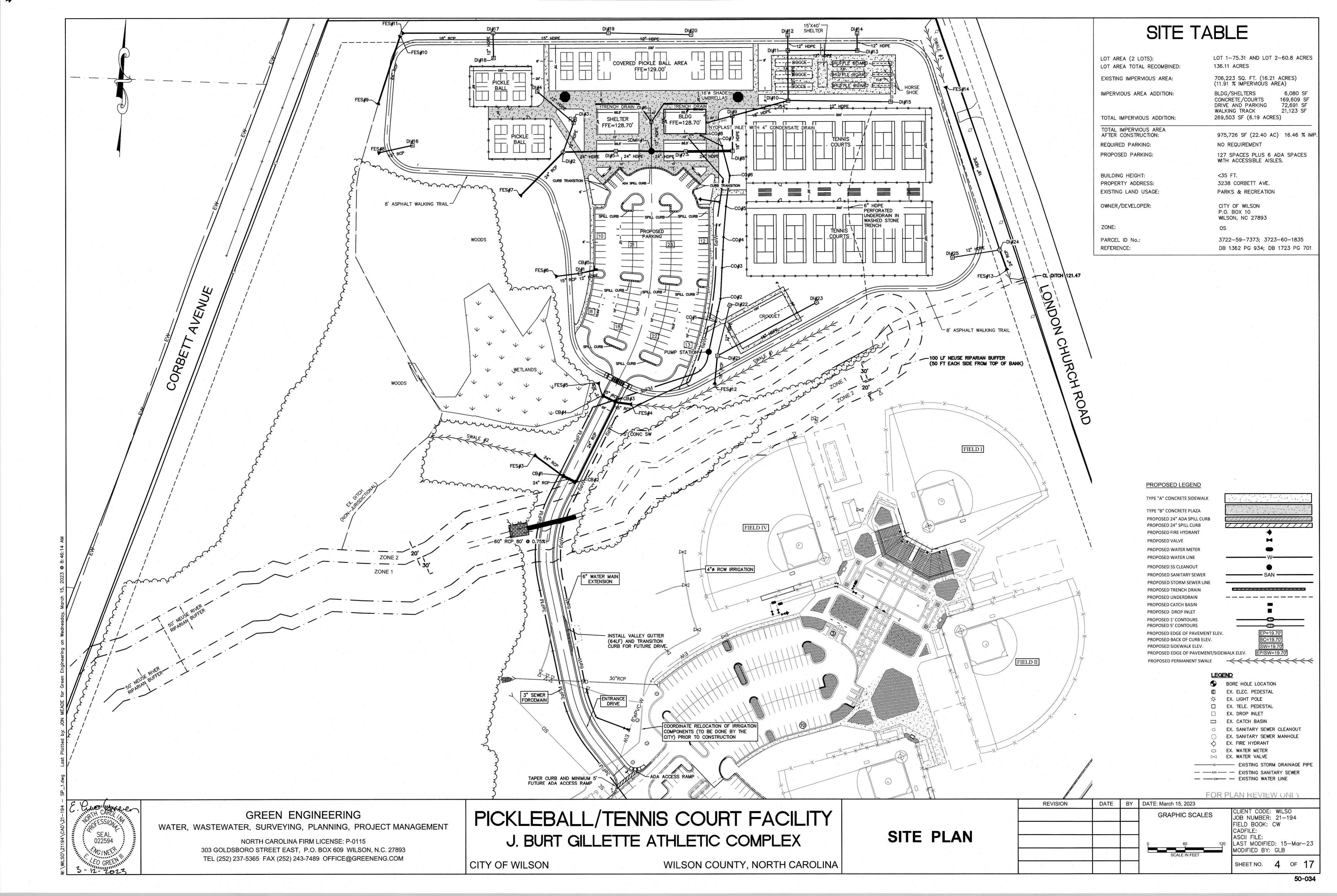
NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM PICKLEBALL/TENNIS COURT FACILITY J. BURT GILLETTEATHLETIC COMPLEX

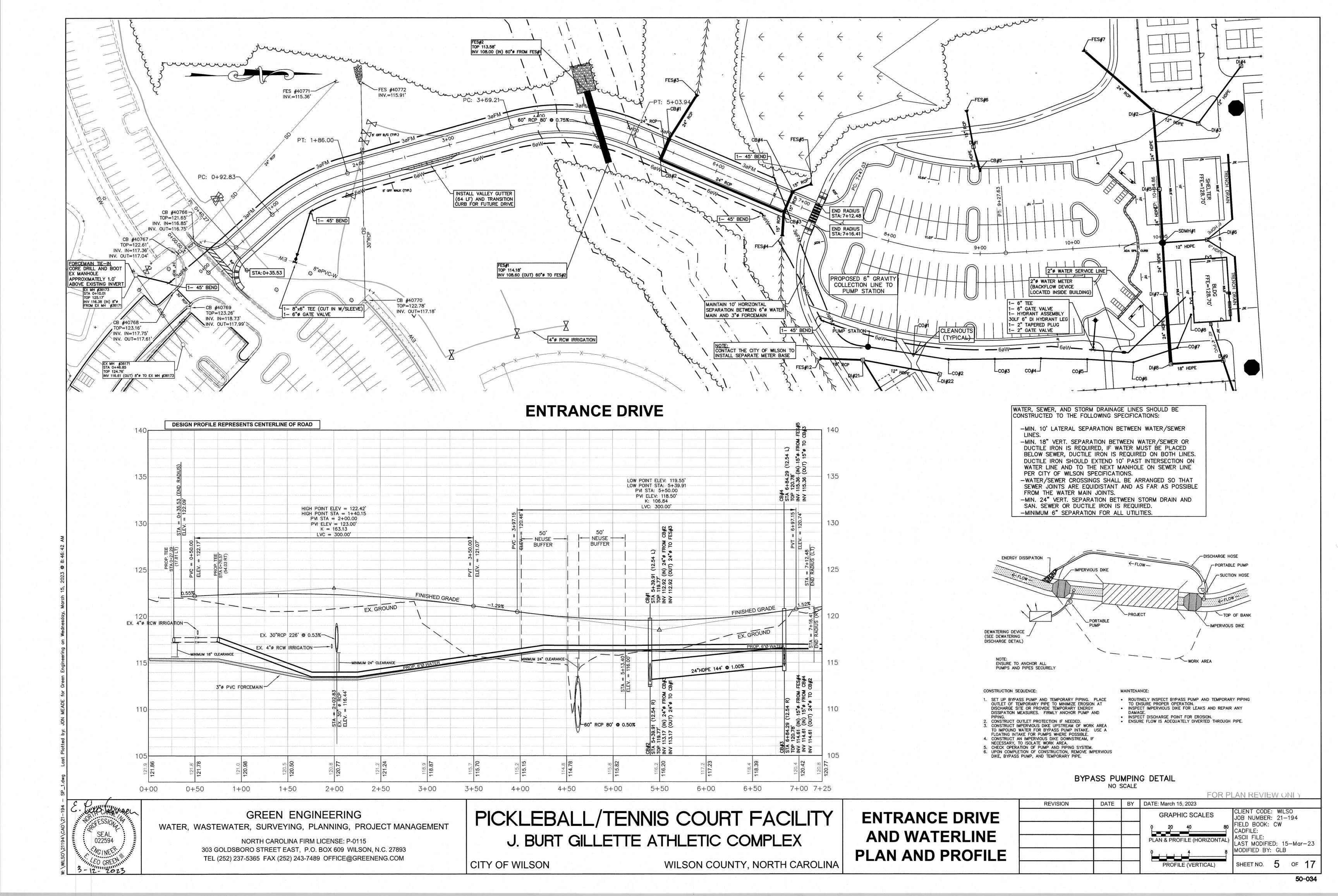
GENERAL CONSTRUCTION NOTES AND CALCULATIONS NORTH CAROLINA

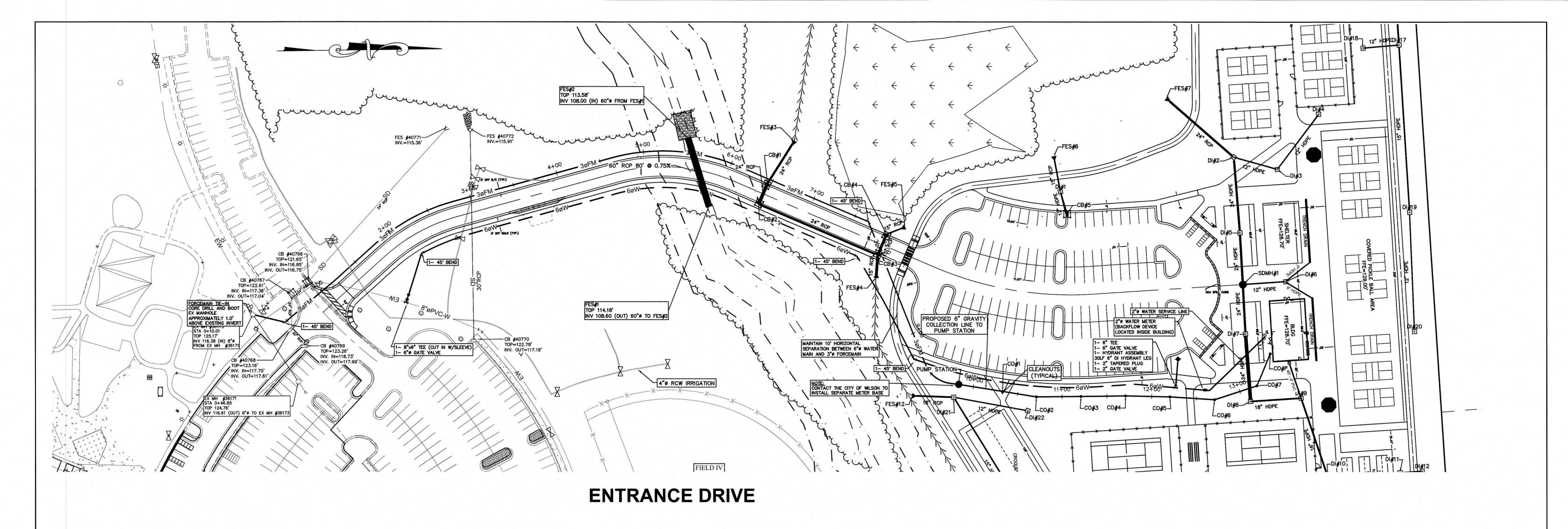
DATE BY DATE: March 14, 2023 CLIENT CODE: WILSO JOB NUMBER: 21-194 FIELD BOOK: XXXXX CADFILE: 21194-CV.dwg ASCII FILE: AS SHOWN LAST MODIFIED: 14-Mar-23 MODIFIED BY: JM

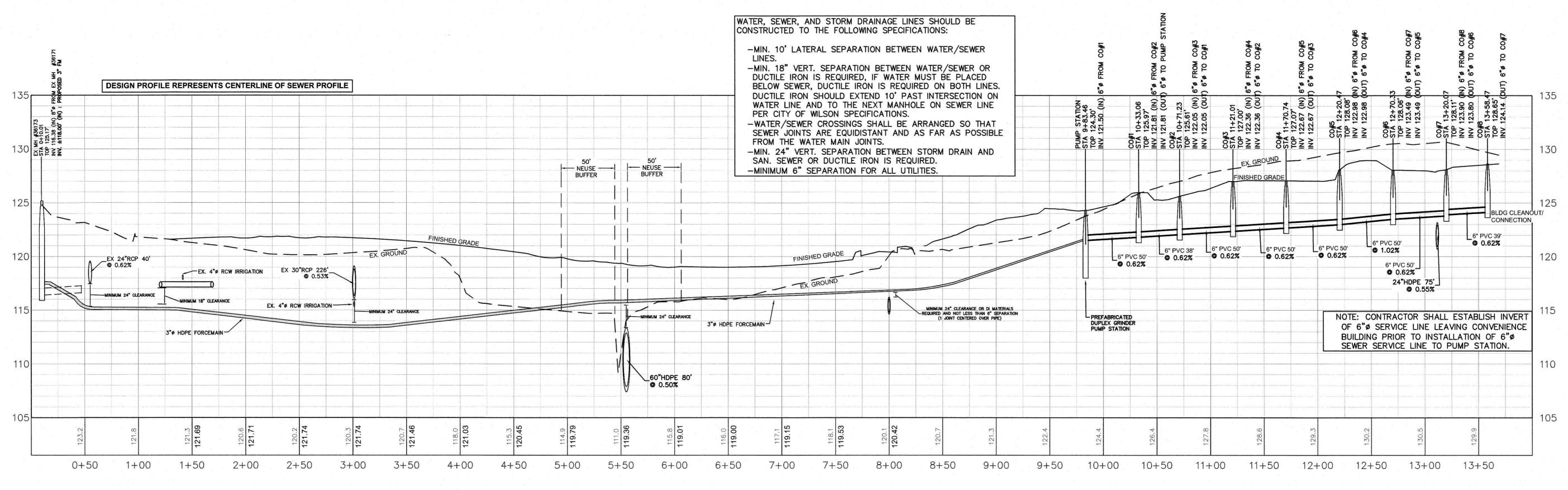
SHEET NO. 2 OF 17











022594

GREEN ENGINEERING

WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM

PICKLEBALL/TENNIS COURT FACILITY J. BURT GILLETTE ATHLETIC COMPLEX

WILSON COUNTY, NORTH CAROLINA CITY OF WILSON

SANITARY SEWER PLAN AND PROFILE

	REVISION	DATE	BY	DATE: March 15, 2023	
2	2 1			GRAPHIC SCALES	CLIENT CODE: WILS JOB NUMBER: 21-
					FIELD BOOK: CW CADFILE:
5 A 2 II		3 8 8		PLAN & PROFILE (HORIZONTAL)	ASCII FILE: LAST MODIFIED: 15
v edi	a fi = 4 3 2	a 11.5		0 5 10	MODIFIED BY: GLB
-				PROFILE (VERTICAL)	SHEET NO. 6

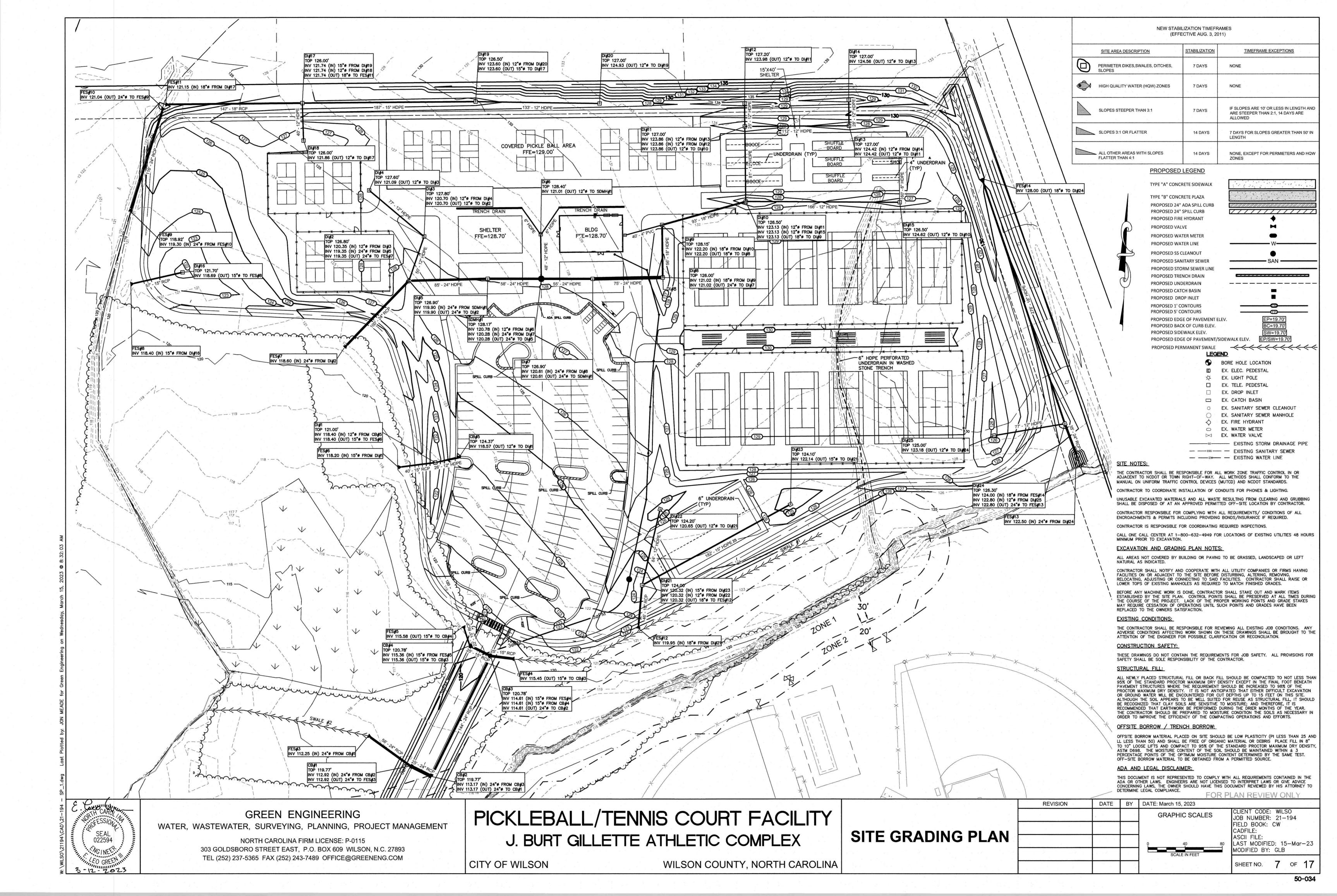
FOR PLAN REVIEW ONLY

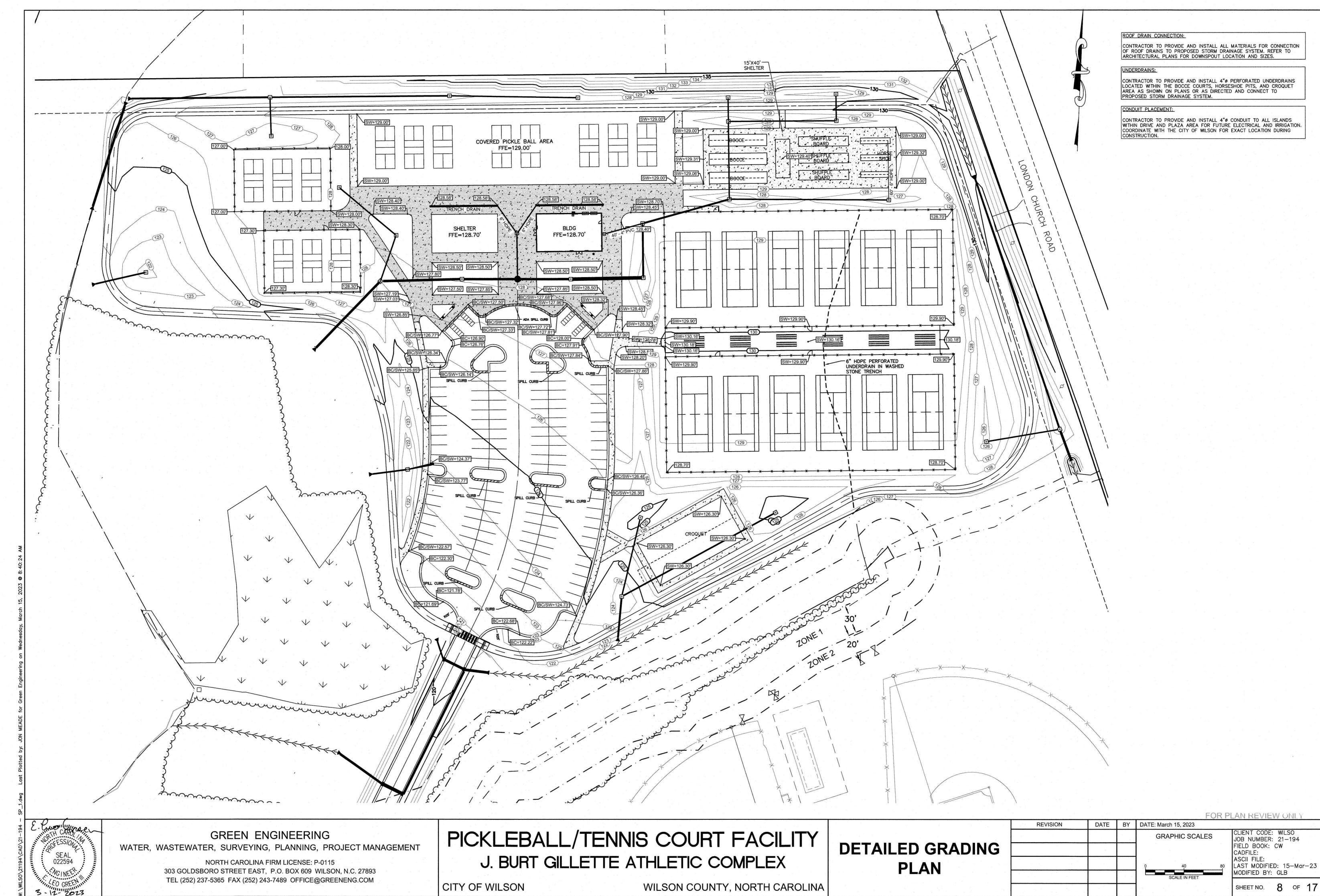
CLIENT CODE: WILSO

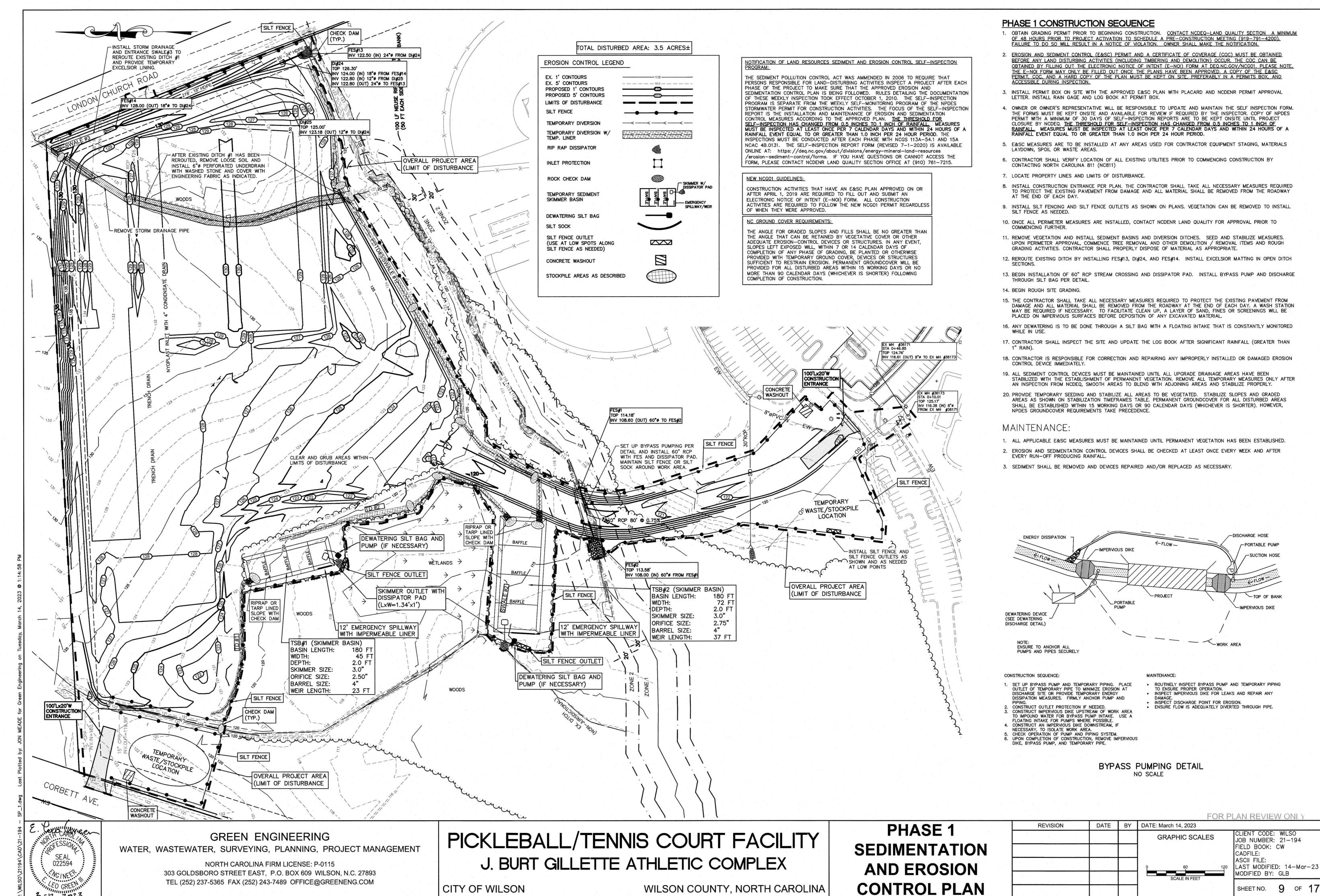
JOB NUMBER: 21-194

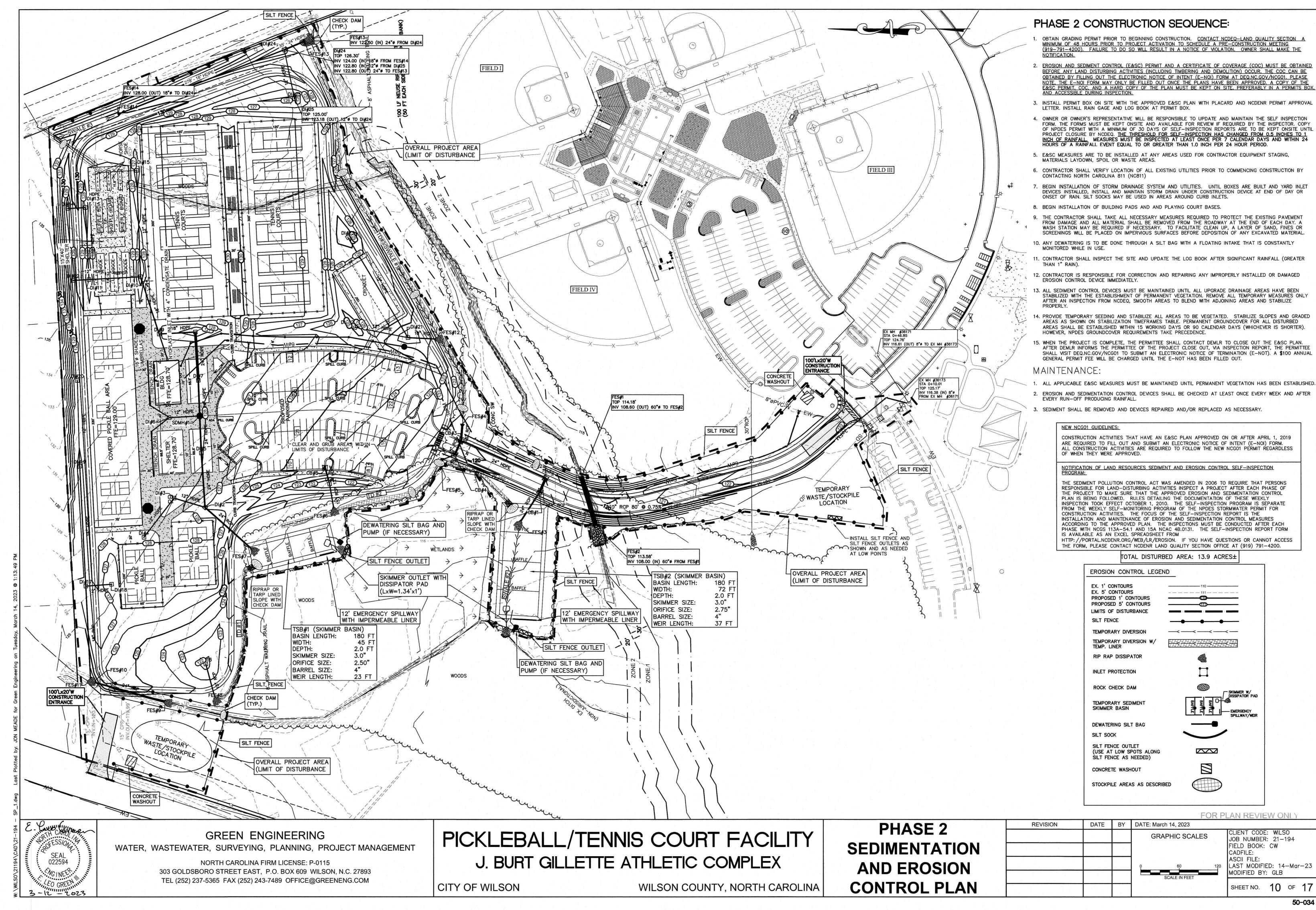
LAST MODIFIED: 15-Mar-23

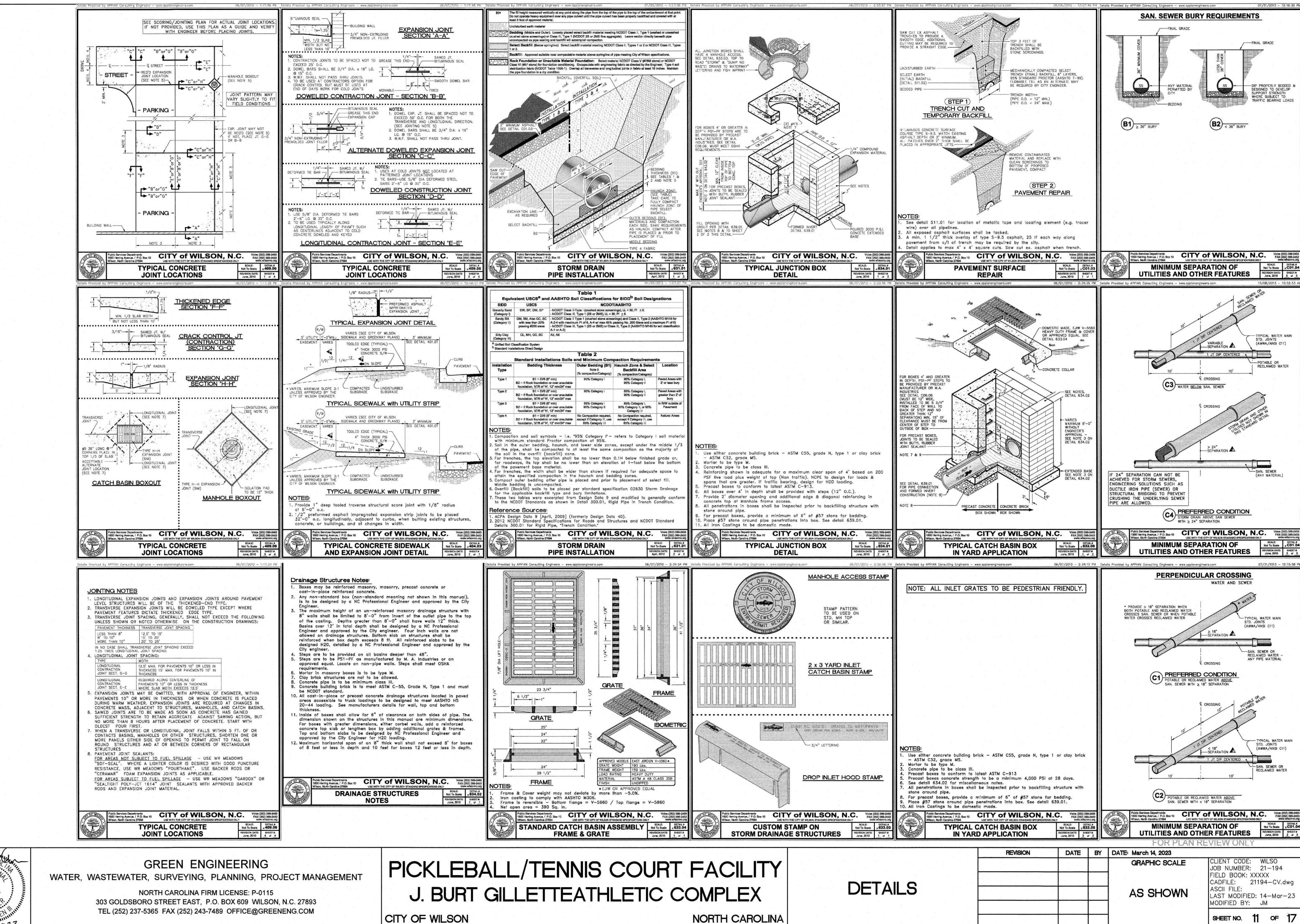
SHEET NO. 6 OF 17





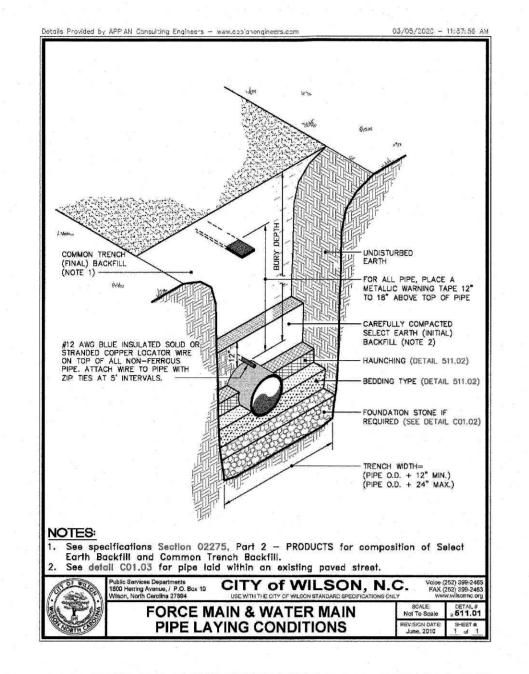


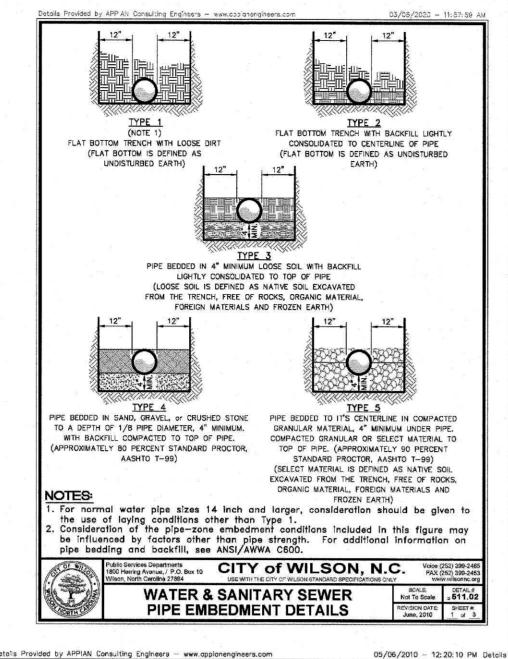


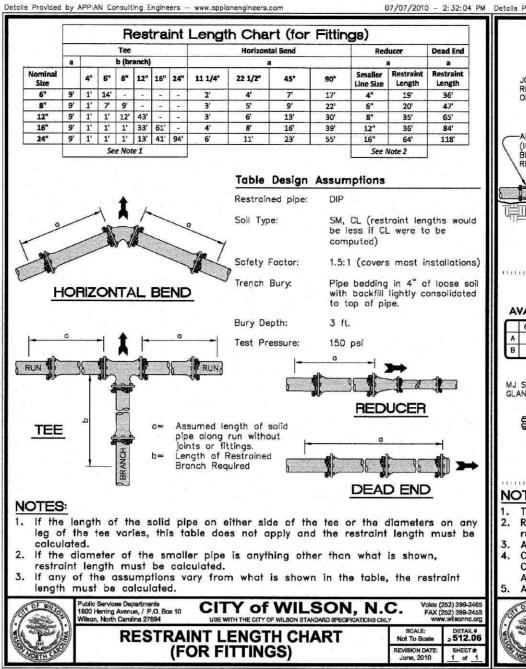


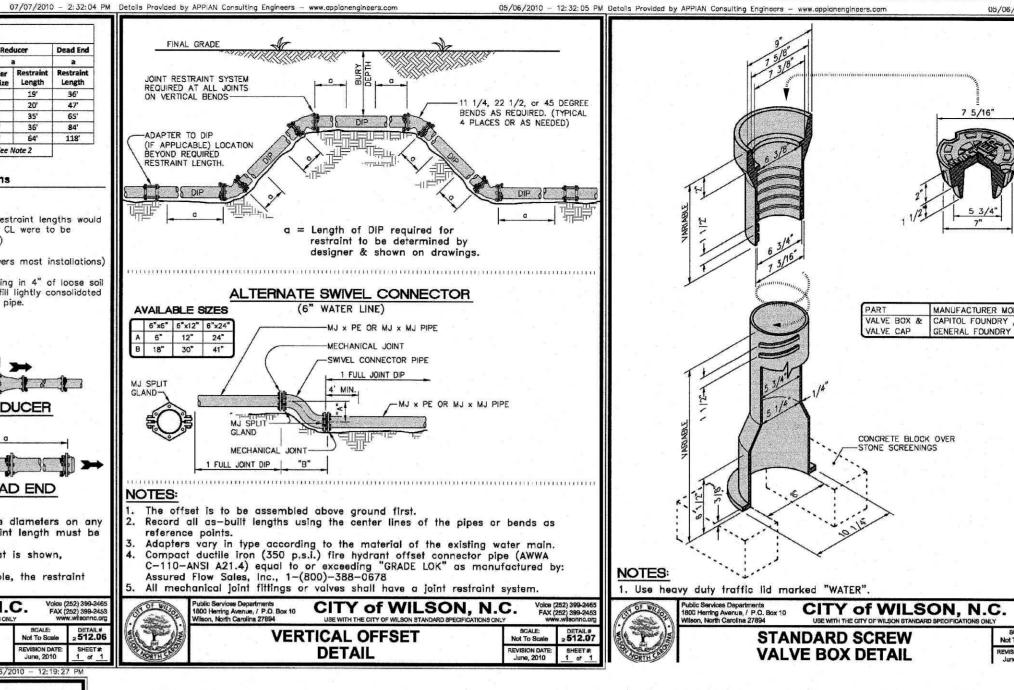
022594

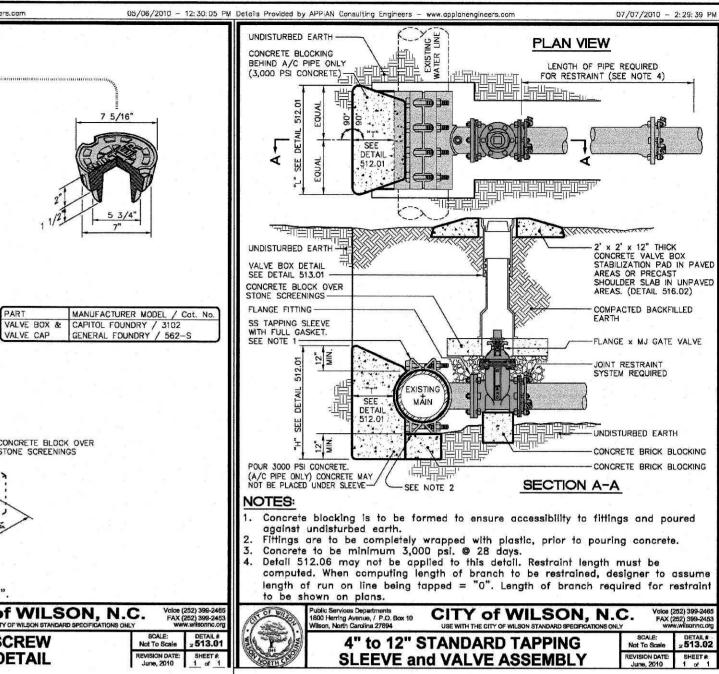
-034



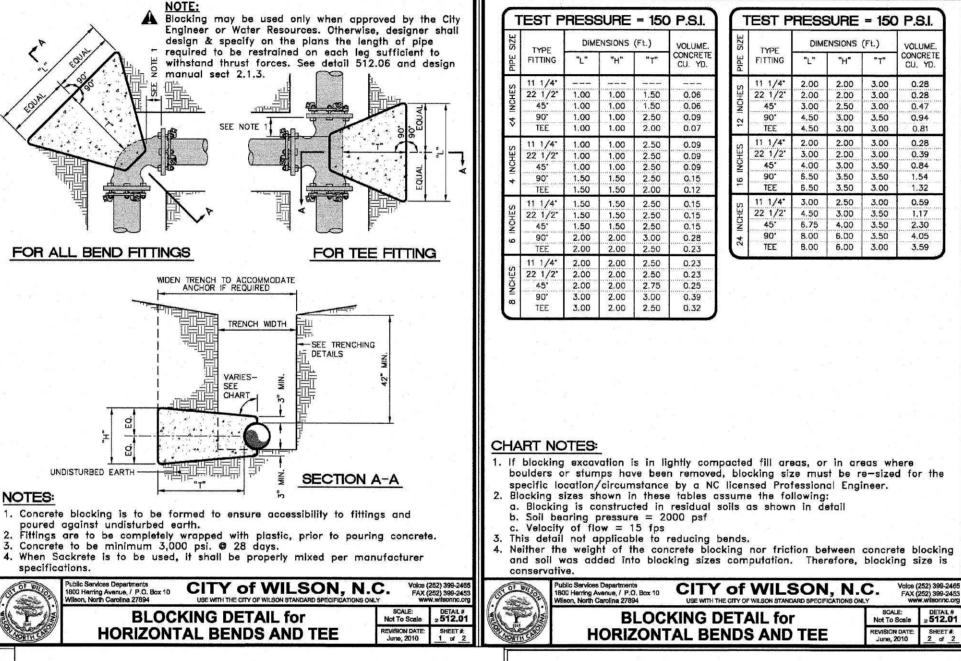


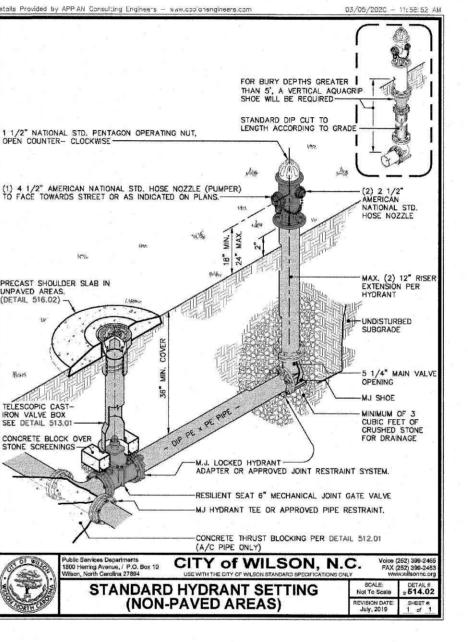


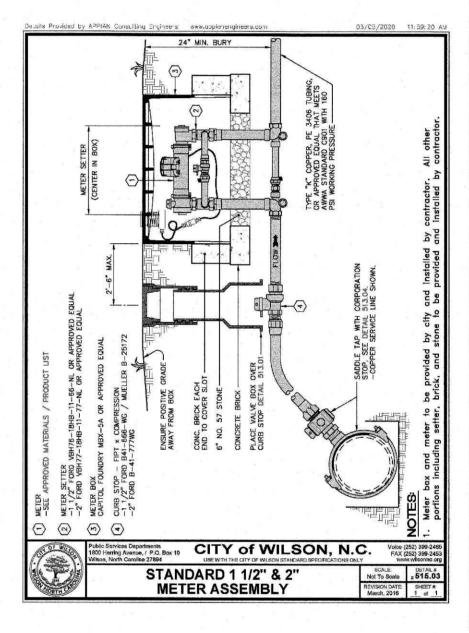


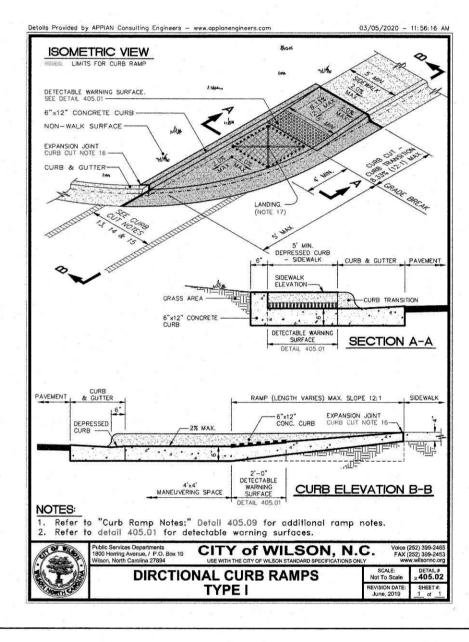


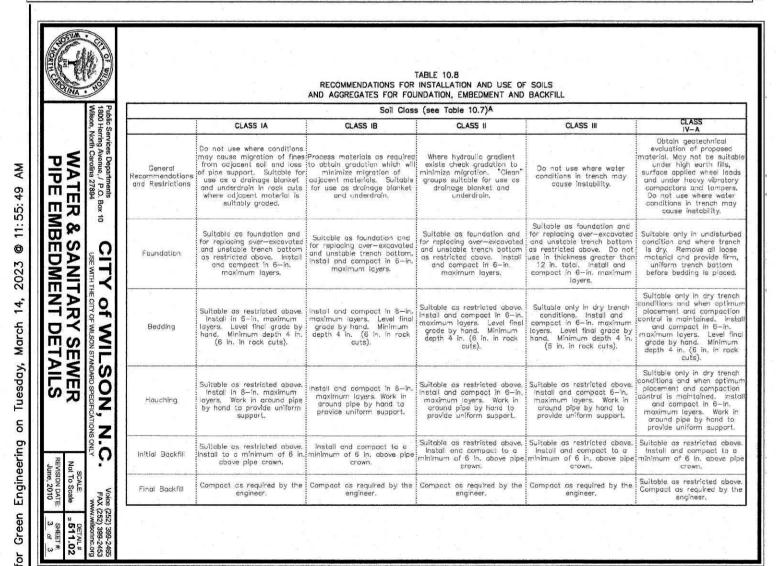
-	13					Percent	tage Passing Siev	e Sizes	Atterbre	g Limits	Coeffi	icients
S. Contract	or Will	Class	Туре	Soil Symbol Group	Description ASTM D 2487	1.5 in (40 mm)	No. 4	No. 200 (0.075 mm)	ш	PL	Uniformity Cu	5
NA NA	18 P	IA.	Monufactured Aggregates: open graded, clean	None	Angular, crushed stone or rock, crushed gravel, broken coral, crushed slag, cinders or shells: large vaid content, contain little or no fines	100	≤10%	<5	Non Plastic			
son, North Car	Public Services Departments 1800 Herring Avenue, / P.O.	18	Manufactured, Processed Aggregates: dense graded, clean	None	Angular, crushed stone (or ather Class IA materials) and stone/sond mixtures with gradations selected to minimize migration of adjacent soils: contain little or no fines	100	≤50%	<5	Non Plastic			
Olina 27	epartme	11	Coarse-Grained Soils, clean	GW	Well-graded gravels and gravel-sand mixtures; little or no fines	100	<50% of Coarse Fraction	<5	Non Plastic		>4	1 to
894	ants O.O. Box		·	GP	Poorly-graded gravels and gravel-sand mixtures; little or no fines						<4	<1 or
	× 10			SW	Well-graded sands and gravelly sands; little or no fines		>50% of Coarse Fraction				>6	1 to
	ဂ		54 B	SP	Poorly—graded sands and gravelly sands; little or no fines				-		<6	<1 or
SE WITH THE	Ţ		Coarse-Grained Salls, borderline clean to w/fines	e.g. GW-GC, SP-SM	Sonds and gravels which are borderline between clean and with fines	100	Varies	5% to 12%	Non Plastic	8	Same as GP, SW	
THE CIT	0	B)	Coarse-Grained Soils with Fines	GM	Silty gravels, gravel—sand—silt mixtures	100	>50% of Coarse Fraction	>12% to <50%		<4 or <"A" Line		
CITY OF WI			. c v	GC	Clayey gravels, gravel—sand—clay mixtures				-	<7 or >"A" Line		
SON STANDARD	S		0	SM	Silty sonds, sand-silt mixtures		>50% of Coarse Fraction			>4 or <"A" Line		
ANDAR	S			sc	Clayey sands, sand—silt mixtures					>7 or. >"A" Line		
USPECIFI	2	IV	Fine-Grained Soils (inorganic)	ML	inorganic silts and very fine sands, rack flour, silty or clayey fine sands, silts with slight plasticity	100	100	>50	<50	<4 or <"A" Line	13	,,,,
CATIONS	Z		va .	CL.	Inorganic clays of low to medium plasticity, gravely clays, sandy clays, silty clays, lean clays					>7 or >"A" Line		
ONLY	S	IVB	Fine-Grained Soils (inorganic)	ИН	Inorganic silts, micaceaus or diatomaceaus fine sondy or silty soile, elastic silts	100	100	>50	>50	<"A" Line		
	∓ Vo			СН	inorganic clays of high plasticity, fat clays					>"A" Line		
N.MAMAN	Voice (25: FAX (25:	٧	Organic Soils	OL	Organic silts and organic silty clays of low plasticity	100	100	>50	<50	<4 or <"A" Line		
MISOUR	(252) 399-2465 (252) 399-2453			ОН	Organic aloys of medium to high plasticity, organic silts				>50	<"A" Line		
Sio.s	45.6		Highly Organic	PT	Peat and other high organic soils							

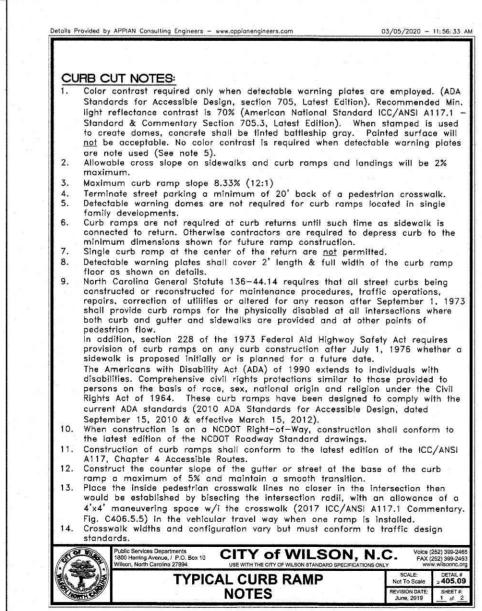


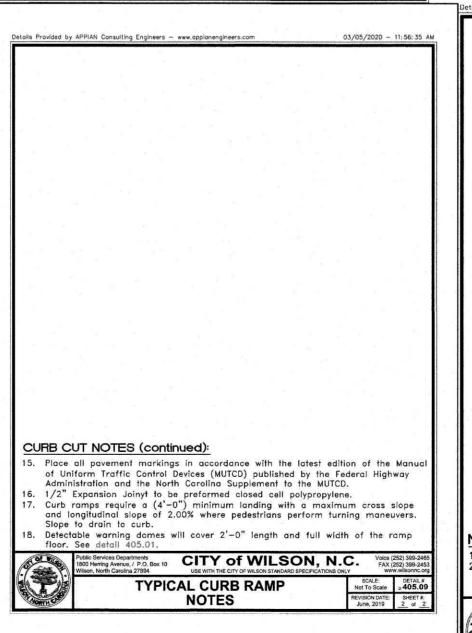


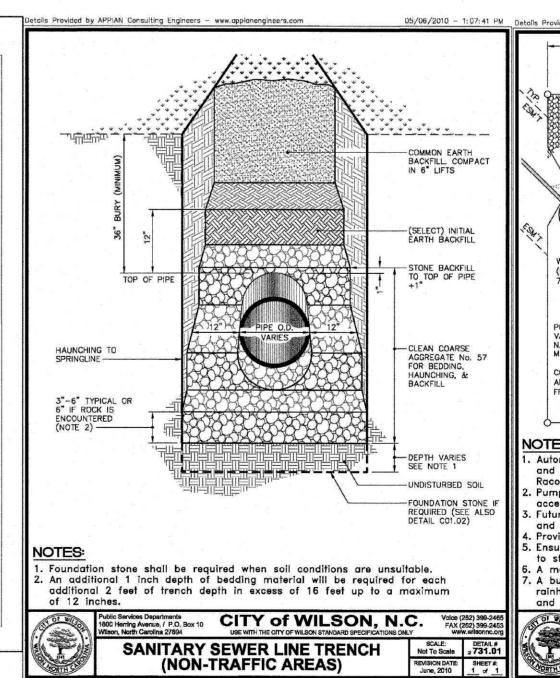


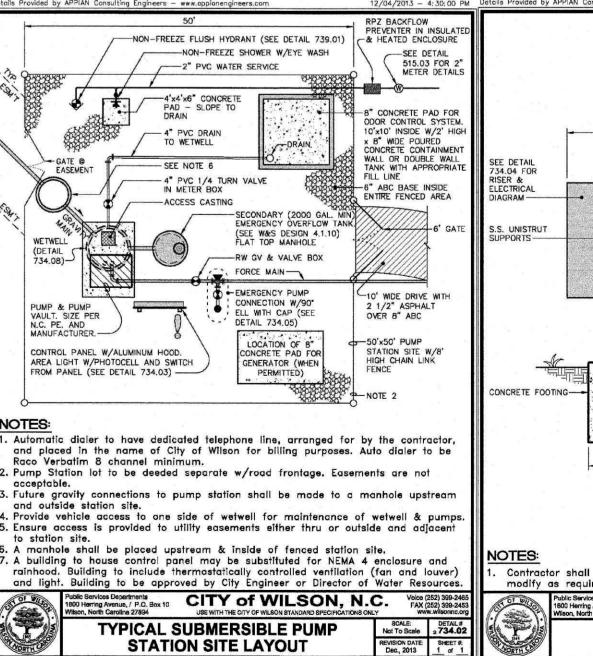


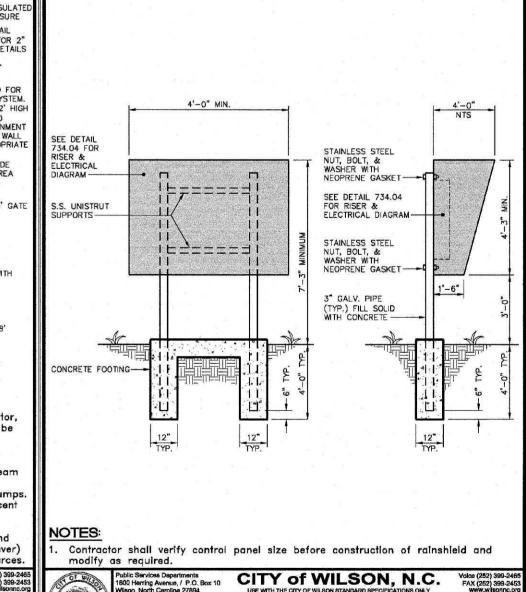




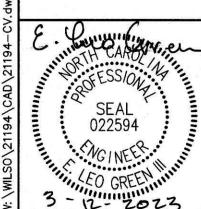








RAINHOOD



GREEN ENGINEERING
WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM PICKLEBALL/TENNIS COURT FACILITY

J. BURT GILLETTEATHLETIC COMPLEX

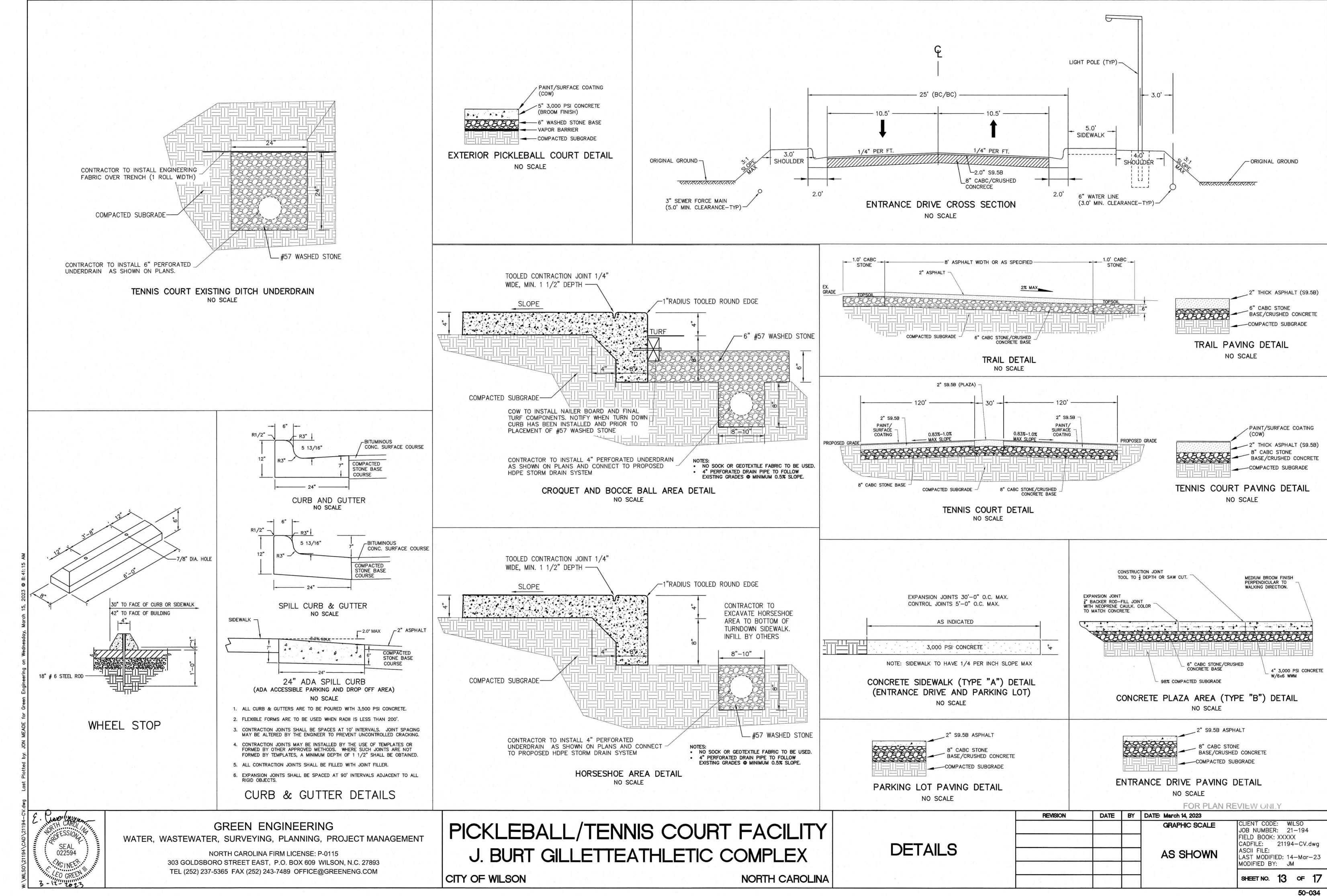
DETAILS

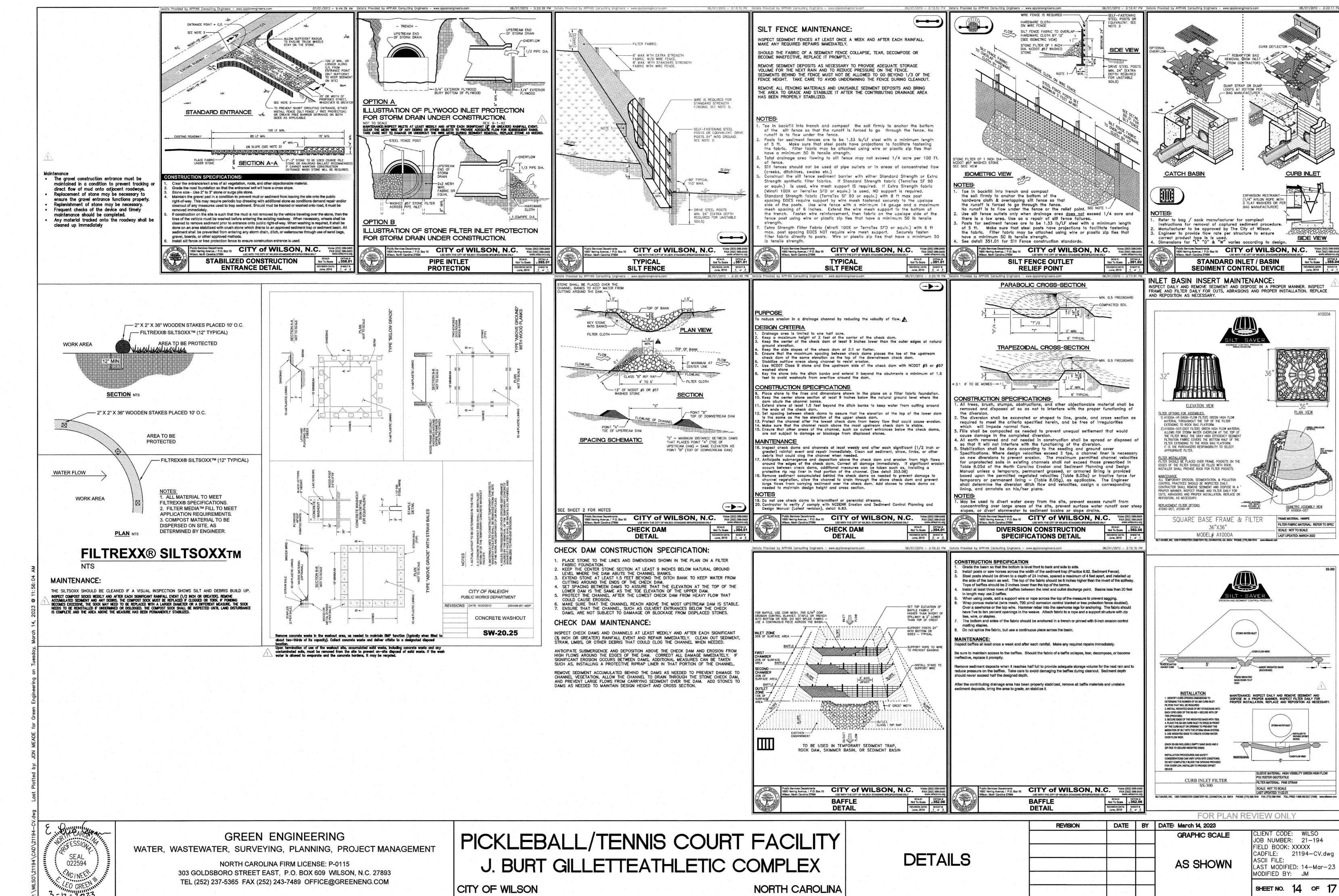
TE LAYOUT	Dec., 2013 1	of 1	DE	ETAIL	June, 2010 1 of
			FOR PLAI	N REVIEW ONLY	
REVISION	DATE	BY	DATE: March 14, 2023		LIGHTING AND THE COURT OF STREET
			AS SHOWN	CLIENT CODE: JOB NUMBER: FIELD BOOK: X> CADFILE: 21 ASCII FILE: LAST MODIFIED MODIFIED BY:	(XXX 194-CV.dwg
				SHEET NO. 1	2 of 17

CITY OF WILSON

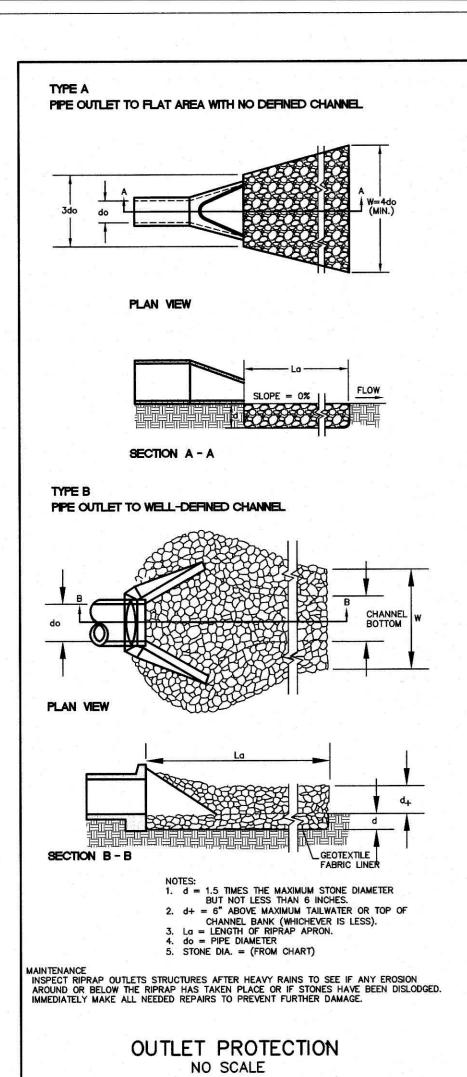
NORTH CAROLINA

FO 004





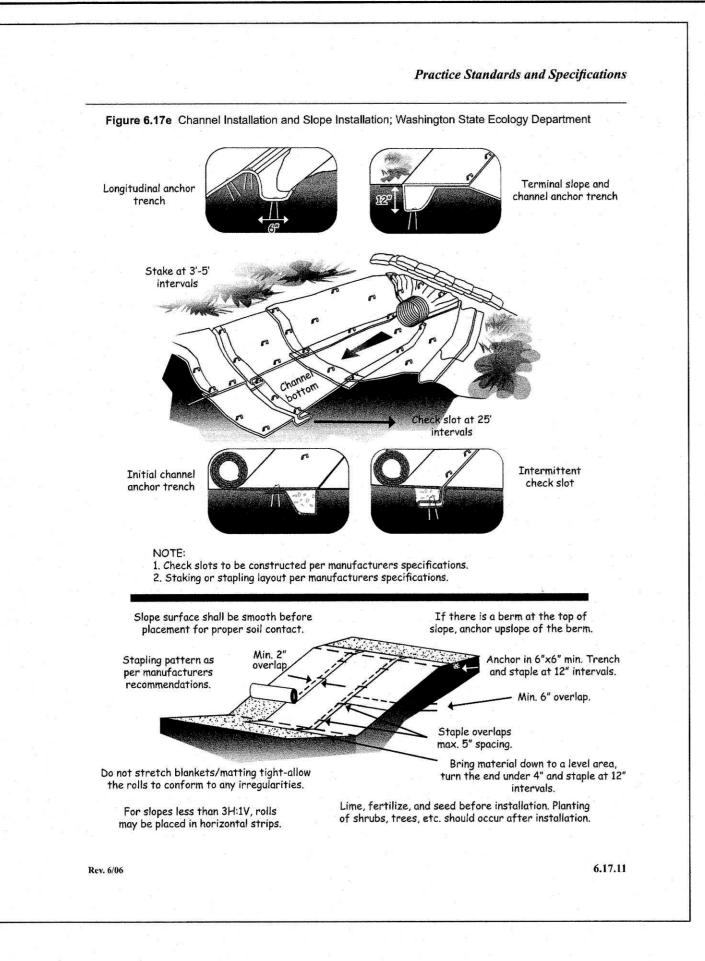
-034

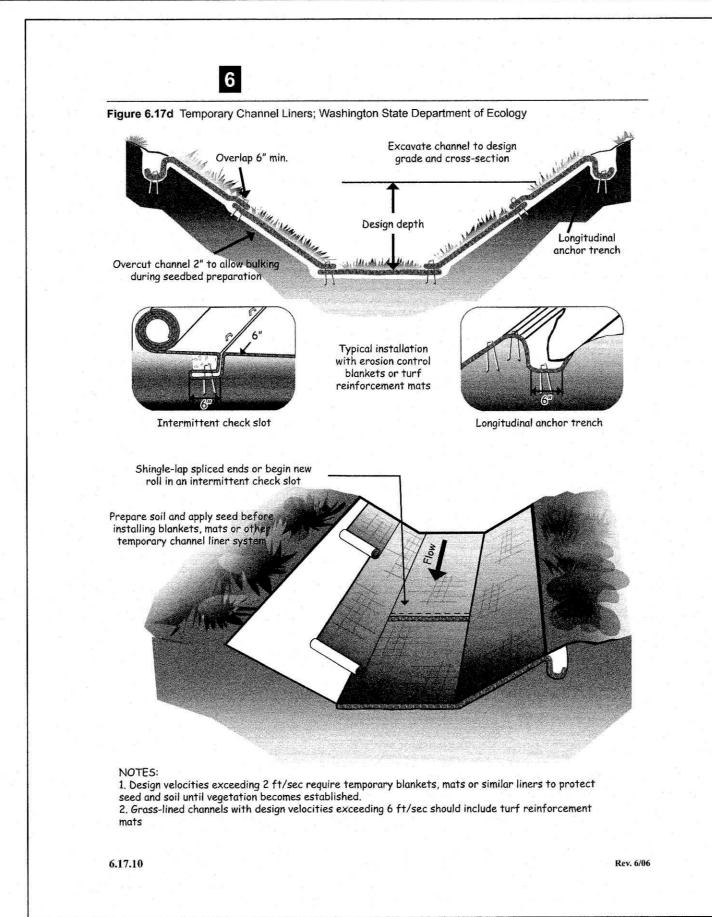


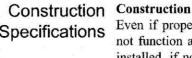
~ ~~

#57 WASHED STONE PLACED TO -/ A HEIGHT OF 12"-18" MIN. ABOVE TOP OF BOX

CATCH BASIN / YARD INLET PROTECTION DETAIL







Specifications Even if properly designed, if not properly installed, RECP's will probably not function as desired. Proper installation is imperative. Even if properly installed, if not properly timed and nourished, vegetation will probably not grow as desired. Proper seed/vegetation selection is also imperative.

> Grade the surface of installation areas so that the ground is smooth and loose. When seeding prior to installation, follow the steps for seed bed preparation, soil amendments, and seeding in Surface Stabilization, 6.1. All gullies, rills, and any other disturbed areas must be fine graded prior to installation. Spread seed before RECP installation. (Important: Remove all large rocks, dirt clods, stumps, roots, grass clumps, trash, and other obstructions from the soil surface to allow for direct contact between the soil surface and the RECP.)

Terminal anchor trenches are required at RECP ends and intermittent trenches must be constructed across channels at 25-foot intervals. Terminal anchor trenches should be a minimum of 12 inches in depth and 6 inches in width, while intermittent trenches need be only 6 inches deep and 6 inches wide.

Installation for Slopes—Place the RECP 2-3 feet over the top of the slope and into an excavated end trench measuring approximately 12 inches deep by 6 inches wide. Pin the RECP at 1 foot intervals along the bottom of the trench, backfill, and compact. Unroll the RECP down (or along) the slope maintaining direct contact between the soil and the RECP. Overlap adjacent rolls a minimum of 3 inches. Pin the RECP to the ground using staples or pins in a 3 foot center-to-center pattern. Less frequent stapling/pinning is acceptable on moderate slopes.

Installation in Channels—Excavate terminal trenches (12 inches deep and 6 inches wide) across the channel at the upper and lower end of the lined channel sections. At 25-foot intervals along the channel, anchor the RECP across the channel either in 6 inch by 6 inch trenches or by installing two closely spaced rows of anchors. Excavate longitudinal trenches 6 inches deep and wide along channel edges (above water line) in which to bury the outside RECP edges. Place the first RECP at the downstream end of the channel. Place the end of the first RECP in the terminal trench and pin it at 1 foot intervals along the bottom of the trench.

Note: The RECP should be placed upside down in the trench with the roll on the downstream side of the bench.

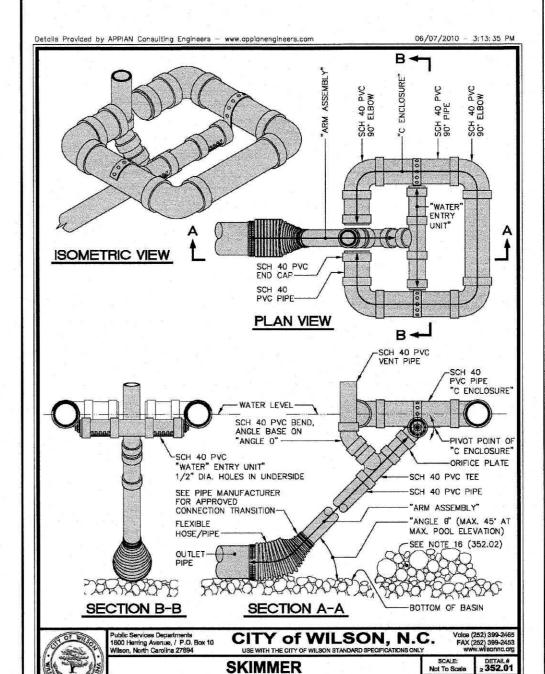
Once pinned and backfilled, the RECP is deployed by wrapping over the top of the trench and unrolling upstream. If the channel is wider than the provided rolls, place ends of adjacent rolls in the terminal trench, overlapping the adjacent rolls a minimum of 3 inches. Pin at 1 foot intervals, backfill, and compact. Unroll the RECP in the upstream direction until reaching the first intermittent trench. Fold the RECP back over itself, positioning the roll on the downstream side of the trench, and allowing the mat to conform to the

Then pin the RECP (two layers) to the bottom of the trench, backfill, and compact. Continue up the channel (wrapping over the top of the intermittent trench) repeating this step at other intermittent trenches, until reaching the upper terminal trench.

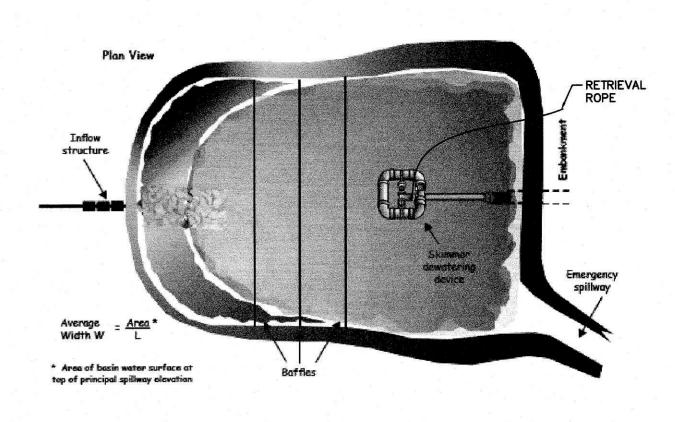
At the upper terminal trench, allow the RECP to conform to the trench, secure with pins or staples, backfill, compact and then bring the mat back over the top of the trench and onto the existing mat (2 to 3 feet overlap in the downstream direction), and pin at 1 foot intervals across the RECP. When starting installation of a new roll, begin in a trench or shingle-lap ends of rolls a minimum of 1 foot with upstream RECP on top to prevent uplifting. Place the outside edges of the RECP(s) in longitudinal trenches, pin, backfill, and

Anchoring Devices—11 gauge, at least 6 inches length by 1 inch width staples or 12 inch minimum length wooden stakes are recommended for anchoring the RECP to the ground.

Drive staples or pins so that the top of the staple or pin is flush with the ground surface. Anchor each RECP every 3 feet along its center. Longitudinal overlaps must be sufficient to accommodate a row of anchors and uniform along the entire length of overlap and anchored every 3 feet along the overlap length. Roll ends may be spliced by overlapping 1 foot (in the direction of water flow), with the upstream/upslope mat placed on top of the downstream/ downslope RECP. This overlap should be anchored at 1 foot spacing across the RECP. When installing multiple width mats heat seamed in the factory, all factory seams and field overlaps should be similarly anchored.



DETAIL



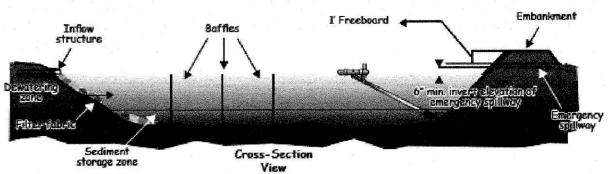


Figure 6.64c Example of a sediment basin with a skimmer outlet and emergency spillway. From Pennsylvania

CONSTRUCTION SPECIFICATION

- CLEAR, GRUB, AND STRIP THE AREA UNDER THE EMBANKMENT OF ALL VEGETATION AND ROOT MAT. REMOVE ALL SURFACE SOIL CONTAINING HIGH AMOUNTS OF ORGANIC MATTER AND STOCKPILE OR DISPOSE OF IT PROPERLY. HAUL ALL OBJECTIONABLE MATERIAL TO THE DESIGNATED DISPOSAL AREA. PLACE TEMPORARY SEDIMENT CONTROL MEASURES ENSURE THAT FILL MATERIAL FOR THE EMBANKMENT IS FREE OF ROOTS, WOODY VEGETATION, ORGANIC MATTER, AND
- OVERFILL THE EMBANKMENT 6 INCHES TO ALLOW FOR SETTIEMENT. SHAPE THE BASIN TO THE SPECIFIED DIMENSIONS. PREVENT THE SKIMMING DEVICE FROM SETTLING INTO THE MUCH B EXCAVATING A SHALLOW PIT UNDER THE SKIMMER OR PROVIDING A LOW SUPPORT UNDER THE SKIMMER OF STONE O
- 4. PLACE THE BARREL (TYPICALLY 4-INCH SCHEDULE 40 PVC PIPE) ON A FIRM, SMOOTH FOUNDATION OF IMPERVIOUS SOIL DO NOT USE PERVIOUS MATERIAL SUCH AS SAND, GRAVEL OR CRUSHED STONE AS BACKFILL AROUND THE PIPE. PLACE THE FILL MATERIAL AROUND THE PIPE SPILLWAY IN 4-INCH LAYERS AND COMPACT IT UNDER AND AROUND THE PIPE TO AT LEAST THE SAME DENSITY AS THE ADJACENT EMBANKMENT. CARE MUST BE TAKEN NOT TO RAISE THE PIPE FROM THE FIRM CONTACT WITH ITS FOUNDATION WHEN COMPACTING UNDER THE PIPE HAUNCHES. PLACE A MINIMUM DEPTH OF 2 FEET OF COMPACTED BACKFILL OVER THE PIPE SPILLWAY BEFORE CROSSING IT WITH CONSTRUCTION EQUIPMENT. IN NO CASE SHOULD THE PIPE CONDUIT BE INSTALLED BY CUTTING A TRENCH THROUGH THE DAM AFTER THE EMBANKMENT IS
- ASSEMBLE THE SKIMMER FOLLOWING THE MANUFACTURERS INSTRUCTIONS, OR AS DESIGNED. LAY THE ASSEMBLED SKIMMER ON THE BOTTOM OF THE BASIN WITH THE FLEXIBLE JOINT AT THE INLET OF THE BARREL PIPE. ATTACH THE FLEXIBLE JOINT TO THE BARREL PIPE AND POSITION THE SKIMMER OVER THE EXCAVATED PIT OR
- SUPPORT. BE SURE TO ATTACH A ROPE TO THE SKIMMER AND ANCHOR IT TO THE SIDE OF THE BASIN. THIS WILL BE USED TO PULL THE SKIMMER TO THE SIDE FOR MAINTENANCE.

 EARTHEN SPILLWAYS—INSTALL THE SPILLWAY IN UNDISTURBED SOIL TO THE GREATEST EXTENT POSSIBLE. THE
 ACHIEVEMENT OF PLANNED ELEVATIONS, GRADE, DESIGN WDTH, AND ENTRANCE AND EXIT CHANNEL SLOPES ARE CRITICAL
 TO THE SUCCESSFUL OPERATION OF THE SPILLWAY. THE SPILLWAY SHOULD BE LINED WITH LAMINATED PLASTIC OR
 IMPERMEABLE GEOTEXTILE FABRIC. THE FABRIC MUST BE WIDE AND LONG ENOUGH TO COVER THE BOTTOM AND SIDES
 AND EXTEND ONTO THE TOP OF THE DAM FOR ANCHORING IN A TRENCH. THE EDGES MAY BE SECURED WITH 8—INCH
 STAPLES OR PINS. THE FABRIC MUST BE LONG ENOUGH TO EXTEND DOWN THE SLOPE MAY BE ONE STABLE GROUND. THE WIDTH OF THE FABRIC MUST BE ONE PIECE, NOT JOINED OR SPLICED: OTHERWISE WATER CAN GET UNDER THE FABRIC. IF THE LENGTH OF THE FABRIC IS INSUFFICIENT FOR THE ENTIRE LENGTH OF THE SPILLWAY, MULTIPLE SECTIONS, SPANNING THE COMPLETE WIDTH, MAY BE USED. THE UPPER SECTIONS(S) SHOULD OVERLAP THE LOWER SECTION(S) SO THAT WATER CANNOT FLOW UNDER THE FABRIC. SECURE THE UPPER EDGE AND SIDES OF THE FABRIC IN A TRENCH WITH STAPLES OR PINS. 8. INLETS—DISCHARGE WATER INTO THE BASIN IN A MANNER TO PREVENT EROSION. USE
- TEMPORARY SLOPE DRAINS OR DIVERSIONS WITH OUTLET PROTECTION TO DIVERT SEDIMENT-LADEN WATER TO THE UPPER END OF THE POOL AREA TO IMPROVE BASIN TRAP EFFICIENCY. EROSION CONTROL-CONSTRUCT THE STRUCTURE SO THAT THE DISTURBED AREA IS MINIMIZED. DIVERT SURFACE WATER AWAY FROM BARE AREAS. COMPLETE THE EMBANKMENT BEFORE THE AREA IS CLEARED. STABILIZE THE EMERGENCY SPILLWAY EMBANKMENT AND ALL OTHER DISTURBED AREAS ABOVE THE CREST OF THE PRINCIPAL SPILLWAY IMMEDIATELY
- O. INSTALL POROUS BAFFLES AS SPECIFIED IN PRACTICE, 6.65 . AFTER ALL THE SEDIMENT-PRODUCING AREAS HAVE BEEN PERMANENTLY STABILIZED, REMOVE THE STRUCTURE AND ALL THE UNSTABLE SEDIMENT. SMOOTH THE AREA TO BLEND WITH THE ADJOINING AREAS AND STABILIZE PROPERLY.

MAINTENANCE

INSPECT SKIMMER SEDIMENT BASINS AT LEAST WEEKLY AND AFTER EACH SIGNIFICANT (ONE-HALF INCH OR GREATER) RAINFALL EVENT AND REPAIR IMMEDIATELY. REMOVE SEDIMENT AND RESTORE THE BASIN TO ITS ORIGINAL DIMENSIONS WHEN SEDIMENT ACCUMULATES TO ONE-HALF THE HEIGHT OF THE FIRST BAFFLE. PULL THE SKIMMER TO ONE SIDE SO THAT THE SEDIMENT UNDERNEATH IT CAN BE EXCAVATED. EXCAVATE THE SEDIMENT FROM THE ENTIRE BASIN, NOT JUST AROUND THE SKIMMER OR THE FIRST CELL. MAKE SURE VEGETATION GROWING IN THE BOTTOM OF THE BASIN DOES NOT

REPAIR THE BAFFLES IF THEY ARE DAMAGED. RE-ANCHOR THE BAFFLES IF WATER IS FLOWING UNDERNEATH OR AROUND

IF THE SKIMMER IS CLOGGED WITH TRASH AND THERE IS WATER IN THE BASIN, USUALLY JERKING ON THE ROPE WILL MAKE THE SKIMMER BOB UP AND DOWN AND DISLODGE THE DEBRIS AND RESTORE FLOW. IF THESE DOES NOT WORK, PULL THE SKIMMER OVER TO THE SIDE OF THE BASIN AND REMOVE THE DEBRIS. ALSO CHECK THE ORIFICE INSIDE THE SKIMMER TO SEE IF IT IS CLOGGED; IF SO REMOVE THE DEBRIS.

IF THE SKIMMER ARM OR BARREL PIPE IS CLOGGED, THE ORIFICE CAN BE REMOVED AND THE OBSTRUCTION CLEARED WITH A PLUMBER'S SNAKE OR BY FLUSHING WITH WATER. BE SURE AND REPLACE THE ORIFICE BEFORE REPOSITIONING THE CHECK THE FABRIC LINED SPILLWAY FOR DAMAGE AND MAKE ANY REQUIRED REPAIRS WITH FABRIC THAT SPANS THE FULL WIDTH OF THE SPILLWAY. CHECK THE EMBANKMENT, SPILLWAYS, AND OUTLET FOR EROSION DAMAGE, AND INSPECT THE EMBANKMENT FOR PIPING AND SETTLEMENT. MAKE ALL NECESSARY REPAIR IMMEDIATELY. REMOVE ALL TRASH AND OTHER DEBRIS FROM THE SKIMMER AND POOL AREAS.

FREEZING WEATHER CAN RESULT IN ICE FORMING IN THE BASIN. SOME SPECIAL PRECAUTIONS SHOULD BE TAKEN IN THE WINTER TO PREVENT THE SKIMMER FROM PLUGGING WITH ICE.

SEDIMENT BASIN WITH SKIMMER OUTLET AND EMERGENCY SPILLWAY

FOR PLAN REVIEW ONLY

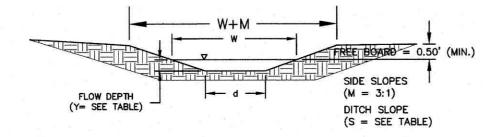
SEAL 022594

/2 DIAMETER OF PIPE OR 12" WHICHEVER IS GREATER CAPACITY OF PIPE = DITCH CAPACITY INSPECT TEMPORARY STREAM/DITCH CROSSINGS AFTER RUNOFF-PRODUCING RAINS TO CHECK FOR BLOCKAGE IN CHANNEL, EROSION OF ABUTMENTS, CHANNEL SCOUR, RIPRAP DISPLACEMENT, OR PIPING. MAKE ALL REPAIRS IMMEDIATELY TO PREVENT FURTHER DAMAGE TO THE TEMPORARY DITCH CROSSING

TO PUBLIC ROAD --

NCDOT #5 WASHED STONE

TEMPORARY EROSION CONTROL MATTING NO SCALE



TEMPORARY DIVERSIONS NO SCALE

shape: parabolic, trapezoidal, or V-shaped side slope: 2:1 or flatter 3:1 or flatter where vehicles cross

Grades— Either a uniform or a gradually increasing grade is preferred. Sudden decreases in grade accumulate sediment and should be expected to cause overtopping. A large increase in grade may erode.

Outlet—Design the outlet to accept flow from the diversion plus any other contributing areas. Divert sediment-laden runoff and release through a sediment-trapping device (Practice 6.60, Temporary Sediment Trap and Practice 6.61, Sediment Basin). Flow from undisturbed areas can be dispersed by a level spreader (Practice 6.40, Level Spreader).

Small diversions—Where the diversion channel grade is between 0.2 and 3%, a permanent vegetative cover is required. A parabolic channel and ridge 1.5 feet deep and 12 feet wide may be used for diversions with flows up to 5 cfs. This depth does not include freeboard or settlement. Side slopes should be 3:1 or flatter, and the top of the dike must be at least 2 feet wide.

Specifications

Construction 1. Remove and properly dispose of all trees, brush, stumps, and other objectionable material 2. Ensure that the minimum constructed cross section meets all design

> 3. Ensure that the top of the dike is not lower at any point than the design elevation plus the specified settlement.

> 4. Provide sufficient room around diversions to permit machine regrading and

5. Vegetate the ridge immediately after construction, unless it will remain in place less than 30 working days.

Maintenance Inspect temporary diversions once a week and after every rainfall. Immediately remove sediment from the flow area and repair the diversion ridge. Carefully check outlets and make timely repairs as needed. When the area protected is permanently stabilized, remove the ridge and the channel to blend with the natural ground level and appropriately stabilize it.

Dewatering Bag/Tube[™] Standard Drawing

occur beneath the RECP.

ground shall be repaired and stapled.

fixed and the eroded area protected.

Maintenance 1. Inspect Rolled Erosion Control Products at least weekly and after each

significant (1/2 inch or greater) rain fall event repair immediately.

2. Good contact with the ground must be maintained, and erosion must not

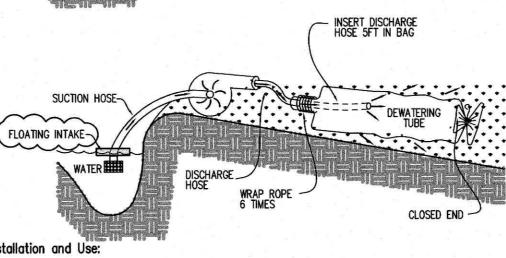
3. Any areas of the RECP that are damaged or not in close contact with the

4. If erosion occurs due to poorly controlled drainage, the problem shall be

5. Monitor and repair the RECP as necessary until ground cover is

SedCatch Dewatering Bag FLOATING INTAKE ~~~~ DISCHARGE ___

A SedCatch® Dewatering Bag/Tube™ or approved equal should be used anytime water is pumped on the site.



1. Place SedCatch Dewatering Bag/Tube on the ground or on a trailer over a relatively level, stabilized area.

2. Insert discharge pipe a minimum of 5ft. inside SedCatch Dewatering Bag/Tube and secure with a rope (included) wrapped 6 times around the snout over a 6 inch width of the bag. To close Open end of the Dewatering Tube: Overlap the tube 2 ft. from the end. Gather the center of the doubled-up portion forming a bow tie. Secure with a rope wrapped multiple times.

3. Replace SedCatch Dewatering Bag when half full of sediment or when the sediment has reduced the flow rate of the pump discharge to an impractical amount, OR; empty SedCatch Sediment Tube when half full of sediment or when the sediment has reduced the flow rate of the pump discharge to an impractical rate.

Remove and dispose of accumulated sediment away from waterways or environmentally sensitive areas. Slit open Dewatering Bag and remove accumulated sediment. Dispose of bag at an appropriate recycling or solid waste facility, OR: open both ends of the SedCatch Dewatering Tube , pick it up in the center and dump accumulated sediment out of both ends. Allow to dry and store for re-use, OR; as directed by engineer or inspector.

PICKLEBALL/TENNIS COURT FACILITY J. BURT GILLETTEATHLETIC COMPLEX

NORTH CAROLINA

DETAILS

DATE BY DATE: March 14, 2023 CLIENT CODE: WILSO **GRAPHIC SCALE** JOB NUMBER: 21-194 FIELD BOOK: XXXXX CADFILE: 21194-CV.dwg ASCII FILE: AS SHOWN LAST MODIFIED: 14-Mar-23 MODIFIED BY: JM

NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM

GREEN ENGINEERING

WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

CITY OF WILSON

SHEET NO. 15 OF 17

SECTION A: SELF-INSPECTION

Self-inspections are required during normal business hours in accordance with the table below. When adverse weather or site conditions would cause the safety of the inspection personnel to be in jeopardy, the inspection may be delayed until the next business day on which it is safe to perform the inspection. In addition, when a storm event of equal to or greater than 1.0 inch occurs outside of normal business hours, the self-inspection shall be performed upon the commencement of the next business day. Any time when inspections were delayed shall be noted in the Inspection Record.

Inspect	Frequency (during normal business hours)	Inspection records must include:
(1) Rain gauge maintained in good working order	Daily	Daily rainfall amounts. If no daily rain gauge observations are made during weekend or holiday periods, and no individual-day rainfall information is available, record the cumulative rain measurement for those unattended days (and this will determine if a site inspection is needed). Days on which no rainfall occurred shall be recorded as "zero." The permittee may use another rain-monitoring device approved by the Division.
(2) E&SC Measures	At least once per 7 calendar days and within 24 hours of a rain event ≥ 1.0 inch in 24 hours	Identification of the measures inspected, Date and time of the inspection, Name of the person performing the inspection, Indication of whether the measures were operating properly, Description of maintenance needs for the measure, Description, evidence, and date of corrective actions taken.
(3) Stormwater discharge outfalls (SDOs)	At least once per 7 calendar days and within 24 hours of a rain event ≥ 1.0 inch in 24 hours	 Identification of the discharge outfalls inspected, Date and time of the inspection, Name of the person performing the inspection, Evidence of indicators of stormwater pollution such as oil sheen, floating or suspended solids or discoloration, Indication of visible sediment leaving the site, Description, evidence, and date of corrective actions taken.
(4) Perimeter of site	At least once per 7 calendar days and within 24 hours of a rain event ≥ 1.0 inch in 24 hours	If visible sedimentation is found outside site limits, then a record of the following shall be made: Actions taken to clean up or stabilize the sediment that has left the site limits, Description, evidence, and date of corrective actions taken, and An explanation as to the actions taken to control future releases.
(5) Streams or wetlands onsite or offsite (where accessible)	At least once per 7 calendar days and within 24 hours of a rain event ≥ 1.0 inch in 24 hours	If the stream or wetland has increased visible sedimentation or a stream has visible increased turbidity from the construction activity, then a record of the following shall be made: 1. Description, evidence and date of corrective actions taken, and 2. Records of the required reports to the appropriate Division Regional Office per Part III, Section C, Item (2)(a) of this permit of this permit.
(6) Ground stabilization measures	After each phase of grading	The phase of grading (installation of perimeter E&SC measures, clearing and grubbing, installation of storm drainage facilities, completion of all land-disturbing activity, construction or redevelopment, permanent ground cover). Documentation that the required ground stabilization measures have been provided within the required

soon as possible.

NOTE: The rain inspection resets the required 7 calendar day inspection requirement.

timeframe or an assurance that they will be provided as

SELF-INSPECTION, RECORDKEEPING AND REPORTING

SECTION B: RECORDKEEPING

L. E&SC Plan Documentation The approved E&SC plan as well as any approved deviation shall be kept on the site. The approved E&SC plan must be kept up-to-date throughout the coverage under this permit. The following items pertaining to the E&SC plan shall be documented in the manner

Item to Document	Documentation Requirements
(a) Each E&SC Measure has been installed and does not significantly deviate from the locations, dimensions and relative elevations shown on the approved E&SC Plan.	Initial and date each E&SC Measure on a copy of the approved E&SC Plan or complete, date and sign an inspection report that lists each E&SC Measure shown on the approved E&SC Plan. This documentation is required upon the initial installation of the E&SC Measures or if the E&SC Measures are modified after initial installation.
(b) A phase of grading has been completed.	Initial and date a copy of the approved E&SC Plan or complete, date and sign an inspection report to indicate completion of the construction phase.
(c) Ground cover is located and installed in accordance with the approved E&SC Plan.	Initial and date a copy of the approved E&SC Plan or complete, date and sign an inspection report to indicate compliance with approved ground cover specifications.
(d) The maintenance and repair requirements for all E&SC Measures have been performed.	Complete, date and sign an inspection report.
(e) Corrective actions have been taken to E&SC Measures.	Initial and date a copy of the approved E&SC Plan or complete, date and sign an inspection report to indicate the completion of the corrective action.

requirement not practical:

In addition to the E&SC Plan documents above, the following items shall be kept on the and available for agency inspectors at all times during normal business hours, unless the

Division provides a site-specific exemption based on unique site conditions that make this

- (a) This general permit as well as the certificate of coverage, after it is received.
- (b) Records of inspections made during the previous 30 days. The permittee shall record the required observations on the Inspection Record Form provided by the Division or a similar inspection form that includes all the required elements. Use of electronically-available records in lieu of the required paper copies will be allowed if shown to provide equal access and utility as the hard-copy records.
- All data used to complete the Notice of Intent and older inspection records shall be maintained for a period of three years after project completion and made available upon request. [40 CFR 122.41]

SELF-INSPECTION, RECORDKEEPING AND REPORTING

SECTION C: REPORTING

1. Occurrences that must be reported Permittees shall report the following occurrences:

- (a) Visible sediment deposition in a stream or wetland.
- (b) Oil spills if:
- They are 25 gallons or more,
- They are less than 25 gallons but cannot be cleaned up within 24 hours,
- They cause sheen on surface waters (regardless of volume), or They are within 100 feet of surface waters (regardless of volume).
- (a) Releases of hazardous substances in excess of reportable quantities under Section 311 of the Clean Water Act (Ref: 40 CFR 110.3 and 40 CFR 117.3) or Section 102 of CERCLA (Ref: 40 CFR 302.4) or G.S. 143-215.85.
- (b) Anticipated bypasses and unanticipated bypasses.
- (c) Noncompliance with the conditions of this permit that may endanger health or the

2. Reporting Timeframes and Other Requirements

After a permittee becomes aware of an occurrence that must be reported, he shall contact the appropriate Division regional office within the timeframes and in accordance with the other requirements listed below. Occurrences outside normal business hours may also be reported to the Division's Emergency Response personnel at (800) 662-7956, (800) 858-0368 or (919) 733-3300.

Occurrence	Reporting Timeframes (After Discovery) and Other Requirements
(a) Visible sediment deposition in a stream or wetland	 Within 24 hours, an oral or electronic notification. Within 7 calendar days, a report that contains a description of the sediment and actions taken to address the cause of the deposition. Division staff may waive the requirement for a written report on a case-by-case basis. If the stream is named on the NC 303(d) list as impaired for sediment-
	related causes, the permittee may be required to perform additional monitoring, inspections or apply more stringent practices if staff determine that additional requirements are needed to assure compliance with the federal or state impaired-waters conditions.
(b) Oil spills and release of hazardous substances per Item 1(b)-(c) above	Within 24 hours, an oral or electronic notification. The notification shall include information about the date, time, nature, volume and location of the spill or release.
(c) Anticipated bypasses [40 CFR 122.41(m)(3)]	 A report at least ten days before the date of the bypass, if possible. The report shall include an evaluation of the anticipated quality and effect of the bypass.
(d) Unanticipated	Within 24 hours, an oral or electronic notification.

Within 24 hours, an oral or electronic notification.

quality and effect of the bypass.

case-by-case basis.

Within 7 calendar days, a report that includes an evaluation of the

Within 7 calendar days, a report that contains a description of the

including exact dates and times, and if the noncompliance has not

continue; and steps taken or planned to reduce, eliminate, and

prevent reoccurrence of the noncompliance. [40 CFR 122.41(I)(6).

Division staff may waive the requirement for a written report on a

been corrected, the anticipated time noncompliance is expected to

noncompliance, and its causes; the period of noncompliance,

· Uniform and evenly distributed ground cover · Structural methods such as concrete, asphalt or

Appropriately applied straw or other mulch

GROUND STABILIZATION SPECIFICATION

· Rolled erosion control products with or

techniques in the table below:

other mulches and tackifiers

without temporary grass seed

Hydroseeding

· Plastic sheeting

THE NCG01 CONSTRUCTION GENERAL PERMI

SECTION E: GROUND STABILIZATION

Site Area Description

perimeter slopes

(b) High Quality Water

(c) Slopes steeper than

(HQW) Zones

(d) Slopes 3:1 to 4:1

e) Areas with slopes

flatter than 4:1

(a) Perimeter dikes, swales, ditches, an

Implementing the details and specifications on this plan sheet will result in the construction

activity being considered compliant with the Ground Stabilization and Materials Handling

sections of the NCG01 Construction General Permit (Sections E and F, respectively). The

delegated authority having jurisdiction. All details and specifications shown on this sheet

may not apply depending on site conditions and the delegated authority having jurisdiction.

Required Ground Stabilization Timeframes

Timeframe variations

not steeper than 2:1, 14 days are

-7 days for slopes greater than 50' in

7 days for perimeter dikes, swales,

ditches, perimeter slopes and HQW

-10 days for Falls Lake Watershed

there is zero slope

Note: After the permanent cessation of construction activities, any areas with temporary

ground stabilization shall be converted to permanent ground stabilization as soon as practicable but in no case longer than 90 calendar days after the last land disturbing

activity. Temporary ground stabilization shall be maintained in a manner to render the

Stabilize the ground sufficiently so that rain will not dislodge the soil. Use one of the

Temporary grass seed covered with straw or
 Permanent grass seed covered with straw or

surface stable against accelerated erosion until permanent ground stabilization is achieved.

7 days for perimeter dikes, swales,

ditches, perimeter slopes and HQW Zones

-10 days for Falls Lake Watershed unless

Permanent Stabilization

Geotextile fabrics such as permanent soil

Shrubs or other permanent plantings covered

Rolled erosion control products with grass seed

reinforcement matting

sufficient to restrain erosion

Hydroseeding

with mulch

retaining walls

length and with slopes steeper than 4:1

permittee shall comply with the Erosion and Sediment Control plan approved by the

Stabilize within th

days after ceasing

and disturbance

many calendar

- **POLYACRYLAMIDES (PAMS) AND FLOCCULANTS** Select flocculants that are appropriate for the soils being exposed during construction, selecting from the NC DWR List of Approved PAMS/Flocculants.
- Apply flocculants at or before the inlets to Erosion and Sediment Control Measures. Apply flocculants at the concentrations specified in the NC DWR List of Approved PAMS/Flocculants and in accordance with the manufacturer's instructions.
- Provide ponding area for containment of treated Stormwater before discharging
- Store flocculants in leak-proof containers that are kept under storm-resistant cover or surrounded by secondary containment structures.

GROUND STABILIZATION AND MATERIALS HANDLING PRACTICES FOR COMPLIANCE WITH EQUIPMENT AND VEHICLE MAINTENANCE

- Maintain vehicles and equipment to prevent discharge of fluids.
- Provide drip pans under any stored equipment. Identify leaks and repair as soon as feasible, or remove leaking equipment from the
- Collect all spent fluids, store in separate containers and properly dispose as
- hazardous waste (recycle when possible). 5. Remove leaking vehicles and construction equipment from service until the problem
- has been corrected. 6. Bring used fuels, lubricants, coolants, hydraulic fluids and other petroleum products to a recycling or disposal center that handles these materials.

LITTER, BUILDING MATERIAL AND LAND CLEARING WASTE

- Never bury or burn waste. Place litter and debris in approved waste containers. . Provide a sufficient number and size of waste containers (e.g dumpster, trash
- receptacle) on site to contain construction and domestic wastes.
- Locate waste containers at least 50 feet away from storm drain inlets and surface waters unless no other alternatives are reasonably available. Locate waste containers on areas that do not receive substantial amounts of runoff
- from upland areas and does not drain directly to a storm drain, stream or wetland. Cover waste containers at the end of each workday and before storm events or
- provide secondary containment. Repair or replace damaged waste containers. Anchor all lightweight items in waste containers during times of high winds.
- Empty waste containers as needed to prevent overflow. Clean up immediately if
- containers overflow. Dispose waste off-site at an approved disposal facility.

9. On business days, clean up and dispose of waste in designated waste containers.

- Do not dump paint and other liquid waste into storm drains, streams or wetlands. 2. Locate paint washouts at least 50 feet away from storm drain inlets and surface waters unless no other alternatives are reasonably available.
- Contain liquid wastes in a controlled area.
- 4. Containment must be labeled, sized and placed appropriately for the needs of site.

Prevent the discharge of soaps, solvents, detergents and other liquid wastes from

construction sites.

- Install portable toilets on level ground, at least 50 feet away from storm drains, streams or wetlands unless there is no alternative reasonably available. If 50 foot offset is not attainable, provide relocation of portable toilet behind silt fence or place on a gravel pad and surround with sand bags.
- Provide staking or anchoring of portable toilets during periods of high winds or in high
- Monitor portable toilets for leaking and properly dispose of any leaked material. Utilize a licensed sanitary waste hauler to remove leaking portable toilets and replace with properly operating unit.

EARTHEN STOCKPILE MANAGEMENT

- Show stockpile locations on plans. Locate earthen-material stockpile areas at least 50 feet away from storm drain inlets, sediment basins, perimeter sediment controls and surface waters unless it can be shown no other alternatives are reasonably
- Protect stockpile with silt fence installed along toe of slope with a minimum offset of five feet from the toe of stockpile
- Provide stable stone access point when feasible
- 4. Stabilize stockpile within the timeframes provided on this sheet and in accordance with the approved plan and any additional requirements. Soil stabilization is defined as vegetative, physical or chemical coverage techniques that will restrain accelerated erosion on disturbed soils for temporary or permanent control needs.

CONCRETE NOTING DEVICE (18"X24" MIN.) THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIGUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY. J.CONCRETE WASHOUT STRUCTURE HEEDS TO BE CLEARY MANGED WITH SIGNAGE NOTING DEVICE. BELOW GRADE WASHOUT STRUCTURE

CONCRETE WASHOUTS

- Do not discharge concrete or cement slurry from the site. 2. Dispose of, or recycle settled, hardened concrete residue in accordance with local
- and state solid waste regulations and at an approved facility. Manage washout from mortar mixers in accordance with the above item and in addition place the mixer and associated materials on impervious barrier and within
- lot perimeter silt fence. 4. Install temporary concrete washouts per local requirements, where applicable. If an alternate method or product is to be used, contact your approval authority for review and approval. If local standard details are not available, use one of the two
- types of temporary concrete washouts provided on this detail. 5. Do not use concrete washouts for dewatering or storing defective curb or sidewalk sections. Stormwater accumulated within the washout may not be pumped into or discharged to the storm drain system or receiving surface waters. Liquid waste must
- be pumped out and removed from project. Locate washouts at least 50 feet from storm drain inlets and surface waters unless it can be shown that no other alternatives are reasonably available. At a minimum, install protection of storm drain inlet(s) closest to the washout which could receive
- Locate washouts in an easily accessible area, on level ground and install a stone entrance pad in front of the washout. Additional controls may be required by the
- approving authority. Install at least one sign directing concrete trucks to the washout within the project
- limits. Post signage on the washout itself to identify this location. Remove leavings from the washout when at approximately 75% capacity to limit overflow events. Replace the tarp, sand bags or other temporary structural components when no longer functional. When utilizing alternative or proprietary
- products, follow manufacturer's instructions. 10. At the completion of the concrete work, remove remaining leavings and dispose of in an approved disposal facility. Fill pit, if applicable, and stabilize any disturbance caused by removal of washout.

HERBICIDES, PESTICIDES AND RODENTICIDES

- 1. Store and apply herbicides, pesticides and rodenticides in accordance with label
- Store herbicides, pesticides and rodenticides in their original containers with the label, which lists directions for use, ingredients and first aid steps in case of accidental poisoning.
- . Do not store herbicides, pesticides and rodenticides in areas where flooding is possible or where they may spill or leak into wells, stormwater drains, ground water
- or surface water. If a spill occurs, clean area immediately. 4. Do not stockpile these materials onsite.

HAZARDOUS AND TOXIC WASTE

- Create designated hazardous waste collection areas on-site.
- Place hazardous waste containers under cover or in secondary containment. 3. Do not store hazardous chemicals, drums or bagged materials directly on the ground.

NCG01 SELF-INSPECTION, RECORDKEEPING AND REPORTING EFFECTIVE: 04/01/19

122.41(m)(3)]

of this permit that

may endanger

environment[40

CFR 122.41(I)(7)

health or the

NCG01 GROUND STABILIZATION AND MATERIALS HANDLING

EFFECTIVE: 04/01/19

Mulching Materials and Application Rate Organic Mulches Should come from wheat or oats: (ANCHOR STRAW BY TACKING WITH unweathered; avoid spread by hand or machine; must be tacked down. ASPHALT OR NETTING) Treat with 12 lbs nitrogen/ton. Apply Wood chips 5-6 tons with mulch blower, chip handler, or by hand. Not for use in fine turf. Also referred to as wood cellulose. Wood fiber May be hydroseeded. Do not use it hot, dry weather Apply with mulch blower, chip 35 cubic vards Air dry, shredded or hammer-milled, or chips. handler, or by hand. Do not use asphalt tack. Cut or shredded in 4-6 Apply with mulch blower or by hand Not for use in fine turf. in lengths. Green or dry; should lespedeza seed-bearing stems ets and Mats Jute net Heavy, uniform; woven Withstands waterflow. Best when of single jute yarn. used with organic mulch. Withstands waterflow. Best when Fiberglass ne used with organic mulch. Withstands waterflow. (wood fiber) Continuous fibers of Apply with a compressed air ejector Fiberglass roving 0.5-1 tons Tack with emulsified asphalt at a drawn glass bound together with a non-toxic rate of 25-35 gal/1,000 sq ft. Not beneficial to plant growth Curasol AK specifications Petroset SB Terra Tack Crust 500 Genaqua 743 Refer to Practice No. 6.30, Grass Lined Channels. 2Use of trade names does not imply endorsement of produc

dislocation or failure. Where erosion is observed, apply additional mulch. If

washout occurs, repair the slope grade, reseed and reinstall mulch. Continue

inspections until vegetation is firmly established.

SEEDING MAINTENANCE: REFERTILIZE IF GROWTH IS NOT FULLY ADEQUATE. RESEED, REFERTILIZE AND MULCH IMMEDIATELY FOLLOWING EROSION OF PERMANENT SEEDING Summer = March 1 - August 31 MIN. OF 2 TONS PER ACRE WITH 3 TONS PER ACRE IN CLAY SOILS OR PER SOILS TEST. Bermudagrass (hulled)... Winter = September 1 - February 28MIN. OF 2 TONS PER ACRE WITH 3 TONS PER ACRE IN CLAY SOILS OR PER SOILS TEST. Bermudagrass (unhulled) *Straw Mulch TEMPORARY SEEDING Summer - March 1 - August 3MIN. OF 2 TONS PER ACRE WITH 3 TONS PER ACRE IN CLAY SOILS OR PER SOILS TEST. Winter = September 1 - February 28MIN. OF 2 TONS PER ACRE WITH 3 TONS PER ACRE IN CLAY SOILS OR PER SÓILS TEST. *Straw Mulch . * Mulch will be doubled if crimping is the method used. . Any variation from these specs must have approval of the Stormwater Program Manager or his/her duty authorized agent. Services Departments
Herring Avenue, / P.O. Box 10
n, North Carolina 27894

CITY of WILSON, N.C.
USE WITH THE CITY OF WILSON STANDARD SPECIFICATIONS ONLY Maintenance Inspect all mulches periodically, and after rainstorms to check for rill erosion,

SEEDING

SPECIFICATIONS

SEEDBED PREPARATION

- CHISEL COMPACTED AREAS AND SPREAD TOPSOIL 3 INCHES DEEP OVER ADVERSE SOIL CONDITIONS, IF AVAILABLE
- . RIP THE ENTIRE AREA TO 6 INCHES DEPTH.
- REMOVE ALL LOOSE ROCK, ROOTS, AND OTHER OBSTRUCTIONS LEAVING SURFACE REASONABLY SMOOTH AND UNIFORM.
- . APPLY AGRICULTURAL LIME, FERTILIZER, AND SUPERPHOSPHATE UNIFORMLY AND MIX WITH SOIL (SEE BELOW).
- CONTINUE TILLAGE UNTIL A WELL-PULVERIZED, FIRM, REASONABLY UNIFORM SEEDBED IS PREPARED 4 TO 6 INCHES DEEP.
- SEED ON A FRESHLY PREPARED SEEDBED AND COVER SEED LIGHTLY WITH SEEDING
- EQUIPMENT OR CULTIPACK AFTER SEEDING. MULCH IMMEDIATELY AFTER SEEDING AND ANCHOR MULCH.
- 8. INSPECT ALL SEEDED AREAS AND MAKE NECESSARY REPAIRS OR RESEEDINGS WITHIN THE PLANTING SEASON, IF POSSIBLE. IF STAND SHOULD BE EVER 60% DAMAGED, REESTABLISH FOLLOWING ORIGINAL LIME, FERTILIZER AND SEEDING
- CONSULT CONSERVATION INSPECTOR ON MAINTENANCE TREATMENT AND FERTILIZATION AFTER PERMANENT COVER IS ESTABLISHED.

AGRICULTURAL LIMESTONE - 2 TONS/ACRE FERTILIZER - 10-10-10 ANALYSIS AT 800 - 1000 LBS./ACRE SUPERPHOSPHATE - 500 LBS./ACRE OF 20% ANALYSIS SUPERPHOSPHATE MULCH - 2 TONS SMALL GRAIN STRAW/ACRE ANCHOR - TACK WITH LIQUID ASPHALT AT 400 GALLONS/ACRE OR EMULSIFIED ASPHALT AT 400 GALLONS/ACRE

EROSION CONTROL NOTES

- 1. STABILIZATION AREAS ACCORDING TO THE REQUIREMENTS OF THE NPDES GROUNDCOVER STABILIZATION TIMETABLE (SEE NEW STABILIZATION TIMEFRAMES).
- 2. SILT FENCE TO BE INSTALLED AS SHOWN ON THE PLANS OR AS DEEMED NECESSARY BY VISUAL OBSERVATION.

HYDROSEEDING

SURFACE ROUGHENING IS PARTICULARLY IMPORTANT WHEN HYDROSEEDING, AS A ROUGHENED SLOPE WILL PROVIDE SOME NATURAL COVERAGE FOR LIME, FERTILIZER, AND SEED. THE SURFACE SHOULD NOT BE COMPACTED OR SMOOTH. FINE SEEDBED PREPARATION IS NOT NECESSARY FOR HYDROSEEDING OPERATIONS: LARGE CLODS, STONES, AND IRREGULARITIES PROVIDE CAVITIES IN WHICH SEEDS CAN LODGE.

RATE OF WOOD FIBER (CELLULOSE) APPLICATION SHOULD BE AT LEAST 2,000 LB/ACRE.

APPLY LEGUME INOCULANTS AT FOUR TIMES THE RECOMMENDED RATE WHEN ADDING INOCULANT TO A HYDROSEEDER SLURRY.

IF A MACHINERY BREAKDOWN OF 1/2 TO 2 HOURS OCCURS, ADD 50% MORE SEED TO THE TASK, BASED ON THE PROPORTION OF THE SLURRY REMAINING. THIS SHOULD COMPENSATE FOR DAMAGE TO SEED. BEYOND 2 HOURS, FULL RATE OF NEW SEED MAY BE

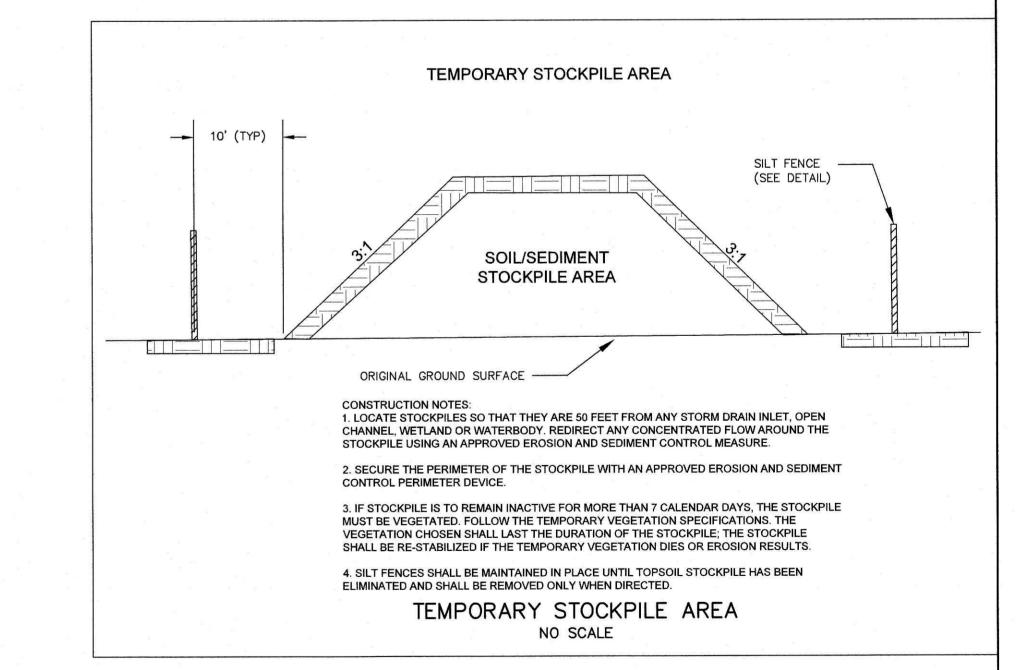
LIME IS NOT NORMALLY APPLIED WITH A HYDRAULIC SEEDER BECAUSE IT IS ABRASIVE. IT CAN BE BLOWN ONTO STEEP SLOPES IN DRY FORM.

CRIMPING STRAW MULCH

IF CRIMPING IS TO BE USED, APPLY $\frac{1}{2}$ THE MULCH, THEN CRIMP, AND INSTALL THE OTHER HALF. CRIMPING CAN BE APPLIED TO AREAS ADJACENT TO ANY SECTION OF THE ROADWAY WHERE TRAFFIC IS TO BE MAINTAINED OR ALLOWED DURING CONSTRUCTION. IN AREAS WITHIN SIX FEET OF THE EDGE OF PAVEMENT, STRAW IS TO BE APPLIED AND THEN CRIMPED. AFTER THE CRIMPING OPERATION IS COMPLETE, AN ADDITIONAL APPLICATION OF STRAW SHALL BE APPLIED AND IMMEDIATELY TACKED WITH A SUFFICIENT AMOUNT OF UNDILUTED EMULSIFIED

STRAW MULCH SHALL BE OF SUFFICIENT LENGHT AND QUALITY TO WITHSTAND THE CRIMPING OPERATION.

CRIMPING EQUIPMENT INCLUDING POWER SOURCE SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER PROVIDING THAT MAXIMUM SPACING OF CRIMPER BLADES SHALL NOT EXCEED 8".



FOR PLAN REVIEW ONLY

GREEN ENGINEERING

WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM PICKLEBALL/TENNIS COURT FACILITY J. BURT GILLETTEATHLETIC COMPLEX

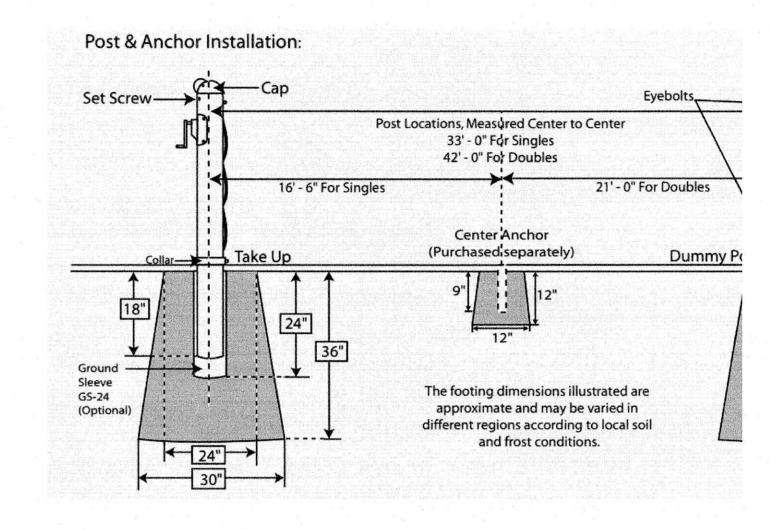
NORTH CAROLINA

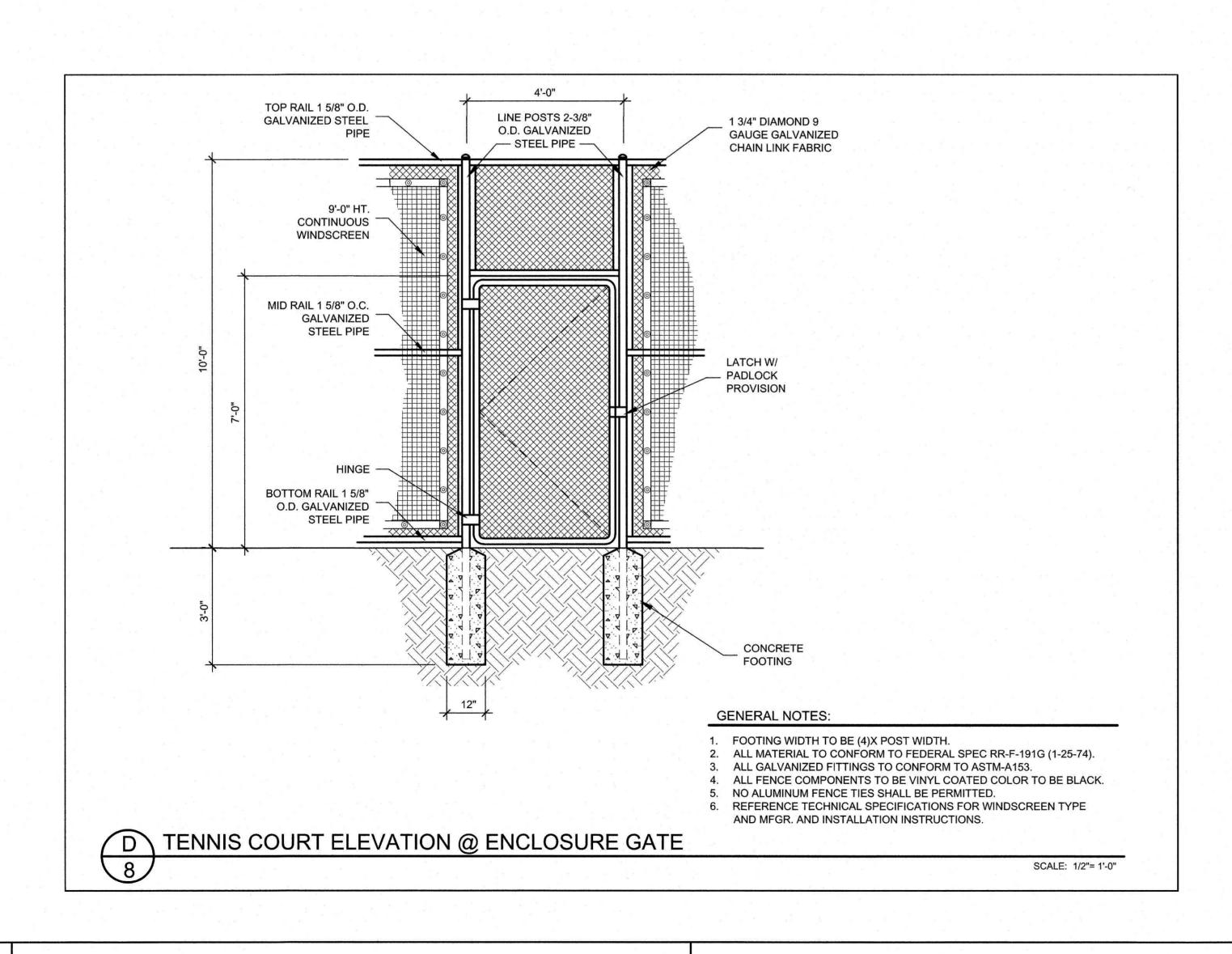
DETAILS

DATE BY DATE: March 14, 2023 CLIENT CODE: GRAPHIC SCALE JOB NUMBER: 21-194 FIELD BOOK: XXXXX CADFILE: 21194—CV.dwg ASCII FILE: AS SHOWN LAST MODIFIED: 14-Mar-23 MODIFIED BY: JM SHEET NO. 16 of 17

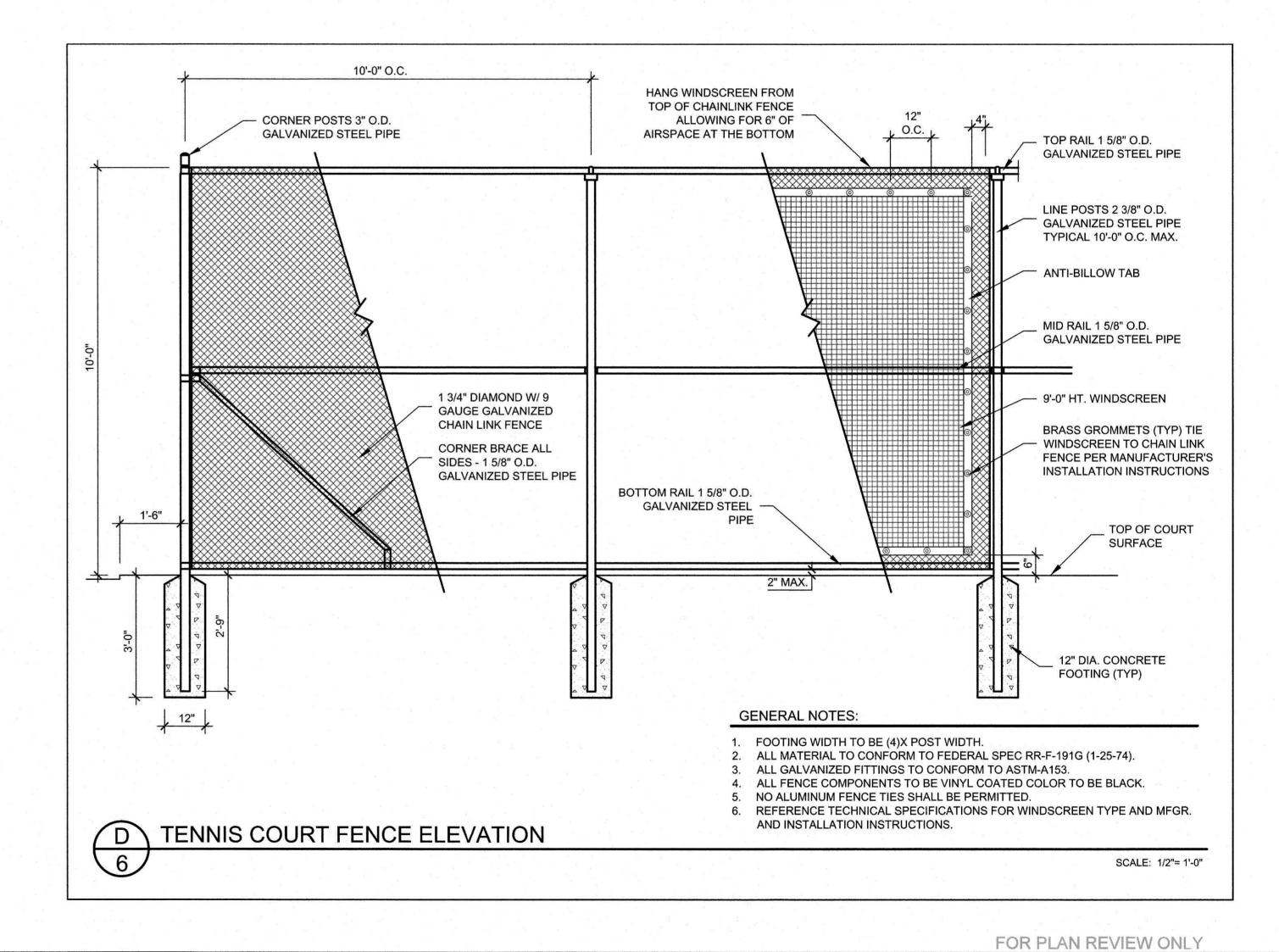
022594

CITY OF WILSON





CITY OF WILSON





GREEN ENGINEERING

WATER, WASTEWATER, SURVEYING, PLANNING, PROJECT MANAGEMENT

NORTH CAROLINA FIRM LICENSE: P-0115 303 GOLDSBORO STREET EAST, P.O. BOX 609 WILSON, N.C. 27893 TEL (252) 237-5365 FAX (252) 243-7489 OFFICE@GREENENG.COM PICKLEBALL/TENNIS COURT FACILITY

J. BURT GILLETTEATHLETIC COMPLEX

DETIALS

NORTH CAROLINA

REVISION DATE BY DATE: March 14, 2023
GRAPHIC SCALE

AS SHOWN

CLIENT CODE: WILSO
JOB NUMBER: 21-194
FIELD BOOK: XXXXX
CADFILE: 21194-CV.dwg
ASCII FILE:
LAST MODIFIED: 14-Mar-23
MODIFIED BY: JM

SHEET NO. 17 OF 17

0-034